

AGENDA
CITY OF TAYLOR, TEXAS
PLANNING & ZONING COMMISSION MEETING
CITY HALL COUNCIL CHAMBERS
400 PORTER STREET, TAYLOR, TX, 76574
OCTOBER 8, 2024, 6:00 PM

For Citizens Communication and to speak during the public hearings, please contact the Development Services office at 512-365-3863 prior to 5:30 p.m. on Tuesday, October 8, 2024.

The agenda packet is on the City of Taylor's [website](#).

I. CALL TO ORDER AND DECLARE A QUORUM

II. CITIZEN COMMUNICATION

(The Planning and Zoning Commission welcomes public comments on items not listed on the agenda. However, the Commission cannot respond until an item is posted on a future meeting agenda. Registration forms are available at the sign in table)

III. CONSENT AGENDA

(The Consent Agenda includes non-controversial and routine items that the Commission may act on with a single vote. The Chair or a Commissioner may pull any item from the Consent Agenda to discuss and act upon it individually as part of the Regular Agenda.)

1. Review and approve minutes from the meeting held on September 10, 2024
2. **PZ-2024-2143** Consider Approval of The Retreat at Mallard Final Plat, generally located near the intersection of Mallard Lane and Jason Drive, legally described as a 19.617-acre tract of land part of and out of the William J. Baker Survey, Abstract No. 65, more particularly described by Williamson Central Appraisal District Parcel R018622, Taylor, Williamson County, Texas. *Courtney Peres*
3. **PZ-2024-2216** Consider Disapproval of the Taylor Pointe Subdivision Preliminary Plat generally located at 715 Carlos G Parker Blvd. SW, consisting of approximately 39.015 acres of land part of the G.M. Reese Survey, Abstract No. 533, more particularly described by Williamson Central Appraisal District Parcel R331170, Taylor, Williamson County, Texas. *Parker McDowell*
4. **PZ-2024-2254** Consider Disapproval of the Taylor North Bull Branch Creek Townhomes, Subdivision Improvement Plan, generally located along the South right-of-way of N.W. Carlos G. Parker Boulevard, approximately 1,400 feet west of the intersection of N.W. Carlos G. Parker Boulevard and North Drive, consisting of approximately 7.747 acres of land part of and out of the William J Baker Survey, Abstract No. 65, more particularly described by the Williamson Central Appraisal District Parcel R018638, Taylor, Williamson County, Texas. *Courtney Peres*
5. **PZ-2024-2291** Consider Disapproval of the Nyle Maxwell Minor Subdivision Plat, generally located at 14150 Hwy 79, legally described as 8.010 acres of land part of the William Kincaid

Survey, Abstract No. 0373, more particularly described by Williamson Central Appraisal District Parcels R311124, R019359, R331352, Taylor, Williamson County, Texas *Courtney Peres*

6. **PZ-2024-2215** Consider Conditional Approval of the RCR- Taylor Logistics Park Replat of Phase Two, Block Two, Lots 1, 2, and 3A, generally located at 201 FM 3349, legally described as RCR-Taylor Logistics Park Phase Two Minor Plat, Block Two, Lots 1-2 and RCR-Taylor Logistics Park Phase Two Replat, Block Two, Lot 3A, more particularly described by the Williamson Central Appraisal District Parcels R637527, R648674, R648675, Taylor, Williamson County, Texas. *Shai Roos*
7. **PZ-2024-2105** Consider Disapproval of Gateway Tract Preliminary Plat, generally located at 505 Carlos G. Parker SW, legally described as 38.09-acre tract of land part of and out of the George M. Reese Survey, Abstract No. 533, more particularly described by Williamson Central Appraisal District Parcels R331030, R019657, R020063, Taylor, Williamson County, Texas. *Shai Roos*

IV. REGULAR AGENDA

8. **PZ-2024-2238** Hold a public hearing and consider action regarding a request for a Subdivision Variances related to the installation of sidewalks generally located at 14501 Hwy 79, legally described as a 69.015-acre tract of land part of and out of the H.G. Johnson Survey, Section 6, Abstract No. 348, more particularly described by the Williamson Central Appraisal District Parcels R019338 and R019992, Taylor, Williamson County, Texas. *Courtney Peres*
9. **WITHDRAWN PZ-2024-2067** Hold a public hearing and consider making a recommendation regarding a request for an Neighborhood Plan generally located at 602 Carlos G Parker Blvd, legally described as a 9.735 acres tract of land part of and out of George M. Reese Survey, Abstract No. 533, more particularly described by the Williamson Central Appraisal District Parcels R311755 and R019660, Taylor, Williamson County, Texas. *Shai Roos*
10. **PZ-2024-2212** Hold a public hearing and consider making a recommendation regarding a request to allow an Infill Neighborhood Plan generally located at 1120 W. Lake Drive, to modify permitted building types and heights within the P3: Neighborhood place type, legally described as approximately 1.24 acres of land situated in Hague Heights, Block One, Lots 8 through 13, more particularly described by the Williamson Central Appraisal District Parcel R017151, Taylor, Williamson County, Texas. *Parker McDowell*

V. DISCUSSION ITEMS

11. Update regarding City Council actions on items referred by the Planning and Zoning Commission
12. Discussion on current processes for approval of short term rentals.

VI. ADJOURN

The Commission may vote and/or act upon each of the items listed in this Agenda. The Commission reserves the right to retire into executive session concerning any of the items listed on this Agenda, whenever it is considered necessary and legally Justified under the Open Meetings Act.

I certify that the notice of this meeting was posted in the City of Taylor Hall Lobby continuously for 72 continuous hours before October 8, 2024. I further certify that the following news media was notified of this meeting: Taylor Daily Press.

Posted by:  Date: October 4, 2024
Victoria Winchester, Administrative Assistant

MINUTES

CITY OF TAYLOR, TEXAS PLANNING AND ZONING COMMISSION MEETING

September 10, 2024, at 6:00 P.M.

City Hall Council Chambers

400 Porter Street, Taylor, TX 76574

<u>PRESENT</u>	<u>ABSENT</u>	<u>STAFF PRESENT</u>
Amy Everhart		Victoria Winchester, Administrative Assistant II
Nora Roy	Mike Eaton	Preston Gunn, Planner
Kellie Billings-Ray	Donna Frazier	Parker McDowell, Senior Planner Consultant
Annette Maruska	Barbara-Aviles	Shai Roos, Planning Consultant
Jim Newman	Torsberg	Scott Dunlop, Director of Development Services
Alexander Allrich		

I. CALL TO ORDER AND DECLARE A QUORUM

Chair, Amy Everhart called to order and declared a quorum at 6:00 P.M.

II. CITIZENS COMMUNICATION

(The Planning and Zoning Commission welcomes public comments on items not listed on the Agenda. However, the Commission cannot respond until an item is posted on a future meeting Agenda. Registration forms are available at the sign in table.)

Chair, Amy Everhart opened the citizens communication at this time.

City of Taylor Administrative Assistant, Victoria Winchester introduced the new Director of Development Services, Scott Dunlop.

III. CONSENT AGENDA

(The Consent Agenda includes non-controversial and routine items that the Commission may act on with one single vote. The Chair or a Commissioner may pull any item from the Consent Agenda to discuss and act upon it individually as part of the Regular Agenda.)

1. Review and approve minutes from the meeting held on August 13, 2024.
2. **PZ-2024-2088** Consider Disapproval of the Davis Tract Phase 1 Preliminary Plat, generally located north of Elm Branch Trail, legally described as approximately 25.52 acres of land part of the James C. Eaves Survey, Abstract No. 214, more particularly

described by the Williamson Central Appraisal District Parcel R656448, Taylor, Williamson County, Texas.

3. **PZ-2024-2143** Consider Disapproval of The Retreat at Mallard Final Plat, generally located near the intersection of Mallard Lane and Jason Drive, legally described as a 19.617-acre tract of land part of and out of the William J. Baker Survey, Abstract No. 65, more particularly described by Williamson Central Appraisal District Parcel R018622, Taylor, Williamson County, Texas.
4. **PZ-2024-2216** Consider Disapproval of the Taylor Pointe Subdivision Preliminary Plat generally located at 715 Carlos G. Parker Blvd. SW, consisting of approximately 39.015 acres of land part of the G.M. Reese Survey, Abstract No. 533, more particularly described by Williamson Central Appraisal District Parcel R331170, Taylor, Williamson County, Texas.
5. **PZ-2024-2254** Consider Disapproval of the Taylor North Bull Branch Creek Townhomes, Subdivision Improvement Plan, generally located along the South right-of-way of N.W. Carlos G. Parker Boulevard, approximately 1,400 feet west of the intersection of N.W. Carlos G. Parker Boulevard and North Drive, consisting of approximately 7.747 acres of land part of and out of the William J Baker Survey, Abstract No. 65, more particularly described by the Williamson Central Appraisal District Parcel R018638, Taylor, Williamson County, Texas.
6. **PZ-2024-2255** Consider Disapproval of the Gradient Technology Park/iMarket Final Plat, generally located at the intersection of Carlos G. Parker Boulevard Southeast and FM 112, legally described as a 211.84-acre tract of land in the Parthinia Coursey Survey, Abstract No. 131, more particularly described by Williamson Central Appraisal District Parcels R357492, R318771, R006142, R620065, R624522, R620064, Taylor, Williamson County, Texas.
7. **PZ-2024-2256** Consider Disapproval of the Gradient Technology Park/iMarket Subdivision Improvement Plan, generally located at the intersection of Carlos G. Parker Boulevard Southeast and FM 112, legally described as a 211.84-acre tract of land in the Parthinia Coursey Survey, Abstract No. 131, more particularly described by Williamson Central Appraisal District Parcels R357492, R318771, R006142, R620065, R624522, R620064, Taylor, Williamson County, Texas.
8. **PZ-2024-2258** Consider Disapproval of the Gradient Technology Park/iMarket Preliminary Plat, generally located at the intersection of Carlos G. Parker Boulevard Southeast and FM 112, legally described as a 211.84-acre tract of land in the Parthinia Coursey Survey, Abstract No. 131, more particularly described by Williamson Central Appraisal District Parcels R357492, R318771, R006142, R620065, R624522, R620064, Taylor, Williamson County, Texas.

Commissioner Jim Newman made a motion to approve the Consent Agenda as presented. Commissioner, Alexander Allrich seconded the motion. Motion passed unanimously (6-0).

IV. REGULAR AGENDA

9. **WITHDRAWN - PZ-2024-2249** Hold a public hearing and consider making a recommendation for a Special Use Permit to allow lodging without a full-time, on-site operator generally located at 120 Mammoth, legally described as Northpark Phase Five, Block B, Lot 15, approximately 0.18 acres, more particularly described by the Williamson Central Appraisal District Parcel R552906, Taylor, Williamson County, Texas. (Parker)
This agenda item was formally withdrawn from consideration by the applicant. A public hearing and a recommendation were not conducted.

10. **PZ-2024-2109** Hold a public hearing and consider making a recommendation regarding a request for an Infill Neighborhood Plan generally located at 215 Commercial Drive, legally described as Lake Aire Subdivision, Block One, Northern Part of Lot One, approximately 4.68 acres, more particularly described by Williamson Central Appraisal District Parcel R048754, Taylor, Williamson County, Texas (Parker)

Chair, Amy Everhart opened the public hearing at 6:04 PM.

Consultant Planner, Parker McDowell, made a presentation to consider making a recommendation of approval for an Infill Neighborhood Plan located at 215 Commercial Drive.

The applicant, Adam Smith, was present to answer questions or concerns regarding the Infill Neighborhood Plan. No further discussion was made.

Chair, Amy Everhart Closed the public hearing at 6:12 PM.

Motion was made by Vice Chair, Nora Roy, to approve the Infill Neighborhood Plan at 215 Commercial Drive as presented. Commissioner, Alexander Allrich seconded the motion. Motion passed unanimously (6-0).

11. **PZ-2024-2102** Hold a public hearing and consider making a recommendation regarding a request for an amendment to the Neighborhood Plan referred to as Davis Tract Phase 2, generally located west of Backwood Pass, legally described as approximately 99 acres of land, part of and out of the James C. Eaves Survey, Abstract No. 533, and George M. Reese Survey, Abstract No. 214, more particularly described by the Williamson Central Appraisal District Parcels R019951, R019181, R019651, Taylor, Williamson County, Texas. (Shai)

Chair, Amy Everhart opened the public hearing at 6:15 PM.

Consultant Planner, Shai Roos, made a presentation regarding an amendment to the Neighborhood Plan referred to as Davis Tract Ph 2. Staff recommend approval of the proposed neighborhood plan referred to as Davis Tract Phase Two Amendment with the stipulated warrants and variances.

Speaker, Steven Tinnin, a representative with the developer was present to answer any questions the board had regarding the Neighborhood Plan amendment.

Chair, Amy Everhart Closed the Public Hearing at 6:29 PM.

Commissioner Newman made a motion to approve the proposed neighborhood plan referred to as Davis Tract Phase Two Amendment with the stipulated warrants and variances. Commissioner, Alexander Allrich seconded the motion. Motion passed unanimously (6-0).

V. DISCUSSION ITEMS

12. Update regarding City Council actions on items referred to by the Planning and Zoning Commission.

Shai Roos, Senior Planner made a presentation regarding actions on items made by City Council Meeting.

VI. ADJOURN *Chair, Amy Everhart has adjourned the meeting at 6:31 PM.*

Approved by Chair: _____

Date: _____

Attest by Administrative Assistant: _____

Date: _____

City of Taylor
PZ-2024-2143
The Retreat at Mallard Final Plat
Staff Report

Item Details

Agenda No.	2
Requested Action:	Consider Approval of The Retreat at Mallard Final Plat
Address/Location:	Generally located along Mallard Lane
Legal Description:	19.617-acre tract of land part of and out of the William J. Baker Survey, Abstract No 65, more particularly described by Williamson Central Appraisal District Parcel R018622, Taylor, Williamson County, Texas.
Current Zoning:	P2.5 – Large Lot
Current Use:	Vacant Acreage
Applicant:	Trevor Tast, TX2 Engineering
Case History:	This is the third submission of the final plat.
Staff Recommendation:	The submitted Final Plat meets the requirements of the Land Development Code and Engineering Manual. Therefore, staff recommends Approval of the proposed Final Plat.

Attachments

1. Staff Comments
2. Location Map
3. Proposed Final Plat



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

Trevor Tast
TX2 Engineering.com
645 Floral Ave Ste C
New Braunfels TX 78130
Trevor@TX2engineering.com

Date: Tuesday, October 1, 2024

Address: Mallard Lane, Taylor 76574

Permit Number PZ-2024-2143

Dear Trevor Tast,

Staff has completed its review of plans for the The Retreat at Mallard Final Plat that is to be located at Mallard Lane, Taylor, TX 76574. Staff has recommended Approval of the Final Plat pending consideration by the Planning and Zoning Commission on October 8, 2024. Ensure all outstanding documents required by Engineering are completed and uploaded to the MyGovernment portal.

Planning Department Comments

The following comments have been provided by Courtney Peres. Should you have any questions or require additional information regarding any of these comments, please contact Courtney Peres by email at courtney.peres@taylortx.gov.

1. Comments resolved with resubmission on 9/13/24 - Approved.

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier Vasquez by email at javier.vasquez@hdrinc.com.

Previous comments have been addressed. Plat uploaded on 9/13/24 has been reviewed and approved with the following notes:

1. The outfall for the subdivision and maintenance within the existing drainage easement are being coordinated with the respective owner. Please provide final resolution of this item once an agreement and improvements (rip rap) have been completed.
2. Acceptance of the subdivision improvements shall be coordinated prior to final plat recordation.

Fire Department Comments

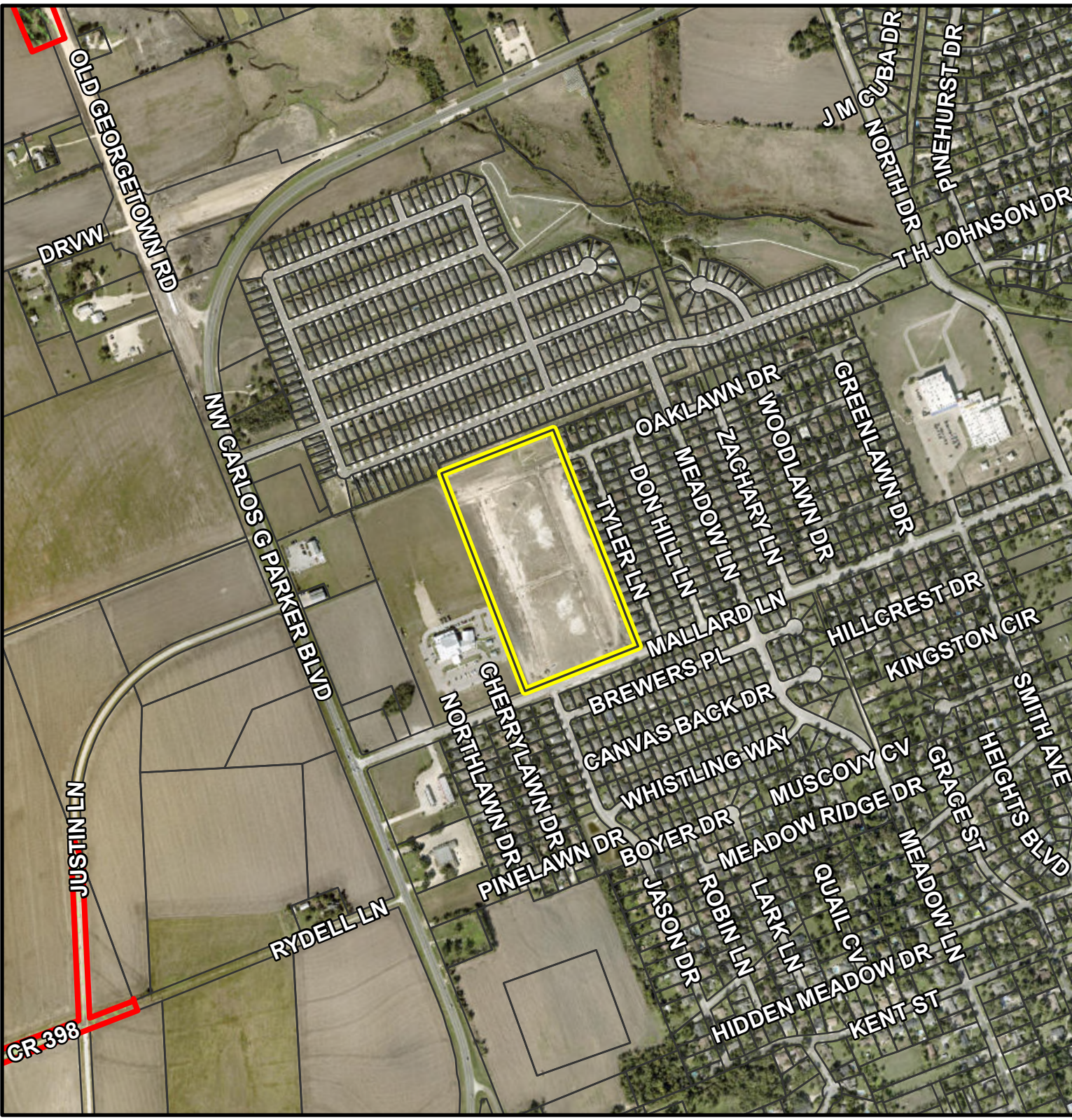
The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylortx.gov.

1. Previously reviewed with no comments.

Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs. Should you have questions or require additional information regarding the plan review process itself, please feel free to contact me directly. I can be reached by e-mail at courtney.peres@taylortx.gov.

Thank you,

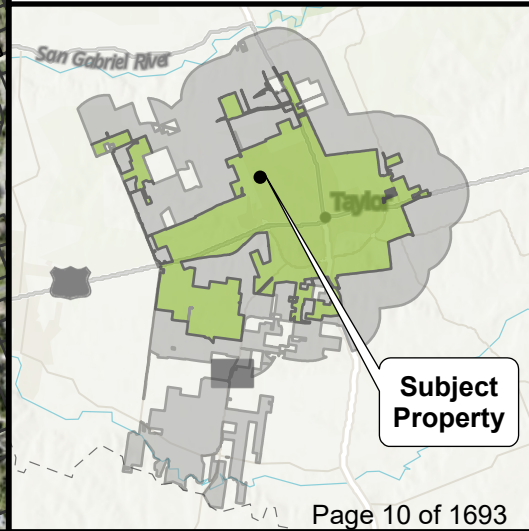
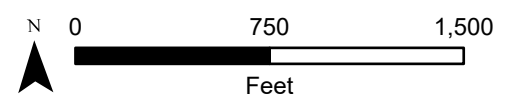
Courtney Peres, CNU-A
Planning Manager
City of Taylor



PZ-2024-2143

**Mallard Ln.
Final Plat
Location Map
Approximately 19.6 acres**

- City Limits
- ETJ Boundary
- Subject Property
- Parcels



STREET TYPE	STREET NAME	STREET LENGTH (FT)	PAVEMENT AREA (BC-BC)(SQ FT)
YIELD STREET	OAKLAWN DRIVE	671	19450
YIELD STREET	JASON DRIVE	1151	33392
YIELD STREET	CAYUGA DRIVE	1151	33369
YIELD STREET	PEKIN WAY	351	10168
TOTAL		3323	96379

ROW	STREET LENGTH (FT)	AREA (SQ. FT.)
TOTAL	3323	157772

DETENTION RESERVE	SQ. FT.
1	36687

LOT AND BLOCK TABLE		
BLOCK #	SQ. FT.	LENGTH (FT)
1	141987	1086
2	156418	521
3	159031	515
4	146505	1086
5	56146	671

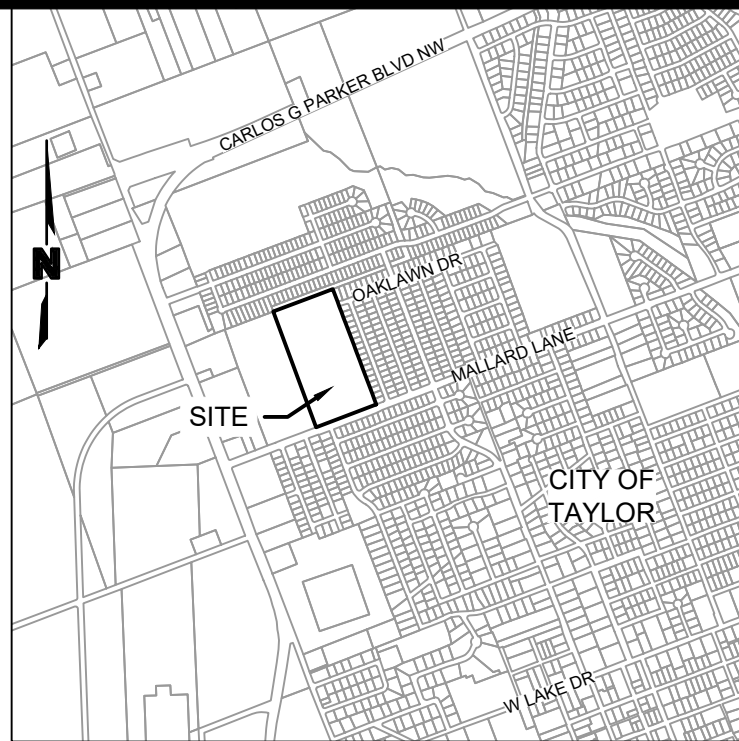
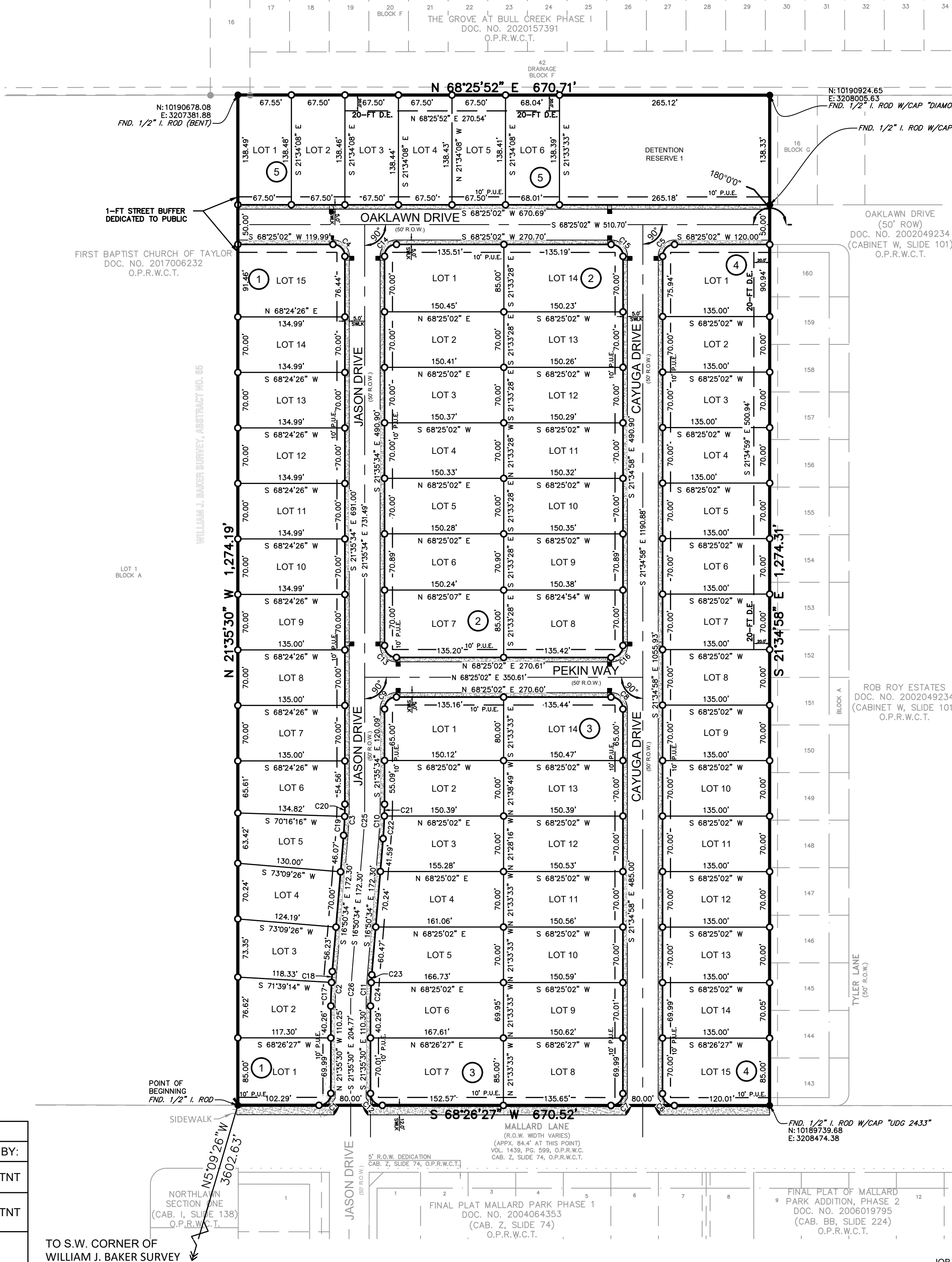
BLOCK 1		BLOCK 2		BLOCK 3		BLOCK 4		BLOCK 5	
LOT	SQ. FT.	LOT	SQ. FT.	LOT	SQ. FT.	LOT	SQ. FT.	LOT	SQ. FT.
1	9222	1	12745	1	11963	1	12229	1	9351
2	8610	2	10530	2	10512	2	9450	2	9347
3	8674	3	10527	3	10679	3	9450	3	9345
4	8897	4	10524	4	11072	4	9450	4	9345
5	8839	5	10521	5	11476	5	9450	5	9343
6	9153	6	10653	6	11711	6	9450		
7	9450	7	12721	7	14197	7	9450		
8	9450	8	12735	8	12756	8	9450		
9	9450	9	10660	9	10539	9	9450		
10	9449	10	10523	10	10540	10	9450		
11	9449	11	10521	11	10538	11	9450		
12	9449	12	10519	12	10532	12	9450		
13	9449	13	10517	13	10530	13	9450		
14	9449	14	12719	14	11988	14	9453		
15	12296					15	11427		

CURVE TABLE				
CURVE #	RADIUS	DIRECTION	LENGTH	CHORD
C1	15.00'	N23°25'29"E	23.57'	21.22'
C2	525.00'	N19°13'02"W	43.51'	43.50'
C3	475.00'	N19°13'02"W	39.37'	39.36'
C4	15.00'	N86°30'16"W	23.56'	21.21'
C5	15.00'	S23°25'02"W	23.56'	21.21'
C6	15.00'	S86°34'16"E	23.56'	21.21'
C7	15.00'	N23°25'44"E	23.57'	21.22'
C8	15.00'	N86°34'58"W	23.56'	21.21'
C9	15.00'	S23°24'44"W	23.56'	21.22'
C10	525.00'	S19°13'02"E	43.51'	43.50'
C11	475.00'	S19°13'02"E	39.37'	39.36'
C12	15.00'	S86°34'31"E	23.55'	21.21'
C13	15.00'	S86°35'14"E	23.56'	21.21'
C14	15.00'	S23°24'46"W	23.56'	21.21'
C15	15.00'	N86°34'58"W	23.56'	21.21'
C16	15.00'	N23°25'01"E	23.56'	21.21'
C17	525.00'	S19°58'08"E	29.74'	29.73'
C18	525.00'	S17°35'40"E	13.77'	13.77'
C19	475.00'	S18°17'09"E	23.93'	23.92'
C20	475.00'	S20°39'37"E	15.44'	15.44'
C21	525.00'	N20°46'40"W	14.91'	14.91'
C22	525.00'	N18°24'13"W	28.60'	28.60'
C23	475.00'	N17°25'53"W	9.76'	9.76'
C24	475.00'	N19°48'21"W	29.61'	29.60'
C25	500.00'	S19°13'02"E	41.44'	41.43'
C26	500.00'	S19°13'02"E	42.48'	42.46'

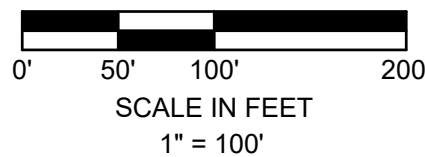
REVISION TABLE			
REV. NO.	DATE	DESCRIPTION	BY:
1	6/5/2024	RESUBMITTAL OF PLAT; UPDATED PER CITY STAFF COMMENTS	TNT
2	8/2/2024	RESUBMITTAL OF PLAT; UPDATED PER CITY STAFF COMMENTS	TNT

THE RETREAT AT MALLARD FINAL PLAT

BEING A FINAL PLAT CONSISTING OF APPROXIMATELY 19.62 ACRES, PART OF AND OUT OF THE WILLIAM J. BAKER SURVEY, ABSTRACT NO. 65, MORE CLEARLY DESCRIBED IN THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, DOCUMENT NUMBER 2021110292, TAYLOR, WILLIAMSON COUNTY, TEXAS.



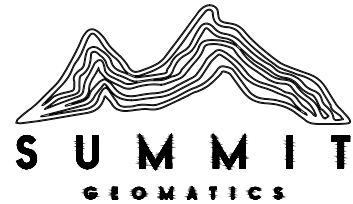
LOCATION MAP
SCALE: 1" = 2000'



LEGEND	
●	1/2" IRON ROD FOUND UNLESS OTHERWISE NOTED
■	SPINDLE FOUND
●	60D NAIL FOUND
⊙	4" FENCE POST FOUND
○	5/8" IRON ROD W/CAP STAMPED "SUMMIT GEOMATICS, INC" SET
D.E.	DRAINAGE EASEMENT
P.U.E.	PUBLIC UTILITY EASEMENT
---	ADJACENT PROPERTY LINES
---	EXISTING CONTOURS
---	SURVEY LINES
---	RIGHT-OF-WAY
O.P.R.W.C.T.	OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, TEXAS
---	PEDESTRIAN SIDEWALK
#	BLOCK NUMBER

SUMMIT GEOMATICS, INC.

4603 N Stahl Park Suite 103
San Antonio, Texas 78217
Tel: 210-971-4870 • summit-geomatics.com
TBPELS Firm No. 10194657
SGI Job No. 21.0004



DEVELOPMENT TEAM CONTACT INFORMATION		
OWNER		
	CASTLEROCK COMMUNITIES, LLC CONTACT: LANCE WRIGHT	2401 FOUNTAIN VIEW DR #215, HOUSTON, TX 77057 LWRIGHT@C-ROCK.COM
CIVIL ENGINEER		
	TREVOR TAST, P.E. TX2 ENGINEERING	645 FLORAL AVE, STE. C NEW BRAUNFELS, TX 78130 (816) 510-9151 TREVOR@TX2ENGINEERING.COM
SURVEYOR		
	ROBERT A. HARPER, RPLS, PLS SUMMIT GEOMATICS, INC.	4603 N. STAHL PARK SUITE 103 SAN ANTONIO, TX 78217 (210) 971-4870 RHARPER@SUMMIT-GEOMATICS.COM

JOB #: 021-001

SEP 05 2024

SHEET 1 OF 2

NOTES:

1. REFERENCE SHEET 1 FOR LINE TABLE & CURVE TABLE INFORMATION.
2. UTILITY PROVIDERS -- WATER: CITY OF TAYLOR; WASTEWATER: CITY OF TAYLOR; ELECTRICITY: ONCOR ELECTRIC DELIVERY COMPANY
3. ALL STRUCTURES AND/OR OBSTRUCTIONS ARE PROHIBITED IN DRAINAGE EASEMENTS.
4. THERE ARE NO AREAS WITHIN THE BOUNDARIES OF THIS SUBDIVISION IN THE 100-YEAR FLOODPLAIN AS DEFINED BY FIRM MAP NO 48491C0533F AND NO. 48491C0530F, EFFECTIVE DATE DECEMBER 20, 2019, AS PREPARED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY.
5. A 10-FT PUBLIC UTILITY EASEMENT (P.U.E.) SHALL BE IN ACCORDANCE WITH THE CITY OF TAYLOR ZONING ORDINANCE, AS AMENDED.
6. ALL MINIMUM YARD (BUILDING) SETBACKS SHALL BE IN ACCORDANCE WITH THE CITY OF TAYLOR ZONING ORDINANCE, AS AMENDED.
7. MINIMUM FIRE FLOW OF 500 GALLONS PER MINUTES FOR NOT LESS THAN TWO (2) HOURS SHALL BE PROVIDED.
8. BEARING ORIENTATION IS BASED UPON THE TEXAS COORDINATE SYSTEM, SOUTH CENTRAL ZONE 4204, NORTH AMERICAN DATUM OF 1983 (NAD 83), 2011 ADJUSTMENT, EPOCH 2010.00. DISTANCES SHOWN HEREON ARE IN SURFACE. TO CONVERT DISTANCES TO GRID MULTIPLY BY THE COMBINED SCALE FACTOR OF 0.9998392637.
9. THIS M&B DESCRIPTION WAS PREPARED IN CONJUNCTION WITH A SURVEY OF EVEN DATE.
10. MONUMENTS WERE FOUND OR SET AT EACH CORNER OF THE SURVEY BOUNDARY OF THE SUBDIVISION. MONUMENTS AND LOT MARKERS SET WITH 1/2" IRON PIN PLASTIC CAP LABELED SUMMIT GEOMATICS.
11. PROJECT NOTE: 64 LOTS, 5 BLOCKS BEING 19.617 ACRES.
12. PROPERTY OWNERS ARE ADVISED THAT THEY ARE RESPONSIBLE FOR MAINTENANCE OF DEDICATED EASEMENTS ON THEIR PROPERTY AND MAY NOT UTILIZE THESE EASEMENTS FOR ANY PURPOSE DETRIMENTAL TO THEIR INTENDED USE (I.E. NO STRUCTURES, SEPTIC TANKS FIELDS, ETC.). GRANTEES OF SAID DEDICATED EASEMENTS RESERVE THE RIGHT OF ACCESS TO SUCH EASEMENTS.
13. THE SUBDIVIDER IS RESPONSIBLE TO NOTE THE LOCATION OF SIDEWALKS ON THE PLAT. PUBLIC SIDEWALKS THAT ARE NOT IN FRONT OF INDIVIDUAL RESIDENTIAL LOTS MUST BE SHOWN ON THE PLAT/SUBDIVISION IMPROVEMENT PLANS & BUILD DURING SUBDIVISION IMPROVEMENT CONSTRUCTION. THE HOMEBUILDER SHALL BE RESPONSIBLE FOR CONSTRUCTING SIDEWALKS ADJACENT TO INDIVIDUAL HOUSE LOTS IN ALL RESIDENTIAL DISTRICTS AND THE SUBDIVIDER SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF ALL OTHER SIDEWALKS INCLUDING ALL REQUIRED ADA RAMPS. SIDEWALKS SHALL BE INSTALLED ON AT LEAST ONE (1) SIDE OF THE STREET. BE A MINIMUM OF FIVE (5) FEET IN WIDTH AND BE IN COMPLIANCE WITH THE AMERICANS WITH DISABILITIES ACT (ADA). ALL SIDEWALKS MUST BE IN COMPLIANCE WITH SPECIFICATIONS PROVIDED BY THE CITY ENGINEER.
14. ALL DRAINAGE AND DETENTION RESERVES SHOWN HEREIN ARE PRIVATE FACILITIES AND SHALL BE THE RESPONSIBILITY OF THE HOMEOWNERS ASSOCIATION. THE HOMEOWNERS ASSOCIATION SHALL BE REQUIRED TO MEET THE STANDARDS SPECIFIED BY THE CITY OF TAYLOR FOR DETENTION FACILITIES IN ACCORDANCE WITH CITY OF TAYLOR LAND DEVELOPMENT CODE CHAPTER 3.8.4 SECTION 2. THE CITY OF TAYLOR, TEXAS SHALL NOT BE RESPONSIBLE FOR MAINTENANCE ACTIVITIES FOR DRAINAGE AND DETENTION FACILITIES SHOWN HEREIN.
15. ALL STREET NAMES SHOWN HEREIN (OAKLAWN DRIVE, JASON DRIVE, PEKIN PLACE, AND CAYUGA COVE) HAVE BEEN APPROVED BY THE WILLIAMSON COUNTY 9-1-1 ADDRESSING DEPARTMENT.

THE RETREAT AT MALLARD FINAL PLAT

BEING A FINAL PLAT CONSISTING OF APPROXIMATELY 19.62 ACRES, PART OF AND OUT OF THE WILLIAM J. BAKER SURVEY, ABSTRACT NO. 65, MORE CLEARLY DESCRIBED IN THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, DOCUMENT NUMBER 2021110292, TAYLOR, WILLIAMSON COUNTY, TEXAS.

DESCRIPTION OF A 19.62 ACRE TRACT OF LAND
SITUATED IN THE WILLIAM J. BAKER SURVEY, ABSTRACT NO. 65
WILLIAMSON COUNTY, TEXAS

BEING A 19.62 ACRE TRACT OF LAND OUT OF THE WILLIAM J. BAKER SURVEY, ABSTRACT NO. 65, WILLIAMSON COUNTY, TEXAS, TRACT BEING ALL OF A CALLED 19.617 ACRE TRACT OF LAND TO CASTLE ROCK COMMUNITIES, L.P. RECORDED IN DOCUMENT NO. 2021110292 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS (O.P.R.W.C.T.) AND BEING DESCRIBED BY METES AND BOUNDS AS FOLLOWS:

BEGINNING AT A 1/2-INCH IRON ROD FOUND ON THE NORTH RIGHT-OF-WAY (R.O.W.) LINE OF MALLARD LANE (APPROXIMATELY 84.4' AT THIS POINT AS SHOWN IN FINAL PLAT MALLARD PARK PHASE 1, A SUBDIVISION PLAT RECORDED IN DOCUMENT NO. 2004064353 (CABINET Z, SLIDE 74), PLAT RECORDS OF WILLIAMSON COUNTY, TEXAS (P.R.W.C.T.)) BEING THE SOUTHEAST CORNER OF LOT 1, BLOCK A OF FIRST BAPTIST CHURCH OF TAYLOR, A SUBDIVISION PLAT RECORDED IN DOCUMENT NO. 2017006232 O.P.R.W.C.T., MARKING THE SOUTHWEST CORNER OF SAID 19.617 ACRE TRACT AND THE HEREIN DESCRIBED TRACT;

THENCE, NORTH 21°35'30" WEST, DEPARTING FROM THE NORTH RIGHT-OF-WAY LINE OF SAID MALLARD LANE, WITH THE COMMON LINE OF SAID LOT 1 AND SAID 19.617 ACRE TRACT, A DISTANCE OF 1,274.19 FEET TO A 1/2-INCH IRON ROD BENT FOUND ON THE SOUTH LINE OF LOT 42, DRAINAGE LOT, BLOCK F OF THE GROVE AT BULL CREEK PHASE I, A SUBDIVISION PLAT RECORDED IN DOCUMENT NO. 2020157391, O.P.R.W.C.T. BEING THE NORTHEAST CORNER OF SAID LOT 1 MARKING THE NORTHWEST CORNER OF SAID 19.617 ACRE TRACT AND THE HEREIN DESCRIBED TRACT;

THENCE, NORTH 68°25'52" EAST, WITH THE COMMON LINE OF SAID DRAINAGE LOT AND SAID 19.617 ACRE TRACT, A DISTANCE OF 670.71 FEET TO A 1/2-INCH IRON ROD WITH CAP STAMPED "DIAMOND SURVEYING" FOUND FOR THE NORTHWEST CORNER OF LOT 16, BLOCK "G" OF ROB ROY ESTATES, A SUBDIVISION PLAT RECORDED IN DOCUMENT NO. 2002049234 (CABINET W, SLIDE 102), P.R.W.C.T., MARKING THE NORTHEAST CORNER OF SAID 19.617 ACRE TRACT AND THE HEREIN DESCRIBED TRACT;

THENCE, SOUTH 21°34'58" EAST, ALONG AND WITH THE COMMON LINE OF SAID ROB ROY ESTATES AND SAID 19.617 ACRE TRACT, A DISTANCE OF 1,274.31 FEET TO A 1/2-INCH IRON ROD WITH CAP STAMPED "UDG 2433" FOUND ON THE NORTH RIGHT-OF-WAY LINE SAID MALLARD LANE, FOR THE SOUTHWEST CORNER OF LOT 143, BLOCK "A", MARKING THE SOUTHEAST CORNER OF SAID 19.617 ACRE TRACT AND THE HEREIN DESCRIBED TRACT;

THENCE, SOUTH 68°26'27" WEST, ALONG AND WITH THE NORTH RIGHT-OF-WAY LINE OF SAID MALLARD LANE AND THE SOUTH LINE OF SAID 19.617 ACRE TRACT, A DISTANCE OF 670.52 FEET TO THE POINT OF BEGINNING AND CONTAINING 19.62 ACRES OF LAND.

STATE OF TEXAS
COUNTY OF WILLIAMSON

KNOW ALL MEN BY THESE PRESENTS:

I, CASTLEROCK COMMUNITIES LLC, SOLE OWNER OF THE CERTAIN 19.62 ACRE TRACT OF LAND SHOWN HEREON AND DESCRIBED IN DEEDS RECORDED IN DOCUMENT #2021110292, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS OF THE CERTAIN TRACT OF LAND, AND DO HEREBY SUBVIDIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR THE STREETS, ALLEYS, RIGHTS-OF-WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS THE RETREAT AT MALLARD.

TO CERTIFY WHICH, WITNESS BY MY HAND THIS ____ DAY OF ____, 20__.

LANCE WRIGHT, CO-CEO
CASTLEROCK COMMUNITIES LLC
FORMERLY KNOWN AS CASTLEROCK COMMUNITITES LP

STATE OF TEXAS {
COUNTY OF WILLIAMSON { KNOW ALL MEN BY THE PRESENTS

BEFORE ME, THE UNDERSIGNED, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN TO ME TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS ____ DAY OF ____, 20__.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES _____

STATE OF TEXAS {
COUNTY OF BEXAR { KNOW ALL MEN BY THE PRESENTS

I, ROBERT A. HARPER, LICENSED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS PLAT IS TRUE AND CORRECTLY MADE FROM AN ACTUAL SURVEY MADE ON THE GROUND OF THE PROPERTY LEGALLY DESCRIBED HEREON, AND THAT THERE ARE NO APPARENT DISCREPANCIES, CONFLICTS, OVERLAPPING OF IMPROVEMENTS, VISIBLE UTILITY LINES, OR ROADS IN PLACE, EXCEPT AS SHOWN ON THE ACCOMPANYING PLAT, AND THAT THE CORNER MONUMENTS SHOWN THEREON WERE PROPERLY PLACED UNDER MY SUPERVISION IN ACCORDANCE WITH THE SUBDIVISION REGULATIONS OF THE CITY OF TAYLOR, TEXAS.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT _____
COUNTY, TEXAS, THIS ____ DAY OF ____, 20__.

ROBERT A. HARPER
RPLS NO. 6582 (SEALED)
SUMMIT GEOMATICS, INC.
4603 N. STAHL PARK SUITE 103
SAN ANTONIO, TEXAS 78217

I, TREVOR TAST, LICENSED PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS SUBDIVISION IS NOT ENCRoACHED BY A ZONE A FLOOD AREA, AS DENOTED HEREIN, AND AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD BOUNDARY MAP, COMMUNITY PANEL NUMBER 48491C0533F & 48491C0530F, EFFECTIVE DATE OF DECEMBER 20, 2019, AND THAT EACH LOT CONFORMS TO THE CITY OF TAYLOR REGULATIONS. THE FULLY DEVELOPED, CONCENTRATED STORMWATER RUNOFF RESULTING FROM THE ONE HUNDRED (100) YEAR FREQUENCY STORM IS CONTAINED WITHIN THE DRAINAGE EASEMENTS SHOWN AND/OR PUBLIC RIGHTS-OF-WAY DEDICATED BY THIS PLAT.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT _____
NEW BRAUNFELS, COMAL COUNTY, TEXAS, THIS ____ DAY OF ____, A.D., 20__.

TREVOR TAST
LICENSED PROFESSIONAL ENGINEER (SEALED)
NO. 124101 STATE OF TEXAS

DEVELOPMENT SERVICES DIRECTOR'S CERTIFICATION

I, SCOTT DUNLOP, DEVELOPMENT SERVICES DIRECTOR FOR THE CITY OF TAYLOR, TEXAS, DO HEREBY CERTIFY THIS PLAT IS APPROVED FOR FILING OF RECORD WITH THE COUNTY CLERK OF WILLIAMSON COUNTY, TEXAS

SCOTT DUNLOP, DIRECTOR DATE

PLANNING AND ZONING COMMISSION

THE SUBDIVISION TO BE KNOWN AS THE RETREAT AT MALLARD HAS BEEN ACCEPTED AND APPROVED, ACCORDING TO THE MINUTES OF THE MEETING OF THE TAYLOR PLANNING AND ZONING COMMISSION ON THE ____ DAY OF ____, 20__, A.D.

_____, DATE
_____, CHAIRMAN
_____, DATE
_____, SECRETARY

STATE OF TEXAS
COUNTY OF WILLIAMSON

I, NANCY E. RISTER, COUNTY CLERK OF SAID COUNTY, DO HEREBY CERTIFY THAT THE FOREGOING INSTRUMENT IN WRITING, WITH ITS CERTIFICATE OF AUTHENTICATION WAS FILED FOR RECORD IN MY OFFICE ON THE ____ DAY OF ____, 20__, A.D., AT ____ M. AND DULY RECORDED THIS THE ____ DAY OF ____, 20__, A.D., ____ O'CLOCK ____ M. IN THE OFFICIAL PUBLIC RECORDS OF SAID COUNTY, IN DOCUMENT # _____.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT THE COUNTY COURT OF SAID COUNTY, AT MY OFFICE IN GEORGETOWN, TEXAS, THE DATE LAST SHOWN ABOVE WRITTEN.

NANCY E. RISTER
CLERK COUNTY COURT
OF WILLIAMSON COUNTY, TEXAS

BY: _____
DEPUTY

DEVELOPMENT TEAM CONTACT INFORMATION		
OWNER		
	CASTLEROCK COMMUNITIES, LLC CONTACT: LANCE WRIGHT	2401 FOUNTAIN VIEW DR #215, HOUSTON, TX 77057 LWRIGHT@C-ROCK.COM
CIVIL ENGINEER		
	TREVOR TAST, P.E. TX2 ENGINEERING	645 FLORAL AVE, STE. C NEW BRAUNFELS, TX 78130 (816) 510-9151 TREVOR@TX2ENGINEERING.COM
SURVEYOR		
	ROBERT A. HARPER, RPLS, PLS SUMMIT GEOMATICS, INC.	4603 N. STAHL PARK SUITE 103 SAN ANTONIO, TX 78217 (210) 971-4870 RHARPER@SUMMIT-GEOMATICS.COM

**City of Taylor
PZ-2024-2216
Taylor Pointe Preliminary Plat
Staff Report**

Item Details

Agenda No.	3
Requested Action:	Consider Disapproval of Taylor Pointe Preliminary Plat
Address/Location:	Generally located at 715 Carlos G. Parker Blvd. SW
Legal Description:	Approximately 39.015 acres of land part of the G.M. Reese Survey, Abstract No. 533, more particularly described by Williamson Central Appraisal District Parcel R331170, Taylor, Williamson County, Texas.
Current Zoning:	P4 – Mix, P5 – Urban Center, EC – Employment Center, CS – Civic Space
Current Use:	Vacant Acreage
Applicant:	Sandy Brantley, Kirkman Engineering
Case History:	This is the third submission of the Preliminary Plat.
Staff Recommendation:	The submitted Preliminary Plat does not meet the minimum requirements of the Land Development Code and Engineering Manual. Therefore, staff recommends Disapproval of the proposed Preliminary Plat.

Attachments

1. Staff Comments
2. Location Map
3. Preliminary Plat Package



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

Sandy Brantley
Kirkman Engineering
1130 Cottonwood Creek Trail, Suite C3
Cedar Park TX 78613
sandy.brantley@trustke.com

Date: Monday, September 30,
2024

Address: 715 Carlos G Parker Blvd SW, Taylor 76574

Permit Number PZ-2024-2216

Dear Sandy Brantley,

Staff has completed its review of plans for the Taylor Pointe Subdivision Preliminary Plat that is to be located at 715 Carlos G Parker Blvd SW, Taylor 76574. Staff's recommendation is for Denial at the October 8, 2024, Planning and Zoning Commission Meeting. There are some corrections that need to be made prior to recordation of the plat. A comment resolution meeting is strongly recommended to address the remaining comments. Comments from this review follow.

Planning Department Comments

The following comments have been provided by Parker McDowell. Should you have any questions or require additional information regarding any of these comments, please contact Parker McDowell at (512) 309-6797 or by email at parker.mcdowell@hdrinc.com.

1. Revise owner information on the first page to include authorized agents and title.
2. add the topography lines back to the plat in a light greyscale
3. Street table is missing the thoroughfare type classification. Refer to approved neighborhood plan.
4. Add Lot 2, Block A to phase one of the phasing plan. Easements shown a fire lane, utility, and drainage easement for Lot 1 that cross over lot 2 from Feather street that will need to be done at the same time.
5. Do not use color on the phasing plan. County scans everything in black and white, use Hatching.
6. field notes still call it 38.979 acres. update notes or revise title acreage.
7. See attached Urban Design Consultant Comments - separate PDF.

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier Vasquez at (713) 576-3691 or by email at javier.vasquez@hdrinc.com.

Preliminary Plat - Engineering Review 03

Sheet 1 of 7

1. Sheet - [1] 1 PRELIM PLAN - Replace and Label all contours
2. Sheet - [1] 1 PRELIM PLAN - Verify and provide dedication of fire alleys on the plat taken from the neighborhood plan (Typical)
3. Sheet - [1] 1 PRELIM PLAN - Verify placement of parking area between lot 5/6 of Block D. Regulating Plan shows a space between these lots for a 20-ft fire apparatus access
4. Sheet - [1] 1 PRELIM PLAN - Caden Way to terminate at Civic Space per Regulating Plan. Sidewalk to be provided in Civic Space to connect pedestrian pathway.
5. Sheet - [1] 1 PRELIM PLAN - Will Lot 2, Block H Civic Space provide enough acreage for a pool plaza for the multifamily lots?
6. Sheet - [1] 1 PRELIM PLAN - Street type to follow engineering manual terminology (e.g. neighborhood street, commercial street, etc.)

Sheet 3 of 7

1. Sheet - [3] 3 EASEMENTS - Verify and provide dedication of fire alleys on the plat taken from the neighborhood plan (Typical)
2. Sheet - [3] 3 EASEMENTS - Where are the limits of the proposed 20-ft water easement?
3. Sheet - [3] 3 EASEMENTS - Label this easement consistently for each block
4. Sheet - [3] 3 EASEMENTS - Recommend sliding plat drawing up to allow more room along FM 973 or aligning north limits with sheet north

Sheet 5 of 7

1. Sheet - [5] 5 PHASING - Phasing to be coordinated for formatting and layout
2. Sheet - [5] 5 PHASING - How will Lot 1, Blk A have access with Lot 2 platted in Phase 2

Sheet 7 of 7

1. Sheet - [7] 7 SIGS - Licensed
2. Sheet - [7] 7 SIGS - Review and revise engineer certificate language
3. Sheet - [7] 7 SIGS - Bryan
4. Sheet - [7] 7 SIGS - Preliminary plat P&Z certificate to be used
5. Sheet - [7] 7 SIGS - Provide the Director of Development Services certificate

[*Please see the Associated Documents section of the permit page to download a copy of the study PDF containing the following engineering comments:](#)

Drainage Study - Engineering Review 03

1. General - Information for sites (e.g. buildings , paving, etc.) on plan sheet exhibits to state as shown for information purposes only on how drainage was calculated and is not being considered for any type of site or building plan approvals.
2. Page 5 of 19 - 3.1 Drainage Impacts - Please provide calculations to demonstrate that the post-development 2yr peak Q will be restricted to Qcritical and that the pond volume is sufficient.
3. Page 10 of 19 - Proposed Drainage Area Map - Please update the proposed DA map to match the tables in Sheet V1.0 and the latest engineering plans.
4. Page 11 of 19 - Detention Exhibit - Pursuant to EM 3.6.5.B, CNs should be reflective of undeveloped conditions and only the impervious % is updated between existing and proposed conditions to not double count impervious cover. Please verify compliance and update table and model accordingly.
5. [Page 11 of 19 - Detention Exhibit](#) - Please verify the impervious cover for DA-5.
6. [Page 11 of 19 - Detention Exhibit](#) - The lag times don't match the engineering plans. Please verify and update the table and model accordingly.
7. [Page 11 of 19 - Detention Exhibit](#) - Please correct 100-yr WSEL

*[*Please see the Associated Documents section of the permit page to download a copy of the Memo PDF containing the following engineering comments:](#)*

Traffic Impact Analysis (TIA) - Engineering Review 03

1. *Please download a copy of the review memo prepared by HDR dated 9/24/24 to detail comments and response to applicant's responses for 16 comments pertaining to the TIA Report review. Also provided is a PDF containing the scoping of the TIA from 2/1/24.*

*[*Please see the Associated Documents section of the permit page to download a copy of the PDF containing the following engineering comments:](#)*

Utility Schematics - Engineering Review 03 (Submittal 02)

Sheet 1 of 2

1. *Utility locations are to follow the ROW assignments found in the Taylor Engineering Manual Chpt. 2.2.*
2. *Verify if 12-inch water line is required for entire development. What is the estimated demand for the*

development.

3. *Water line to be placed to perimeter line to allow for continuation to adjacent property.*
4. *Verify to have to this water line routed along the front perimeter in a dedicated easement.*

Sheet 2 of 2

1. *Utility locations are to follow the ROW assignments found in the Taylor Engineering Manual Chpt. 2.2.*
2. *An alternative connection to the line here may be required due to topography for these 3 locations along Carlos G. Parker.*

Fire Department Comments (PZ)

The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylor.tx.gov.

Reviewed the plat notes on the resubmitted plans and the spelling has been corrected on plat note #7 from plans with upload date of 9/13/24.

Please revise the plat to address the comments noted above. Following revision, submit the plat electronically with a resolution letter in PDF format.

Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs. Should you have questions or require additional information regarding the plan review process itself, please feel free to contact me directly. I can be reached by telephone at (512) 309-6797, or by e-mail at parker.mcdowell@hdrinc.com.

Thank you,



Parker McDowell

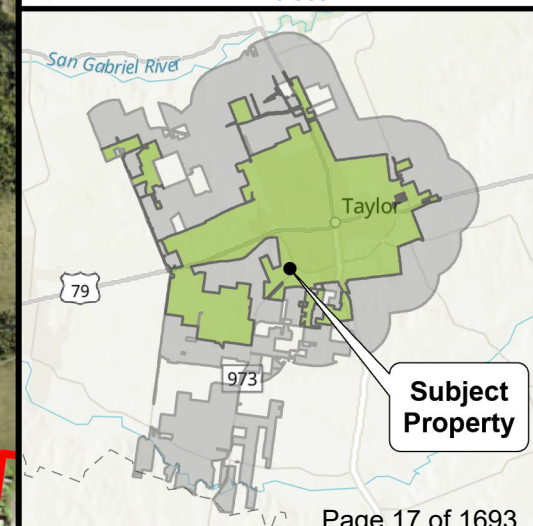
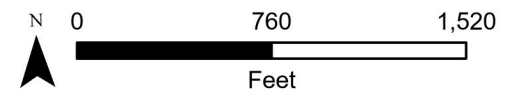
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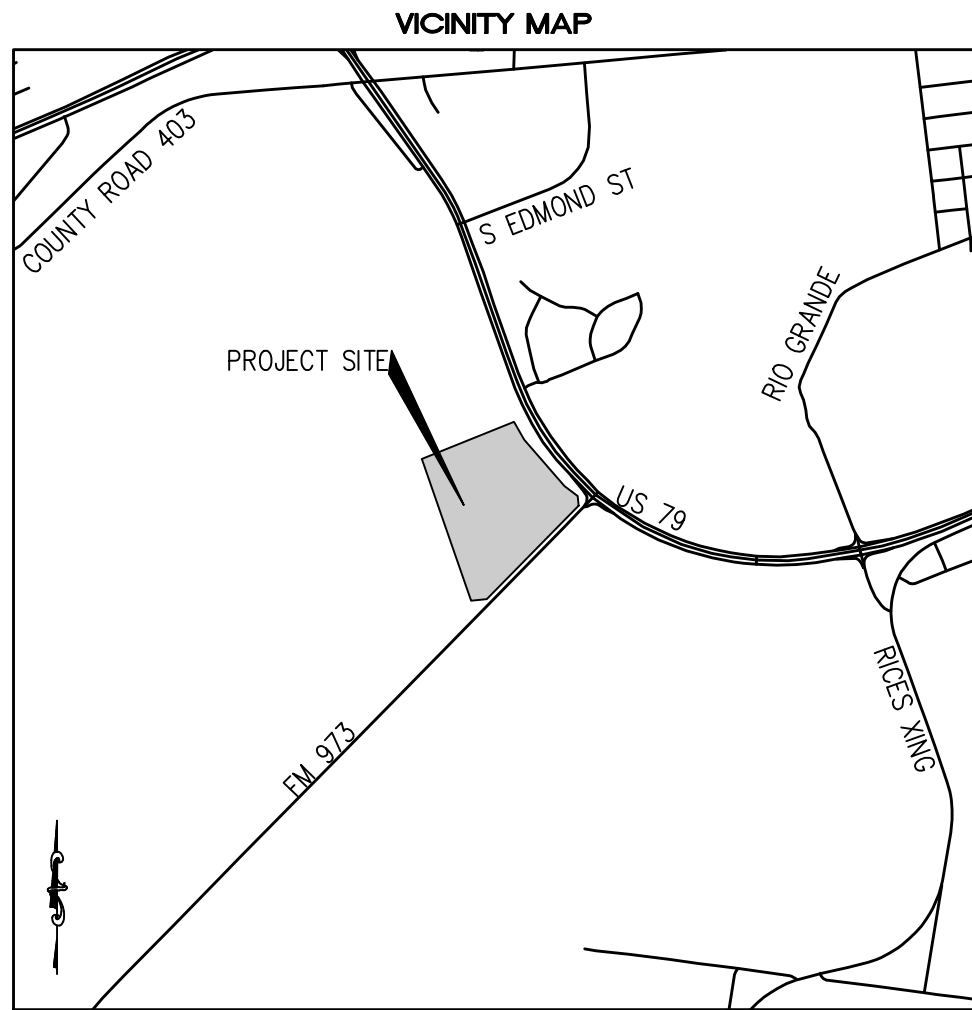


PZ-2024-2216

**715 Carlos G Parker Blvd.
Preliminary Subdivision Plat
Location Map
Approximately 39 acres**

- City Limits
- ETJ Boundary
- Subject Property
- Parcels





SCALE: 1" = 2000'

TAYLOR POINTE SUBDIVISION PRELIMINARY PLAT
BEING A PRELIMINARY PLAT CONSISTING OF 39.015 ACRES, INCLUDING 6.05 ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE 67 LOTS, 7 BLOCKS, 0 RESERVES, PART OF LAND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT NO. 533, TAYLOR, WILLIAMSON COUNTY, TEXAS

LEGEND

- IRON ROD FOUND
- IRON ROD SET
- BENCHMARK

ABBREVIATIONS

FND. — FOUND
I.R. — IRON ROD
N.F. — NOT FOUND
P.O.B. — POINT OF BEGINNING
R.O.W. — RIGHT-OF-WAY
DRWC — DEED RECORDS OF WILLIAMSON COUNTY, TEXAS
ORWC — OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS
OPRWC — OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS
D W/VL — DEED WITH VENDOR'S LIEN
WCR — WILLIAMSON COUNTY ROAD
{xx.xx} — RECORD CALL

OWNERS:
FASH TAYLOR, LTD.
DOSS DEVELOPMENT, LLC
MILAN PROPERTY GROUP LLC
4637 STONE OAK DRIVE
CARROLLTON, TX 75010

SURVEYOR:
BRUCE BRYAN, RPLS
BRYAN TECHNICAL SERVICES, INC.
911 NORTH MAIN
TAYLOR, TEXAS 76574
BRUCE@BRYANTECHNICALSERVICES.COM

ENGINEER:
JONATHAN E. SCHINDLER
5200 STATE HIGHWAY 121
COLLEYVILLE, TX 76034

TBPE FIRM NO.F-15874
SANDY.BRANTLEY@TRUSTKE.COM

PATENT SURVEY:
GEORGE M. REESE SURVEY
ABSTRACT NO. 533

TOTAL NUMBER OF BLOCKS: 7

TOTAL NUMBER OF LOTS: 67

LINEAR FEET OF NEW STREETS: 4746

TOTAL ACREAGE: 39.015 ACRES

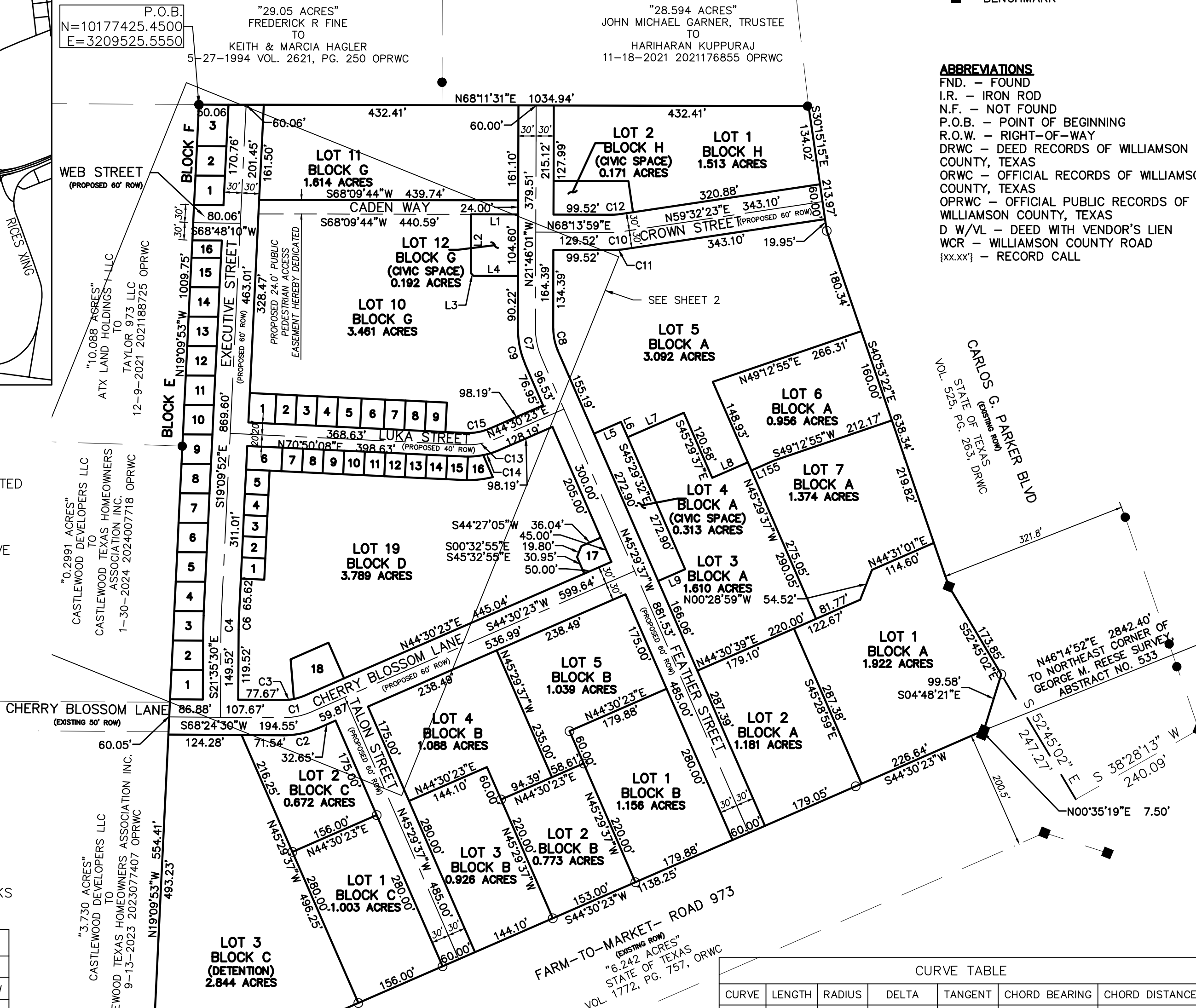
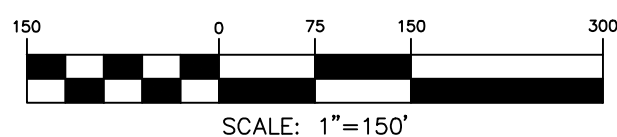
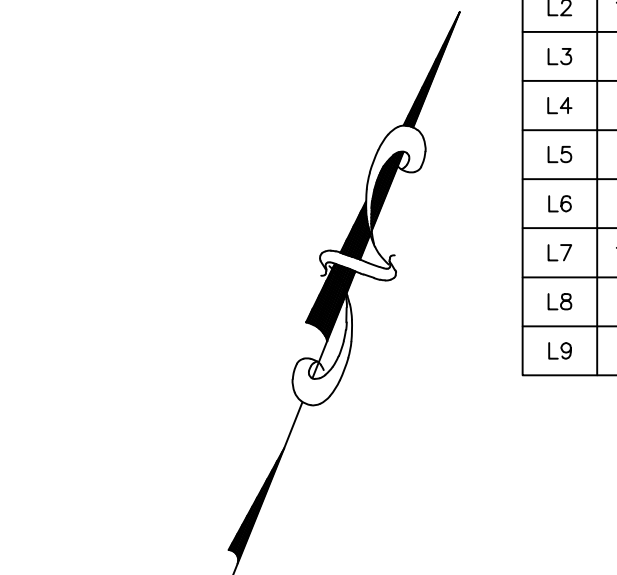
SUBMITTAL DATE:
JULY 5, 2024

BENCHMARK DESCRIPTION:
COTTON SPIKE IN POWER POLE
N = 10177315.040'
E = 3210860.312'
ELEVATION = 607.42
(NAVD 88, GEOD MODEL 2012)

PLAT NOTES:

- UTILITY PROVIDERS —WATER: CITY OF TAYLOR ; WASTEWATER: CITY OF TAYLOR ; ELECTRICITY: ONCOR.
- ALL STRUCTURES AND/OR OBSTRUCTIONS ARE PROHIBITED IN DRAINAGE EASEMENTS.
- THERE ARE NOT AREAS WITHIN THE BOUNDARIES OF THIS SUBDIVISION IN THE 100-YEAR FLOODPLAIN AS DEFINED BY FIRM MAP NUMBER 48491C0541F, EFFECTIVE DATE OF 12/20/2019
- A 10-FT PUBLIC UTILITY EASEMENT (P.U.E.) SHALL BE DEDICATED ALONG ALL STREET FRONTAGES.
- ALL MINIMUM YARD (BUILDING) SETBACKS SHALL BE IN ACCORDANCE WITH THE CITY OF TAYLOR ZONING ORDINANCE, AS AMENDED.
- PARKLAND DEDICATION REQUIREMENTS ARE MET BY: PAYMENT OF FEES IN LIEU PER RESIDENTIAL LOT, PAYABLE AT THE TIME OF FINAL PLAT RECORDATION.
- THE FIRE FLOW SHALL BE ACCORDING TO THE MINIMUM FIRE FLOW REQUIREMENTS PER THE ADOPTED 2018 INTERNATIONAL FIRE CODE.
- DEVELOPMENT PLAN SHALL BE IN ACCORDANCE WITH ORDINANCE 2024-12 — 715 CARLOS G PARKER BLVD. NEIGHBORHOOD PLAN.
- SEE SHEET 3 FOR BEARING AND DISTANCES OF EASEMENTS DEDICATED BY THIS PLAT.
- SEE SHEET 2 FOR LOT TABLE.
- SEE SHEET 2 FOR INSET OF SMALL LOTS WITHIN BLOCKS D, E, F, AND G.

LINE TABLE		
LINE	LENGTH	BEARING
L1	80.00	S68°13'59"W
L2	100.02	S21°46'01"E
L3	6.29	S65°07'38"E
L4	75.68	N68°13'59"E
L5	50.00	N44°30'23"E
L6	28.35	S45°29'32"E
L7	104.00	N44°30'23"E
L8	66.00	N44°30'23"E
L9	49.99	S44°30'23"W



STREET TABLE			
STREET NAME	ROW WIDTH	LENGTH	DESIGNATION
CHERRY BLOSSOM LANE	60'-0"	841.0 LF	THOROUGHFARE
EXECUTIVE STREET	60'-0"	1,039.9 LF	THOROUGHFARE
TALON STREET	60'-0"	485.0 LF	THOROUGHFARE
LUKA STREET	40'-0"	579.2 LF	THOROUGHFARE
FEATHER STREET	60'-0"	1,309.9 LF	THOROUGHFARE
CROWN STREET	60'-0"	490.5 LF	THOROUGHFARE
WEB STREET	60'-0"	80.0 LF	THOROUGHFARE
CADON WAY	24'-0"	502.2 LF	PEDESTRIAN PATHWAY

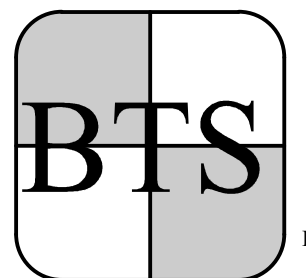
CURVE TABLE						
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD BEARING	CHORD DISTANCE
C1	47.14	113.00	023°54'08"	23.92	S56°27'26"W	46.80
C2	59.66	143.00	023°54'08"	30.27	N56°27'26"E	59.22
C3	16.99	83.00	011°43'32"	8.52	N62°32'44"E	16.96
C4	21.94	518.00	002°25'38"	10.97	S20°22'41"E	21.94
C5	23.21	548.00	002°25'38"	11.61	S20°22'41"E	23.21
C6	20.67	488.00	002°25'38"	10.34	S20°22'41"E	20.67
C7	48.87	118.00	023°43'37"	24.79	N33°37'49"W	48.52
C8	36.44	88.00	023°43'37"	18.49	S33°37'49"E	36.18
C9	60.46	146.00	023°43'37"	30.67	N33°37'49"W	60.03
C10	17.90	118.00	008°41'37"	8.97	N63°53'11"E	17.89
C11	22.46	148.00	008°41'37"	11.25	S63°53'11"W	22.43
C12	13.35	88.00	008°41'37"	6.69	N63°53'11"E	13.34
C13	52.39	114.00	026°19'45"	26.66	N57°40'15"E	51.93
C14	29.04	134.00	012°25'00"	14.58	S50°42'52"W	28.98
C15	43.20	94.00	026°19'45"	21.99	N57°40'15"E	42.82

SHEET 1 OF 7

PRELIMINARY PLAT OF
TAYLOR POINTE SUBDIVISION

39.015 ACRE TRACT OUT OF
THE GEORGE M. REESE SURVEY
ABSTRACT NO. 533
WILLIAMSON COUNTY, TEXAS

BRYAN TECHNICAL SERVICES, INC.



911 NORTH MAIN
TAYLOR, TX 76574

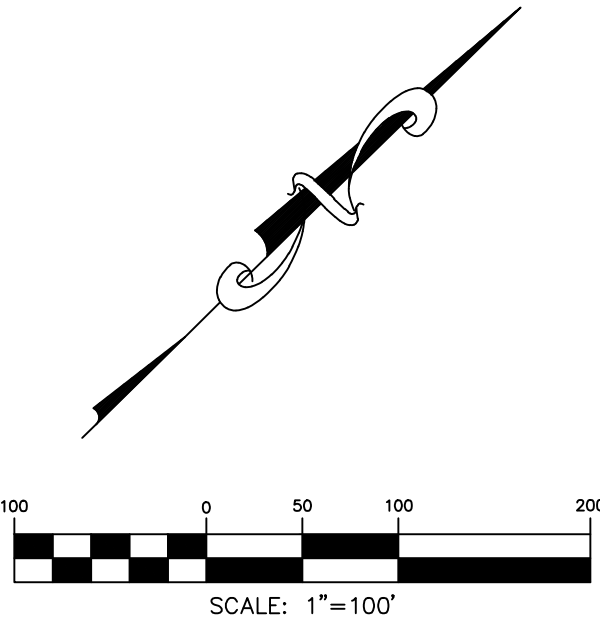
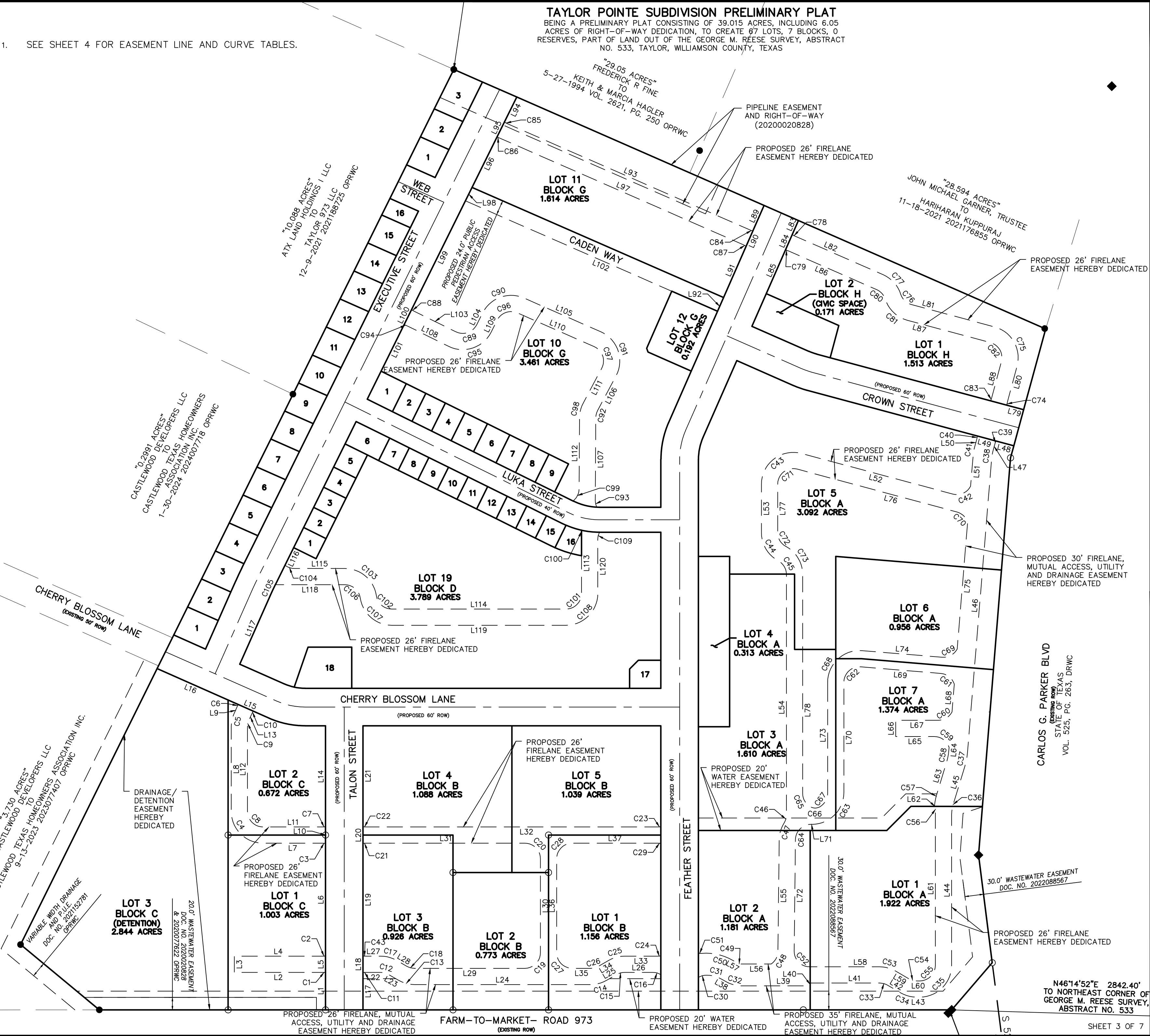
PHONE: (512) 352-9090
FAX: (512) 352-9091

FIRM No. 10128500
surveying@austin.rr.com
www.bryantechnicalservices.com

NO.	DATE	REVISIONS	BY

DRAWN BY: AMR	CHECKED BY: BLB
SCALE: 1" = 150'	APPROVED BY: BLB
PROJECT NO. 24-159	DATE: JUNE 5, 2024

1. SEE SHEET 4 FOR EASEMENT LINE AND CURVE TABLES.



LEGEND

- IRON ROD FOUND
- IRON ROD SET
- BENCHMARK

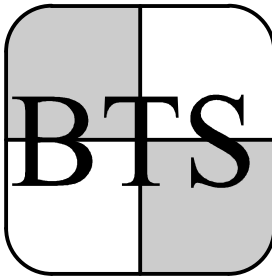
ABBREVIATIONS

FND. - FOUND
I.R. - IRON ROD
N.F. - NOT FOUND
P.O.B. - POINT OF BEGINNING
R.O.W. - RIGHT-OF-WAY
DRWC - DEED RECORDS OF WILLIAMSON COUNTY, TEXAS
ORWC - OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS
OPRWC - OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS
D W/VL - DEED WITH VENDOR'S LIEN
WCR - WILLIAMSON COUNTY ROAD
{xx.xx} - RECORD CALL

PRELIMINARY PLAT OF
TAYLOR POINTE SUBDIVISION

39.015 ACRE TRACT OUT OF
THE GEORGE M. REESE SURVEY
ABSTRACT NO. 533
WILLIAMSON COUNTY, TEXAS

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NO.	DATE	REVISIONS	BY

DRAWN BY: AMR	CHECKED BY: BLB
SCALE: 1" = 100'	APPROVED BY: BLB
PROJECT NO. 24-159	DATE: JUNE 5, 2024

TAYLOR POINTE SUBDIVISION PRELIMINARY PLAT
BEING A PRELIMINARY PLAT CONSISTING OF 39.015 ACRES, INCLUDING 6.05
ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE 67 LOTS, 7 BLOCKS, 0
RESERVES, PART OF LAND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT
NO. 533, TAYLOR, WILLIAMSON COUNTY, TEXAS

LINE TABLE		
LINE	LENGTH	BEARING
L1	59.51	N45°29'37"W
L2	141.00	S44°30'23"W
L3	26.00	N45°29'00"W
L4	141.00	N44°30'23"E
L5	27.01	N45°29'37"W
L6	179.97	N45°29'37"W
L7	95.00	S44°30'23"W
L8	135.30	N45°29'37"W
L9	12.71	N21°35'30"W
L10	27.01	N45°29'37"W
L11	95.00	N44°30'23"E
L12	135.30	S45°29'37"E
L13	12.71	S21°35'30"E
L14	161.49	N45°29'37"W
L15	71.54	N68°24'31"E
L16	124.28	N68°24'33"E
L17	59.49	N45°29'37"W
L18	27.01	N45°29'37"W
L19	179.99	N45°29'37"W
L20	27.01	N45°29'37"W
L21	161.49	N45°29'37"W
L22	23.40	N44°30'24"E
L23	18.64	N74°30'16"E
L24	282.12	N44°30'24"E
L25	18.64	N14°30'24"E
L26	45.19	N44°30'24"E
L27	23.40	S44°30'24"W
L28	18.37	S74°30'16"W
L29	176.29	S44°30'25"W
L30	150.50	S45°29'37"E
L31	254.10	N44°30'23"E
L32	466.99	S44°30'23"W
L33	52.62	S44°30'24"W
L34	18.64	S14°30'24"W
L35	29.56	S44°30'24"W
L36	150.50	N45°29'37"W
L37	136.88	N44°30'23"E
L38	7.11	N74°30'24"E
L39	109.48	N44°31'01"E
L40	4.50	N45°28'59"W
L41	111.44	N44°31'01"E
L42	23.95	N74°51'00"E
L43	5.00	N44°31'01"E
L44	241.34	N45°28'59"W
L45	58.09	N28°33'56"W
L46	473.30	N40°47'05"W
L47	8.00	N30°27'37"W
L48	29.57	S59°32'23"W
L49	31.01	S59°32'23"W
L50	11.34	S30°27'37"E

LINE TABLE		
LINE	LENGTH	BEARING
L51	47.00	S40°47'05"E
L52	250.69	S59°32'23"W
L53	77.66	S45°29'37"E
L54	384.96	S45°29'37"E
L55	179.19	S44°50'36"E
L56	47.14	S44°31'01"W
L57	7.11	S74°30'24"W
L58	111.44	N44°31'01"E
L59	23.95	N74°51'00"E
L60	5.00	N44°31'01"E
L61	245.21	N45°28'59"W
L62	2.01	S44°31'01"W
L63	55.38	N28°33'56"W
L64	6.64	N40°47'05"W
L65	61.69	S44°31'01"W
L66	26.00	N45°29'37"W
L67	68.03	N44°31'01"E
L68	44.85	N40°47'05"W
L69	128.55	S49°12'55"W
L70	176.35	S45°29'37"E
L71	1.49	S44°33'01"W
L72	178.32	S44°50'36"E
L73	176.35	N45°29'37"W
L74	128.55	N49°12'55"E
L75	203.87	N40°47'05"W
L76	264.53	S59°32'23"W
L77	77.66	S45°29'37"E
L78	350.56	S45°29'37"E
L79	27.86	S59°32'23"W
L80	59.94	S30°25'56"E
L81	137.69	N59°34'04"E
L82	154.33	N68°11'23"E
L83	24.51	N21°46'01"W
L84	27.01	N21°46'01"W
L85	76.47	S21°46'01"E
L86	154.38	S68°11'23"W
L87	137.69	S59°34'04"W
L88	59.97	N30°25'56"W
L89	71.46	N21°46'01"W
L90	116.65	N21°46'01"W
L91	116.65	N21°46'01"W
L92	24.00	N21°46'01"W
L93	423.32	N68°09'44"E
L94	44.46	N19°09'52"W
L95	27.09	N19°09'52"W
L96	89.95	N19°09'52"W
L97	426.77	N68°09'44"E
L98	24.02	S19°43'21"E
L99	194.33	S19°05'44"E
L100	27.01	S19°09'52"E

LINE TABLE		
LINE	LENGTH	BEARING
L101	83.11	S19°09'52"E
L102	440.59	S68°09'44"W
L103	74.28	S70°50'08"W
L104	21.28	S18°53'57"E
L105	134.39	S68°13'59"W
L106	62.99	N17°29'46"W
L107	132.18	N45°10'53"W
L108	74.28	N70°50'08"E
L109	21.28	N18°53'57"W
L110	134.39	N68°13'59"E
L111	62.99	S17°29'46"E
L112	113.65	S45°10'53"E
L113	97.34	S45°29'37"E
L114	278.83	S44°29'42"W
L115	80.57	S44°30'29"W
L116	31.40	S19°09'52"E
L117	119.52	N21°35'30"W
L118	111.37	N44°30'27"E
L119	278.83	N44°29'42"E
L120	86.98	N45°29'37"W
L149	80.06	S68°48'10"W

CURVE TABLE						
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD BEARING	CHORD DISTANCE
C1	5.03	25.00	011°32'13"	2.53	S50°16'29"W	5.03
C2	5.03	25.00	011°32'13"	2.53	N38°44'16"E	5.03
C3	5.03	25.00	011°32'13"	2.53	S50°16'29"W	5.03
C4	80.11	51.00	090°00'00"	51.00	S89°30'23"W	72.12
C5	21.28	51.00	023°54'07"	10.79	N33°32'34"W	21.12
C6	3.89	25.00	008°54'47"	1.95	N26°02'53"W	3.89
C7	5.03	25.00	011°32'13"	2.53	N38°44'16"E	5.03
C8	39.27	25.00	090°00'00"	25.00	N89°30'23"E	35.36
C9	10.43	25.00	023°54'07"	5.29	S33°32'34"E	10.35
C10	3.89	25.00	008°54'47"	1.95	S17°08'07"E	3.89
C11	5.04	25.00	011°32'27"	2.53	N38°44'11"E	5.03
C12	13.09	25.00	029°59'51"	6.70	N59°30'20"E	12.94
C13	26.70	51.00	029°59'52"	13.66	N59°30'20"E	26.40
C14	26.70	51.00	030°00'00"	13.67	N29°30'24"E	26.40
C15	13.09	25.00	030°00'00"	6.70	N29°30'24"E	12.94
C16	13.67	25.00	031°19'54"	7.01	N60°10'22"E	13.50
C17	26.70	51.00	029°59'51"	13.66	S59°30'20"W	26.40
C18	13.61	26.00	029°59'51"	6.97	S59°30'20"W	13.46
C19	39.27	25.00	090°00'01"	25.00	S00°29'36"E	35.36
C20	39.27	25.00	090°00'01"	25.00	N89°30'23"E	35.36
C21	5.03	25.00	011°32'19"	2.53	N38°44'13"E	5.03
C22	5.03	25.00	011°32'17"	2.53	S50°16'31"W	5.03
C23	5.03	25.00	011°32'13"	2.53	S38°44'16"W	5.03
C24	5.62	25.00	012°52'13"	2.82	S38°04'18"W	5.60
C25	26.70	51.00	030°00'00"	13.67	S29°30'24"W	26.40
C26	13.09	25.00	030°00'00"	6.70	S29°30'24"W	12.94
C27	39.27	25.00	089°59'59"	25.00	S89°30'24"W	35.36
C28	39.27	25.00	089°59'59"	25.00	N00°29'37"W	35.36
C29	5.03	25.00	011°32'13"	2.53	N50°16'29"E	5.03
C30	13.56	25.00	031°05'05"	6.95	N28°53'40"E	13.40
C31	26.21	49.89	030°06'18"	13.42	N59°29'21"E	25.91
C32	26.69	51.00	029°59'23"	13.66	N59°30'43"E	26.39
C33	13.24	25.00	030°19'59"	6.78	N59°41'00"E	13.08
C34	27.00	51.00	030°19'59"	13.82	N59°41'00"E	26.69
C35	80.11	51.00	090°00'00"	51.00	N00°28'59"W	72.12
C36	22.78	59.24	022°02'08"	11.53	N34°27'55"W	22.64
C37	21.33	100.00	012°13'09"	10.70	N34°40'31"W	21.29
C38	15.95	73.22	012°29'08"	8.01	N34°32'31"W	15.92
C39	5.03	25.00	011°32'13"	2.53	N24°41'31"W	5.03
C40	5.03	25.00	011°32'13"	2.53	S36°13'44"E	5.03
C41	18.02	100.00	010°19'28"	9.03	S35°37'21"E	18.00
C42	33.71	25.00	077°15'02"	19.98	S20°54'52"W	31.21
C43	5.03	25.00	011°32'21"	2.53	S50°16'35"W	5.03
C43	93.49	51.00	105°02'00"	66.50	S07°01'23"W	80.94
C44	54.94	51.00	061°43'35"	30.48	S76°21'25"E	52.32
C45	26.93	25.00	061°43'35"	14.94	S76°21'25"E	25.65
C46	13.95	25.00	031°58'53"	7.16	S29°30'11"E	13.77
C47	27.87	51.07	031°16'08"	14.29	S29°08'48"E	27.53
C48	23.76	25.00	054°26'42"	12.86	S17°37'15"E	22.87
C49	8.37	16.00	029°59'23"	4.29	S59°30'43"W	8.28
C50	52.50	87.05	034°33'09"	27.07	S57°13'50"W	51.70
C51	5.05	14.02	020°38'24"	2.55	S50°16'28"W	5.02
C52	40.19	25.01	092°04'50"	25.93	N89°06'59"E	36.00
C53	27.00	51.00	030°19'59"	13.82	N59°41'00"E	26.69
C54	13.24	25.00	030°19'59"	6.78	N59°41'00"E	13.08

CURVE TABLE						
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD BEARING	CHORD DISTANCE
C55	39.27	25.00	090°00'00"	25.00	N00°28'59"W	35.36
C56	9.35	100.00	005°21'30"	4.68	N42°48'14"W	9.35
C57	20.77	102.00	011°39'52"	10.42	N34°23'52"W	20.73
C58	14.93	70.00	012°13'09"	7.49	N34°40'31"W	14.90
C59	31.49	25.00	072°10'04"	18.22	S80°36'03"W	29.45
C60	27.15	25.00	062°13'40"	15.09	N13°24'11"E	25.84
C61	29.20	25.00	066°55'34"	16.52	S82°40'42"W	27.57
C62	41.32	25.00	094°42'32"	27.14	S01°51'39"W	36.78
C63	80.15	51.00	090°02'39"	51.04	S00°28'18"E	72.15
C64	39.01	25.00	089°23'38"	24.74	S00°08'47"E	35.17
C65	26.18	25.00	060°00'23"	14.44	S75°29'49"E	25.00
C66	15.71	15.00	060°00'01"	8.66	N44°29'59"E	15.00
C67	27.52	25.15	062°41'01"	15.32	N16°57'48"W	26.17
C68	84.30	51.00	094°42'35"	55.37	N01°51'37"E	75.03
C69	29.20	25.00	066°55'34"	16.52	N15°45'08"E	27.57
C70	24.70	25.00	056°36'06"	13.46	S87°50'26"W	23.71
C71	45.83	25.00	105°02'00"	32.60	S07°01'23"W	39.68
C72	26.93	25.00	061°43'35"	14.94	S76°21'25"E	25.65
C73	54.94	51.00	061°43'35"	30.48	S76°21'25"E	52.32
C74	5.05	25.00	011°33'54"	2.53	S36°12'53"E	5.04
C75	80.11	51.00	090°00'00"	51.00	S75°25'56"E	72.12
C76	24.98	25.00	057°14'37"	13.64	N88°11'22"E	23.95
C77	43.28	51.00	048°37'18"	23.04	S87°29'58"E	41.99
C78	5.05	25.00	011°34'50"	2.54	N73°58'48"E	5.04
C79	5.01	25.00	011°29'37"	2.52	S62°26'35"W	5.01
C80	21.22	25.00	048°37'18"	11.29	N87°29'58"W	20.58
C81	50.95	51.00	057°14'37"	27.83	S88°11'22"W	48.86
C82	39.27	25.00	090°00'00"	25.00	N75°25'56"W	35.36
C83	5.02	25.00	011°30'32"	2.52	N24°40'40"W	5.01
C84	5.00	25.00	011°27'57"	2.51	N62°25'45"E	4.99
C85	6.20	25.00	014°12'37"	3.12	N75°16'02"E	6.18
C86	3.87	25.00	008°51'49"	1.94	N63°43'49"E	3.86
C87	5.06	25.00	011°36'29"	2.54	N73°57'58"E	5.06
C88	5.03	25.00	011°32'13"	2.53	S76°36'15"W	5.03
C89	39.15	25.00	089°44'05"	24.88	S25°58'06"W	35.27
C90	77.56	51.00	087°07'56"	48.51	S24°40'01"W	70.30
C91	83.91	51.00	094°16'15"	54.95	N64°37'53"W	74.76
C92	12.08	25.00	027°41'08"	6.16	N31°20'19"W	11.96
C93	11.42	25.00	026°09'53"	5.81	N58°15'50"W	11.32
C94	5.03	25.00	011°32'13"	2.53	N65°04'01"E	5.03
C95	79.87	51.00	089°44'05"	50.76	N25°58'06"E	71.96
C96	38.02	25.00	087°07'56"	23.78	N24°40'01"E	34.46
C97	41.13	25.00	094°16'15"	26.94	S64°37'53"E	36.65
C98	24.64	51.00	027°41'08"	12.57	S31°20'19"E	24.40
C99	22.74	25.00	052°07'15"	12.23	S19°07'16"E	21.97
C100	3.86	25.00	008°50'49"	1.93	S49°55'02"E	3.86
C101	39.26	25.00	089°59'19"	25.00	S00°29'58"E	35.35
C102	35.65	25.00	081°42'03"	21.62	S85°20'43"W	32.70
C103	72.67	51.00	081°38'10"	44.05	S85°22'40"W	66.67
C104	16.52	25.00	037°51'58"	8.58	S63°26'22"W	16.22
C105	20.01	488.00	002°20'59"	10.01	N20°25'00"W	20.01
C106	35.62	25.00	081°38'10"	21.59	N85°22'45"E	32.68
C107	72.72	51.00	081°42'03"	44.10	N85°20'43"E	66.72
C108	80.10	51.00	089°59'19"	50.99	N00°29'58"W	72.12
C109	11.47	22.92	028°40'58"	5.86	N31°09'08"W	11.35

TAYLOR POINTE SUBDIVISION PRELIMINARY PLAT
BEING A PRELIMINARY PLAT CONSISTING OF 39.015 ACRES, INCLUDING 6.05
ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE 67 LOTS, 7 BLOCKS, 0
RESERVES, PART OF LAND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT
NO. 533, TAYLOR, WILLIAMSON COUNTY, TEXAS

39.015 ACRES

THESE NOTES DESCRIBE THAT CERTAIN TRACT OF LAND SITUATED IN THE **GEORGE M. REESE SURVEY, ABSTRACT NO. 533** LOCATED IN WILLIAMSON COUNTY, TEXAS; SUBJECT TRACT BEING ALL OF A CALLED “38.979 ACRES” AS CONVEYED IN A WARRANTY DEED (WITH VENDOR’S LIEN) FROM MARIO D. CRUZ TO FASH TAYLOR, LTD, DATED 06–29–2023 AS RECORDED IN DOCUMENT NO. 2023055749, OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY (OPRWC) BEING SURVEYED ON THE GROUND UNDER THE DIRECT SUPERVISION OF BRUCE LANE BRYAN, REGISTERED PROFESSIONAL LAND SURVEYOR NO. 4249; SUBJECT TRACT BEING MORE FULLY DESCRIBED AS FOLLOWS {BEARINGS AND COORDINATES DESCRIBED HEREIN ARE BASED ON THE TEXAS STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE (4203), NAD 83, 93 {ADJ.};

BEGINNING AT THE NORTHWEST CORNER OF SAID “38.979”, SAME BEING AN EXTERIOR CORNER OF A CALLED “85.047 ACRES” CONVEYED IN A WARRANTY DEED FROM FREDERICK R. FINE TO KEITH HAGLER DATED 5–27–1994 AND RECORDED IN VOLUME 2621, PAGE 250, OFFICIAL RECORDS OF WILLIAMSON COUNTY (ORWC), SAME BEING THE NORTHEAST CORNER OF A CALLED “10.088 ACRES” AS CONVEYED FROM ATX LAND HOLDINGS I, LLC TO TAYLOR 973, LLC DATED 12–09–2021 AND RECORDED IN DOCUMENT NO. 2021188725, OPRWC; FOUND A ½” IRON ROD (AT FENCE CORNER POST) AT SAME (NORTH = 10,177,425.45’, EAST = 3,209,525.55); A ½” IRON ROD FOUND AT AN INTERIOR CORNER OF SAID “85.047 ACRES”, IN OR NEAR THE COMMON SURVEY LINE OF THE **GEORGE M. REESE SURVEY, ABSTRACT NO. 533** AND THE **JAMES C. EAVES SURVEY, A–214**, BEARS NORTH 22° 21’ 55” WEST, 40.00 FEET;

THENCE NORTH 68° 11’ 31” EAST WITH THE COMMON LINE OF SAID “38.979” AND EASTERNMOST SOUTH LINE OF SAID “85.047 ACRES”, GENERALLY ALONG A FENCE, A DISTANCE OF **1034.94 FEET** TO A CALCULATED POINT AT THE NORTHEAST CORNER OF SUBJECT TRACT IN THE SOUTHWESTERN RIGHT–OF–WAY LINE OF CARLOS PARKER BLVD, ALSO KNOWN AS LOOP NO. 427 AS RECORDED IN VOLUME 525, PAGE 263, DRWC; ½” IRON ROD FOUND AT THE SOUTHEAST CORNER OF SAID “85.047 ACRES” BEARS SOUTH 68° 11’ 31” WEST, 1.71 FEET; FOUND A TYPE I TXDOT CONCRETE MONUMENT (ENGINEER’S STATION 206+28.98) BEARING NORTH 30° 15’ 15” WEST, 402.92 FEET;

THENCE SOUTH 30° 15’ 15” EAST WITH SAID SOUTHWESTERN RIGHT–OF–WAY LINE OF CARLOS PARKER BLVD, A DISTANCE OF **213.97 FEET** TO A ½” IRON ROD SET (CAPPED “BRYAN TECHNICAL SERVICES”) AT AN ANGLE POINT IN SAME;

THENCE SOUTH 40° 47’ 05” EAST WITH SAID SOUTHWESTERN RIGHT–OF–WAY LINE OF CARLOS PARKER BLVD, A DISTANCE OF **638.55 FEET** TO A FOUND A TYPE I TXDOT CONCRETE MONUMENT (ENGINEER’S STATION 218+00) AT AN ANGLE POINT IN SAME;

THENCE SOUTH 52° 38’ 45” EAST WITH SAID SOUTHWESTERN RIGHT–OF–WAY LINE OF CARLOS PARKER BLVD, A DISTANCE OF **176.08 FEET** TO A ½” IRON ROD SET (CAPPED “BRYAN TECHNICAL SERVICES”) AT THE INTERSECTION WITH THE NORTHWESTERN RIGHT–OF–WAY OF FARM TO MARKET ROAD NO. 973, REFERRED TO AS A CALLED “6.242 ACRES” AS RECORDED IN VOLUME 1772, PAGE 757, ORWC;

THENCE SOUTH 04° 09’ 48” EAST WITH SAID NORTHWESTERN RIGHT–OF–WAY OF FARM TO MARKET ROAD NO. 973, A DISTANCE OF **95.59 FEET** TO A ½” IRON ROD SET (CAPPED “BRYAN TECHNICAL SERVICES”) AT AN ANGLE POINT IN SAME;

THENCE SOUTH 44° 27’ 05” WEST WITH SAID NORTHWESTERN RIGHT–OF–WAY OF FARM TO MARKET ROAD NO. 973, A DISTANCE OF **1365.60 FEET** TO A ½” IRON ROD SET (CAPPED “BRYAN TECHNICAL SERVICES”) AT AN EXTERIOR CORNER OF SUBJECT TRACT, SAME BEING THE EASTERNMOST SOUTHEAST CORNER OF CASTLEWOOD SUBDIVISION PHASE I FINAL PLAT, A DEDICATED PLAT AS RECORDED IN DOCUMENT NO. 2021152781, OPRWC;

THENCE SOUTH 84° 10’ 07” WEST WITH THE COMMON LINE OF SUBJECT TRACT AND CASTLEWOOD SUBDIVISION PHASE I FINAL PLAT, A DISTANCE OF **163.72 FEET** TO A FOUND ½” IRON ROD AT A COMMON CORNER OF BOTH TRACTS;

THENCE NORTH 19° 09’ 53” WEST WITH THE COMMON LINE OF SAID SUBJECT TRACT AND CASTLEWOOD SUBDIVISION PHASE I FINAL PLAT, ALONG AN OLD TREE LINE, PASSING A FOUND ½” IRON ROD AT THE NORTHEAST CORNER OF SAID CASTLEWOOD SUBDIVISION PHASE I FINAL PLAT, SAME BEING THE SOUTHEAST CORNER OF AFOREMENTIONED “10.088 ACRES” AT 983.69 FEET AND CONTINUING AN ADDITIONAL 580.47 FEET FOR A TOTAL DISTANCE OF **1564.16 FEET** TO THE PLACE OF BEGINNING CONTAINING ACCORDING TO THE DIMENSIONS HEREIN STATED AN AREA OF **39.015 ACRES** OF LAND.

STATE OF TEXAS:

KNOW ALL MEN BY THESE PRESENTS:

COUNTY OF WILLIAMSON

THAT FASH TAYLOR, LTD, CO–OWNER OF THE CERTAIN 38.979 ACRES TRACT OF LAND SHOWN HEREON AND DESCRIBED IN A DEED RECORDED IN DOCUMENT NUMBER 2023055749 OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR TO THE STREETS, ALLEYS, RIGHTS–OF–WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS TAYLOR POINTE SUBDIVISION.

TO CERTIFY WHICH, WITNESS BY MY HAND, THIS THE _____ DAY OF _____, 20____.

FASH TAYLOR, LTD
812 W. MCDERMOTT DRIVE
PMB 1305
ALLEN, TEXAS 75013

STATE OF TEXAS

COUNTY OF WILLIAMSON

BEFORE ME, THE UNDERSIGNED AUTHORITY, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN BY ME TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 20____.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

STATE OF TEXAS:

KNOW ALL MEN BY THESE PRESENTS:

COUNTY OF WILLIAMSON

THAT DOSS DEVELOPMENT, LLC, CO–OWNER OF THE CERTAIN 0.773 ACRES TRACT OF LAND SHOWN HEREON AND DESCRIBED IN A DEED RECORDED IN DOCUMENT NUMBER 2024030082 OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR TO THE STREETS, ALLEYS, RIGHTS–OF–WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS TAYLOR POINTE SUBDIVISION.

TO CERTIFY WHICH, WITNESS BY MY HAND, THIS THE _____ DAY OF _____, 20____.

DOSS DEVELOPMENT, LLC
4637 STONE OAK DRIVE
CARROLLTON, TX 75010

STATE OF TEXAS

COUNTY OF WILLIAMSON

BEFORE ME, THE UNDERSIGNED AUTHORITY, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN BY ME TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 20____.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

STATE OF TEXAS:

KNOW ALL MEN BY THESE PRESENTS:

COUNTY OF WILLIAMSON

THAT MILAN PROPERTY GROUP LLC, CO–OWNER OF THE CERTAIN 3.085 ACRES TRACT OF LAND SHOWN HEREON AND DESCRIBED IN A DEED RECORDED IN DOCUMENT NUMBER 2024049722 OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS OF THE CERTAIN TRACT OF LAND, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR TO THE STREETS, ALLEYS, RIGHTS–OF–WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS TAYLOR POINTE SUBDIVISION.

TO CERTIFY WHICH, WITNESS BY MY HAND, THIS THE _____ DAY OF _____, 20____.

MILAN PROPERTY GROUP LLC
4125 E. BELKNAP STREET, SUITE 109
HALTOM CITY, TEXAS 76111

STATE OF TEXAS

COUNTY OF WILLIAMSON

BEFORE ME, THE UNDERSIGNED AUTHORITY, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN BY ME TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 20____.

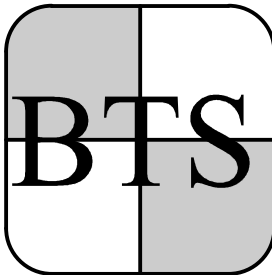
NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

PRELIMINARY PLAT OF
TAYLOR POINTE SUBDIVISION

39.015 ACRE TRACT OUT OF
THE GEORGE M. REESE SURVEY
ABSTRACT NO. 533
WILLIAMSON COUNTY, TEXAS

BRYAN TECHNICAL SERVICES, INC.



911 NORTH MAIN
TAYLOR, TX 76574

PHONE: (512) 352-9090
FAX: (512) 352-9091

FIRM No. 10128500

surveying@austin.rr.com
www.bryantechanicalservices.com

NO.	DATE	REVISIONS	BY

DRAWN BY: AMR	CHECKED BY: BLB
SCALE: 1” = NTS	APPROVED BY: BLB
PROJECT NO. 24–159	DATE: JUNE 5, 2024

SHEET 6 OF 7

CITY PROJECT NO: PZ–2024–2216

TAYLOR POINTE SUBDIVISION PRELIMINARY PLAT
BEING A PRELIMINARY PLAT CONSISTING OF 39.015 ACRES, INCLUDING 6.05
ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE 67 LOTS, 7 BLOCKS, 0
RESERVES, PART OF LAND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT
NO. 533, TAYLOR, WILLIAMSON COUNTY, TEXAS

COUNTY OF WILLIAMSON {

KNOW ALL MEN BY THESE PRESENTS

COUNTY OF WILLIAMSON {

I, BRUCE LANE BYRAN, REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS PLAT IS TRUE AND CORRECTLY MADE FROM AN ACTUAL SURVEY MADE ON THE GROUND OF THE PROPERTY LEGALLY DESCRIBED HEREON, AND THAT THERE ARE NO APPARENT DISCREPANCIES, CONFLICTS, OVERLAPPING OF IMPROVEMENTS, VISIBLE UTILITY LINES OR ROADS IN PLACE, EXCEPT AS SHOWN ON THE ACCOMPANYING PLAT, AND THAT THE CORNER MONUMENTS SHOWN THEREON WERE PROPERLY PLACED UNDER MY SUPERVISION IN ACCORDANCE WITH THE SUBDIVISION REGULATIONS OF THE CITY OF TAYLOR, TEXAS.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT _____,
_____ COUNTY, TEXAS, THIS _____ DAY OF _____, 20____.

REGISTERED PROFESSIONAL SURVEYOR
NO. 4249 STATE OF TEXAS



I, JONATHAN E. SCHINDLER, DO HEREBY CERTIFY THAT THIS SUBDIVISION IS NOT ENCROACHED BY A ZONE A FLOOD AREA, AS DENOTED HEREIN, AND AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD BOUNDARY MAP, COMMUNITY PANEL NUMBER 48491C0541F, EFFECTIVE DATE 12/20/2019, AND THAT EACH LOT CONFORMS TO THE CITY OF TAYLOR REGULATIONS. THE FULLY DEVELOPED, CONCENTRATED STORMWATER RUNOFF RESULTING FROM THE ONE HUNDRED (100) YEAR FREQUENCY STORM IS CONTAINED WITHIN THE DRAINAGE EASEMENTS SHOWN AND/OR PUBLIC RIGHTS-OF-WAY DEDICATED BY THIS PLAT.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT _____,
_____ COUNTY, TEXAS, THIS _____ DAY OF _____, 20____.

REGISTERED PROFESSIONAL ENGINEER
NO. 108864 STATE OF TEXAS

FOR REVIEW.
THIS DOCUMENT IS RELEASED
FOR THE PURPOSE OF REVIEW
UNDER THE AUTHORITY OF
JONATHAN E. SCHINDLER, P.E.
NO. 108864. IT IS NOT TO BE
USED FOR BIDDING, PERMIT OR
CONSTRUCTION.

THIS SUBDIVISION TO BE KNOWN AS TAYLOR POINTE SUBDIVISION HAS BEEN ACCEPTED AND APPROVED FOR FILING OF RECORD WITH THE COUNTY CLERK OF WILLIAMSON COUNTY, TEXAS, ACCORDING TO THE MINUTES OF THE MEETING OF THE TAYLOR PLANNING AND ZONING COMMISSION ON THE _____ DAY OF _____, 20____, A.D.

_____, CHAIRMAN

_____, SECRETARY

STATE OF TEXAS {

COUNTY OF WILLIAMSON {

KNOW ALL MEN BY THESE PRESENTS

I, NANCY RISTER, CLERK OF THE COUNTY COURT OF SAID COUNTY, DO HEREBY CERTIFY THAT THE FOREGOING INSTRUMENT IN WRITING, WITH ITS CERTIFICATE OF AUTHENTICATION WAS FILED FOR RECORD IN MY OFFICE ON THE ____ DAY OF _____, 20____ A.D., AT ____O'CLOCK, ____M., AND DULY RECORDED THIS THE DAY OF _____, 20____ A.D., AT ____O'CLOCK, ____M., IN THE OFFICIAL PUBLIC RECORDS OF SAID COUNTY IN INSTRUMENT NO. _____.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT THE COUNTY COURT OF SAID COUNTY, AT MY OFFICE IN GEORGETOWN, TEXAS THE DATE LAST SHOWN ABOVE WRITTEN.

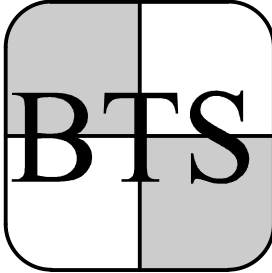
NANCY RISTER, CLERK COUNTY COURT
OF WILLIAMSON COUNTY, TEXAS

BY: _____
DEPUTY

PRELIMINARY PLAT OF
TAYLOR POINTE SUBDIVISION

39.015 ACRE TRACT OUT OF
THE GEORGE M. REESE SURVEY
ABSTRACT NO. 533
WILLIAMSON COUNTY, TEXAS

BRYAN TECHNICAL SERVICES, INC.



911 NORTH MAIN
TAYLOR, TX 76574

PHONE: (512) 352-9090
FAX: (512) 352-9091

FIRM No. 10128500
surveying@austin.rr.com
www.bryantechnicalservices.com

NO.	DATE	REVISIONS	BY

DRAWN BY: AMR	CHECKED BY: BLB
SCALE: 1" = N/A	APPROVED BY: BLB
PROJECT NO. 24-159	DATE: JUNE 5, 2024



BM NO. 1 TEMPORARY BENCH MARK 1/2" IRON,
N=10177819.470' E=3210509.089'
ELEVATION = 610.34' (NAD '83)

BM NO. 2 TEMPORARY BENCH MARK COTTON SPIKE IN
POWER POLE.
N=10177315.040' E=3210860.312'
ELEVATION = 607.42' (NAD '83)

PROPOSED FIRE HYDRANT	
PROPOSED SANITARY MAN-HOLE	
PROPOSED CURB INLET	
PROPOSED GRATE INLET	
PROPERTY BOUNDARY	
PROPOSED PAVEMENT	
PROPOSED SIDEWALK	

NOTE: THESE PLANS ARE FOR CONCEPTUAL REVIEW ONLY

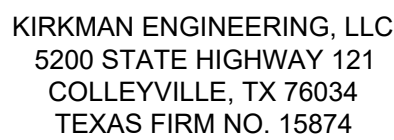
P R E L I M I N A R Y
F O R R E V I E W O N L Y
T H E S E D O C U M E N T S A R E F O R
D E S I G N R E V I E W O N L Y A N D
N O T I N T E N D E D F O R T H E P U R P O S E S
O F C O N S T R U C T I O N , B I D D I N G
O R P E R M I T . T H E Y W E R E P R E P A R E D
B Y , O R U N D E R T H E S U P E R V I S I O N O F :

JONATHAN E. SCHINDLER
P.E.# 108864
DATE: September 13, 2024

FASH TAYLOR, LTD
812 W. MCDERMOTT DRIVE
PMB 1305
ALLEN, TX 75013

DEVELOPER:
CONTACT: JOHN CHONG

TAYLOR POINTE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS

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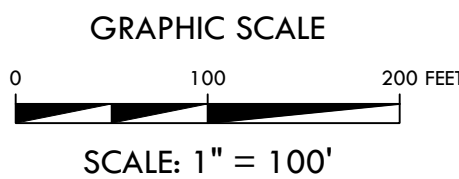
JOB NUMBER: FSH24001

ISSUE DATE:

PRELIMINARY ROADWAY CONCEPT PLAN

SHEET:

C3.0





LEGEND

BASIN NAME	DA1
ACRES	0.63 ac.
TIME OF CONCENTRATION	5.57

TING DRAINAGE AREA LABEL

TING DRAINAGE AREA DIVIDE

NG DRAINAGE FLOW ARROW

EXISTING CONTOUR

EXISTING STORM PIPE

PROPERTY BOUNDARY

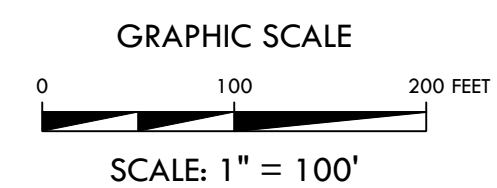
POINT OF INTEREST

FLOW PATH

Existing Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
EX-1	290,110	6.66	80	0.00	8.87	0.0104063	0%	5.32
EX-2	215,186	4.94	80	0.00	15.33	0.0077188	0%	9.20
EX-3	788,872	18.11	80	0.00	15.82	0.0282969	0%	9.49
EX-4	627,700	14.41	80	0.00	22.09	0.0225156	0%	13.26

Existing Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
EX-1	290,110	6.66	80	0.00	8.87	0.0104063	0%	5.32
EX-2	215,186	4.94	80	0.00	15.33	0.0077188	0%	9.20
EX-3	788,872	18.11	80	0.00	15.82	0.0282969	0%	9.49
EX-4	627,700	14.41	80	0.00	22.09	0.0225156	0%	13.26

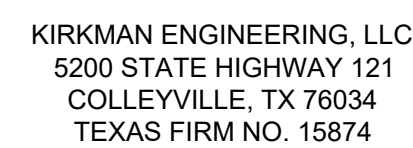
NOTE: DRAINAGE CRITERIA TAKEN FROM CITY OF TAYLOR ENGINEERING DESIGN MANUAL DATED JANUARY 2024.



FASH TAYLOR, LTD
812 W. MCDERMOTT DRIVE
PMB 1305
ALLEN, TX 75013

DEVELOPER:
CONTACT: JOHN CHONG

TAYLOR POINTE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS

[illegible]

PRELIMINARY EXISTING DRAINAGE AREA MAP

C6.0



P R E L I M I N A R Y
F O R R E V I E W O N L Y
T H E S E D O C U M E N T S A R E F O R
D E S I G N R E V I E W O N L Y A N D
N O T I N T E N D E D F O R T H E P U R P O S E S
O F C O N S T R U C T I O N, B I D D I N G
O R P E R M I T. T H E Y W E R E P R E P A R E D
B Y, O R U N D E R T H E S U P E R V I S I O N O F:

JONATHAN E. SCHINDLER
P.E.# 108864
DATE: September 13, 2024

FASH TAYLOR, LTD
812 W. MCDERMOTT DRIVE
PMB 1305
ALLEN, TX 75013

DEVELOPER:
CONTACT: JOHN CHONG

TAYLOR POINTE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS

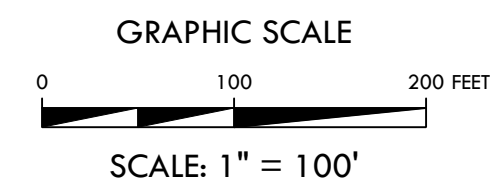
Proposed Hydrology Summary				
Analysis Point	Peak Flow (cfs)			
	Q ₂	Q ₁₀	Q ₂₅	Q ₁₀₀
POI-1	12.7	18.6	22.5	28.9
POI-2	9.4	15.3	19.2	26.7
POI-3	25.4	38.8	47.4	61.5
POI-4	20.3	40.6	53.1	74.4

Existing Hydrology Summary				
Analysis Point	Peak Flow (cfs)			
	Q ₂	Q ₁₀	Q ₂₅	Q ₁₀₀
POI-1	14.2	26.1	34.0	46.9
POI-2	9.8	18.1	23.6	32.6
POI-3	33.9	62.5	81.6	112.9
POI-4	21.9	41.1	54.0	75.2

Proposed Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table		HEC-HMS Inputs	
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
DA-1	159,865	3.67	93	127892.16	8.00	0.0057344	80%	3.00
DA-2	148,975	3.42	83	44692.56	5.33	0.0053438	30%	5.00
DA-3	173,804	3.99	93	147733.74	5.00	0.0062344	85%	3.00
DA-4	1,169,586	26.85	93	935666.80	7.44	0.0419531	80%	4.46
DA-5	171,626	3.94	83	8581.32	11.27	0.0061563	5%	6.76
DA-6	44,431	1.02	83	444.31	11.48	0.0015938	1%	6.89

NOTE: THESE PLANS ARE FOR CONCEPTUAL REVIEW ONLY

NOTE: DRAINAGE CRITERIA TAKEN FROM CITY OF TAYLOR ENGINEERING DESIGN MANUAL DATED JANUARY 2024.



PRELIMINARY PROPOSED DRAINAGE AREA MAP

SHEET:

C6.1



SITE BENCHMARKS

BM NO. 1	TEMPORARY BENCH MARK 1/2" IRON, N=10777819.470' E=3210509.089' ELEVATION = 610.34' (NAD '83)
BM NO. 2	TEMPORARY BENCH MARK COTTON SPIKE IN POWER POLE. N=10777315.040' E=3210860.312' ELEVATION = 607.42' (NAD '83)

JONATHAN E. SCHINDLER
 P.E.# 108864
 DATE: September 13, 2024

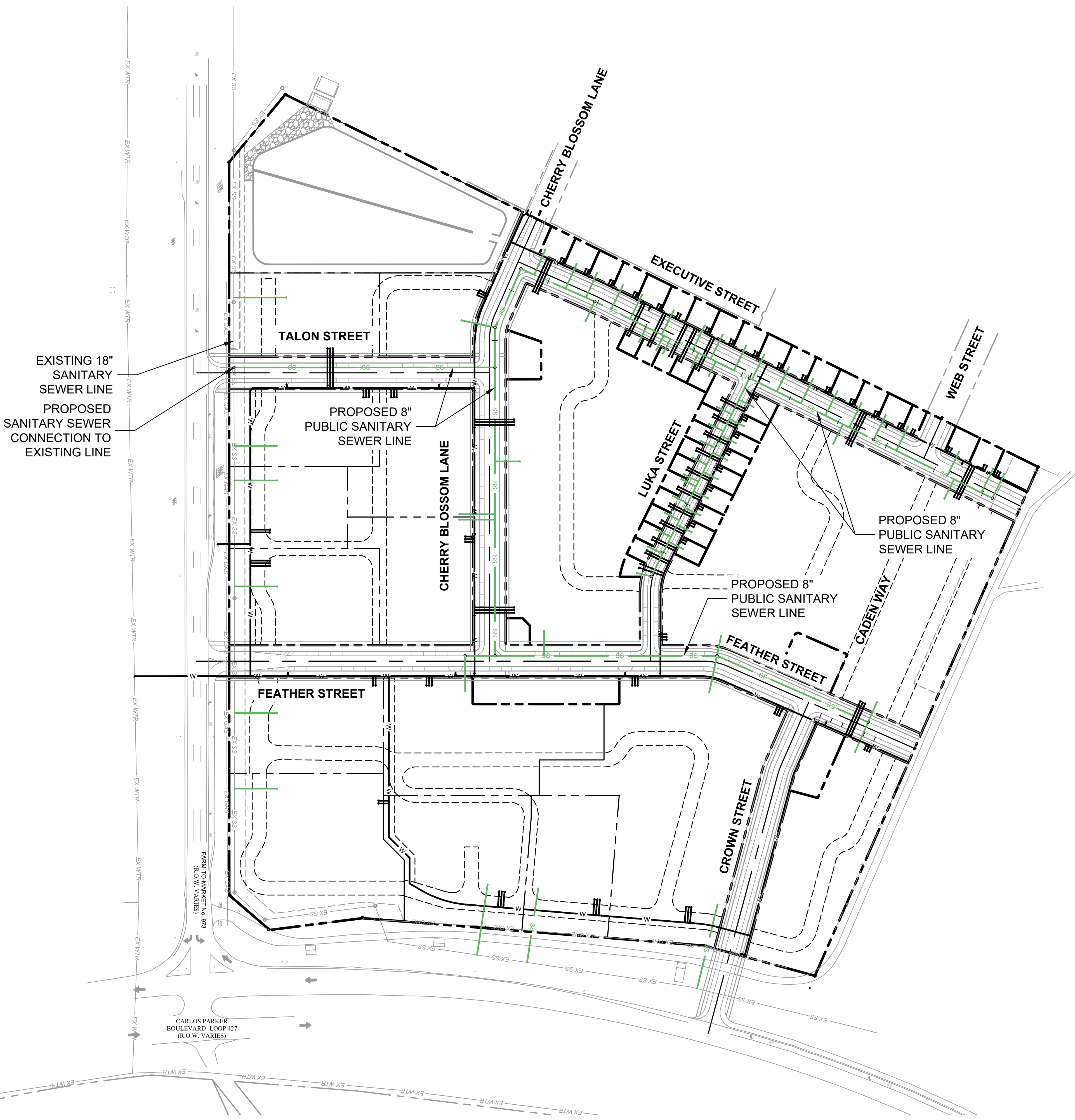
DEVELOPER:
CONTACT: JOHN CHONG

TAYLOR POINTE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS

[illegible]

SHEET:

C7.11



LEGEND

EXISTING SANITARY SEWER	— EX SS —
PROPOSED SANITARY SEWER	— SS —
EXISTING WATER	— EX WTR —
PROPOSED WATER	— W — W —
EXISTING SANITARY SEWER MANHOLE	
PROPOSED SANITARY SEWER MANHOLE	
PROPOSED SANITARY SEWER CLEANOUT	
EXISTING FIRE HYDRANT	 FH
PROPOSED FIRE HYDRANT	
PROPOSED GAS	— GAS —
EXISTING GAS	— EX GAS —
PROPOSED UNDERGROUND ELECTRIC	— UGE — UGE — UGE —
EXISTING UNDERGROUND ELECTRIC	— EX UGE —
PROPOSED OVERHEAD ELECTRIC	— OHE — OHE — OHE —
EXISTING OVERHEAD ELECTRIC	— EX OHE —
PROPOSED TELEPHONE	— TELE — TELE — TELE —
EXISTING TELEPHONE	— EX UGE —
PROPERTY BOUNDARY	— — — — —

WATER & SANITARY SEWER GENERAL NOTES

1. ALL WATER MAINS, UNLESS OTHERWISE NOTED, SHALL HAVE A MINIMUM COVER OF 48" BELOW FINISHED GRADE. PROVIDE VALVE EXTENSIONS TO ALL VALVES ON LINES DEEPER THAN 48".
2. WHEN WATER AND SEWER SERVICE LINES ARE INSTALLED, THEY SHALL BE INSTALLED PER TCEO SEPARATION REQUIREMENTS.
3. CONTRACTOR TO VERIFY ALL EXISTING SEWER FLOW LINES BEFORE BEGINNING CONSTRUCTION.
4. ALL SANITARY SEWER LINES SHALL BE TESTED IN ACCORDANCE WITH THE STANDARD CITY SPECIFICATIONS.
5. THE UTILITY CONTRACTOR SHALL INSTALL THE WATER SERVICES TO A POINT AS SHOWN ON THE PLANS. THE METER BOX SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR AFTER FINE GRADING HAS BEEN COMPLETED.
6. ALL METER BOXES SHALL BE LOCATED IN NON-TRAFFIC AREAS.
7. CONTRACTOR SHALL RAISE/LOWER OR ADJUST ALL EXISTING FRANCHISE UTILITY LINES IN CONFLICT WITH PROPOSED UTILITIES AS PART OF THE BASE BIDS FOR ALL KNOWN OR UNKNOWN LINES.
8. REFER TO PLAN PL-100 FOR EXACT SIZE, LOCATION, AND INVERT OF UTILITIES AT BUILDING.
9. FIRE SPRINKLER LINES AND FIRE DEPARTMENT CONNECTIONS SHALL BE SIZE, DESIGNED, AND INSTALLED BY A LICENSED FIRE SPRINKLER CONTRACTOR.
10. BELOW-GRADE SERVICE FOR DOMESTIC AND FIRE SERVICE TO BE LOCATED INSIDE THE BUILDING PER CITY OF TAYLOR STANDARDS UNLESS OTHERWISE NOTED.

NOTE TO CONTRACTOR

NOTE TO CONTRACTOR
THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) PRIOR TO COMMENCING CONSTRUCTION. IF FIELD CONDITIONS DIFFER FROM LOCATIONS SHOWN ON THE PLANS, THE CONTRACTOR SHALL CONTACT THE PROJECT ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.

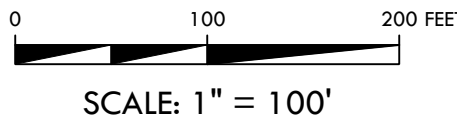
NOTE TO CONTRACTOR - FRANCHISE UTILITIES

FRANCHISE UTILITIES ARE SHOWN FOR SCHEMATIC REFERENCE ONLY.
CONTRACTOR SHALL COORDINATE ALL FRANCHISE UTILITY DESIGN AND
INSTALLATION WITH THE SPECIFIC UTILITY PROVIDER AND DESIGNER.
CONTRACTOR SHALL COORDINATE ALL UTILITY INSTALLATIONS TO AVOID
CONFLICTS AT EXISTING AND PROPOSED UTILITY CROSSINGS.

NOTE: THESE PLANS ARE FOR CONCEPTUAL REVIEW ONLY



GRAPHIC SCALE



TAYLOR POINTE

CITY OF TAYLOR

WILLIAMSON COUNTY, TEXAS

PRELIMINARY DRAINAGE REPORT

JULY 2, 2024

Developer:

Fash Taylor, LTD

825 Watters Creek Boulevard, Suite 250

Allen, Texas 75013

John Chong

(469) 267-1520

Engineer:

Kirkman Engineering, LLC

Texas Firm No. 15874

Jonathan Schindler, P.E.

1130 Cottonwood Creek Trail, Suite C3

Cedar Park, TX 78613

(817) 488-4960

Date Prepared: July 5, 2024

DATE: 9/13/2024

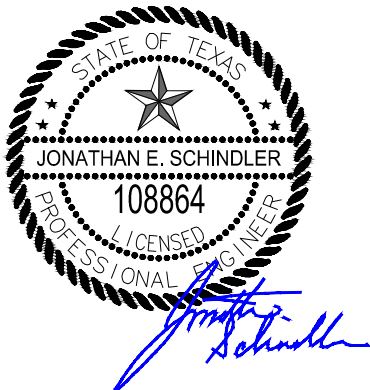


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1.0 INTRODUCTION

This preliminary drainage report was prepared in support of the preliminary plat application for the master development called Taylor Mixed Use located in Taylor, Texas. The project site is located at the Southwest corner of FM 973 and Loop 427 (Carlos G Parker Boulevard). This report has been prepared to address the site's hydrologic and hydraulic impacts, stormwater mitigation and conformance to the city's stormwater design requirements. This proposed development will consist of mixed land uses including multi-family, single-family residential, hotel, and general commercial. The total site area for this development is 38.979 acres. The existing zoning of the site is P5: Urban Center. A master plan has previously been approved for this mixed-use development.

2.0 PROJECT DESCRIPTION

2.1 SURVEY BENCHMARK INFORMATION

Two temporary benchmarks were set on the northern side of the property. Cotton spike in power pole with an elevation of 607.42' (Temporary benchmark #1) and a ½" iron rod with an elevation of 610.34' (Temporary benchmark #2).

2.2 PROPOSED LAND USE

The subject tract will be a mixed-use development consisting of, but not limited to, multi-family, single-family residential, retail, restaurant, office, and hotel uses.

2.3 FEMA MAP & FLOOD ZONE

The limits of this project site are not located within a floodplain per the latest FEMA FIRM Map Number 48491C0541F. See Appendix B – FEMA FIRM Map.

2.4 METHODOLOGY

Existing and proposed stormwater analysis was derived using approved methods as outlined in the City of Taylor Engineering Manual & Details, dated January 2024. Drainage area calculations used the standard SCS methodology. Rainfall data was taken from the City of Taylor Engineering Manual Table 3-1. HEC-HMS Version 4.12 was used for detention modeling and is attached with this report. The development is required to mitigate stormwater runoff from the 2-yr, 10-yr, 25-yr, and 100-yr storm events. The total drainage area for this site is +/- 39 acres. For the existing site, the runoff CN values of 80 has been used for each point of interest. CN values for the proposed site are a mixture of 93 for onsite and 89 for channelized flow. For all four (4) points of interest, runoff coefficients are shown for each basin on the Existing and Proposed Drainage Area Maps, Refer to Appendix C and D, respectively.

2.5 EXISTING CONDITIONS

The project site is located at the Southwest corner of FM 973 and Loop 427 (Carlos G Parker Boulevard) in Taylor, Texas. Refer to “Appendix A” for the project site location map. The existing condition of this site is currently vacant, consisting of cultivated farmland. There are no trees on site. The site is generally draining from the north corner of the property (approximate elevation of 611.2’) and splits into three separate drainage basins sheet flowing to the western corner (approximate elevation of 598.65’), southern corner (approximate elevation of 596.63’), and the southeastern side of the property (approximate elevation of 596.92’). The slope of the existing site ranges from 0.5% to 5%. Point of interest 1 will consist of 6.64 acres and have a flow of 46.9 cfs at the 100-yr storm event. Point of interest 2 will consist of 4.97 acres and have a flow of 32.6 cfs at the 100-yr storm event. Point of interest 3 will consist of 17.43 acres and have a flow of 112.9 cfs at the 100-yr storm event. Point of interest 4 will consist of 14.28 acres and have a flow of 75.2 cfs at the 100-yr storm event. Refer to “Appendix C – Existing Drainage Area Map” for the existing topography and drainage patterns for this site and table 1.1 for existing drainage calculations.

Existing Hydrology Summary				
Analysis Point	Peak Flow (cfs)			
	Q ₂	Q ₁₀	Q ₂₅	Q ₁₀₀
POI-1	14.2	26.1	34.0	46.9
POI-2	9.8	18.1	23.6	32.6
POI-3	33.9	62.5	81.6	112.9
POI-4	21.9	41.1	54.0	75.2

Table 1.1

2.6 PROPOSED CONDITIONS

Point of interest 1 will consist of 4.11 acres of runoff flow and outflow to the western corner of the property. Point of interest 1 will have 28.9 cfs at the 100-yr storm event. Point of interest 2 will consist of 3.46 acres of runoff flow going to the northern corner of the site. Point of interest 2 will have 26.7 cfs at the 100-yr storm event. Point of interest 3 will consist of 7.94 acres and flow to the southeastern side of the property into an existing drainage culvert. Point of interest 3 will have 61.5 cfs at the 100-yr storm event. Point of interest 4 will consist of 27.61 acres and flow to an on-site detention pond at the southern corner of the property and will outfall into an existing drainage culvert crossing under FM 973. Point of interest 4 will have 74.4 cfs at the 100-yr storm event. Runoff coefficients were calculated based on the total of impervious and pervious areas. Refer to “Appendix D – Proposed Drainage Area Maps” and Table 1.2.

Proposed Hydrology Summary				
Analysis Point	Peak Flow (cfs)			
	Q ₂	Q ₁₀	Q ₂₅	Q ₁₀₀
POI-1	12.7	18.6	22.5	28.9
POI-2	9.4	15.3	19.2	26.7
POI-3	25.4	38.8	47.4	61.5
POI-4	29.3	33.6	46.0	74.4

Table 1.2

3.0 PROPOSED DRAINAGE DESIGN

3.1 DRAINAGE IMPACTS

The existing topography for this site generally slopes from the north to the south. The proposed site will be designed to collect stormwater runoff from 26.49 acres within catch basins and storm inlets. The stormwater discharge will be routed through an underground stormwater drainage system to a detention pond in the southern corner of the site. The remaining 12.49 acres will consist of three separate drainage basins which will bypass the proposed regional detention pond. There shall be no increase in stormwater discharge in the 2, 10, 25, and 100-yr storm events at Points of interest 2-4. Hydrologic calculations were performed using HEC-HMS Version 4.12. Storage volumes were calculated for the 2-yr, 10-yr, 25-yr, and 100-yr storm events. On-site analysis consists of four (4) existing sub-basins and four (4) proposed sub-basins. The site is located within the Blackland Prairie which requires that channel erosion protection will have the 2-year Q critical flow to be less than 2% of the pre-development 2-year frequency storm peak flow rate. (Section 3.1.4.D of the City of Taylore Engineering Manual) Riprap will be placed where channels are being used to reduce the 2-year peak flow rate to below the 2% Q critical flow.

3.2 STORMWATER MITIGATION

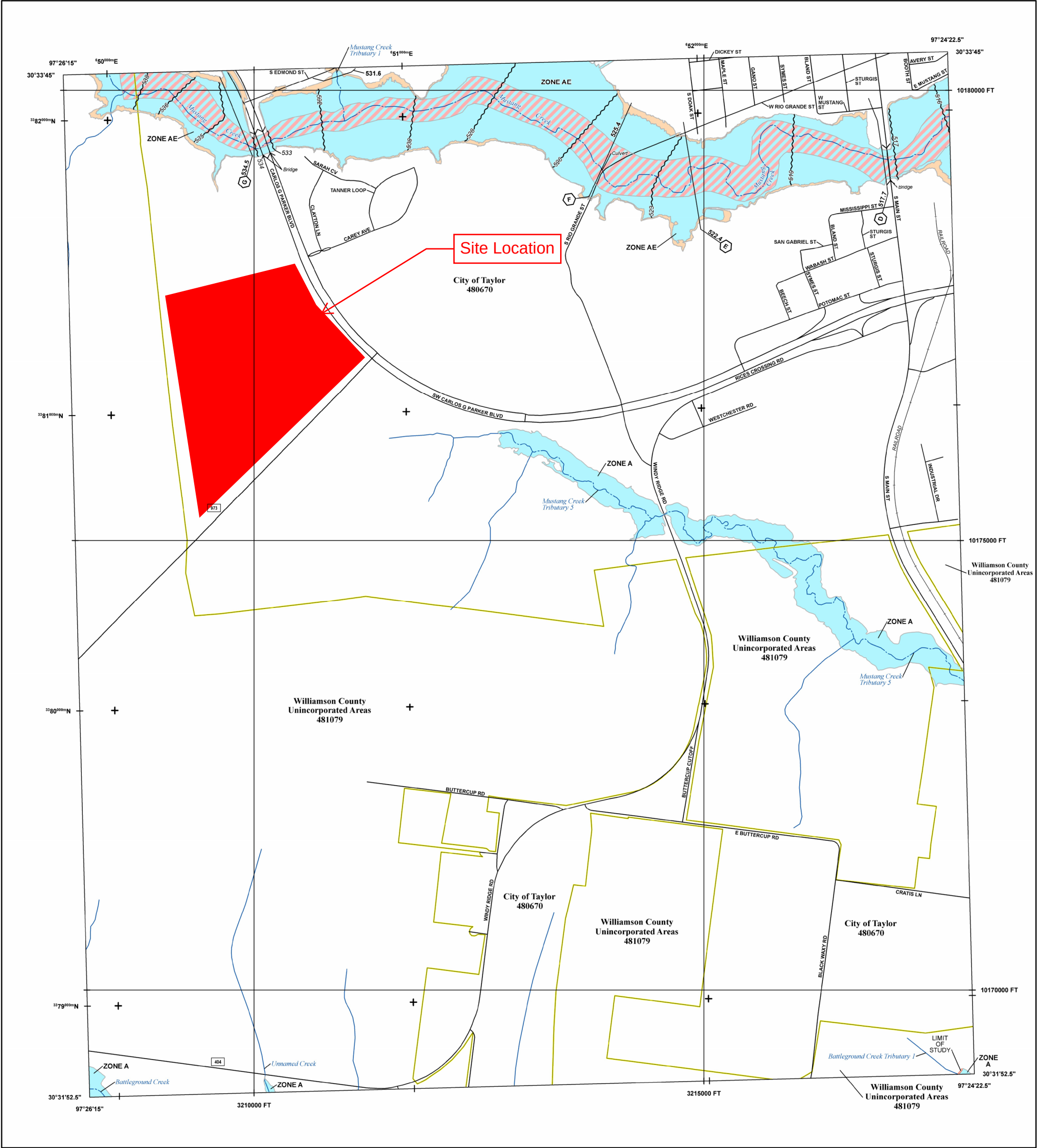
The proposed development is shown on the Proposed Drainage Area Map "Exhibit D". Calculations were performed to calculate the required detention volume. An above ground stormwater detention pond was designed to mitigate the increased stormwater discharge developed by the proposed development. The required detention volume is 351,584.1 cubic feet of storage. The amount of detention volume provided is 439,741.1 cubic feet that includes 1-ft of freeboard. A weir outfall system was designed to reduce the stormwater discharge to maintain or be less than the existing conditions. All drainage calculations are shown in "Appendices C & D – Existing and Proposed Drainage Area Maps".

Appendices

Appendix A: Site Map



Appendix B: FEMA Map



FLOOD HAZARD INFORMATION

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT
THE INFORMATION DEPICTED ON THIS MAP AND SUPPORTING DOCUMENTATION ARE ALSO AVAILABLE IN DIGITAL FORMAT AT [HTTPS://MSC.FEMA.GOV](https://msc.fema.gov)

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A/V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
OTHER AREAS OF FLOOD HAZARD		Regulatory Floodway
		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee See Notes. Zone X
OTHER AREAS		Area with Flood Risk due to Levee Zone D
		NO SCREEN Area of Minimal Flood Hazard Zone X
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
OTHER FEATURES		Levee, Dike, or Floodwall
		Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
	Base Flood Elevation Line (BFE)	
	Limit of Study	
	Jurisdiction Boundary	

NOTES TO USERS

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at <https://msc.fema.gov>. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates refer to the Flood Insurance Study Report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6629.

Base map information shown on this FIRM was derived from digital data obtained from Texas Natural Resource Information Systems (TNRIS), dated 2000; United States Census Bureau, dated 2015; United States Geological Survey, dated 2005; and the Williamson County Geographic Information Systems (GIS) Department, dated 2014 and 2017.

SCALE

Map Projection:
Texas State Plane Central (FIPS ZONE 4203); North American Datum 1983;
Western Hemisphere; Vertical Datum: NAVD 88

1 inch = 500 feet 1:6,000

0 250 500 750 1,000 2,000 feet

0 125 250 500 meters

PANEL LOCATOR

FEMA

National Flood Insurance Program

NATIONAL FLOOD INSURANCE PROGRAM
FLOOD INSURANCE RATE MAP

WILLIAMSON COUNTY, TEXAS
and Incorporated Areas

PANEL 541 OF 750

Panel Contains:
COMMUNITY
TAYLOR, CITY OF
WILLIAMSON COUNTY

NUMBER PANEL SUFFIX
480670 0541 F
481079 0541 F

VERSION NUMBER
2.3.3.3

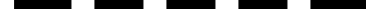
MAP NUMBER
48491C0541F

MAP REVISED
DECEMBER 20, 2019



LEGEND

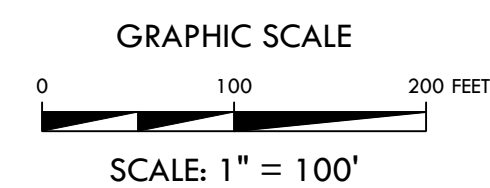
BASIN NAME	DA1
ACRES	0.63 ac.
Q100 (cfs)	5.57

TYPING DRAINAGE AREA LABEL	
TYPING DRAINAGE AREA DIVIDE	
TYPING DRAINAGE FLOW ARROW	
EXISTING CONTOUR	
EXISTING STORM PIPE	
PROPERTY BOUNDARY	
POINT OF INTEREST	

Existing Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
EX-1	289,238	6.64	80	0.00	8.87	0.0103750	0%	5.32
EX-2	216,493	4.97	80	0.00	16.71	0.0077656	0%	10.03
EX-3	759,251	17.43	80	0.00	15.82	0.0272344	0%	9.49
EX-4	622,037	14.28	80	0.00	22.09	0.0223125	0%	13.26

Existing Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
EX-1	289,238	6.64	80	0.00	8.87	0.0103750	0%	5.32
EX-2	216,493	4.97	80	0.00	16.71	0.0077656	0%	10.03
EX-3	759,251	17.43	80	0.00	15.82	0.0272344	0%	9.49
EX-4	622,037	14.28	80	0.00	22.09	0.0223125	0%	13.26

NOTE: DRAINAGE CRITERIA TAKEN FROM CITY OF TAYLOR ENGINEERING DESIGN MANUAL DATED JANUARY 2024.



JONATHAN E. SCHINDLER
P.E.# 108864
DATE: June 26, 2024

TAYLOR MIXED USE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS



Kirkman
ENGINEERING

KIRKMAN ENGINEERING, LLC
5200 STATE HIGHWAY 121
COLLEEVILLE, TX 76034
TEXAS FIRM NO. 15874

ISSUE DATE:

SHEET:

APPENDIX C

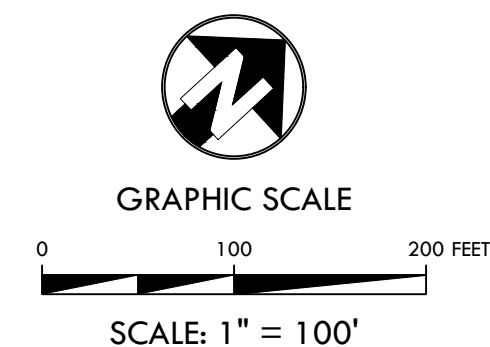


P R E L I M I N A R Y
F O R R E V I E W O N L Y
T H E S E D O C U M E N T S A R E F O R
D E S I G N R E V I E W O N L Y A N D
N O T I N T E N D E D F O R T H E P U R P O S E S
O F C O N S T R U C T I O N , B I D D I N G
O R P E R M I T . T H E Y W E R E P R E P A R E D
B Y , O R U N D E R T H E S U P E R V I S I O N O F :

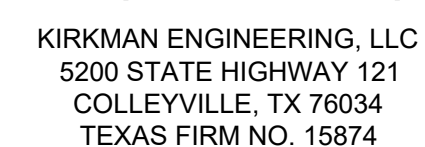
J O N A T H A N E . S C H I N D L E R
P . E . # 1 0 8 8 6 4
D A T E : J u n e 2 6 , 2 0 2 4

825 WATTERS CREEK
BLVD #STE 250
ALLEN, TX 75013

Proposed Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
DA-1	174,240	4	93	139392.00	5.00	0.0062500	80%	3.00
DA-2	32,234	0.74	93	25787.52	5.00	0.0011563	80%	3.00
DA-3	284,447	6.53	93	227557.44	5.00	0.0102031	80%	3.00
DA-4	1,178,734	27.06	93	942986.88	7.44	0.0422813	80%	4.46
OFF-1	104,544	2.4	89	5227.20	11.27	0.0037500	5%	6.76
OFF-2	63,162	1.45	89	3158.10	11.48	0.0022656	5%	6.89
OFF-3	45,302	1.04	89	2265.12	5.00	0.0016250	5%	3.00



TAYLOR MIXED USE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLIAMSON COUNTY, TEXAS

[illegible]

JOB NUMBER: FSH24001
ISSUE DATE:

PROPOSED DRAINAGE AREA MAP

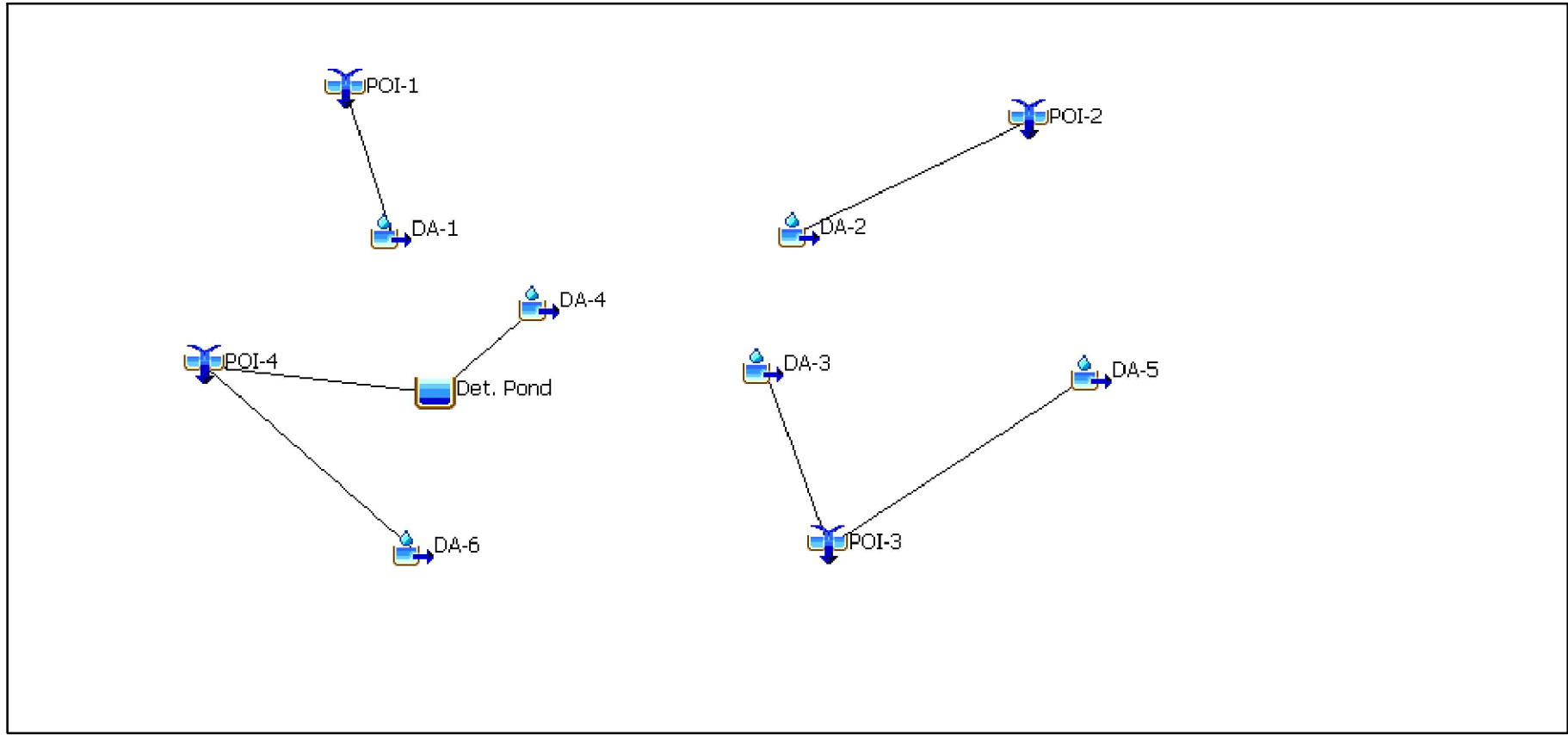
SHEET:

APPENDIX D

FULL PATH: K:\30852_Pond Exhibit\FSH24001_Taylor\Drawings\02_Ext\04 - Exhibit\30852_Pond Exhibit_FSH24001

PLANS: 30852_Pond Exhibit_FSH24001.dwg
PLOTTER: 30852_Pond Exhibit_FSH24001.dwg
PLOT DATE: 9/13/2024

PROPOSED HEC-HMS DIAGRAM



Detention Pond Stage Storage						
Elevation *	Depth	Area	Avg. Area	Inc. Vol.	Total Vol.	Total Vol.
ft. msl	ft.	S. F.	S. F.	C. F.	C. F.	A.c. Ft.
597.85	0.00	0.0		-	-	0.00
598.00	0.15	222.0	111.0	16.6	16.6	0.00
599.00	1.15	11,382.0	5,802.0	5,802.0	5,818.7	0.13
600.00	2.15	38,233.0	24,807.5	24,807.5	30,626.2	0.70
601.00	3.15	67,378.0	52,805.5	52,805.5	83,431.7	1.92
602.00	4.15	83,047.0	75,212.5	75,212.5	158,644.2	3.64
603.00	5.15	88,157.0	85,602.0	85,602.0	244,246.2	5.61
604.00	6.15	88,157.0	88,157.0	88,157.0	332,403.2	7.63
605.00	7.15	88,157.0	88,157.0	88,157.0	420,560.2	9.65

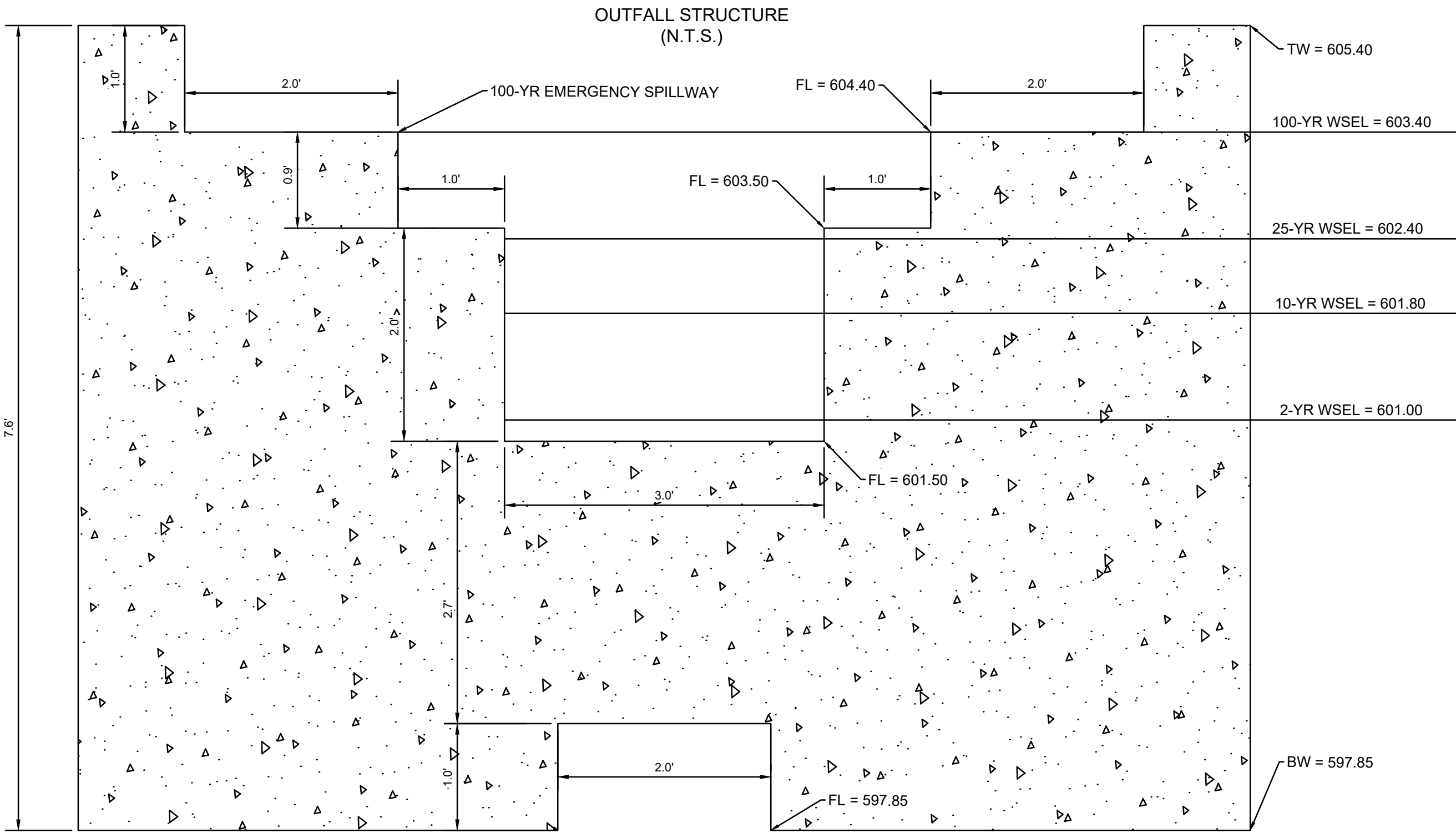
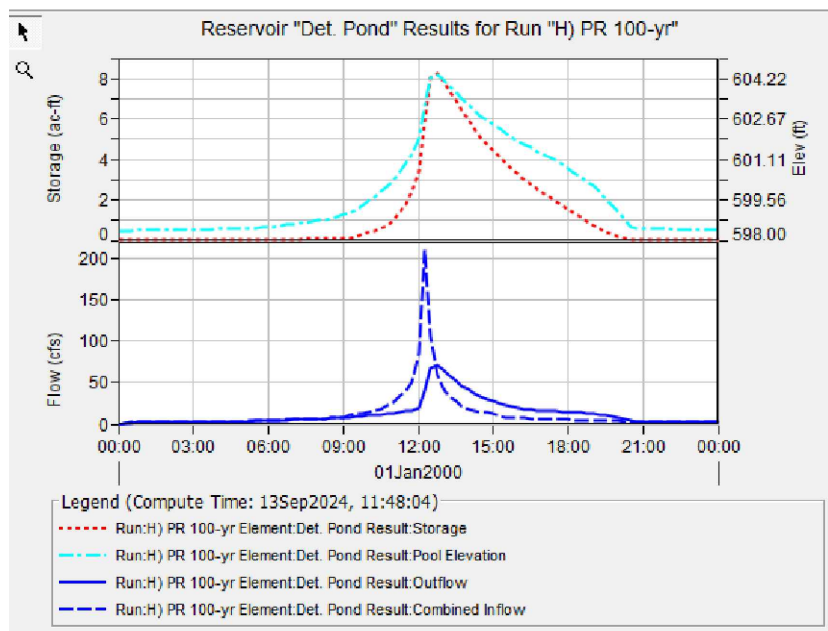
Proposed Hydrology Summary				
Point of Interest	Peak Flow (cfs)			
	Q ₂	Q ₁₀	Q ₂₅	Q ₁₀₀
POI-1	12.7	18.6	22.5	28.9
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Existing Hydrology Summary				
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POI-2	9.8	18.1	23.6	32.6
POI-3	33.9	62.5	81.6	112.9
POI-4	21.9	41.1	54.0	75.2

Proposed Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
DA-1	159,865	3.67	93	127892.16	5.00	0.0057344	80%	3.00
DA-2	148,975	3.42	83	44692.56	5.00	0.0053438	30%	3.00
DA-3	173,804	3.99	93	147733.74	5.00	0.0062344	85%	3.00
DA-4	1,169,586	26.85	93	935668.80	5.00	0.0419531	80%	3.00
DA-5	171,626	3.94	83	8581.32	5.00	0.0061563	5%	3.00
DA-6	44,431	1.02	83	444.31	5.00	0.0015938	1%	3.00
Existing Conditions - Routing Analysis Inputs								
Drainage Areas			Land Use		TOC Calculation Table	HEC-HMS Inputs		
Contributing Area	Area (sf)	Area (ac)	Base Curve Number	Impervious Cover (sf)	TOC (min)	Area (sq. mi.)	Impervious Cover (%)	Lag Time
EX-1	290,110	6.66	80	0.00	8.87	0.0104063	0%	5.32
EX-2	215,186	4.94	80	0.00	15.33	0.0077188	0%	9.20
EX-3	788,872	18.11	80	0.00	15.82	0.0282969	0%	9.49
EX-4	627,700	14.41	80	0.00	22.09	0.0225156	0%	13.26

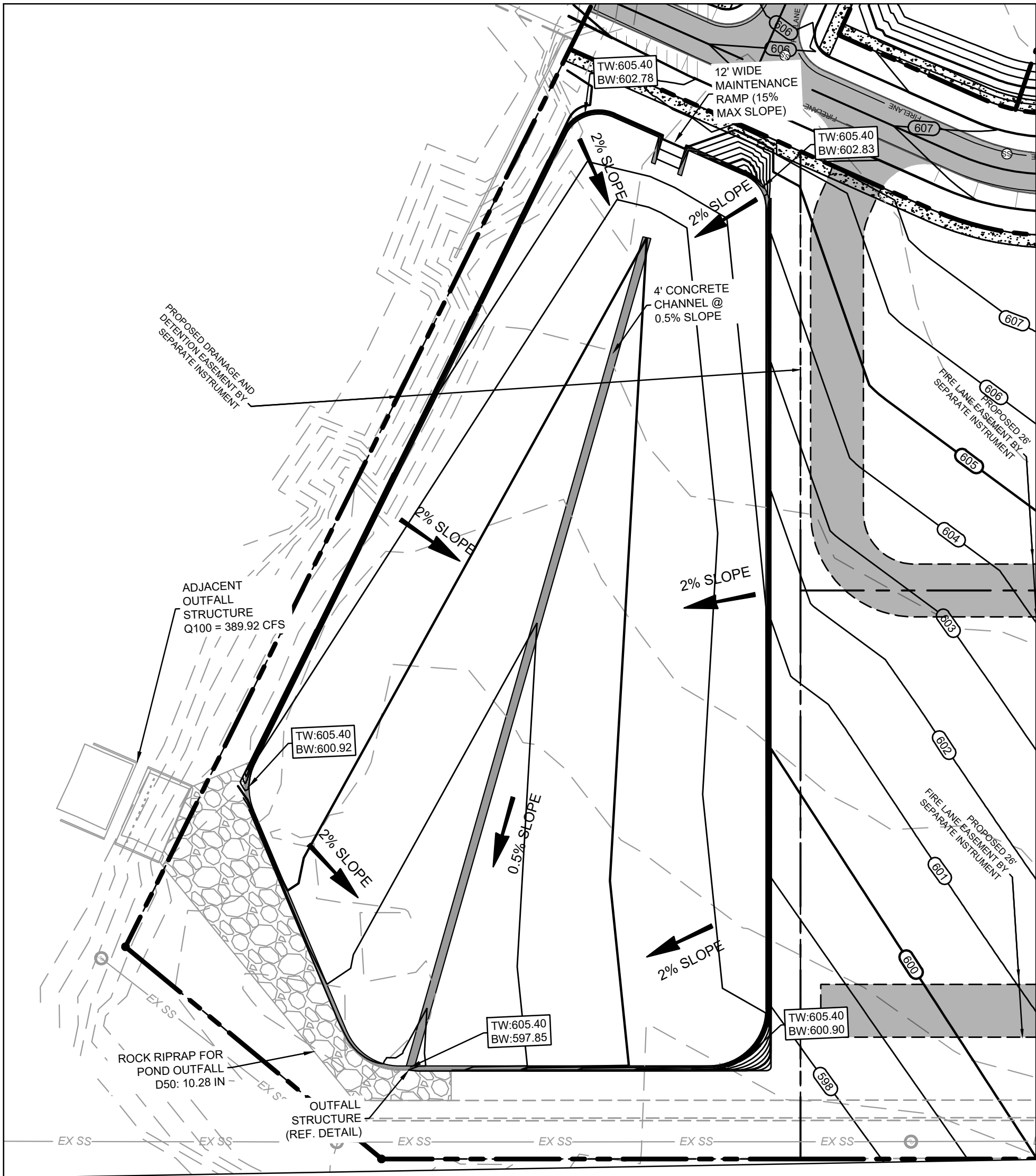
Time of Concentration Calculations - Existing Conditions							
Contributing Area	Sheet Flow				Shallow Concentrated Flow (Unpaved)		
	Length	Slope (ft/ft)	Roughness Coefficient	T _{sheet}	Length (ft)	Slope (ft/ft)	T _{unpaved}
EX-1	100	0.01	0.06	5.99	440	0.025	2.87
EX-2	100	0.01	0.06	5.99	700	0.006	9.34
EX-3	100	0.02	0.06	4.54	1196	0.012	11.28
EX-4	100	0.01	0.06	5.99	1454	0.0087	16.10

Rip Rap Calculator	
D50	8.71 in
Q	74.4 cfs
D	3.5 ft
WSEL	604.4 ft
INV	597.85 ft

DETENTION POND DRAWDOWN TIME



Water Surface Elev.	
Storm Event	Elevation
2-Year	601.7
10-Year	602.7
25-Year	603.4
100-Year	604.4



PRELIMINARY
FOR REVIEW ONLY
THESE DOCUMENTS ARE FOR
DESIGN REVIEW ONLY AND
NOT INTENDED FOR THE PURPOSES
OF CONSTRUCTION, BIDDING
OR PERMIT. THEY WERE PREPARED
BY, OR UNDER THE SUPERVISION OF:
JONATHAN E. SCHINDLER
P.E.# 108864
DATE: September 13, 2024

FASH TAYLOR, LTD
812 W. MCDERMOTT DRIVE
PMB 1305
ALLEN, TX 75013
DEVELOPER:
CONTACT: JOHN CHONG

TAYLOR POINTE
715 CARLOS G PARKER BLVD
CITY OF TAYLOR
WILLAMSON COUNTY, TEXAS

REV. DATE. DESCRIPTION:

K
kirkman
ENGINEERING
KIRKMAN ENGINEERING, LLC
5200 STATE HIGHWAY 121
COLLEYVILLE, TX 76034
TEXAS FIRM NO. 15874

JOB NUMBER: FSH24001
ISSUE DATE:

DETENTION
EXHIBIT

SHEET:

V1.0

Soil Map—Williamson County, Texas
(FSH24001_Taylor)



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Williamson County, Texas

Survey Area Data: Version 24, Sep 5, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 2, 2022—May 17, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BrB	Branyon clay, 1 to 3 percent slopes	42.2	93.5%
HedC2	Heiden clay, 2 to 5 percent slopes, moderately eroded	2.9	6.5%
Totals for Area of Interest		45.2	100.0%

Engineering Properties

This table gives the engineering classifications and the range of engineering properties for the layers of each soil in the survey area.

Hydrologic soil group is a group of soils having similar runoff potential under similar storm and cover conditions. The criteria for determining Hydrologic soil group is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Listing HSGs by soil map unit component and not by soil series is a new concept for the engineers. Past engineering references contained lists of HSGs by soil series. Soil series are continually being defined and redefined, and the list of soil series names changes so frequently as to make the task of maintaining a single national list virtually impossible. Therefore, the criteria is now used to calculate the HSG using the component soil properties and no such national series lists will be maintained. All such references are obsolete and their use should be discontinued. Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonal high water table, saturated hydraulic conductivity after prolonged wetting, and depth to a layer with a very slow water transmission rate. Changes in soil properties caused by land management or climate changes also cause the hydrologic soil group to change. The influence of ground cover is treated independently. There are four hydrologic soil groups, A, B, C, and D, and three dual groups, A/D, B/D, and C/D. In the dual groups, the first letter is for drained areas and the second letter is for undrained areas.

The four hydrologic soil groups are described in the following paragraphs:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Depth to the upper and lower boundaries of each layer is indicated.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."

Classification of the soils is determined according to the Unified soil classification system (ASTM, 2005) and the system adopted by the American Association of State Highway and Transportation Officials (AASHTO, 2004).

The Unified system classifies soils according to properties that affect their use as construction material. Soils are classified according to particle-size distribution of the fraction less than 3 inches in diameter and according to plasticity index, liquid limit, and organic matter content. Sandy and gravelly soils are identified as GW, GP, GM, GC, SW, SP, SM, and SC; silty and clayey soils as ML, CL, OL, MH, CH, and OH; and highly organic soils as PT. Soils exhibiting engineering properties of two groups can have a dual classification, for example, CL-ML.

The AASHTO system classifies soils according to those properties that affect roadway construction and maintenance. In this system, the fraction of a mineral soil that is less than 3 inches in diameter is classified in one of seven groups from A-1 through A-7 on the basis of particle-size distribution, liquid limit, and plasticity index. Soils in group A-1 are coarse grained and low in content of fines (silt and clay). At the other extreme, soils in group A-7 are fine grained. Highly organic soils are classified in group A-8 on the basis of visual inspection.

If laboratory data are available, the A-1, A-2, and A-7 groups are further classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-7-5, or A-7-6. As an additional refinement, the suitability of a soil as subgrade material can be indicated by a group index number. Group index numbers range from 0 for the best subgrade material to 20 or higher for the poorest.

Percentage of rock fragments larger than 10 inches in diameter and 3 to 10 inches in diameter are indicated as a percentage of the total soil on a dry-weight basis. The percentages are estimates determined mainly by converting volume percentage in the field to weight percentage. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Percentage (of soil particles) passing designated sieves is the percentage of the soil fraction less than 3 inches in diameter based on an oven-dry weight. The sieves, numbers 4, 10, 40, and 200 (USA Standard Series), have openings of 4.76, 2.00, 0.420, and 0.074 millimeters, respectively. Estimates are based on laboratory tests of soils sampled in the survey area and in nearby areas and on estimates made in the field. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Liquid limit and plasticity index (Atterberg limits) indicate the plasticity characteristics of a soil. The estimates are based on test data from the survey area or from nearby areas and on field examination. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

References:

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Report—Engineering Properties

Absence of an entry indicates that the data were not estimated. The asterisk "*" denotes the representative texture; other possible textures follow the dash. The criteria for determining the hydrologic soil group for individual soil components is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Engineering Properties—Williamson County, Texas														
Map unit symbol and soil name	Pct. of map unit	Hydrologic group	Depth	USDA texture	Classification		Pct Fragments		Percentage passing sieve number—				Liquid limit	Plasticity index
					Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		
			<i>In</i>				<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>
BrB—Branyon clay, 1 to 3 percent slopes														
Branyon	85	D	0-12	Clay	CH	A-7-6	0- 0- 0	0- 0- 0	94-96-100	78-92-100	73-90-100	64-79-92	59-63-69	35-39-43
			12-72	Clay, silty clay	CH	A-7-6	0- 0- 0	0- 0- 0	95-96-100	78-92-100	69-90-100	62-82-94	59-69-74	39-43-47
			72-80	Clay, silty clay, silty clay loam, clay loam	CH, CL	A-7-6, A-6	0- 0- 0	0- 0- 0	95-96-100	79-92-100	65-91-100	60-85-97	40-60-64	23-34-47
HedC2—Heiden clay, 2 to 5 percent slopes, moderately eroded														
Heiden, moderately eroded	85	D	0-6	Clay	CH	A-7-6	0- 0- 0	0- 0- 0	96-98-100	90-96-100	80-94-100	65-81-94	50-60-80	30-40-55
			6-18	Clay, silty clay	CH	A-7-6	0- 0- 0	0- 0- 0	96-98-100	90-96-100	80-94-100	65-81-98	50-60-80	30-40-55
			18-58	Clay, silty clay	CH	A-7-6	0- 0- 0	0- 0- 0	96-98-100	90-96-100	80-94-100	65-81-98	50-60-80	30-40-55
			58-80	Clay	CH	A-7-6	0- 0- 0	0- 0- 0	98-100-100	97-100-100	86-98-100	71-86-95	50-70-80	30-45-55



Data Source Information

Soil Survey Area: Williamson County, Texas
Survey Area Data: Version 24, Sep 5, 2023



City of Taylor
PZ-2024-2254
North Bull Branch Creek Townhomes Subdivision Improvement Plan
Staff Report

Item Details

Agenda No.	4
Requested Action:	Consider Disapproval of North Bull Branch Creek Townhomes Subdivision Improvement Plan
Address/Location:	Generally located along the South right-of-way of N.W. Carlos G. Parker Boulevard, approximately 1,400 feet west of the intersection of N.W. Carlos G. Parker Boulevard and North Drive
Legal Description:	Approximately 7.747 acres of land part of and out of the William J Baker Survey, Abstract No. 65, more particularly described by the Williamson Central Appraisal District Parcel R018638, Taylor, Williamson County, Texas
Current Zoning:	P2.5 – Large Lot
Current Use:	Vacant Acreage
Applicant:	Sam Dehaybi, Paradise Engineers, LLC
Case History:	This is the second submission of the Subdivision Improvement Plans
Staff Recommendation:	The submitted Subdivision Improvement Plans do not meet the minimum requirements of the Land Development Code and Engineering Manual. Therefore, staff recommends Disapproval of the proposed Subdivision Improvement Plans.

Attachments

1. Staff Comments
2. Location Map



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

Sam Dehaybi
Paradise Engineers LLC
5900 Balcones Drive Suite 100
Austin TX 78731
sdehaybi@paradise-engineers.com

Date: Friday, September 27, 2024

Address: TBD CARLOS G PARKER, Wichita 67207

Permit Number PZ-2024-2254

Dear Sam Dehaybi,

Staff has completed its review of plans for the North BB Creek Townhouses - SIP that is to be located at TBD CARLOS G PARKER, Wichita 67207. There are some corrections that need to be made prior to acceptance of the plans. Comments from this review follow.

Planning Department Comments

The following comments have been provided by Courtney Peres. Should you have any questions or require additional information regarding any of these comments, please contact Courtney Peres at (512) 309-6268 or by email at courtney.peres@taylor.tx.gov.

1. The extension of the trail corridor must be incorporated into the Subdivision Improvements. Please provide details to identify the infrastructure for a 10–12-foot paved trail through the site as shown on the site plan.

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier Vasquez by email at javier.vasquez@hdrinc.com.

*See Associated Documents section of the permit page to download a PDF containing the following engineering comments:

Drainage Report - Engineering Review 02

1. General - As discussed in the Jan 2023 DIA scoping meeting, a drainage study is to be prepared pursuant to section 3.22 of the EM which includes a hydrologic analysis of the upstream drainage areas.
2. Page 2 - Per EM 3.20/3.22, the developer and engineer are responsible for determining the extents of the floodplain and providing supporting hydrology. Please provide a hydrologic analysis that supports the flows entered in the hydraulic model.
3. Page 4 - Please provide further detail or supporting data to explain how the creek elevation was determined.
4. Page 4 - Please verify this elevation matches the plans.
5. Page 4 - Please provide further detail or supporting calculations used to determine the total drawdown time with the tailwater considerations.
6. Page 166 of 182 - Orifice Drawdown Calculations - Please provide more detail as to how the volume provided and depth were calculated.

*See Associated Documents section of the permit page to download a PDF containing the following engineering comments:

Geotechnical Report - Engineering Review 02

1. Sheet - 4 of 67 - How was design ESAL determined and does it meet representative values provided in CoA TCM?
2. Sheet - 13 of 67 - Were pH and sulfate tests run per CoT EM?
3. Sheet - 24 of 67 - Include input parameters in Table 2 as stated.
4. Sheet - 25 of 67 - Define street type per current CoT EM.
5. Sheet - 25 of 67 - How was design subgrade modulus determined?
6. Sheet - 25 of 67 - What is the 2% cement based on?

7. Sheet - 26 of 67 - What materials parameters are being achieved by this lime rate?
8. Sheet - 28 of 67 - Do these construction guidelines meet the CoT EM requirements?
9. Sheet - 29 of 67 - Where is select fill being used related to the streets?
10. Sheet - 30 of 67 - Do pavement recommendations need to be re-evaluated based on the general fill recommendation?
11. Sheet - 30 of 67 - % Compaction and water content should be in accordance with the current CoT EM.
12. Sheet - 42 of 67 - Provide boring location plan with current street layout, do boring spacings meet the requirements per the the current CoT EM?
13. Sheet - 50 of 67 - Is the pavement design based on 1 sulfate content test?
14. Sheet - 53 of 67 - Does required percent lime provide this elastic modulus and are you following TxDOT Test Method Tex-121-E?
15. Sheet - 60 of 67 - How was 12,000 determined from a CBR of 3, too high for CH clay.
16. Sheet - 60 of 67 - Does required percent cement provide this elastic modulus and are you following TxDOT Test Method Tex-120-E?

*See Associated Documents section of the permit page to download a PDF containing the following engineering comments:

Subdivision Improvement Plans - Engineering Review 02

1. Sheet - [05] 5 SITE PLAN - Verify sidewalk requirements and planter area for this portion of the project. Refer to snippet taken from the approved amended RPD. It appears the planter area is to be closest to the perimeter line and then placement of sidewalk. The required buffer shall also be considered within this space.
2. Sheet - [05] 5 SITE PLAN - Label Civic Space
3. Sheet - [05] 5 SITE PLAN - Label trail and width of proposed trail
4. Sheet - [05] 5 SITE PLAN - Label section of sidewalk that vary from 5-ft
5. Sheet - [05] 5 SITE PLAN - Show sidewalk and connect all sidewalk. Curb ramps to be provided accordingly.
6. Sheet - [06] 6 SIGNAGE & STRIPING PLAN - Verify and notate signage and striping at driveway entrance to Carlos G. Parker (e.g. yield or stop, right only, do not enter, etc.). Should a separate plan apply here, place note accordingly
7. Sheet - [09] 9 WALTER HEAP STREET PLAN & PROFILE - Per PD respective street types to reflect parking lane width accordingly (7' and 8'). Recommend clarifying this in a striping plan
8. Sheet - [12] 12 DETENTION POND - Retaining wall design to follow requirements found under Taylor EM 7.8 Retaining Walls
9. Sheet - [13] 13 STORM PLAN - Street Connection Area - Reference proposed information or ancillary planset for this culvert installation
10. Sheet - [13] 13 STORM PLAN - Acceptance of the non-standard inlet is being reviewed by COT Public Works staff for conditional approval. A waiver letter may be required.
11. Sheet - [13] 13 STORM PLAN - Revise text shown for Rim information
12. Sheet - [15] 15 WATER PLAN - Block A water meters are graphically shown skewed; however, these water meters to be perpendicular to ROW line.
13. Sheet - [15] 15 WATER PLAN - Bends no longer shown, remove callout.
14. Sheet - [15] 15 WATER PLAN - Only two valves required here.
15. Sheet - [15] 15 WATER PLAN - Only two valves required here.
16. Sheet - [18] 18 SEWER PLAN - Verify proposed sewer line is within ROW or civic space.
17. Sheet - [18] 18 SEWER PLAN - Proposed layout is not acceptable. Provide one main line and have dual services connected to proposed main. An end of line cleanout will be acceptable only if Taylor EM Sec. 6.8 is followed.
18. Sheet - [21] 21 SITE DETAILS (1 OF 3) - ADA Ramp Detail was removed. Coordination to be made for acceptable details not available within City Standard Details.
19. Sheet - [21] 21 SITE DETAILS (1 OF 3) - Include details for typical street ROW sections and assignments used (all types including private alley)
20. Sheet - [23] 23 SITE DETAILS (3 OF 3) - Provide street section to include typical ROW assignments for water/wastewater/storm sewer (See Taylor Engineering Manual Chapter 2-2). Include a section for private alleys.
21. Sheet - [21] 21 SITE DETAILS (1 OF 3) - Define geogrid type.
22. Sheet - [21] 21 SITE DETAILS (1 OF 3) - Are prime and tack coats required, and what type of Asphalt is required? Specify on detail.

Fire Department Comments (PZ)

The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylorlx.gov.

1. 2 of the 3 previous comments have been addressed. I spoke with Sam Dehaybi on 9/16/2024 and informed him that any units that exceed 2 units connected shall have fire suppression systems. Sam stated that he would inform the client and evaluate the water supply lines for the units that require fire suppression systems.

Previous comment not address:

2. 3. Attached units that are more than 2connected living units shall incorporate fire suppression systems to be installed. The plans show the individual water lines to each units are 1 inch. A fire suppression company shall be consulted to ensure the 1-inch water line is sufficient to supply the fire suppression system to each unit.

Please revise the plans to address the comments noted above. Following revision, submit the plans electronically with a resolution letter in PDF format. Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs. Should you have questions or require additional information regarding the plan review process itself, please feel free to contact me directly. I can be reached by telephone at (512) 309-6268, or by e-mail at courtney.peres@taylortx.gov.

Thank you,



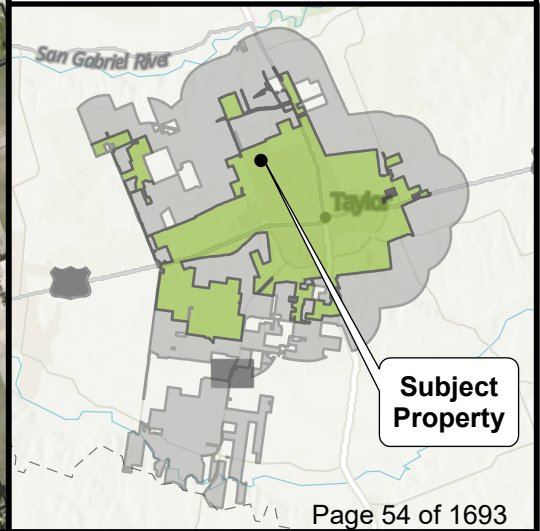
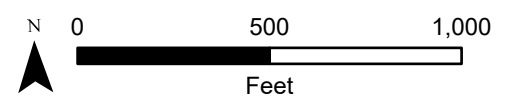
Courtney Peres, CNU-A
Planning Manager
City of Taylor



PZ-2024-2254

TBD Carlos G Parker Blvd.
Subdivision Improvement Plans
Location Map
Approximately 7 acres

- City Limits
- ETJ Boundary
- Subject Property
- Parcels



City of Taylor
PZ-2024-2291
Nyle Maxwell – Minor Subdivision Plat
Staff Report

Item Details

Agenda No.	5
Requested Action:	Consider Disapproval of Nyle Maxwell Minor Subdivision Plat
Address/Location:	Generally located at 14150 Highway 79
Legal Description:	8.010 acres of land part of the William Kincaid Survey, Abstract No. 0373, more particularly described by Williamson Central Appraisal District Parcels R311124, R019359, R331352, Taylor, Williamson County, Texas.
Current Zoning:	EC – Employment Center
Current Use:	Car Dealership
Applicant:	James Barker – Texas Land Surveying
Case History:	This is the first submission of the Minor Subdivision plat.
Staff Recommendation:	The submitted Minor Subdivision plat does not meet the minimum requirements of the Land Development Code and Engineering Manual. Therefore, staff recommends Disapproval of the proposed Minor Subdivision Plat.

Attachments

- 1. Staff Comments
- 2. Location Map
- 3. Proposed Minor Plat



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

JAMES BARKER
TEXAS LAND SURVEYING
3613 WILLIAMS DR #903
GEORGETOWN TX 78628
james@texas-ls.com

Date: October 2, 2024

Address: 14150 HWY 79, TAYLOR 76574

Permit Number PZ-2024-2291

Dear JAMES BARKER,

Staff has completed its review of plans for the Nyle Maxwell - 14150 Hwy 79 - Minor Subdivision Plat that is to be located at 14150 HWY 79, TAYLOR 76574. There are some corrections that need to be made prior to recordation of the plat. Comments from this review follow.

Planning Department Comments

The following comments have been provided by Courtney Peres. Should you have any questions or require additional information regarding any of these comments, please contact Courtney Peres by email at courtney.peres@taylortx.gov.

1. The Legal Description alludes to X acres of right-of-way dedication but there is not any dedication shown on the plat. Please identify the total right-of-way of the bounding roadways (CR 402 and State Highway 79). Label as "Right-of-way Width". Along County Road 402 it is identified as a Community Boulevard in the City's Transportation Master Plan. Community Boulevard are required to be 100' right-of-way. Please determine if dedication is needed and revise accordingly.
2. The Plat identifies 2.999 and 5.011 acres as separate acreages. Is the plat not going to be combined into 1 lot from 3? The total acreage should be reflected only.
3. What are the dashed lines bisecting the southern part of the property?
4. Remove "14150" from the State Highway 79 road name.
5. Provide Engineer's company name and contact information.
6. Clarify Plat Note #3 – is this required?
7. Revise Plat Note #4 Septic is Williamson County.
8. Remove Plat Note #9 – not applicable to this property.
9. Remove Plat Note #12 – not applicable to this property.
10. Revise Plat Note #14 – confirm that there is a sewer tap...
11. Remove Planning and Zoning Commission Signature Block.
12. Revise Development Services Director's name to Scott Dunlop

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier Vasquez by email at javier.vasquez@hdrinc.com.

Minor Plat - Engineering Review 01

Sheet 1 of 2

1. Update permit number.
2. Place plat heading and legal description top center of the plat document.
3. Remove asphalt and only show plat items such as street name, ROW information, and dimensioning.
4. Orient plat drawing with true north matching top of sheet.
5. Location of existing or proposed detention to be provided as a drainage easement.
6. Remove asphalt and only show plat items such as street name, ROW information, and dimensioning.
7. Label these areas on plat (railroad?).
8. Verify and update this section of the scale bar. It appears to be slightly off from 1"=50'.
9. Notate point of beginning on the plat drawing.

10. Show location of existing creek/water feature and provide a dedicated easement for this facility and crossing to CR 402

Sheet 2 of 2

1. Verify and update plat note. Septic is permitted by Williamson County and an OSSF plat certificate is to be provided below.
2. The domestic water is currently served by Jonah. Please verify the requirement of both domestic and fire being provided by City of Taylor. Extension of water will be required.
3. Engineer's Certificate - Provide name in blank.
4. Update name for Development Services Director.
5. Verify and update to match plat drawing which shows County Road 402.

Fire Department Comments (PZ)

The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylor.tx.gov.

1. The plat shall correct the wording of plat note #13. The plat note shall read: The minimum fire flow shall be in accordance with the current adopted International Fire Code or recognized standards for fire flows.

Please revise the plat to address the comments noted above. Following revision, submit the plat electronically with a resolution letter in PDF format. Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs.

Thank you,



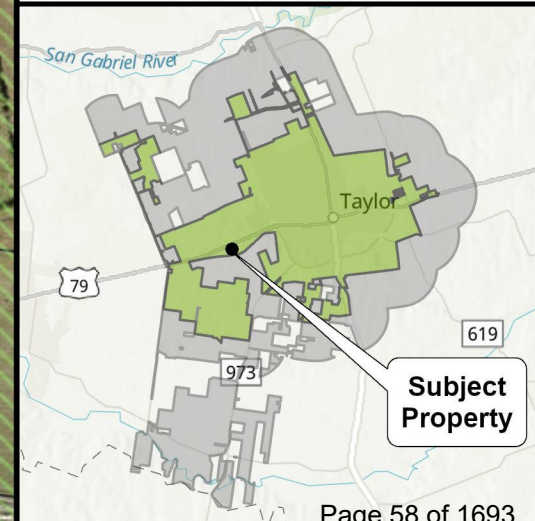
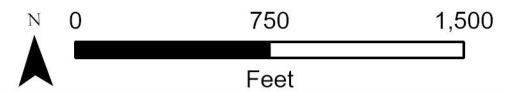
Courtney Peres, CNU-A
Planning Manager
City of Taylor



PZ-2024-2291

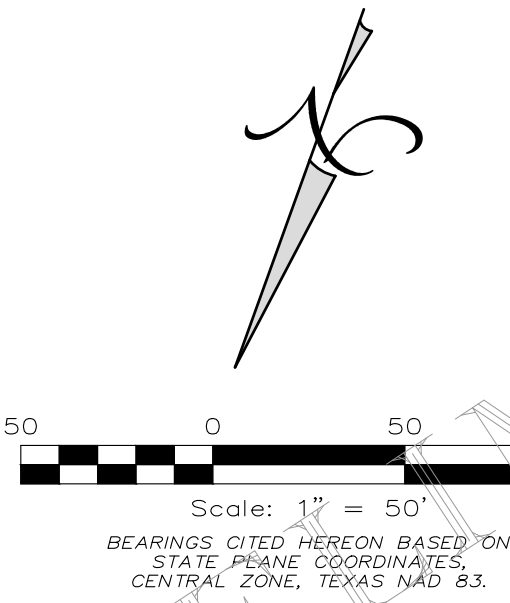
14150 Hwy 79
Minor Subdivision Plat
Location Map
Approximately 8 acres

-  City Limits
-  ETJ Boundary
-  Subject Property
-  Parcels



MINOR PLAT
NYLE MAXWELL TAYLOR

BEING A MINOR PLAT CONSISTING OF 8.010 ACRES OF LAND, MORE OR LESS, INCLUDING _____ ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE ONE LOT, WITH NO RESERVES, OUT OF THE WILLIAM KINCAID SURVEY, ABSTRACT NO. 373, AND THE JAMES C. EAVES SURVEY, ABSTRACT NO. 214, TAYLOR, WILLIAMSON COUNTY, TEXAS, CONSISTING OF THE FOLLOWING TWO TRACTS: BEING ALL OF THAT TRACT CALLED 5.010 ACRES AS CONVEYED TO AUSTIN MKA INVESTMENTS, LTD. AND RECORDED IN DOCUMENT NO. 2010078781, OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, TEXAS, ALSO BEING ALL OF THAT TRACT CALLED 3.00 ACRES AS CONVEYED TO NYLE MAXWELL OF TAYLOR, LLC, AND RECORDED IN DOCUMENT NO. 2009075788, OF SAID OFFICIAL PUBLIC RECORDS,



COUNTY ROAD 402
(A.K.A. HERMANN SONS ROAD)
PAVED ROAD

MELVIN RAY RANDIG & DORIS RANDIG
REMAINDER OF (5 ACRES)
VOLUME 461, PAGE 116
D.R.W.C.

2.999
ACRES

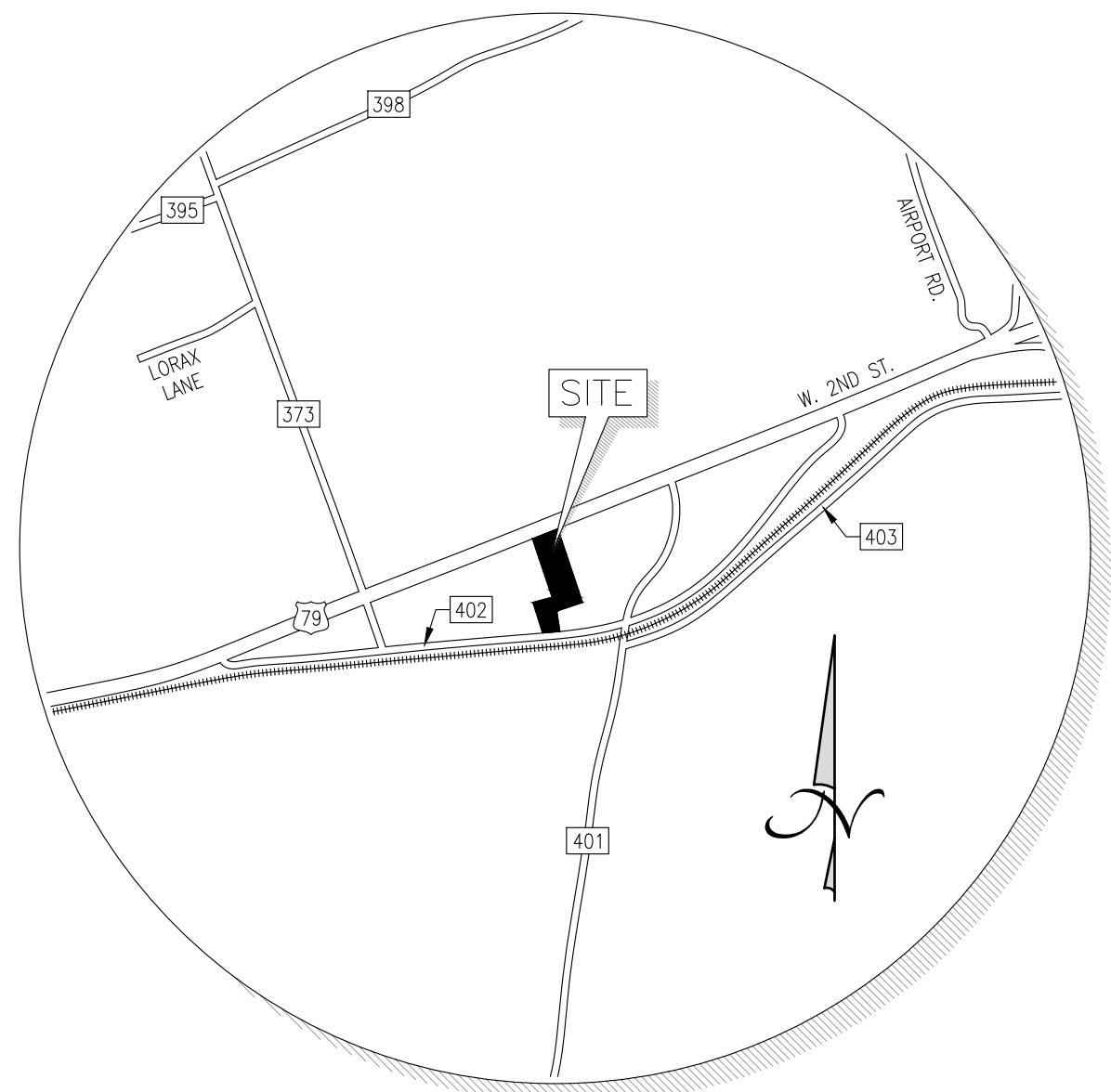
NYLE MAXWELL OF TAYLOR, LLC
(3.00 ACRES)
DOCUMENT NO. 2009075788
O.P.R.W.C.

LOT 1
BLOCK "A"

5.011
ACRES

AUSTIN MKA INVESTMENTS LTD.
(5.010 ACRES)
DOCUMENT NO. 2010078781
O.P.R.W.C.

14150 STATE HIGHWAY 79
PAVED ROAD



LOCATION MAP
SCALE: 1"=2000'

LEGEND	
●	1/2" IRON ROD FOUND (UNLESS OTHERWISE NOTED)
○	1/2" PIPE FOUND
⊗	X FOUND IN CONCRETE
○	1/2" IRON ROD SET WITH PINK CAP STAMPED 'TLS'
—	PAVEMENT
■	FENCE POST
XX	CHAIN LINK FENCE
X	PIPE FENCE
W	WOOD FENCE
●	GUARD POST
—	GUARD RAIL
U	UTILITY POLE
K	GUY ANCHOR
E	ELECTRIC LINES
—	ELECTRIC/TELEPHONE LINES
T	TELEPHONE LINES
—	AIR CONDITIONER
—	CELANOLIT
—	ELECTRIC
—	FLAG POLE
—	LIGHT POLE
—	MAIL BOX
—	SATELLITE DISH POLE
—	SEPTIC ALARM
—	SIGN
—	TRANSFORMER
—	UNKNOWN UTILITY (UNLESS OTHERWISE NOTED)
—	WATER METER
—	WATER VALVE
—	RECORD INFORMATION
—	CONCRETE
—	RIGHT-OF-WAY
—	DEED RECORDS OF WILLIAMSON COUNTY, TEXAS
—	OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS
—	OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS
—	PLAN RECORDS OF WILLIAMSON COUNTY, TEXAS
—	P.R.W.C.

LINE TABLE		
NUMBER	DIRECTION	DISTANCE
L1	N 66°59'02" E	308.03'

(DOCUMENT NO. 2010078781)

RECORD LINE TABLE		
NUMBER	DIRECTION	DISTANCE
L1	N 69°38'00" E	308.06'

LOT SUMMARY TABLE	
TOTAL SITE	8.010 ACRES (348,918.28 SQ. FT.)
LOT 1, BLOCK A	8.010 ACRES (348,918.28 SQ. FT.)

NOTES:

—SETBACKS NOT SHOWN ON LOTS SHALL CONFORM TO THE CITY OF TAYLOR ZONING ORDINANCE.

MINOR PLAT
NYLE MAXWELL TAYLOR

OWNERS: AUSTIN MKA INVESTMENTS, LTD.
NYLE MAXWELL, REGISTERED AGENT
3400 VINTAGE DRIVE
ROUND ROCK, TX 78664

NYLE MAXWELL OF TAYLOR, LLC
T. NYLE MAXWELL, JR., MANAGER
13401 N. FM-620
AUSTIN, TX 78717

SUBMISSION DATE: AUGUST 30, 2024

SURVEYOR: Texas Land Surveying, Inc.
3613 Williams Drive, Ste 903
Georgetown, Texas 78628
512-930-1600 - phone
EMAIL: Rod@Texas-LS.com

SHEET INDEX

SHEET NO.	TITLE
1	MINOR PLAT
2	PLAT NOTES, PERIMETER FIELD NOTES & SIGNATURE BLOCKS

PZ-2024-XXXX

Texas Land Surveying, Inc.

—A Land Surveying Firm—
3613 Williams Drive, Suite 903 Georgetown, Texas 78628
(512) 930-1600 www.texas-ls.com
TBPLS FIRM NO.10056200

IF THIS DOCUMENT DOES NOT CONTAIN THE STAMPED SEAL OF THE UNDERSIGNED SURVEYOR, IT IS AN UNAUTHORIZED/ILLEGAL COPY. TEXAS LAND SURVEYING, INC. ASSUMES NO LIABILITY FROM THE USE OF ANY UNAUTHORIZED/ILLEGAL DOCUMENT.

SHEET

1 OF 2

MINOR PLAT
NYLE MAXWELL TAYLOR

BEING A MINOR PLAT CONSISTING OF 8.010 ACRES OF LAND, MORE OR LESS, INCLUDING ----- ACRES OF RIGHT-OF-WAY DEDICATION, TO CREATE ONE LOT, WITH NO RESERVES, OUT OF THE WILLIAM KINCAID SURVEY, ABSTRACT NO. 373, AND THE JAMES C. EAVES SURVEY, ABSTRACT NO. 214, TAYLOR, WILLIAMSON COUNTY, TEXAS, CONSISTING OF THE FOLLOWING TWO TRACTS: BEING ALL OF THAT TRACT CALLED 5.010 ACRES AS CONVEYED TO AUSTIN MKA INVESTMENTS, LTD. AND RECORDED IN DOCUMENT NO. 2009075788, OF SAID OFFICIAL PUBLIC RECORDS, AS SURVEYED ON THE GROUND BY TEXAS LAND SURVEYING, INC. ON APRIL 22ND, 2024, AND FURTHER DESCRIBED BY METES AND BOUNDS AS FOLLOWS:

BEGINNING AT AN "X" FOUND IN CONCRETE IN THE SOUTH RIGHT-OF-WAY LINE OF STATE HIGHWAY 79, AT THE NORTHWEST CORNER OF THAT TRACT CALLED THE REMAINDER OF 290.77 ACRES AS CONVEYED TO PREWITT FARMS, L.T.D., AND RECORDED IN DOCUMENT NO. 2007076169, OF SAID OFFICIAL PUBLIC RECORDS, AND AT THE NORTHEAST CORNER OF SAID 5.010 ACRE TRACT AS CONVEYED TO AUSTIN MKA INVESTMENTS, LTD AND RECORDED IN DOCUMENT NO. 2010078781 OF THE OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, FOR THE NORTHEAST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: S 19°46'57"E, WITH THE WEST LINE OF THE SAID 290.77 ACRE TRACT AND THE EAST LINE OF THE SAID 5.010 ACRE TRACT, FOR A DISTANCE OF 727.18' FEET, TO A 1/2 INCH PIPE FOUND, AT THE SOUTHEAST CORNER OF SAID 5.010 ACRE TRACT, AND AT THE NORTHEAST CORNER OF A 3.00 ACRE TRACT, AS CONVEYED TO NYLE MAXWELL AND RECORDED IN DOCUMENT #2009075788 OF THE OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, TEXAS, FOR AN ANGLE POINT IN THE HEREIN DESCRIBED TRACT;

THENCE: S 19°39'52"E, WITH THE WEST LINE OF THE SAID 290.77 ACRE TRACT AND THE EAST LINE OF THE SAID 3.00 ACRE TRACT, FOR A DISTANCE OF 195.13' FEET, TO A 3/8 INCH IRON ROD FOUND, AT THE EAST SOUTHEAST CORNER OF THE SAID 3.00 ACRE TRACT, AND AT THE NORTHEAST CORNER OF A REMAINDER OF 5 ACRES AS CONVEYED TO MELVIN RAY RANDIG, ET UX, AND DESCRIBED IN VOLUME 461, PAGE 116, DEED RECORDS, WILLIAMSON COUNTY, TEXAS, FOR THE EAST SOUTHEAST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: S 70°17'34"W, WITH THE EASTERLY SOUTH LINE OF THE SAID 3.00 ACRE TRACT AND THE NORTH LINE OF THE SAID REMAINDER OF 5 ACRE TRACT, FOR A DISTANCE OF 305.50' FEET, TO A 3/8 INCH IRON ROD FOUND, AT AN ELL CORNER OF THE SAID 3.00 ACRE TRACT AND AT THE NORTHWEST CORNER OF THE SAID 5 ACRE TRACT, FOR AN ELL CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: S 09°46'49"E, WITH THE EAST LINE OF THE SAID 3.00 ACRE TRACT AND THE WEST LINE OF THE SAID 5 ACRE TRACT, FOR A DISTANCE OF 228.46' FEET, TO AN IRON ROD WITH YELLOW CAP STAMPED "ALL POINTS" FOUND IN THE NORTH LINE OF COUNTY ROAD 403 (HERMANN SONS ROAD), AT THE SOUTHEAST CORNER OF THE SAID 3.00 ACRE TRACT, AND AT THE SOUTHWEST CORNER OF THE SAID 5 ACRE TRACT, FOR THE SOUTHEAST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: S 82°59'30"W, WITH THE NORTH LINE OF SAID COUNTY ROAD 403 AND THE SOUTH LINE OF THE SAID 3.00 ACRE TRACT, FOR A DISTANCE OF 150.23' FEET, TO A 1/2 INCH IRON ROD FOUND, AT THE SOUTHEAST CORNER OF A 0.33 ACRE TRACT, AS CONVEYED TO JESUS LOPEZ, ET UX, AND RECORDED IN DOCUMENT NO. 2002097812, OF SAID OFFICIAL PUBLIC RECORDS, AND AT THE SOUTHWEST CORNER OF THE SAID 3.00 ACRE TRACT, FOR THE SOUTHWEST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: N 18°59'50"W, WITH THE EAST LINE OF THE SAID 0.33 OF AN ACRE TRACT AND THE WEST LINE OF THE SAID 3.00 ACRE TRACT, FOR A DISTANCE OF 361.12 FEET, TO A 1/2 INCH IRON ROD WITH PINK CAP STAMPED "TLS" SET IN THE SOUTH LINE OF A 8.02 ACRE TRACT AS CONVEYED TO CLARENCE H. WALTHER, ET UX, AND RECORDED IN DOCUMENT NO. 9722470, OFFICIAL RECORDS, WILLIAMSON COUNTY, TEXAS, AT THE NORTHEAST CORNER OF SAID 0.33 ACRE TRACT, AND AT THE NORTHWEST CORNER OF SAID 3.00 ACRE TRACT, FOR THE MOST WEST NORTHWEST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: N 73°18'29"E, WITH THE SOUTH LINE OF SAID 8.02 ACRE TRACT AND THE NORTH LINE OF SAID 3.00 ACRE TRACT, FOR A DISTANCE OF 223.66 FEET, TO A 1/2 INCH IRON ROD FOUND, AT THE SOUTHEAST CORNER OF THE SAID 8.02 ACRE TRACT, AND AT THE SOUTHWEST CORNER OF THE SAID 5.010 ACRE TRACT, FOR AN ELL CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: N 19°47'06"W, WITH THE EAST LINE OF THE SAID 8.02 ACRE TRACT AND THE WEST LINE OF THE SAID 5.010 ACRE TRACT, FOR A DISTANCE OF 692.44 FEET, TO A 1/2 INCH IRON ROD FOUND IN THE SOUTH LINE OF SAID STATE HIGHWAY 79, AT THE NORTHEAST CORNER OF SAID 8.02 ACRE TRACT, AND AT THE NORTHWEST CORNER OF THE SAID 5.010 ACRE TRACT, FOR THE MOST NORTHERLY NORTHWEST CORNER OF THE HEREIN DESCRIBED TRACT;

THENCE: N 66°59'02"E, WITH THE SOUTH LINE OF THE SAID STATE HIGHWAY 79 AND THE NORTH LINE OF THE SAID 5.010 ACRE TRACT, FOR A DISTANCE OF 308.03' FEET, TO THE PLACE OF BEGINNING, CONTAINING A COMPUTED AREA OF 8.010 ACRES OF LAND MORE OR LESS.

PERIMETER FIELD NOTES

DESCRIPTION OF A 8.010 ACRE TRACT OF LAND OUT OF AND A PART OF THE WILLIAM KINCAID SURVEY, ABSTRACT NO. 373, AND THE JAMES C. EAVES SURVEY, ABSTRACT NO. 214, WILLIAMSON COUNTY, TEXAS, CONSISTING OF THE FOLLOWING TWO TRACTS: BEING ALL OF THAT TRACT CALLED 5.010 ACRES AS CONVEYED TO AUSTIN MKA INVESTMENTS, LTD, AND RECORDED IN DOCUMENT NO. 2010078781, OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, TEXAS, ALSO BEING ALL OF THAT TRACT CALLED 3.00 ACRES AS CONVEYED TO NYLE MAXWELL OF TAYLOR, LLC, AND RECORDED IN DOCUMENT NO. 2009075788, OF SAID OFFICIAL PUBLIC RECORDS, AS SURVEYED ON THE GROUND BY TEXAS LAND SURVEYING, INC. ON APRIL 22ND, 2024, AND FURTHER DESCRIBED BY METES AND BOUNDS AS FOLLOWS:

BEGINNING AT AN "X" FOUND IN CONCRETE IN THE SOUTH RIGHT-OF-WAY LINE OF STATE HIGHWAY 79, AT THE NORTHWEST CORNER OF THAT TRACT CALLED THE REMAINDER OF 290.77 ACRES AS CONVEYED TO PREWITT FARMS, L.T.D., AND RECORDED IN DOCUMENT NO. 2007076169, OF SAID OFFICIAL PUBLIC RECORDS, AND AT THE NORTHEAST CORNER OF SAID 5.010 ACRE TRACT AS CONVEYED TO AUSTIN MKA INVESTMENTS, LTD AND RECORDED IN DOCUMENT NO. 2010078781 OF THE OFFICIAL PUBLIC RECORDS, WILLIAMSON COUNTY, FOR THE NORTHEAST CORNER OF THE HEREIN DESCRIBED TRACT;

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THENCE: S 09°46'49"E, WITH THE EAST LINE OF THE SAID 3.00 ACRE TRACT AND THE WEST LINE OF THE SAID 5 ACRE TRACT, FOR A DISTANCE OF 228.46' FEET, TO AN IRON ROD WITH YELLOW CAP STAMPED "ALL POINTS" FOUND IN THE NORTH LINE OF COUNTY ROAD 403 (HERMANN SONS ROAD), AT THE SOUTHEAST CORNER OF THE SAID 3.00 ACRE TRACT, AND AT THE SOUTHWEST CORNER OF THE SAID 5 ACRE TRACT, FOR THE SOUTHEAST CORNER OF THE HEREIN DESCRIBED TRACT;

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THENCE: N 66°59'02"E, WITH THE SOUTH LINE OF THE SAID STATE HIGHWAY 79 AND THE NORTH LINE OF THE SAID 5.010 ACRE TRACT, FOR A DISTANCE OF 308.03' FEET, TO THE PLACE OF BEGINNING, CONTAINING A COMPUTED AREA OF 8.010 ACRES OF LAND MORE OR LESS.

OWNERS' CERTIFICATION:

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

That I (we), Austin MKA Investments, Ltd., owner(s) of that certain 5.011 acre tract of land shown hereon and described in Deed(s) recorded in Document No(s). 2010078781 of the Official Public Records, of Williamson County, Texas, do hereby state that there are no lien holders of the certain tract of land, and do hereby subdivide said tract(s) as shown hereon, and do hereby consent to all plat note requirements shown hereon, and do hereby dedicate to the City of Taylor the streets, alleys, rights-of-way, easements and public places shown hereon for such public purposes as the City of Taylor may deem appropriate. This subdivision is to be known as **NYLE MAXWELL TAYLOR**.

TO CERTIFY WHICH, WITNESS by my hand this _____ day of _____, 20____.

Nyle Maxwell, registered agent
3400 Vintage Drive
Round Rock, TX 78664

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

Before me, the undersigned, a notary public in and for said County and State, on this day personally appeared Nyle Maxwell, GIVEN UNDER MY HAND AND SEAL of office this _____ day of _____, 20____.

Notary Public in and for the State of Texas
My Commission expires on: _____.

OWNERS' CERTIFICATION:

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

That I (we), Nyle Maxwell of Taylor, LLC, owner(s) of that certain 2.999 acre tract of land shown hereon and described in Deed(s) recorded in Document No(s). 2009075788 of the Official Public Records, of Williamson County, Texas, do hereby state that there are no lien holders of the certain tract of land, and do hereby subdivide said tract(s) as shown hereon, and do hereby consent to all plat note requirements shown hereon, and do hereby dedicate to the City of Taylor the streets, alleys, rights-of-way, easements and public places shown hereon for such public purposes as the City of Taylor may deem appropriate. This subdivision is to be known as **NYLE MAXWELL TAYLOR**.

TO CERTIFY WHICH, WITNESS by my hand this _____ day of _____, 20____.

T. Nyle Maxwell, Jr., manager
13401 N. FM-620
Austin, TX 78717

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

Before me, the undersigned, a notary public in and for said County and State, on this day personally appeared T. Nyle Maxwell, Jr. GIVEN UNDER MY HAND AND SEAL of office this _____ day of _____, 20____.

Notary Public in and for the State of Texas
My Commission expires on: _____.

PLAT NOTES

- THE BEARINGS SHOWN HEREON ARE RELATIVE TO THE TEXAS STATE PLANE GRID SYSTEM NAD=83, CENTRAL ZONE 4203, DISTANCES SHOWN ARE GRID DISTANCES. DISTANCES AND COORDINATES SHOWN ARE U.S. SURVEY FEET.
- SUBJECT TO EASEMENTS, CONDITIONS, COVENANTS AND RESTRICTIONS AS STATED OR SHOWN HEREON, AS REQUIRED BY THE CITY OF TAYLOR, TEXAS, AND ANY OF RECORD WHICH MAY BE APPLICABLE.
- A CURRENT TITLE COMMITMENT OR REPORT WAS NOT AVAILABLE OR PROVIDED TO THE UNDERSIGNED SURVEYOR AS OF THE DATE OF THIS SURVEY AND THE UNDERSIGNED SURVEYOR DID NOT ABSTRACT THE SUBJECT PROPERTY.
- UTILITY PROVIDERS – WATER: CITY OF TAYLOR, TEXAS; WASTEWATER / SEPTIC: CITY OF TAYLOR, TEXAS; ELECTRICITY: ONCOR ELECTRIC.
- NO PART OF THE SUBJECT PROPERTY LIES WITHIN A SPECIAL FLOOD HAZARD AREA ACCORDING TO THE FLOOD INSURANCE RATE MAP (FIRM) AS COMPILED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), FIRM PANEL 48491C0540F, REVISED DECEMBER 20, 2019.
- NO STRUCTURES AND/OR OBSTRUCTIONS ARE PERMITTED IN DRAINAGE EASEMENTS WITH PRIOR REVIEW AND APPROVAL BY THE CITY OF TAYLOR, TEXAS DEVELOPMENT SERVICES DEPARTMENT.
- ALL MINIMUM YARD (BUILDING) SETBACKS SHALL BE IN ACCORDANCE WITH CITY OF TAYLOR, TEXAS ORDINANCES AND REQUIREMENTS.
- TYPICAL FIRE SUPPRESSION SYSTEMS THAT MEET CHAPTER 9 FIRE SUPPRESSION SYSTEM OF THE INTERNATIONAL FIRE CODE AND NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) 13 REQUIREMENTS, OR THE MOST RECENT VERSION THEROF, AS REQUIRED BY THE CITY OF TAYLOR, TEXAS WILL BE DESIGNED ACCORDINGLY IN ALL PHASES THAT INCLUDE PLANNED BUILDING CONSTRUCTION.
- THIS SUBDIVISION IS SUBJECT TO CITY OF TAYLOR, TEXAS ORDINANCE 2022=25 (COMMERCIAL PLANNED DEVELOPMENT DISTRICT) AND CORRESPONDING SITE DEVELOPMENT PERMITS.
- ALL DRAINAGE AND DETENTION FACILITIES SHOWN HEREIN ARE PRIVATE FACILITIES. THE LAND OWNER AND/OR DEVELOPER SHALL BE REQUIRED TO MEET THE STANDARDS SPECIFIED IN THE OPERATIONS AND MAINTENANCE PLAN FOR DETENTION FACILITIES. THE CITY OF TAYLOR, TEXAS SHALL NOT BE RESPONSIBLE FOR MAINTENANCE ACTIVITIES FOR DRAINAGE AND DETENTION FACILITIES SHOWN HEREON.
- A 15 FOOT DRAINAGE EASEMENT AND A 20 FOOT PUBLIC UTILITY EASEMENT (P.U.E.) SHALL BE DEDICATED ADJACENT TO ALL APPLICABLE PUBLIC RIGHT-OF-WAYS SHOWN HEREON.
- THE SUBDIVIDER OR LOT OWNER AT THE TIME OF DEVELOPMENT IS RESPONSIBLE TO NOTE THE LOCATION OF SIDEWALKS SHALL BE CONSTRUCTED AND EXTENDED ACROSS THEIR PROPERTY ALONG THE PERIMETER OF ALL PROPERTY BOUNDARIES ADJACENT TO PUBLIC RIGHT-OF-WAYS AS INDICATED IN THE EFFECTIVE COMMERCIAL PLANNED DEVELOPMENT CODES. SIDEWALKS SHALL BE A MINIMUM OF FIVE (5) FEET IN WIDTH AND BE IN COMPLIANCE WITH THE AMERICANS WITH DISABILITIES ACT (ADA). ALL SIDEWALKS MUST BE IN COMPLIANCE WITH SPECIFICATIONS PROVIDED BY THE CITY ENGINEER.
- MINIMUM FIRE FLOW OF 500 GALLONS PER MINUTE FOR NOT LESS THAN TWO (2) HOURS SHALL BE PROVIDED. THE PLATTED PROPERTY HAS ADJACENT ACCESS TO THE CITY OF TAYLOR, TEXAS WATER SYSTEM. TYPICAL FIRE SUPPRESSION SYSTEMS AND FIRE PROTECTION WATER SUPPLY WILL COMPLY WITH THE CITY OF TAYLOR, TEXAS CODE REQUIREMENTS AND WILL BE DESIGNED ACCORDINGLY.
- AN INDIVIDUAL SEWER TAP AND WATER TAP ALREADY EXIST FOR THIS LOT.
- ALL LOT CORNERS DESCRIBED HEREIN AS 1/2" IRON RODS WITH PINK CAPS STAMPED "TLS" SHALL BE SET PRIOR TO ACCEPTANCE OF SUBDIVISION IMPROVEMENTS.

FLOODPLAIN ADMINISTRATOR'S CERTIFICATION:

Based upon the above representations of the Engineer or Surveyor whose seal is affixed hereto, and after a review of the plat as represented by the said Engineer or Surveyor, I find that this plat complies with the City of Taylor Floodplain Regulations. This certification is made solely upon such regulations and should not be relied upon for verifications of the facts alleged. The City of Taylor disclaims any responsibility to any member of the public for independent verification of the representations, factual or otherwise, contained in this plat and the documents associated with it.

Bryan Martin
City of Taylor, Floodplain Administrator

Date

PLANNING AND ZONING COMMISSION CERTIFICATION:

This subdivision to be known as Nyle Maxwell Taylor, has been accepted and approved for filing of record with the County Clerk of Williamson County, Texas according to the minutes of the meeting of the Taylor Planning and Zoning Commission on the _____ day of _____, 20____, A.D.

R. Travis Perthuis, Chairman

Date

Steve Dickey, Secretary

Date

DEVELOPMENT SERVICES DIRECTOR'S CERTIFICATION:

I, Colin Harrison, Development Services Director of the City of Taylor, Texas, do hereby certify this plat is approved for filing of record with the County Clerk of Williamson County, Texas.

Colin Harrison, Director

Date

SURVEYOR'S CERTIFICATION:

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

I, William C Stewart, Registered Professional Land Surveyor in the State of Texas, do hereby certify that this plat is true and correctly made from an actual survey made on the ground of the property legally described hereon, and that there are no apparent discrepancies, conflicts, overlapping of improvements, visible utility lines or roads in place, except as shown on the accompanying plat, and that the corner monument shown thereon will be properly placed under my supervision in accordance with the Subdivision Regulations of the City of Taylor, Texas.

TO CERTIFY WHICH, WITNESS my hand and seal at the City of Taylor, Williamson County, Texas

This _____ day of _____, 20____.

William C Stewart
Registered Professional Land Surveyor
No. 5785, State of Texas

ENGINEER'S CERTIFICATION:

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

I, _____, a Licensed Professional Engineer in the State of Texas, do hereby certify that this Nyle Maxwell Taylor Plat is not encroached by a zone A flood area, as denoted herein, and as defined by Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 48491C0540F, effective on December 20, 2019, and that each lot conforms to the City of Taylor regulations. The fully developed concentrated stormwater runoff resulting from the one hundred (100) year frequency storm is contained within the drainage easements shown and/or public rights-of-way dedicated by this plat.

TO CERTIFY WHICH, WITNESS my hand and seal at the City of Taylor, Williamson County, Texas,

This _____ day of _____, 20____.

ADAY & Associates
K'Kensie Adoy, P.E.
Licensed Professional Engineer No. _____
State of Texas

COUNTY CLERK'S CERTIFICATION:

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS:

I, Nancy Rister, Clerk of the County Court of said County, do hereby certify that the foregoing instrument in writing, with its certificate of authentication was filed for record in my office on the _____ day of _____, 20____, A.D., at _____ o'clock, ____M., and duly recorded this the _____ day of _____, 20____, A.D., at _____ o'clock, ____M in the Official Public Records of said County in instrument No. _____.

TO CERTIFY WHICH, WITNESS my hand and seal at the County Court of said County, at my office in Georgetown, Texas, the date last shown above written.

Nancy E. Rister, Clerk County Court
of Williamson County, Texas

By: _____, Deputy

PZ-2024-XXXX

Texas Land Surveying, Inc.		SHEET	
-A Land Surveying Firm- 3613 Williams Drive, Suite 903 (512) 930-1600 www.texas-ls.com TBPLS FIRM NO.10056200		2	
IF THIS DOCUMENT DOES NOT CONTAIN THE STAMPED SEAL OF THE UNDERSIGNED SURVEYOR, IT IS AN UNAUTHORIZED/ILLEGAL COPY. TEXAS LAND SURVEYING, INC. ASSUMES NO LIABILITY FROM THE USE OF ANY UNAUTHORIZED/ILLEGAL DOCUMENT.		OF	
		2	

City of Taylor
PZ-2024-2215
RCR-Logistics Park Replat Plat of Phase Two, Block Two, Lots 1, 2, and 3A
Staff Report

Item Details

Agenda No.	6
Requested Action:	Consider Conditional Approval of RCR-Logistics Park Replat of Phase Two, Block Two, Lots 1, 2, and 3A
Address/Location:	Generally located at 201 FM 3349
Legal Description:	RCR-Taylor Logistics Park Phase Two Minor Plat, Block Two, Lots 1-2 and RCR-Taylor Logistics Park Phase Two Replat, Block Two, Lot 3A, more particularly described by the Williamson Central Appraisal District Parcels R637527, R648674, R648675, Taylor, Williamson County, Texas
Current Zoning:	EC – Employment Center with Planned Development Overlay
Current Use:	Vacant Acreage
Applicant:	Richard Heiges, RITD
Case History:	This is the second submission of the Replat Plat.
Staff Recommendation:	The submitted Replat Plat meets the minimum requirements of the Land Development Code and Engineering Manual. Therefore, staff recommends Conditional Approval of the proposed Replat pending compliance with all outstanding comments listed in the Comment Review letter dated 09/30/2024.

Attachments

1. Staff Comments
2. Location Map
3. Proposed Plat



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

Richard Heiges
RITD
35505 Cooper Rd.
Brookshire TX 77423

Date: Monday, September 30, 2024

Address: 201 FM 3349, Taylor 76574

Permit Number PZ-2024-2215

Dear Richard Heiges,

Staff has completed its review and conditionally approved the RCR Logistics Replat Phase 2, Block 2, Lots 1, 2, and 3A that is to be located at 201 FM 3349, Taylor 76574. There are some corrections that need to be made prior to recordation of the plat. Comments from this review follow.

Planning Department Comments

The following comments have been provided by Shai Roos. Should you have any questions or require additional information regarding any of these comments, please contact Shai at (972) 960-4077 or by email at shai.roos@hdrinc.com.

Previous comments have been addressed

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier at (713) 576-3691 or by email at javier.vasquez@hdrinc.com.

Conditional approval subject to minor cleanup before recording the plat:
the double comma shown below in the engineer's certificate.
easements note needs to be changed from TBD.

- 1) Remove
- 2) Private utility

Fire Department Comments (PZ)

The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylortx.gov.

Previously reviewed with no comments.

This Replat will be considered by the Taylor Planning and Zoning Commission at their October 2024 meeting. Upon Planning & Zoning Commission action please revise the plat to address the comments noted above prior to recordation.

Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs. Should you have questions or require additional information regarding the plan review process itself, please feel free to contact me directly. I can be reached by telephone at (972) 415-0914, or by e-mail at shai.roos@hdrinc.com.

Thank you,

Shailaja Roos

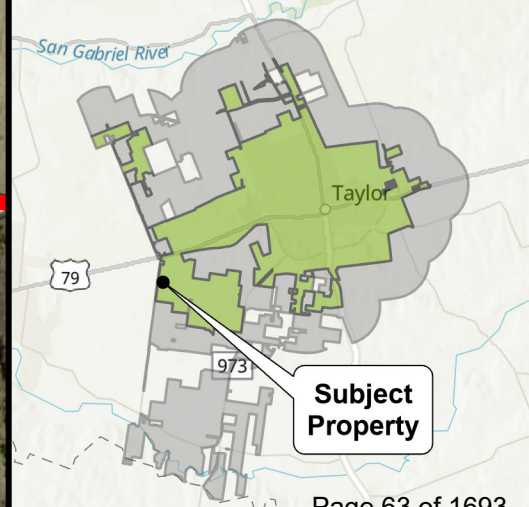
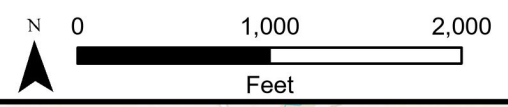
Shai Roos, AICP

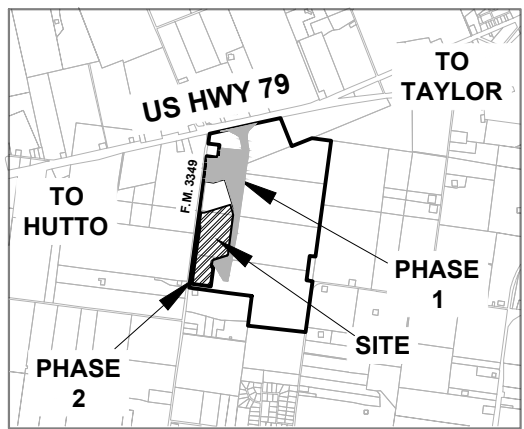


PZ-2024-2215

RCR Phase 2 Block 2
Lots 1, 2, & 3
Replat
Location Map
Approximately 77 acres

- City Limits
- ETJ Boundary
- Subject Property
- Parcels
- Railroad

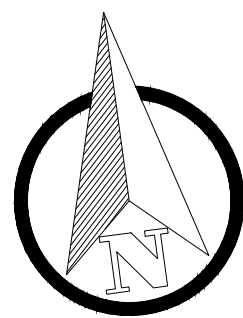




RCR-TAYLOR LOGISTICS PARK

REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A

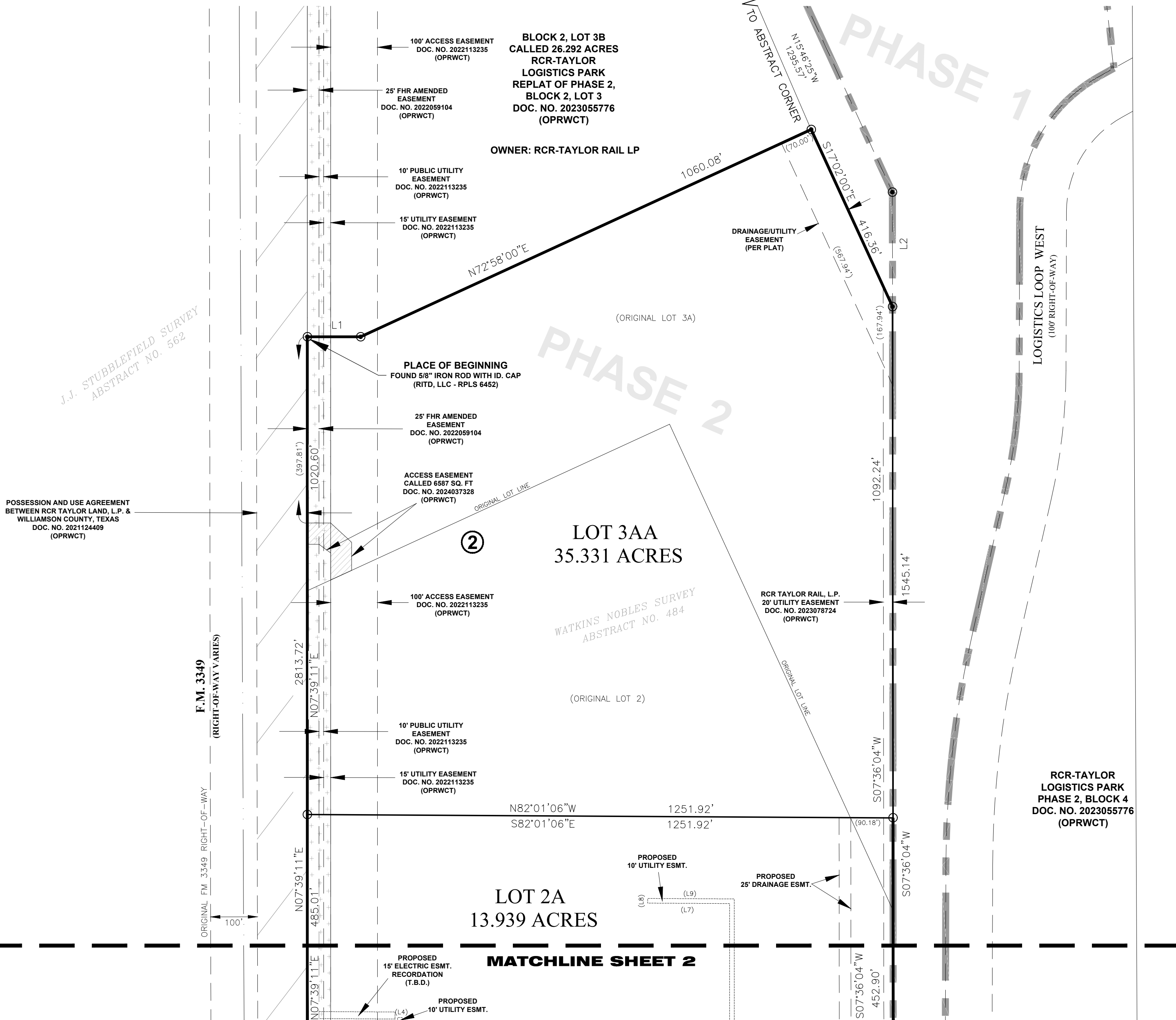
BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK PHASE 2 MINOR PLAT, CONSISTING OF BLOCK 2, LOT 1 (CALLED 21.710 ACRES), AND LOT 2 (CALLED 30.943 ACRES), LYING IN PART OF AND OUT OF THE WATKINS NOBLES SURVEY, ABSTRACT NO. 484, AS RECORDED IN INSTRUMENT NO. 2022113235 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND ALSO BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2, LOT 3, CONSISTING OF BLOCK 2, LOT 3A (CALLED 20.120 ACRES), LYING IN PART OF AND OUT OF SAID WATKINS NOBLES SURVEY, AS RECORDED IN INSTRUMENT NO. 2023055776 OF THE OFFICIAL PUBLIC RECORDS OF TAYLOR, WILLIAMSON COUNTY, TEXAS.



LEGEND

- FOUND 5/8" IRON ROD w/ CAP "RPLS 6452"
- SET 5/8" IRON ROD w/ CAP "RPLS 6452"
- FOUND 5/8" IRON ROD
- ORIGINAL ABSTRACT CORNER
- PHASE LINE
- APPROX. ABSTRACT LINE
- PROPOSED LANDSCAPE BUFFER (SEE NOTE 14)
- 4933 = CORNER MONUMENT AS SHOWN
ALSO HAS A CAP MARKED "ROW 4933"
- 6452 = CORNER MONUMENT AS SHOWN
ALSO HAS A CAP MARKED "RPLS 6452"
- (DRWCT) = DEED RECORDS OF WILLIAMSON COUNTY, TEXAS
- (OPRWCT) = OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS
- P/O = PART OF RES. = RESIDUE BM-BENCHMARK
- (XX) = EASEMENT DIMENSIONS

RCR TAYLOR
LOGISTICS PARK
AMENDING PLAT 3
PHASE 1 LOT 1A3
CALLED 102.19 ACRES
DOC. NO. 2023095246
(OPRWCT)
OWNER: RCR TAYLOR RAIL LP



DATE 07/05/2024	DRAWN RGH	SURVEYOR: R.I.T.D. ENGINEER: R.I.T.D.	PARTNERS DEVELOPMENT COMPANY, LLC. 1360 Post Oak Blvd., Ste. 1900 Houston, Tx 77056	REV.	DATE	BY	DESCRIPTION
CHECK RH	APPRV'D RH			1	9/13/2024	RGH	RESUBMITTAL

SHT 1
OF 3

35505 COOPER ROAD
BROOKSHIRE, TEXAS 77423
(281) 392-4850

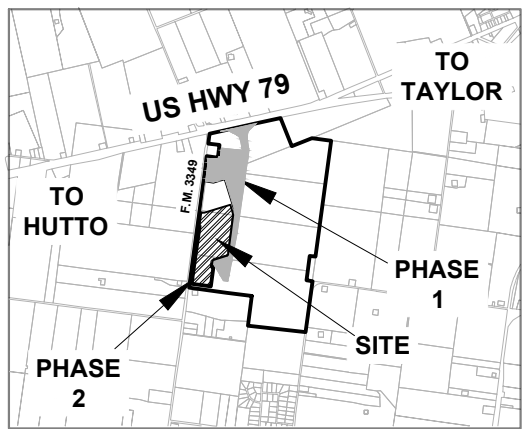
308 WEST 10TH STREET
LAMPASAS, TEXAS 76550
WWW.RITD-LLC.COM FIRM 10194405

150 0 150 300

SCALE: 1" = 150'

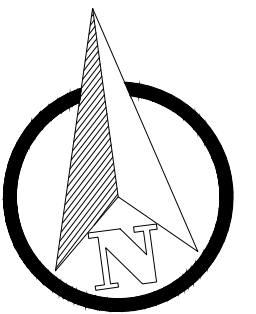
LOCATION AND DESCRIPTION:
RCR - TAYLOR LOGISTICS PARK
781 FM 3349, Taylor, TX 76574

RCR-TAYLOR LOGISTICS PARK
REPLAT OF
PHASE 2, BLOCK 2,
LOTS 1, 2 AND 3A



RCR-TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A

BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK PHASE 2 MINOR PLAT, CONSISTING OF BLOCK 2, LOT 1 (CALLED 21.710 ACRES), AND LOT 2 (CALLED 30.943 ACRES), LYING IN PART OF AND OUT OF THE WATKINS NOBLES SURVEY, ABSTRACT NO. 484, AS RECORDED IN INSTRUMENT NO. 2022113235 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND ALSO BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2, LOT 3, CONSISTING OF BLOCK 2, LOT 3A (CALLED 20.120 ACRES), LYING IN PART OF AND OUT OF SAID WATKINS NOBLES SURVEY, AS RECORDED IN INSTRUMENT NO. 2023055776 OF THE OFFICIAL PUBLIC RECORDS OF TAYLOR, WILLIAMSON COUNTY, TEXAS.



LEGEND

- FOUND 5/8" IRON ROD w/ CAP "RPLS 6452" 4933 = CORNER MONUMENT AS SHOWN ALSO HAS A CAP MARKED "ROW 4933"
- SET 5/8" IRON ROD w/ CAP "RPLS 6452" 6452 = CORNER MONUMENT AS SHOWN ALSO HAS A CAP MARKED "RPLS 6452"
- FOUND 5/8" IRON ROD
- ORIGINAL ABSTRACT CORNER
- PHASE LINE
- APPROX. ABSTRACT LINE
- PROPOSED LANDSCAPE BUFFER (SEE NOTE 14)

EASEMENT LINE TABLE

NUM	BEARING	DISTANCE
(L3)	N07°42'03"E	52.43'
(L4)	S82°17'57"E	10.00'
(L5)	S07°42'03"W	42.43'
(L6)	N07°42'03"E	289.92'
(L7)	N82°01'06"W	175.04'
(L8)	N07°42'03"E	10.00'
(L9)	S82°01'06"E	185.04'
(L10)	S07°42'03"W	299.92'
(L11)	S82°01'06"E	335.19'
(L12)	N07°39'11"E	49.21'
(L13)	N07°39'11"E	15.00'
(L14)	S82°08'16"E	187.42'
(L15)	S07°39'11"W	15.00'
(L16)	N82°08'16"W	187.40'

LINE TABLE

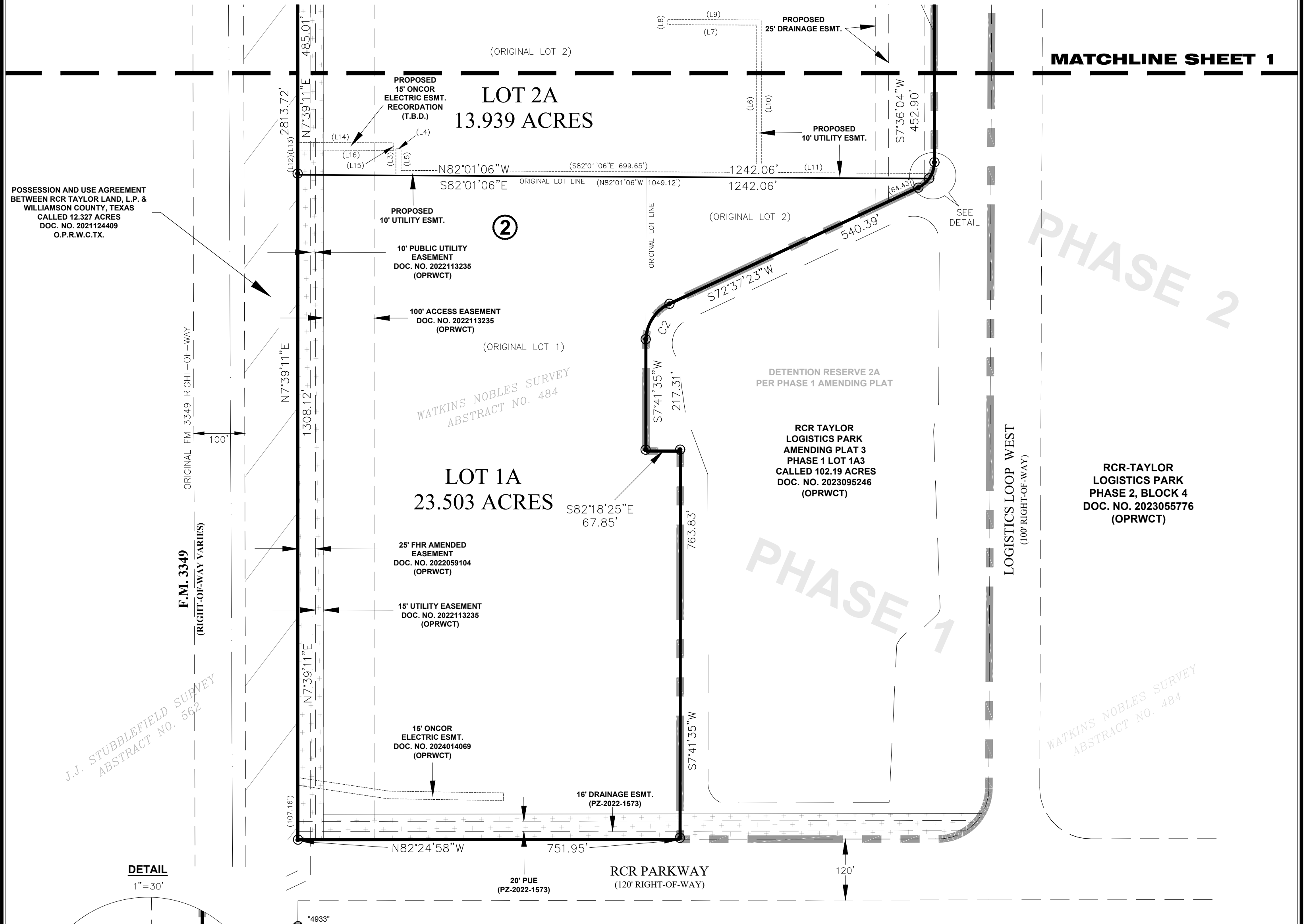
NUM	BEARING	DISTANCE
L1	S82°20'49"E	113.81'
L2	N07°36'04"E	244.73'

CURVE TABLE

NUM	DELTA	ARC	RADIUS	BEARING	DISTANCE
C1	65°01'18"	62.42'	55.00'	S40°06'43"W	59.12'
C2	64°56'10"	90.67'	80.00'	S40°09'35"W	85.89'
C3	35°37'58"	34.21'	55.00'	S25°25'03"W	33.66'
C4	29°23'20"	28.21'	55.00'	S57°55'42"W	27.90'

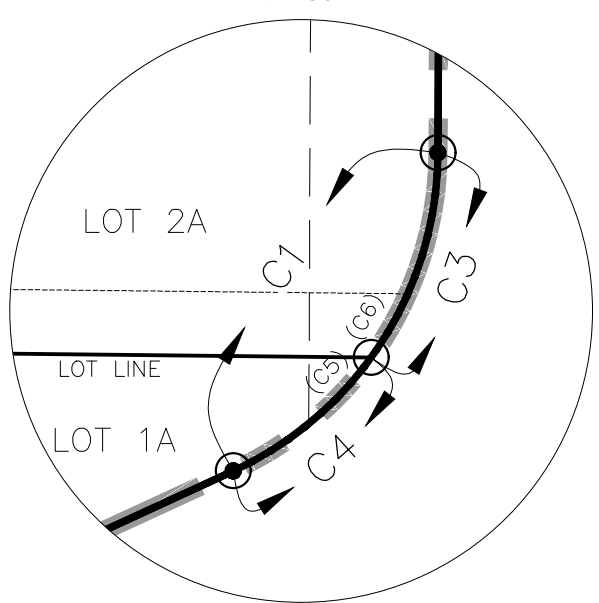
EASEMENT CURVE TABLE

NUM	DELTA	ARC	RADIUS	BEARING	DISTANCE
(C5)	29°23'20"	28.21'	55.00'	N57°55'42"E	27.90'
(C6)	12°02'33"	11.56'	55.00'	S37°25'08"W	11.54'



DETAIL

1" = 30'



DATE 07/05/2024
CHECK RH

DRAWN RGH
APPRV'D RH

SURVEYOR: R.I.T.D.
ENGINEER: R.I.T.D.

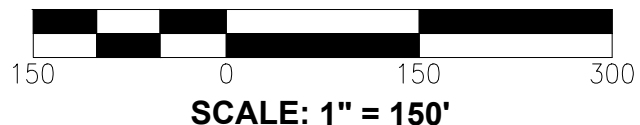
PARTNERS DEVELOPMENT COMPANY, LLC.
1360 Post Oak Blvd., Ste. 1900
Houston, Tx 77056

REV.	DATE	BY	DESCRIPTION
1	9/13/2024	RGH	RESUBMITTAL

SHT 2
OF 3



35505 COOPER ROAD
BROOKSHIRE, TEXAS 77423
(281) 392-4850
308 WEST 10TH STREET
LAMPASAS, TEXAS 76550
WWW.RITD-LLC.COM
FIRM 10194405



LOCATION AND DESCRIPTION:
RCR - TAYLOR LOGISTICS PARK
781 FM 3349, Taylor, TX 76574

RCR-TAYLOR LOGISTICS PARK
REPLAT OF
PHASE 2, BLOCK 2,
LOTS 1, 2 AND 3A

C:\Users\Richard.hermosillo\DC\ACCDocs\RTD\678_PDC Replat of Lots 1, 2 and 3A.dwg * 9/12/2024 4:56 PM * Plotted by Richard Hermosillo

RCR-TAYLOR LOGISTICS PARK

REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A

BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK PHASE 2 MINOR PLAT, CONSISTING OF BLOCK 2, LOT 1 (CALLED 21.710 ACRES), AND LOT 2 (CALLED 30.943 ACRES), LYING IN PART OF AND OUT OF THE WATKINS NOBLES SURVEY, ABSTRACT NO. 484, AS RECORDED IN INSTRUMENT NO. 2022113235 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND ALSO BEING A REPLAT OF RCR-TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2, LOT 3, CONSISTING OF BLOCK 2, LOT 3A (CALLED 20.120 ACRES), LYING IN PART OF AND OUT OF SAID WATKINS NOBLES SURVEY, AS RECORDED IN INSTRUMENT NO. 2023055776 OF THE OFFICIAL PUBLIC RECORDS OF TAYLOR, WILLIAMSON COUNTY, TEXAS.

OWNER'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

WE, PDC TP 01 LOT A, LLC., ACTING THROUGH _____, MANAGER, OWNER OF 21.710 ACRES OF LAND SHOWN HEREON AND BEING CONVEYED IN DEED RECORDED IN DOCUMENT NUMBER 2022076416 OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS OF THE CERTAIN TRACT OF LAND, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR THE STREETS, ALLEYS, RIGHTS-OF-WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS RCR-TAYLOR LOGISTICS PARK, REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A.

TO CERTIFY WHICH, WITNESS BY MY HAND THIS _____ DAY OF _____, 2024.

_____, MANAGER, PDC TP 01 LOT A, LLC.

NOTARY'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

BEFORE ME, THE UNDERSIGNED, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 2024.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

OWNER'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

WE, PDC TP 01 LOT B, LLC., ACTING THROUGH _____, MANAGER, OWNER OF 30.943 ACRES OF LAND SHOWN HEREON AND BEING CONVEYED IN DEED RECORDED IN DOCUMENT NUMBER 2022076405, OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS OF THE CERTAIN TRACT OF LAND, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR THE STREETS, ALLEYS, RIGHTS-OF-WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS RCR-TAYLOR LOGISTICS PARK, REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A.

TO CERTIFY WHICH, WITNESS BY MY HAND THIS _____ DAY OF _____, 2024.

_____, MANAGER, PDC TP 01 LOT B, LLC.

NOTARY'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

BEFORE ME, THE UNDERSIGNED, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 2024.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

OWNER'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

WE, PDC TP 01 LOT C, LLC., ACTING THROUGH _____, MANAGER, OWNER OF 20.120 ACRES OF LAND SHOWN HEREON AND BEING CONVEYED IN DEED RECORDED IN DOCUMENT NUMBER 2024037327 OF THE OFFICIAL RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS OF THE CERTAIN TRACT OF LAND, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR THE STREETS, ALLEYS, RIGHTS-OF-WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS RCR-TAYLOR LOGISTICS PARK, REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A.

TO CERTIFY WHICH, WITNESS BY MY HAND THIS _____ DAY OF _____, 2024.

_____, MANAGER, PDC TP 01 LOT C, LLC.

NOTARY'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS
COUNTY OF _____ {

BEFORE ME, THE UNDERSIGNED, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED _____, KNOWN TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ DAY OF _____, 2024.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON:

DATE 07/05/2024 DRAWN _____ RGH
CHECK _____ RH APPRV'D _____ RH

SURVEYOR: R.I.T.D.
ENGINEER: R.I.T.D.

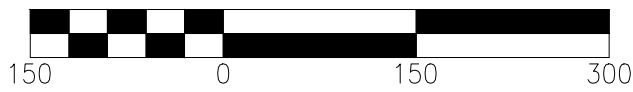
PARTNERS DEVELOPMENT COMPANY, LLC.
1360 Post Oak Blvd., Ste. 1900
Houston, Tx 77056

REV.	DATE	BY	DESCRIPTION
1	9/13/2024	RGH	RESUBMITTAL

SHT 3
OF 3



35505 COOPER ROAD 308 WEST 10TH STREET
BROOKSHIRE, TEXAS 77423 LAMPASAS, TEXAS 76550
(281) 392-4850 WWW.RITD-LLC.COM FIRM 10194405



LOCATION AND DESCRIPTION:
RCR - TAYLOR LOGISTICS PARK
781 FM 3349, Taylor, TX 76574

RCR-TAYLOR LOGISTICS PARK
REPLAT OF
PHASE 2, BLOCK 2,
LOTS 1, 2 AND 3A

LEGAL DESCRIPTION

BEGINNING AT A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR THE SOUTHWEST CORNER OF LOT 3B (CALLED 26.292 ACRES) OF RCR TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2 LOT 3 AS RECORDED IN INSTRUMENT NO. 2023055776 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS FOR THE NORTHWEST CORNER HEREOF, COMMON WITH THE NORTHWEST CORNER OF SAID RCR TAYLOR LOGISTICS PARK LOT 3A (CALLED 20.120 ACRES), ALSO BEING ON AN EAST LINE OF THE POSSESSION AND USE AGREEMENT BETWEEN RCR TAYLOR LAND, L.P. & WILLIAMSON COUNTY, TEXAS TRACT CALLED 12.327 ACRES AS RECORDED IN INSTRUMENT NO. 2021124409 OF SAID OFFICIAL PUBLIC RECORDS, BEING THE EAST RIGHT-OF-WAY LINE OF FM 3349;

THENCE ALONG SOUTH AND WEST LINES OF SAID PHASE 2, BLOCK 2, LOT 3B, FOR NORTH AND EAST LINES HEREOF, BEING ALONG THE NORTH AND EAST LINES OF SAID LOT 3A, THE FOLLOWING CALLS: S 82°20'49" E 113.81 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR AN AN ANGLE POINT HEREOF AND OF SAID LOT 3B, N 72°58'00" E 1060.08 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR AN INTERIOR CORNER OF PHASE 2, LOT 3B COMMON WITH THE NORTH CORNER HEREOF, S 17°02'00" E 416.36 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR AN ANGLE POINT HEREOF AND BEING IN THE WEST LINE OF RCR TAYLOR LOGISTICS PARK AMENDING PLAT 3 PHASE 1, LOT 1A3 (CALLED 102.19 ACRES) RECORDED IN DOCUMENT NO. 2023095246 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, S 07°39'04" W 1545.14 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR A TANGENT POINT HEREOF AND OF PHASE 1 LOT 1A3, BEING AT THE BEGINNING OF A CURVE TO THE RIGHT;

THENCE ALONG AN EAST LINE HEREOF, COMMON WITH A WEST LINE OF PHASE1, LOT 1A3, BEING ALONG SAID CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 65°01'18", HAVING A RADIUS OF 55.00 FEET, A CHORD OF S 40°06'43" W 59.12 FEET, FOR AN ARC DISTANCE OF 62.42 FEET TO A 5/8 INCH IRON ROD FOUND FOR A TANGENT POINT HEREOF, AND OF SAID PHASE 1 LOT 13A, AT THE END OF SAID CURVE TO THE RIGHT;

THENCE ALONG A SOUTH LINE HEREOF AND THE NORTH LINE OF LOT 1A3, S 72°37'23" W 540.39 FEET TO A 5/8 INCH IRON ROD FOUND FOR A TANGENT POINT HEREOF AND OF SAID PHASE 1, LOT 1A3, AT THE BEGINNING OF A CURVE TO THE LEFT;

THENCE ALONG AN EAST LINE HEREOF AND A WEST LINE PHASE 1 LOT 1A3, BEING ALONG SAID CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 64°56'10", HAVING A RADIUS OF 80.00 FEET, A CHORD OF S 40°09'35" W 85.89 FEET, FOR AN ARC DISTANCE OF 90.67 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR A TANGENT POINT HEREOF, AND OF SAID PHASE 1, LOT 1A3, AND AT THE END OF SAID CURVE TO THE LEFT;

THENCE WITH EAST, AND NORTH LINES HEREOF, BEING WEST, AND SOUTH LINE OF LOT 1A3 THE FOLLOWING CALLS: S 07°41'35" W 217.31 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR AN INTERIOR CORNER HEREOF, S 82°18'25" E 67.85 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR A NORTHEAST CORNER HEREOF, AND S 07°41'35" W 763.83 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) ON THE THE NORTH RIGHT-OF-WAY LINE OF RCR PARKWAY (120' RIGHT-OF-WAY) LINE FOR THE SOUTHEAST CORNER HEREOF, BEING THE SOUTHWEST CORNER OF LOT 1A3;

THENCE WITH THE SOUTH LINE HEREOF, AND THE NORTH LINE OF SAID RCR PARKWAY N 82°24'58" W 751.95 FEET TO A 5/8 INCH IRON ROD FOUND WITH ID. CAP (RITD, LLC - RPLS 6452) FOR THE SOUTHWEST CORNER HEREOF ONAN EAST LINE OF SAID POSSESSION AND USE AGREEMENT TRACT CALLED 12.327 ACRES, AND SAID RIGHT-OF-WAY LINE OF FM 3349;

THENCE ALONG THE WEST LINE HEREOF, BEING ALONG A PORTION OF AN EAST LINE OF SAID 12.327 ACRES POSSESSION AND USE AGREEMENT TRACT, AND OF SAID FM 3349, N 07°39'11" E 2813.72 FEET TO THE PLACE OF BEGINNING AND CONTAINING 72.773 ACRES OF LAND.

NOTES:

- THE BEARINGS SHOWN HEREON ARE RELATIVE TO THE TEXAS STATE PLANE GRID SYSTEM, NAD-83 (2011), CENTRAL ZONE 4203, DISTANCES SHOWN ARE GROUND DISTANCES. SCALE FACTOR IS 0.9998800.
- DISTANCES AND COORDINATES SHOWN HEREON ARE U.S. SURVEY FEET.
- SUBJECT TO EASEMENTS, CONDITIONS, COVENANTS AND RESTRICTIONS AS STATED OR SHOWN HEREON, AS REQUIRED BY THE CITY OF TAYLOR, TEXAS, AND ANY OF RECORD WHICH MAY BE APPLICABLE.
- A CURRENT TITLE COMMITMENT OR REPORT WAS NOT AVAILABLE OR PROVIDED TO THE UNDERSIGNED SURVEYOR AS OF THE DATE OF THIS SURVEY AND THE UNDERSIGNED SURVEYOR DID NOT ABSTRACT THE SUBJECT PROPERTY.
- UTILITY PROVIDERS - WATER: CITY OF TAYLOR, TEXAS; WASTEWATER/SEPTIC: CITY OF TAYLOR, TEXAS AND WILLIAMSON COUNTY, TEXAS; ELECTRICITY: ONCOR ELECTRIC.
- NO PART OF THE SUBJECT PROPERTY LIES WITHIN THE SPECIAL FLOOD HAZARD AREA ACCORDING TO THE FLOOD INSURANCE RATE MAP (FIRM) AS COMPILED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY, NATIONAL FLOOD INSURANCE PROGRAM, MAP NUMBER 48491C0540F EFFECTIVE DATE DECEMBER 20, 2019, WILLIAMSON COUNTY, TEXAS.
- NO STRUCTURES AND/OR OBSTRUCTIONS ARE PERMITTED IN DRAINAGE EASEMENTS WITHOUT PRIOR REVIEW AND APPROVAL BY THE CITY OF TAYLOR, TEXAS DEVELOPMENT SERVICES DEPARTMENT.
- ALL MINIMUM YARD (BUILDING) SETBACKS SHALL BE IN ACCORDANCE WITH THE COMMERCIAL PLANNED DEVELOPMENT FOR RCR-TAYLOR LOGISTICS PARK ORDINANCE NUMBER 2022-25 OR AS AMENDED.
- THE CITY OF TAYLOR PUBLIC WORKS SHALL BE ALLOWED TO ACCESS EASEMENTS ADJACENT TO THE 10-FT PUE SHOWN IN ORDER TO MAINTAIN PUBLIC UTILITIES.
- MINIMUM FIRE FLOW OF 750 GALLONS PER MINUTES FOR NOT LESS THAN TWO (2) HOURS SHALL BE PROVIDED. THE PLATTED PROPERTY HAS ADJACENT ACCESS TO THE CITY OF TAYLOR WATER SYSTEM; TYPICAL FIRE SUPPRESSION SYSTEMS AND FIRE PROTECTION WATER SUPPLY WILL COMPLY WITH CITY OF TAYLOR, TEXAS CODE REQUIREMENTS AND WILL BE DESIGNED ACCORDINGLY.
- STRUCTURES AND BUILDINGS CONSTRUCTED NEXT TO THE FLOODPLAIN MUST BE A MINIMUM OF TWO (2) FEET ABOVE THE 100-YEAR FLOODPLAIN BASE FLOOD ELEVATION (BFE).
- ALL DRAINAGE AND DETENTION FACILITIES SHOWN HEREIN ARE PRIVATE FACILITIES. THE LAND OWNER/DEVELOPER SHALL BE REQUIRED TO MEET THE STANDARDS SPECIFIED IN THE OPERATIONS & MAINTENANCE PLAN FOR DETENTION FACILITIES FILED UNDER CITY OF TAYLOR PERMIT PZ-2019-1182 AND PZ-2022-1573. THE CITY OF TAYLOR, TEXAS SHALL NOT BE RESPONSIBLE FOR MAINTENANCE ACTIVITIES FOR DRAINAGE AND DETENTION FACILITIES SHOWN HEREIN.
- SIDEWALKS SHALL NOT BE REQUIRED FOR PROPERTIES WITHIN RCR TAYLOR LOGISTICS PARK THAT HAVE PUBLIC ROAD ACCESS ALONG FM 3349, BUT SIDEWALKS ARE REQUIRED ALONG RCR PARKWAY.
- AN INDIVIDUAL SEWER TAP AND WATER TAP SHALL BE INSTALLED FOR EACH LOT.
- THE LANDSCAPE BUFFER SHOWN HEREON IS 50 FEET WIDE AND ADJACENT TO EXISTING AND PROPOSED RIGHTS-OF-WAY AS SHOWN HEREON. THE LANDSCAPING PLAN AND BUFFER LOCATION SHALL MEET THE REQUIREMENTS STATED IN THE RCR TAYLOR LOGISTICS PARK COMMERCIAL PLANNED DEVELOPMENT CODES THAT ARE IN EFFECT FOR ANY DEVELOPMENT OF THE PLATTED AREA SHOWN HEREON.
- NEXT TO THE 50-FOOT EASEMENT RUNNING ALONG F.M. 3349, THERE IS AN EXISTING 100-FOOT JOINT ACCESS EASEMENT AS ESTABLISHED BY THE RCR TAYLOR LOGISTICS PARK, PHASE 2 MINOR PLAT. WITHIN THE 100-FOOT JOINT ACCESS EASEMENT, A MINIMUM 35 FEET WIDE PAVEMENT MUST BE CONSTRUCTED. ADDITIONALLY, PERMANENT JOINT ACCESS EASEMENTS ARE HEREBY ESTABLISHED WHERE TxDOT DRIVEWAYS ARE APPROVED AND TEMPORARY CONSTRUCTION JOINT ACCESS EASEMENTS ARE HEREBY ESTABLISHED FOR TWO (2) YEARS OR UNTIL THE TIME CERTIFICATES OF OCCUPANCIES ARE OBTAINED FROM THE CITY OF TAYLOR FOR THE USE OF THE LAND OR BUILDINGS, WHICHEVER OCCURS FIRST. JOINT ACCESS EASEMENTS ARE TO PROVIDE ACCESS ACROSS ALL LOTS WITHIN THIS AND FUTURE PLATS ENCOMPASSING THESE PROPOSED LOTS. CROSS-ACCESS CANNOT BE OBSTRUCTED BY VEHICLES, FENCES, DUMPSTERS, CONSTRUCTION MATERIALS, OR ANY OTHER MEANS.
- THIS SUBDIVISION IS SUBJECT TO ALL GENERAL NOTES AND RESTRICTIONS APPEARING ON THE PLAT OF RCR TAYLOR LOGISTICS PARK, PHASE 2, RECORDED AS DOCUMENT #2022113235 OF THE PLAT RECORDS OF WILLIAMSON COUNTY, TEXAS.

ENGINEER'S CERTIFICATION

I, _____, LICENSED PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS RCR-TAYLOR LOGISTICS PARK REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A CITY OF TAYLOR, WILLIAMSON COUNTY, TEXAS IS NOT ENCRoACHED BY A ZONE A FLOOD AREA, AS DENOTED HEREIN, AND AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD BOUNDARY MAP, COMMUNITY PANEL NUMBER 48491C0540F, EFFECTIVE DATE DECEMBER 20, 2019, AND THAT EACH LOT CONFORMS TO THE CITY OF TAYLOR REGULATIONS. THE FULLY DEVELOPED, CONCENTRATED STORMWATER RUNOFF RESULTING FROM THE ONE HUNDRED (100) YEAR FREQUENCY STORM IS CONTAINED WITHIN THE DRAINAGE EASEMENT SHOWN AND/OR PUBLIC RIGHTS-OF-WAY DEDICATED BY THIS PLAT.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT _____, COUNTY, TEXAS, THIS _____ DAY OF _____, 2024

FOR REVIEW

RICHARD W. HEIGES, P.E.
REGISTERED PROFESSIONAL
ENGINEER NO. 123155

WILLIAMSON COUNTY - ON-SITE SEWAGE FACILITIES & FLOODPLAIN ADMINISTRATOR

BASED UPON THE ABOVE REPRESENTATIONS OF THE ENGINEER OR SURVEYOR WHOSE SEAL IS AFFIXED HERETO, AND AFTER A REVIEW OF THE PLAT AS REPRESENTED BY THE SAID ENGINEER OR SURVEYOR, I FIND THAT THIS PLAT COMPLIES WITH THE REQUIREMENTS OF EDWARDS AQUIFER REGULATIONS FOR WILLIAMSON COUNTY AND WILLIAMSON COUNTY ON-SITE SEWAGE FACILITY REGULATIONS. THIS CERTIFICATION IS MADE SOLELY UPON SUCH REPRESENTATIONS AND SHOULD NOT BE RELIED UPON FOR VERIFICATIONS OF THE FACTS ALLEGED. THE WILLIAMSON COUNTY ENGINEER'S OFFICE AND WILLIAMSON COUNTY DISCLAIM ANY RESPONSIBILITY TO ANY MEMBER OF THE PUBLIC FOR INDEPENDENT VERIFICATION OF THE REPRESENTATIONS, FACTUAL OR OTHERWISE, CONTAINED IN THIS BLUE LINE (SURVEY) AND THE DOCUMENTS ASSOCIATED WITH IT.

J. TERRON EVERTSON, PE, DR, CFM
COUNTY ENGINEER

DATE

DEVELOPMENT SERVICES DIRECTOR'S CERTIFICATION

DEVELOPMENT SERVICES DIRECTOR

I, _____, DEVELOPMENT SERVICES DIRECTOR FOR THE CITY OF TAYLOR, TEXAS, DO HEREBY CERTIFY THIS PLAT IS APPROVED FOR FILING OF RECORD WITH THE COUNTY CLERK OF WILLIAMSON COUNTY, TEXAS

SCOTT DUNLOP, DIRECTOR

DATE

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

I, RICHARD W. HEIGES, REGISTERED PROFESSIONAL LAND SURVEYOR NO. 6452 IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS PLAT IS TRUE AND CORRECTLY MADE FROM AN ACTUAL SURVEY MADE ON THE GROUND OF THE PROPERTY LEGALLY DESCRIBED HEREON, AND THAT THERE ARE NO APPARENT DISCREPANCIES, CONFLICTS, OVERLAPPING OF IMPROVEMENTS, VISIBLE UTILITY LINES OR ROADS IN PLACE, EXCEPT AS SHOWN ON THE ACCOMPANYING PLAT, AND THAT THE CORNER MONUMENTS SHOWN THEREON WERE PROPERLY PLACED UNDER MY SUPERVISION IN ACCORDANCE WITH THE SUBDIVISION REGULATIONS OF THE CITY OF TAYLOR, TEXAS.

DATED THIS THE _____ DAY OF _____, 2024, A.D.

FOR REVIEW

RICHARD W. HEIGES
REGISTERED PROFESSIONAL LAND
SURVEYOR NO. 6452, STATE OF TEXAS

PLANNING AND ZONING COMMISSION

THIS SUBDIVISION TO BE KNOWN AS RCR-TAYLOR LOGISTICS PARK, REPLAT OF PHASE 2, BLOCK 2, LOTS 1, 2 AND 3A HAS BEEN ACCEPTED AND APPROVED FOR FILING OF RECORD WITH THE COUNTY CLERK OF WILLIAMSON COUNTY, TEXAS, ACCORDING TO THE MINUTES OF THE MEETING OF THE TAYLOR PLANNING AND ZONING COMMISSION ON THE _____ DAY OF _____, 20____ A.D.

_____, CHAIRMAN

DATE

_____, SECRETARY

DATE

COUNTY CLERK'S CERTIFICATION

STATE OF TEXAS {
KNOW ALL MEN BY THESE PRESENTS

COUNTY OF WILLIAMSON {

I, _____, CLERK OF THE COUNTY COURT OF SAID COUNTY, DO HEREBY CERTIFY THAT THE FOREGOING INSTRUMENT IN WRITING, WITH ITS CERTIFICATE OF AUTHENTICATION WAS FILED FOR RECORD IN MY OFFICE ON THE _____ DAY OF _____, 20____ A.D., AT _____ O'CLOCK, ____ M., AND DULY RECORDED THIS THE _____ DAY OF _____, 20____ A.D., AT _____ O'CLOCK, ____ M., IN THE OFFICIAL PUBLIC RECORDS OF SAID COUNTY IN DOCUMENT NO. _____.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT THE COUNTY COURT OF SAID COUNTY, AT MY OFFICE IN GEORGETOWN, TEXAS, THE DATE LAST SHOWN ABOVE WRITTEN.

_____, CLERK COUNTY COURT
OF WILLIAMSON COUNTY, TEXAS

BY: _____, DEPUTY

City of Taylor
PZ-2024-2105
Gateway Tract - Preliminary Plat
Staff Report

Item Details

Agenda No.:	7
Requested Action:	Consider Disapproval of Gateway Tract Preliminary Plat
Address/Location:	505 Carlos G. Parker SW
Legal Description:	38.09-acre tract of land part of and out of the George M. Reese Survey, Abstract No. 533, Taylor, Williamson County, Texas
Current Zoning:	P5 – Urban Center with a PD Overlay
Current Use:	Vacant
Applicant:	John Pickens, Kimley Horn
Case History:	This is the second submission of the Preliminary Plat
Staff Recommendation:	The submitted Plat does not meet the minimum requirements of the Subdivision Ordinance and Engineering Manual. Therefore, staff recommends disapproval of the proposed Preliminary Plat

Attachments

1. Staff Comments
2. Location Map
3. Proposed Plat



City of Taylor

400 Porter Street
Taylor, TX 76574
(512) 352-3675
www.ci.taylor.tx.us

John Pickens
Kimley Horn

Date: Monday, September 30, 2024

Address: 505 SW Carlos G. Parker Blvd., Taylor 76574

Permit Number PZ-2024-2105

Dear John Pickens,

Staff has completed its review of plans for the Gateway Tract Preliminary Plat that is to be located at 505 SW Carlos G. Parker Blvd., Taylor 76574. There are some corrections that need to be made prior to approval of the plat. Comments from this review follow.

Planning Department Review

The following comments have been provided by Shai Roos. Should you have any questions or require additional information regarding any of these comments, please contact Shai Roos at (972) 415-0914 or by email at shai.roos@hdrinc.com.

*See Associated Documents section to download a PDF(s) containing the following planning comments:

- Page 2 - Show easements through property correctly - easement lines need to close.
- Page 2 - Clean up overlapping labels.
- Page 2 - The verbiage for "vacated through City of Taylor" should be "vacated per this plat."
- Page 2 - Remove pedestrian access easements from the public streets.
- Page 2 - Remove Fire Alley's from the plat drawing - these can be shown on the site plan at a later stage.

Engineering Department Review

The following comments have been provided by Javier Vasquez. Should you have any questions or require additional information regarding any of these comments, please contact Javier Vasquez at (713) 576-3691 or by email at javier.vasquez@hdrinc.com.

*See Associated Documents section to download a PDF(s) containing the following engineering comments:

Preliminary Plat - Engineering Review 03

Sheet 1 of 3

1. Verify and update classification to be Neighborhood Street or Commercial Street respectively with the Planned Development Standards.
2. Provide plat note regarding 1-ft buffer per Taylor LDC 3.8.1.11

Sheet 2 of 3

1. Fire alleys to be dedicated alley type. Remove fire lane designation and update plat. Dimension fire alleys per PD Standards
2. Verify and update Fremi Street termination point to match PD.
3. Verify and update designation per PD for Blocks B, and D (e.g. thoroughfare required here as a "Way". Designation is missing from plat.
4. Notate Civic Space shown near Block E
5. Point leaders to PUE lines.
6. Notate acreage for all Civic Space shown on plat (Typical)
7. Denote Blocks for areas/large lots that do not have it shown. (Typical)
8. Notate name of original survey (abstract)
9. Fix text
10. Verify if this easement is required per PD documents

Traffic Impact Analysis - Engineering Review 04

1. No updated TIA was submitted. Please submit an updated TIA along with a response to comments letter

Drainage Impact Analysis and Modeling - Engineering Review 01 (Submittal 03)

Page 1

1. Please provide a bookmarked document for future submittals. It is difficult to review an 800+ page report when there is no wayfinding.

Page 3

1. Please demonstrate that 2-5'x8' boxes are of sufficient size to bypass A1 and A2 flows entirely for the 100-year event. Otherwise, offsite contributing flow to the detention ponds will need to be considered.

Page 4

1. Please demonstrate that 2-5'x6' boxes are of sufficient size to bypass B flows entirely for the 100-year event. Otherwise, offsite contributing flow to the detention ponds will need to be considered.

Existing Drainage Area Map (Page 6)

1. Please provide a summary table for the following hydrologic data on the Existing Drainage Area Map: - Tc flowpaths - curve numbers
2. Provide contour labels on the proposed drainage area map.
3. The City of Taylor Floodplain shall be delineated for all waterways with a contributing drainage area greater than 64 acres (EM 3.20). Please prepare a floodplain analysis for contributing drainage areas over 64 acres and demonstrate that proposed improvements do not adversely impact COT Floodplain on adjacent properties.

Appendix B (Page 7)

1. Provide a routing diagram for the existing model that is in conformance with the plans. (example shown on Existing DAM)

Proposed Drainage Area Map (Page 389)

1. The City of Taylor Floodplain shall be delineated for all waterways with a contributing drainage area greater than 64 acres (EM 3.20). Please prepare a floodplain analysis for contributing drainage areas over 64 acres.
2. Provide contour labels on the existing drainage area map.
3. Please provide a summary table for the following hydrologic data on the Proposed Drainage Area Map: - Tc flowpaths - curve numbers - pond stage-storage tables - pond elevations for the 2-, 10-, 25-, and 100-year events
4. How is pond B included in the routing diagram for POA D?
5. Proposed detention located in the City of Taylor Floodplain will be required to demonstrate no impact to the floodplain. The pond must be shown to fully drain prior to the peak event in the creek, otherwise any volume below the 100-year elevation in the detention pond cannot be counted towards detention volume.
6. Drainage criteria (Chapter 3) of the City of Taylor's Engineering Manual and demonstrate compliance with Qcritical (EM Section 3.1.4.D) for proposed detention.
7. Proposed detention ponds must be sized appropriately to consider all upstream contributing flow.

Appendix D (Page 390)

1. Provide a routing diagram for the proposed model that is in conformance with the plans.

Phasing Plan - Engineering Review 01 (Submittal 03)

1. Coordination to be made with planning and engineering for phasing plan update and formatting. It appears the current phasing shown does not reflect key infrastructure for the development.

Utility Schematic Plan - Engineering Review 03

PRELIMINARY UTILITY PLAN (SHEET 1 OF 2)

1. This configuration appears unfavorable for maintenance. Please review additional options where wastewater can be routed around proposed drainage crossing or if additional diversion is required within FM 973 ROW (example shown)
2. Proposed drainage and utilities appear to have a large impact to these lots. Will need to verify with planning regarding concerns and if modifications/amendment will be needed for the PD Standards Ordinance.
3. Segment of water line shown extending from Fermi Street will require a separate utility easement through this lot. If this line segment is required for fire requirements, will need to look at end of line requirements, otherwise, line might be able to end with street.
4. Water line stubout/extension required here to provide continuation to adjacent properties
5. Please review the Taylor Engineering Manual Sec. 2.2 for utility assignments within the ROW and update schematic accordingly.
6. Will need to verify if both water and sewer can be extended through the civic spaces
7. How was this wastewater line sized with consideration of existing flow?

PRELIMINARY UTILITY PLAN (SHEET 2 OF 2)

1. Verify and update to have water and sewer placed within the civic space here on Farnsworth Way and not as shown for lots 12-22.

2. Verify this with the regulating plan and primary/secondary frontages shown. Verify and update to have water and sewer placed within the civic space here on Carver Way lots 7-13. Verify this with the regulating plan and primary/secondary frontages shown.
3. Sizing of water line to be coordinated with engineering.

Fire Department Comments (PZ)

The following comments have been provided by Bobby Copeland. Should you have any questions or require additional information regarding any of these comments, please contact Bobby Copeland at (512) 352-6992 or by email at robert.copeland@taylor.tx.gov.

Previously reviewed with comments addressed.

Urban Design Review

The following comments have been provided by Mike Watkins. Should you have any questions or require additional information regarding any of these comments, please contact Mike Watkins at (301) 675-6223 or by email at mike.watkins@me.com.

All urban design comments applicable for this approval have been resolved.

Please revise the plat to address the comments noted above. Following revision, submit the plat electronically with a resolution letter in PDF format.

Should you have questions regarding specific comments, please contact the staff member referenced under the section in which the comment occurs. Should you have questions or require additional information regarding the plan review process itself, please feel free to contact me directly. I can be reached by telephone at (972) 415-0914, or by e-mail at shai.roos@hdrinc.com.

Thank you,



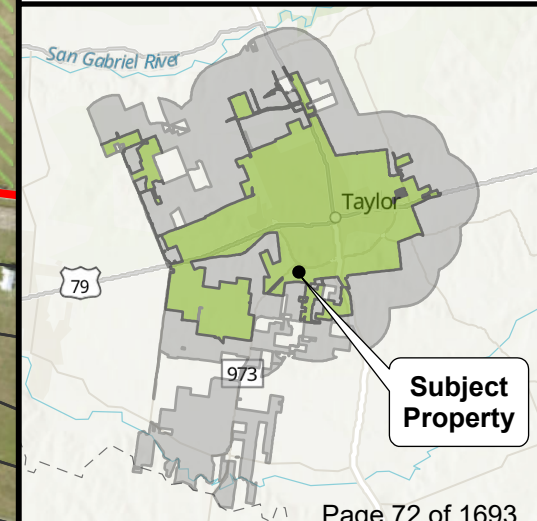
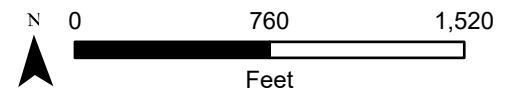
Shai Roos, AICP



PZ-2024-2105

505 SW Carlos G Parker Blvd.
Preliminary Subdivision Plat
Location Map
Approximately 38.1 acres

- City Limits
- ETJ Boundary
- Parcels
- Subject Property



ADDRESS:
505 SW CARLOS G. PARKER BLVD.
TAYLOR, TEXAS 76574

SITE SUMMARY:

- SURVEY:
1. TOTAL ACRES: 38.10
3. PROPOSED RIGHT-OF-WAY AREA: 7.84 ACRES
4. THIS TRACT IS LOCATED ENTIRELY IN THE CITY OF TAYLOR

OWNER/ DEVELOPER:
GATEWAY OZ REAL ESTATE LP,
76 RENFIELD DRIVE
PRINCETON, NEW JERSEY 08540
PHONE: (201) 655-2831
CONTACT: PHILIP GHERARDINI
EMAIL: SERVICES@EVOLVE-REALESTATE.COM

ENGINEER:
KIMLEY-HORN AND ASSOCIATES, INC.
10814 JOLLYVILLE ROAD, CAMPUS IV, SUITE 200
AUSTIN, TEXAS 78759
TBPE FIRM REGISTRATION NO. F-928
PHONE: (512) 720-5466
CONTACT: JOHN PICKENS, P.E.
EMAIL: JOHN.PICKENS@KIMLEY-HORN.COM

SURVEYOR:
KIMLEY-HORN AND ASSOCIATES, INC.
MICHAEL A MONTGOMERY II
10814 JOLLYVILLE ROAD
CAMPUS IV, SUITE 200
AUSTIN, TEXAS 78759
PH. (512) 418-1771
MIKE.MONTGOMERY@KIMLEY-HORN.COM

SITE TABLE			
ZONE	ZONE TYPE	AREA	% AREA
T4-S	NEIGHBORHOOD GENERAL ZONE SMALL-FOOTPRINT	12.49 ACRES	32.8%
T4-L	NEIGHBORHOOD GENERAL ZONE LARGE-FOOTPRINT	6.46 ACRES	16.9%
T5	NEIGHBORHOOD CENTER ZONE	7.63 ACRES	20.0%
SD	SPECIAL DISTRICT	11.52 ACRES	30.3%
	TOTAL AREA	38.10 ACRES	100.0%

STREET SUMMARY TABLE:					
STREET NAME	LENGTH	DESIGN SPEED	ROW WIDTH	PD THOROUGHFARE PLAN CLASSIFICATION	CITY OF TAYLOR STREET CLASSIFICATION
WOZNIAC STREET	670 LF	20 MPH	60 FEET	(CS-60-36)	NEIGHBORHOOD AVENUE
TURING STREET	1260 LF	20 MPH	60 FEET	(CS-60-36)	NEIGHBORHOOD AVENUE
FREMI STREET	333 LF	20 MPH	60 FEET	(CS-60-36)	NEIGHBORHOOD AVENUE
CURIE STREET	301 LF	15 MPH	52 FEET	(CS-52-28)	YIELD STREET
HOPPER STREET	832 LF	20 MPH	60 FEET	(ST-60-34)	NEIGHBORHOOD AVENUE
CARMAK STREET	506 LF	20 MPH	60 FEET	(ST-60-34)	NEIGHBORHOOD AVENUE
GODDARD STREET	467 LF	15 MPH	53 FEET	(ST-53-27)	YIELD STREET
DEERE STREET	507 LF	15 MPH	45 FEET	(ST-45-19)	YIELD STREET
LAMARR STREET	830 LF	20 MPH	60 FEET	(ST-60-34)	NEIGHBORHOOD AVENUE
WILMUT STREET	341 LF	15 MPH	53 FEET	(ST-53-27)	YIELD STREET

NOTES:

- UTILITY PROVIDERS - WATER THE CITY OF TAYLOR. WASTEWATER - THE CITY OF TAYLOR. ELECTRICITY - ONCOR.
- ALL STRUCTURES AND/OR OBSTRUCTIONS ARE PROHIBITED IN DRAINAGE AREAS/RESERVES.
- THERE ARE NO AREAS WITHIN THE BOUNDARIES OF THIS SITE IN THE 100-YEAR FLOODPLAIN AD DEFINED BY FIRM MAP NO. 48491C0541F EFFECTIVE DATE OF DECEMBER 20, 2019.
- MINIMUM FIRE FLOW OF 500 GALLONS PER MINUTE FOR NOT LESS THAN TWO (2) HOURS SHALL BE APPROVED.
- GATEWAY TRACT SUBDIVISION PRELIMINARY PLAT TO BE IN CONFORMANCE WITH THE PLANNED DEVELOPMENT ORDINANCE 2023-34.
- ALL CIVIC SPACE RESERVES SHALL BE OWNED AND MAINTAINED BY THE PROPERTY OWNERS ASSOCIATION.
- ALL CIVIC SPACE RESERVES SHALL HAVE A PUBLIC ACCESS EASEMENT TO PROVIDE ACCESS TO LOTS.
- A 10' P.U.E SHALL BE RESERVED ALONG ALL PUBLIC RIGHTS-OF-WAY.
- SIDEWALKS ARE REQUIRED PER THE PLANNED DEVELOPMENT ORDINANCE 2023-34

COORDINATES: TEXAS STATE COORDINATE SYSTEM, CENTRAL ZONE (4203)
HORIZONTAL DATUM: NAD '83
VERTICAL DATUM: NAD '88

For Review:

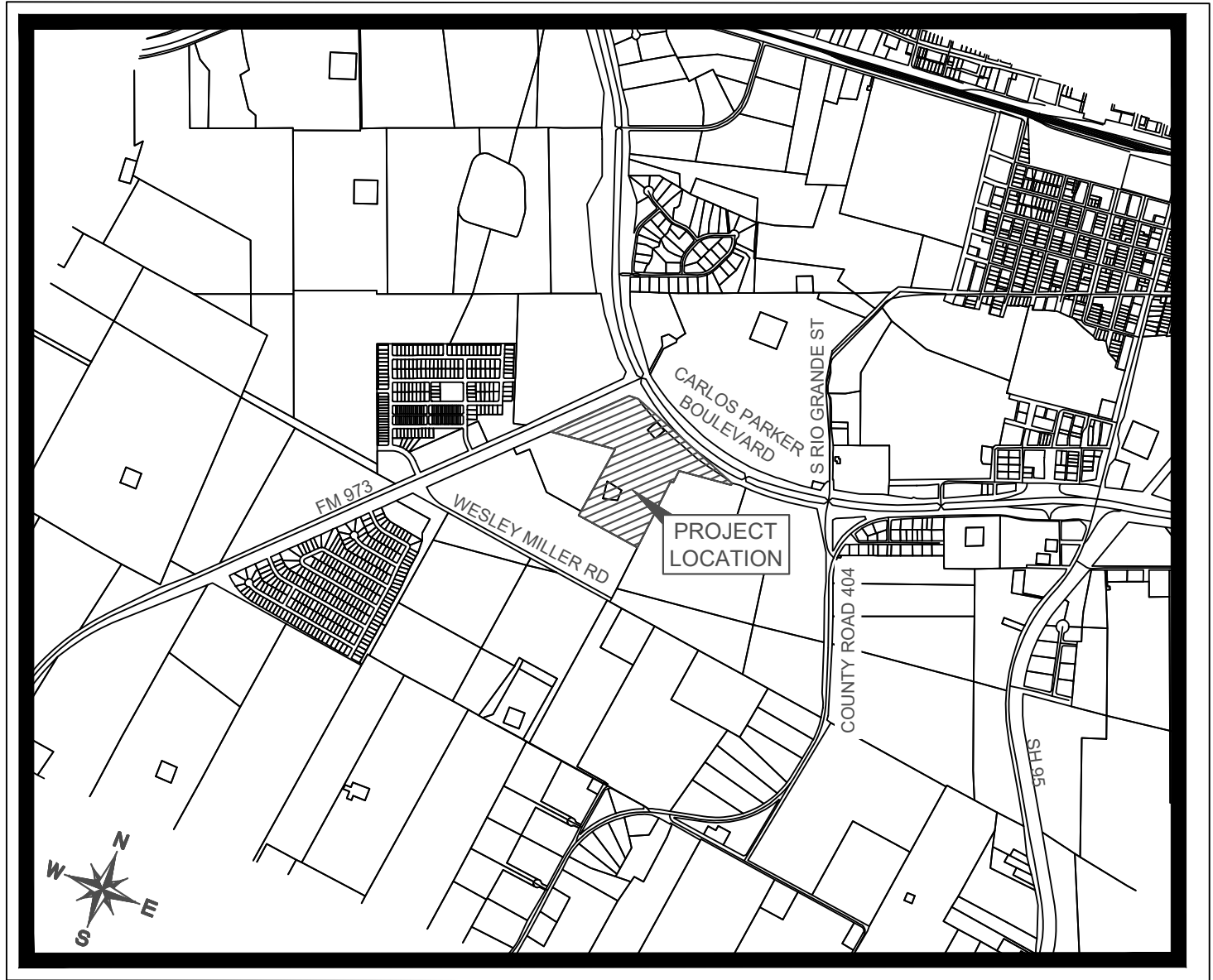
This document is released for the purpose of review under the authority of John Pickens,
Professional Engineer No. 110631 on 07/19/2024. It is not to be used for bidding, permit
or construction.

GATEWAY TRACT SUBDIVISION PRELIMINARY PLAT

BEING A PRELIMINARY PLAT CONSISTING OF
38.10 ACRES, PART OF AND OUT OF THE
GEORGE M. REESE SURVEY, ABSTRACT NO.
533, TAYLOR, WILLIAMSON COUNTY, TEXAS

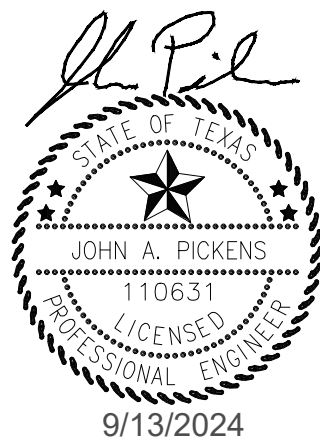
SHEET INDEX

SHEET NO.	DESCRIPTION
1	OVERVIEW, VICINITY MAP, NOTES, SUMMARY
2	PLAT, LOT AREAS, LINE TABLES, CURVE TABLES
3	DEDICATION SHEET



VICINITY MAP

SCALE: 1" = 2,000'



BENCHMARKS

BENCHMARK:	
NGS NO. BM0359	ELEV.: 554.10'
BENCHMARK IS A BRASS DISK STAMPED "E 196 1933" LOCATED 1.1 MILES FROM TYLOR, BEING 1.1 MILES SOUTH OF AND ALONG N.I.C.T. RAILROAD 145 YARDS SOUTH OF MILE POST 500.70 YARDS SOUTH OF ROAD CROSSING ON THE SOUTHWEST END OF A CONCRETE RAILROAD CULVERT.	
SITE BENCHMARK:	
5/8 I.R.	ELEV.: 601.68'
BENCHMARK IS A 5/8 INCH IRON ROD WITH CAP SET, LOCATED NORTH 63°58'55" WEST, A DISTANCE OF 11.34 FEET FROM A UTILITY POWER POLE. BENCHMARK CAN BE FOUND WITH THE FOLLOWING COORDINATES: N:10,175,573.072, E:3,211,584.120	

**GATEWAY TRACT
PRELIMINARY PLAT**
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 9/13/2024

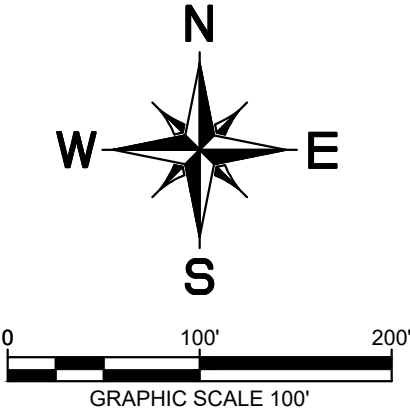
Kimley»Horn

TEXAS REGISTERED ENGINEERING FIRM F-928

Scale	Drawn by	Checked by	Date	KHA Project No.	Sheet No.
	AJM	JAP	SEPTEMBER 2024	069405306	1 OF 3

City Project Number: PZ-2024-2105

GATEWAY TRACT SUBDIVISION PRELIMINARY PLAT
BEING A PRELIMINARY PLAT CONSISTING OF 38.10 ACRES, PART OF AND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT NO. 533,
TAYLOR, WILLIAMSON COUNTY, TEXAS



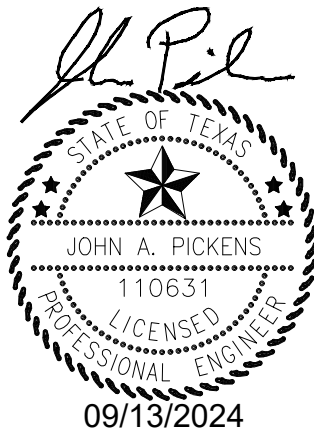
LEGEND	
	PROPERTY BOUNDARY
	PROPOSED RIGHT-OF-WAY
	PROPOSED FIRE LANE
	PROPOSED PUBLIC SIDEWALK
	PROPOSED EASEMENT
	NEIGHBORING PROPERTY BOUNDARY
	EXISTING EASEMENT
	EXISTING CONTOUR
	EXISTING FEMA 100-YR FLOODPLAIN

LINE TABLE		
LINE	LENGTH	BEARING
L1	34.79	S83°46'14.00"W

CURVE TABLE						
CURVE	RADIUS	LENGTH	CHORD BEARING	CHORD	DELTA	TANGENT
C1	2984.93'	1579.99'	S68°32'22"E	1561.61'	30°19'41"	808.97'

BENCHMARKS

BENCHMARK:
NGS NO. BM0359 ELEV.: 554.10'
BENCHMARK IS A BRASS DISK STAMPED "E 196 1933" LOCATED 1.1 MILES FROM TAYLOR, BEING 1.1 MILES SOUTH OF AND ALONG M.K.T. RAILROAD 145 YARDS SOUTH OF MILE POST 920, 70 YARDS SOUTH OF ROAD CROSSING ON THE SOUTHWEST END OF A CONCRETE RAILROAD CULVERT.
SITE BENCHMARK:
516 I.R. ELEV.: 601.68'
BENCHMARK IS A 1/2" INCH IRON ROD WITH CAP SET, LOCATED NORTH 83°58'55" WEST, A DISTANCE OF 11.34 FEET FROM A UTILITY POWER POLE. BENCHMARK CAN BE FOUND WITH THE FOLLOWING COORDINATES: N:10,175,573.072; E:3,211,584.120



**GATEWAY TRACT
PRELIMINARY PLAT**
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 9/13/2024

Kimley»Horn
TEXAS REGISTERED ENGINEERING FIRM F-928

Scale	Drawn by	Checked by	Date	KHA Project No.	Sheet No.
	AJM	JAP	SEPTEMBER 2024	069405306	2 OF 3

City Project Number: PZ-2024-2105

GATEWAY TRACT SUBDIVISION PRELIMINARY PLAT

BEING A PRELIMINARY PLAT CONSISTING OF 38.10 ACRES, PART OF AND OUT OF THE GEORGE M. REESE SURVEY, ABSTRACT NO. 533, TAYLOR, WILLIAMSON COUNTY, TEXAS

OWNER'S DEDICATION STATEMENT

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS

THAT GATEWAY OZ REAL ESTATE, LP SOLE OWNER OF THAT CERTAIN 38.09 ACRE TRACT OF LAND SHOWN HEREON AND DESCRIBED IN DEEDS RECORDED IN DOCUMENT NO. 2021150771, BOTH OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS, AND DO HEREBY STATE THAT THERE ARE NO LIEN HOLDERS ON THAT CERTAIN TRACT OF LAND, AND DO HEREBY SUBDIVIDE SAID TRACT AS SHOWN HEREON, AND DO HEREBY CONSENT TO ALL PLAT NOTE REQUIREMENTS SHOWN HEREON, AND DO HEREBY DEDICATE TO THE CITY OF TAYLOR THE STREETS, ALLEYS, RIGHT-OF-WAY, EASEMENTS AND PUBLIC PLACES SHOWN HEREON FOR SUCH PUBLIC PURPOSES AS THE CITY OF TAYLOR MAY DEEM APPROPRIATE. THIS SUBDIVISION IS TO BE KNOWN AS "GATEWAY TRACT".

TO CERTIFY WHICH, WITNESS BY MY HAND THIS _____ day of _____, 2024.

SANTOSH ENUKONDA
GATEWAY OZ REAL ESTATE, LP
76 RENFIELD DRIVE
PRINCETON, NJ 08540

STATE OF TEXAS §
COUNTY OF _____ §

KNOW ALL MEN BY THESE PRESENTS

BEFORE ME, THE UNDERSIGNED, A NOTARY PUBLIC IN AND FOR SAID COUNTY AND STATE, ON THIS DAY PERSONALLY APPEARED AARON LEVY, KNOWN TO ME TO BE THE PERSON WHOSE NAME IS SUBSCRIBED TO THE FOREGOING INSTRUMENT.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS _____ day of _____, 2024.

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS

MY COMMISSION EXPIRES ON: _____

WILLIAMSON COUNTY CLERK

STATE OF TEXAS §
COUNTY OF WILLIAMSON §

KNOW ALL MEN BY THESE PRESENTS

I, NANCY E. RISTER, CLERK OF THE COUNTY COURT OF SAID COUNTY, DO HEREBY CERTIFY THAT THE FOREGOING INSTRUMENT IS WRITING, WITH ITS CERTIFICATE OF AUTHENTICATION, WAS FILED FOR RECORD IN MY OFFICE ON THE _____ DAY OF _____, 2024 A.D., AT _____ O'CLOCK _____ M., AND DULY RECORDED THIS THE _____ DAY OF _____, 2024 A.D., AT _____ O'CLOCK _____ M., IN THE OFFICIAL PUBLIC RECORDS OF SAID COUNTY IN INSTRUMENT NO. _____.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT THE SAID COUNTY COURT OF SAID COUNTY, AT MY OFFICE IN GEORGETOWN, TEXAS, THE DATE LAST SHOWN ABOVE WRITTEN.

NANCY E. RISTER, CLERK COUNTY COURT
OF WILLIAMSON COUNTY, TEXAS

BY: _____, DEPUTY

PLANNING & ZONING COMMISSION

A PRELIMINARY PLAT FOR A SUBDIVISION TO BE KNOW AS "GATEWAY TRACT" HAS BEEN APPROVED ACCORDING TO THE MINUTES OF THE MEETING OF THE TAYLOR PLANNING AND ZONING COMMISSION OF THE _____ DAY OF _____, 2021.

_____, CHAIRMAN _____ DATE _____
_____, SECRETARY _____ DATE _____

SURVEYORS CERTIFICATION:

STATE OF TEXAS §
COUNTY OF COLLIN §

KNOW ALL MEN BY THESE PRESENTS

I, MICHAEL A. MONTGOMERY II, REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS PLAT IS TRUE AND CORRECTLY MADE FROM AN ACTUAL SURVEY MADE ON THE GROUND OF THE PROPERTY LEGALLY DESCRIBED HEREON, AND THAT THERE ARE NO APPARENT DISCREPANCIES, CONFLICTS, OVERLAPPING OF IMPROVEMENTS, VISIBLE UTILITY LINES OR ROADS IN PLACE, EXCEPT AS SHOWN ON THE ACCOMPANYING PLAT, AND THAT THE CORNER MONUMENTS SHOWN THEREON WERE PROPERLY PLACED UNDER MY SUPERVISION IN ACCORDANCE WITH THE SUBDIVISION REGULATIONS OF THE CITY OF TAYLOR, TEXAS

MICHAEL A. MONTGOMERY, II,
REGISTERED PROFESSIONAL LAND SURVEYOR NO. 6860
KIMLEY-HORN AND ASSOCIATES
10814 JOLLYVILLE ROAD
CAMPUS IV, SUITE 200
AUSTIN, TEXAS 78759
PH. 512-418-1771

ENGINEERS CERTIFICATION:

STATE OF TEXAS §
COUNTY OF TRAVIS §

KNOW ALL MEN BY THESE PRESENTS

I, JOHN PICKENS, LICENSED PROFESSIONAL ENGINEER IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THIS SUBDIVISION IS NOT ENCLOSED BY A ZONE A FLOOD AREA, AS DENOTED HEREIN, AND AS DEFINED BY FEDERAL EMERGENCY MANAGEMENT ADMINISTRATION FLOOD HAZARD BOUNDARY MAP, COMMUNITY PANEL NUMBER 48491C0541F, EFFECTIVE DATE DECEMBER 12, 2019, AND THAT EACH LOT CONFORMS TO THE CITY OF TAYLOR REGULATIONS. THE FULLY DEVELOPED, CONCENTRATED STORMWATER RUNOFF RESULTING FROM THE ONE HUNDRED (100) YEAR FREQUENCY STORM IS CONTAINED WITHIN THE DRAINAGE EASEMENTS SHOWN AND/OR PUBLIC RIGHTS-OF-WAY DEDICATED BY THIS PLAT.

TO CERTIFY WHICH, WITNESS MY HAND AND SEAL AT AUSTIN, TRAVIS COUNTY, TEXAS, THIS _____ DAY OF _____, 2024.

JOHN PICKENS
LICENSED PROFESSIONAL ENGINEER NO. 110631
KIMLEY-HORN AND ASSOCIATES, INC.
10814 JOLLYVILLE ROAD, CAMPUS IV, SUITE 200
AUSTIN, TEXAS 78759
PH. 512-418-1791

LEGAL DESCRIPTION:

38.10 ACRES TRACT
BEING A 38.10 ACRES TRACT OF LAND SITUATED IN THE GEORGE M. REESE SURVEY, ABSTRACT 533, WILLIAMSON COUNTY, TEXAS; AND BEING ALL OF A CALLED 38.100 ACRES TRACT OF LAND DESCRIBED TO GATEWAY OZ REAL ESTATE, LP, AS SHOWN ON INSTRUMENT RECORDED IN DOCUMENT NO. 2021150771 OF THE OFFICIAL PUBLIC RECORDS OF WILLIAMSON COUNTY, TEXAS (O.P.R.W.C.T.); AND BEING ALL OF A CALLED 0.776 ACRES TRACT OF LAND ALSO DESCRIBED TO GATEWAY OZ REAL ESTATE, LP, AS SHOWN ON INSTRUMENT RECORDED IN DOCUMENT NO. 2021150737 OF SAID O.P.R.W.C.T.;

BEGINNING AT A TEXAS DEPARTMENT OF TRANSPORTATION TYPE II MONUMENT FOUND, AT THE INTERSECTION OF THE SOUTHEASTERLY RIGHT-OF-WAY LINE OF F.M. 973, A VARIABLE WIDTH RIGHT-OF-WAY, ACCORDING TO TEXAS DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY MAP CSJ NO. 2285-01-007, WITH THE SOUTHWESTERLY RIGHT-OF-WAY LINE OF LOOP 427 (A.K.A. CARLOS PARKER BOULEVARD), A VARIABLE WIDTH RIGHT-OF-WAY, ACCORDING TO TEXAS DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY MAP CSJ NO. 0320-04-001, SAME BEING THE NORTH CORNER OF SAID 38.100 ACRES TRACT;

THENCE, DEPARTING SAID SOUTHEASTERLY RIGHT-OF-WAY LINE OF F.M. 973, WITH SAID SOUTHWESTERLY RIGHT-OF-WAY LINE OF LOOP 427, SAME BEING THE NORTH BOUNDARY LINE OF SAID 38.100 ACRES TRACT, IN A SOUTHEASTERLY DIRECTION, WITH A CURVE TO THE LEFT, HAVING A CENTRAL ANGLE OF 30°19'41", A RADIUS OF 2,384.62 FEET, A CHORD BEARING AND DISTANCE OF SOUTH 68°32'22" EAST, 1,561.61 FEET, AND AN ARC LENGTH OF 1,579.96 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "CORE LAND SURVEYING" FOUND, FOR THE NORTHEAST CORNER OF THE REMAINDER OF A CALLED 1.35 ACRES TRACT OF LAND DESCRIBED TO C.W. LINDQUIST, AS SHOWN ON INSTRUMENT RECORDED IN DOCUMENT NO. 191301594 OF SAID O.P.R.W.C.T., SAME BEING THE EAST CORNER OF SAID 38.100 ACRES TRACT;

THENCE, SOUTH 83°42'51" WEST, DEPARTING SAID SOUTHWESTERLY RIGHT-OF-WAY LINE OF CARLOS PARKER BOULEVARD, WITH THE NORTH BOUNDARY LINE OF SAID REMAINDER OF 1.35 ACRES TRACT, SAME BEING THE EAST BOUNDARY LINE OF SAID 38.100 ACRES, A DISTANCE OF 680.54 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "CORE 6657" FOUND, FOR THE NORTHWEST CORNER OF SAID REMAINDER OF 1.35 ACRES TRACT;

THENCE, SOUTH 59°22'30" WEST, WITH THE WEST BOUNDARY LINE OF SAID REMAINDER OF 1.35 ACRES TRACT, SAME BEING SAID EAST BOUNDARY LINE OF THE 38.100 ACRES TRACT, PASSING A COTTON SPINDLE WITH WASHER STAMPED "BAKER-AICKLEN & ASSOC" FOUND, FOR THE SOUTHWEST CORNER OF SAID 1.35 ACRES TRACT, SAME BEING THE NORTHWEST CORNER OF A CALLED 68.36 ACRES TRACT OF LAND DESCRIBED TO TEMPLE COLLEGE AT TAYLOR FOUNDATION, INC., AS SHOWN ON INSTRUMENT RECORDED IN DOCUMENT NO. 2008029591 OF SAID O.P.R.W.C.T., CONTINUING WITH THE WEST BOUNDARY LINE OF SAID 68.36 ACRES TRACT, SAME BEING SAID EAST BOUNDARY LINE OF THE 38.100 ACRES TRACT, A DISTANCE OF 211.28 FEET TO A 5/8-INCH IRON ROD WITH PLASTIC CAP STAMPED "CORE 6657" FOUND;

THENCE, CONTINUING WITH SAID WEST BOUNDARY LINE OF THE 68.36 ACRES TRACT AND SAID EAST BOUNDARY LINE OF THE 38.100 ACRES TRACT, THE FOLLOWING TWO (2) COURSES AND DISTANCES:

- SOUTH 83°46'14" WEST, A DISTANCE OF 34.79 FEET TO A 5/8-INCH IRON ROD FOUND;
- SOUTH 09°22'04" WEST, A DISTANCE OF 926.69 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "BAKER-AICKLEN & ASSOC CEDAR PARK" FOUND, FOR THE NORTHEAST CORNER OF LOT 1, BLOCK A, TAYLOR HIGH SCHOOL ADDITION, A LEGAL SUBDIVISION, AS SHOWN ON PLAT RECORDED IN DOCUMENT NO. 201040451 OF SAID O.P.R.W.C.T., SAME BEING THE SOUTHEAST CORNER OF SAID 38.100 ACRES TRACT;

THENCE, NORTH 82°35'57" WEST, DEPARTING SAID WEST BOUNDARY LINE OF THE 68.36 ACRES TRACT, WITH THE NORTH BOUNDARY LINE OF SAID LOT 1, SAME BEING THE SOUTH BOUNDARY LINE OF SAID 38.100 ACRES TRACT, A DISTANCE OF 842.37 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "BAKER-AICKLEN & ASSOC INC CEDAR PARK TEXAS" FOUND;

THENCE, NORTH 07°24'02" EAST, WITH AN EAST BOUNDARY LINE OF SAID LOT 1 AND LOT 2, BLOCK A, SAID TAYLOR HIGH SCHOOL ADDITION, SAME BEING THE WEST BOUNDARY LINE OF SAID 38.100 ACRES TRACT, A DISTANCE OF 950.03 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "CORE LAND SURVEYING" FOUND, FOR THE NORTHEAST CORNER OF SAID LOT 2;

THENCE, SOUTH 83°48'33" WEST, WITH THE NORTH BOUNDARY LINE OF SAID LOT 2, SAME BEING SAID WEST BOUNDARY LINE OF THE 38.100 ACRES, A DISTANCE OF 636.59 FEET TO A 1/2-INCH IRON ROD WITH PLASTIC CAP STAMPED "CORE LAND SURVEYING" FOUND, IN SAID SOUTHEASTERLY RIGHT-OF-WAY LINE OF F.M. 973, FOR THE NORTHWEST CORNER OF SAID LOT 2;

THENCE, WITH SAID SOUTHEASTERLY RIGHT-OF-WAY LINE OF F.M. 973, SAME BEING SAID WEST BOUNDARY LINE OF THE 38.100 ACRES TRACT, THE FOLLOWING TWO (2) COURSES AND DISTANCES:

- NORTH 44°29'20" EAST, A DISTANCE OF 1,141.94 FEET TO A TEXAS DEPARTMENT OF TRANSPORTATION TYPE II MONUMENT FOUND;
- NORTH 85°58'48" EAST, A DISTANCE OF 111.89 FEET TO THE POINT OF BEGINNING AND CONTAINING 38.10 ACRES OF LAND, MORE OR LESS, IN WILLIAMSON COUNTY, TEXAS. THIS DOCUMENT WAS PREPARED IN THE OFFICE OF KIMLEY-HORN INC. IN AUSTIN, TEXAS.

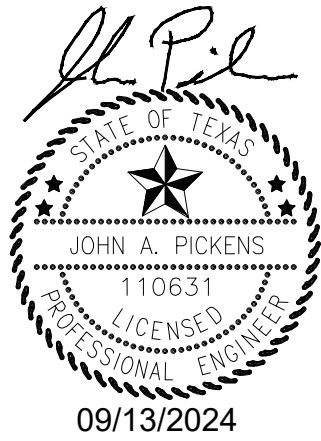
RESERVED SPACES		
PROPOSED RESERVE 1	CIVIC SPACE/SQUARE WITH ACCESS, DRAINAGE, & UTILITY EASEMENT	0.29 AC.
PROPOSED RESERVE 2	CIVIC SPACE WITH ACCESS, DRAINAGE, & UTILITY EASEMENT	1.21 AC.
PROPOSED RESERVE 3	CIVIC SPACE WITH ACCESS, DRAINAGE, & UTILITY EASEMENT	0.76 AC.
PROPOSED RESERVE 4	CIVIC SPACE	0.06 AC.
PROPOSED RESERVE 5	CIVIC SPACE - FARNSWORTH WAY	0.20 AC.
PROPOSED RESERVE 6	CIVIC SPACE WITH ACCESS, DRAINAGE, & UTILITY EASEMENT	1.07 AC.
PROPOSED RESERVE 7	CIVIC SPACE - CARVER WAY	0.05 AC.
PROPOSED RESERVE 8	CIVIC SPACE - CARVER WAY	0.09 AC.
PROPOSED RESERVE 9	CIVIC SPACE - CARVER WAY	0.07 AC.
PROPOSED RESERVE 10	CIVIC SPACE WITH ACCESS, DRAINAGE, & UTILITY EASEMENT	1.66 AC.

PRIVATE ALLEY	
1	FA-30-20
2	FA-30-20
3	FA-30-20
4	FA-30-20
5	FA-30-20
6	FA-30-20
7	FA-30-20
8	FA-30-20

PEDESTRIAN ACCESS		
PEDESTRIAN ACCESS 1	1,000 SF	PT-V-5
PEDESTRIAN ACCESS 2	400 SF	PT-V-5
PEDESTRIAN ACCESS 3	1,155 SF	PT-V-5
PEDESTRIAN ACCESS 4	360 SF	PT-V-5
PEDESTRIAN ACCESS 5	886 SF	PT-V-5
PEDESTRIAN ACCESS 6	1,500 SF	PT-V-5
PEDESTRIAN ACCESS 7	415 SF	PT-V-5
PEDESTRIAN ACCESS 8 - FARNSWORTH WAY	2,400 SF	PT-V-5
PEDESTRIAN ACCESS 9 - CARVER WAY	691 SF	PT-V-5
PEDESTRIAN ACCESS 10 - CARVER WAY	656 SF	PT-V-5

PRIVATE ALLEY	
PRIVATE ALLEY 1 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	3.60 AC.
PRIVATE ALLEY 2 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	1.05 AC.
PRIVATE ALLEY 3 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	0.99 AC.
PRIVATE ALLEY 4 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	0.26 AC.
PRIVATE ALLEY 5 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	0.67 AC.
PRIVATE ALLEY 6 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	1.93 AC.
PRIVATE ALLEY 7 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	2.16 AC.
PRIVATE ALLEY 8 WITH PARKING WITH ACCESS, DRAINAGE & UTILITY EASEMENT	0.41 AC.

BLOCK A		BLOCK D		BLOCK E		BLOCK G	
LOT	SF	LOT	SF	LOT	SF	LOT	SF
1	45565	1	5423	1	3100	1	13549
2	39482	2	3000	2	2500	2	12400
3	7793	3	4979	3	2500	3	12800
BLOCK B		4	2500	4	2500	4	2124
LOT	SF	5	2500	5	2500	5	1888
1	22520	6	2500	6	2425	6	2171
2	32607	7	2500	7	2717	7	2905
3	26053	8	2500	8	3113	8	2445
4	42438	9	2500	9	2754	9	2468
5	10432	10	2500	10	2756	10	2490
6	8151	11	2500	11	2758	11	3605
		12	3100	12	2103	12	5176
		13	3100	13	1403	13	2501
		14	2500	14	1800	14	2501
		15	2500	15	1800	15	2500
		16	2500	16	2237	16	2500
		17	3100	17	1869	17	3749
		18	1840	18	1800	18	3300
		19	1800	19	1800	19	2500
		20	1800	20	1840	20	3000
		21	1800	BLOCK F		21	3000
		22	1800	LOT	SF	22	4741
		23	2842	1	12782	BLOCK I	
		24	3054	2	12400	LOT	SF
		25	1863	3	12400	1	3015
		26	2092	4	12112	2	2512
		27	22037	5	12000	3	2513
		28	2142	6	12023	4	2513
		29	1667	7	1930	5	3015
		30	1620	8	2000	6	3005
		31	2267	9	1600	7	2480
		32	2158	10	1600	8	2458
		33	1620	11	1600	9	2435
		34	1620	12	2303	10	3671
		35	1620	13	1998		
		36	1859	14	1600		
				15	1600		
				16	1636		



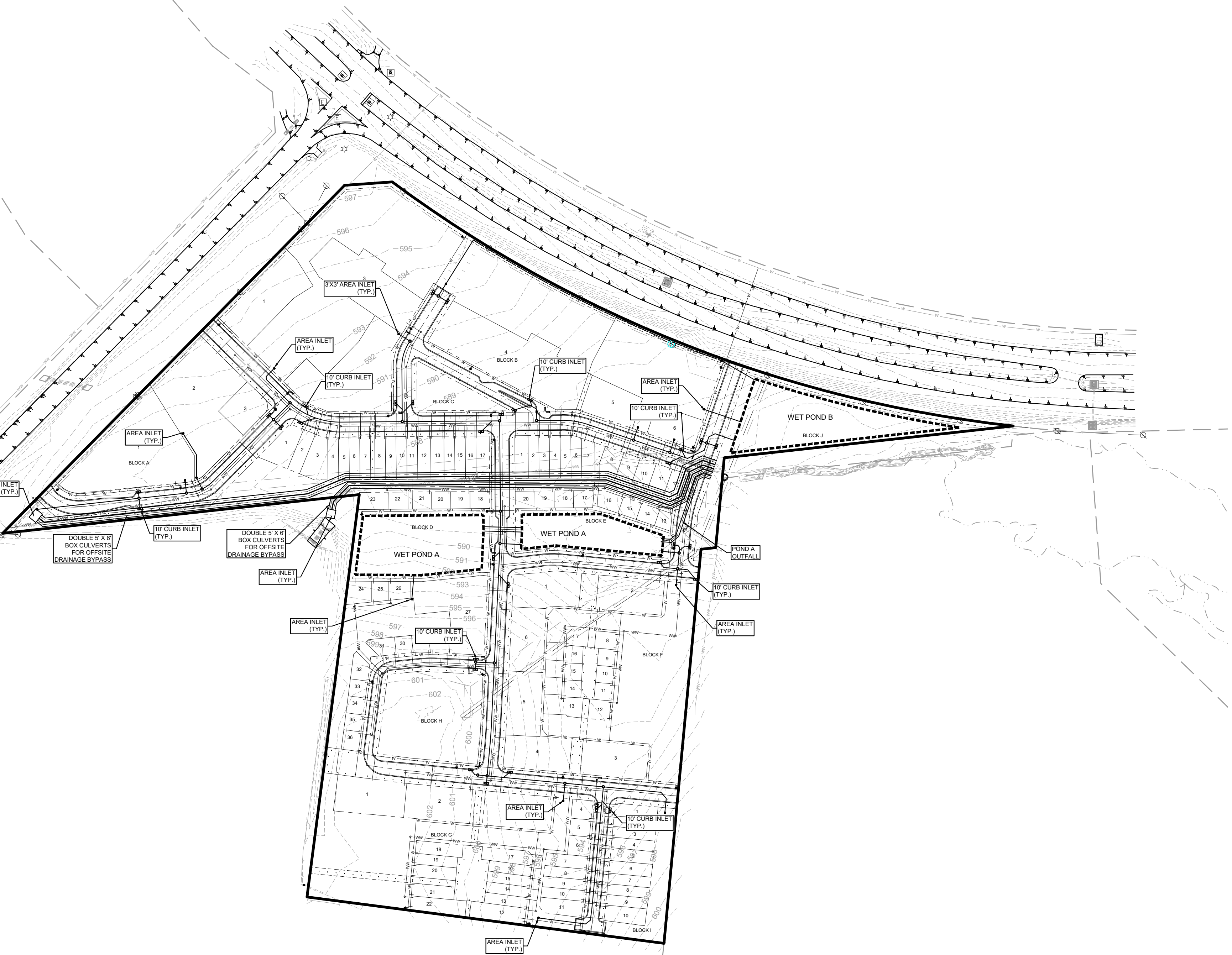
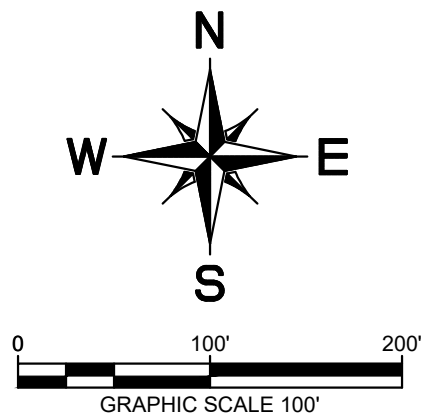
GATEWAY TRACT
PRELIMINARY PLAT
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 9/13/2024

Kimley»Horn

TEXAS REGISTERED ENGINEERING FIRM F-928

Scale	Drawn by	Checked by	Date	KHA Project No.	Sheet No.
	AJM	JAP	SEPTEMBER 2024	069405306	OF 3

City Project Number: PZ-2024-2105



**GATEWAY TRACT
PRELIMINARY DRAINAGE PLAN**
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 09/13/2024



TEXAS REGISTERED ENGINEERING FIRM F-928					
Scale	Drawn by	Checked by	Date	KHA Project No.	Sheet No.
	AJM	JAP	SEPTEMBER 2024	069405306	1 OF 1

August 7, 2024

Technical Memorandum

Response to TIA Comments, Traffic Impact Analysis, Taylor Gateway (v1.2)
Comments by City of Taylor (via HDR) in black text, responses in green.

It was not necessary to produce a revised report in response to these limited number of comments.

Comment Number	Page No.	Comments
1	n/a	Per discussions with TxDOT, it's our understanding that Driveway 1B should be modeled as RIRO in synchro files and in report. V1.2 of the report documented Driveway 1B on FM 973 as right-in/right-out only. See discussion throughout the report and the anticipated buildout year volumes in Exhibits A17 and A18, and the annotated improvement diagram on Exhibit A19.
2	n/a	Per discussions with TxDOT, it's our understanding that they will allow signalization of Driveway 2B when warrants are met. Please move left turns that were utilizing Driveway 1B to Driveway 2B for analysis. Verify any turn lanes requirements for entering/exiting Driveway 2B. See response to comment #1 above and Exhibit A19 documenting a summary of turn lane requirements and Exhibits A20-A, B, and C for KHA's conceptual schematics for the turn lanes in for review by TxDOT.
3	n/a	Driveway 2B must align with Driveway #3 for the QuickTrip #4192 Development, which is located directly across FM 973 from this site. (see figure below). It was our understanding that since the Gateway project was active first that TxDOT would require that the QuickTrip project match the location of Driveway 2B, not vice-versa, particularly given that 1) the spacing between Driveway 2B and other driveways meets TxDOT Access Management Manual guidelines for highways with a 50 mph speed limit, 2) Driveway 2B is appropriately spaced from both Driveway 1B and the Taylor High School north site driveway, and 3) and since the Gateway Project was the initial project in the area we have confirmation from Shane Brown and Scott Cunningham that they planned to discuss adjustments to access locations with the QuickTrip developers. The Gateway project team will further coordinate with TxDOT and report back to the city with the result of those discussions, but would also ask that the city confirm this with TxDOT as well.

Comment Number	Page No.	Comments
4	n/a	For the left-turn lanes at FM 973 and US 79 required by TxDOT, please confirm if those will be built by this project or if there is a pro-rate share contribution that will be calculated and provide calculation. The analysis of the dual left turn configuration on FM 973 at US 79 was completed at the request of HDR as documented in v1.2 of the report. Since this was a new analysis and finding, the project team will approach TxDOT with the information and coordinate on whether this will be TxDOT driven, developer driven, or some combination of the two if required by TxDOT. The project team will reach back out to the City of Taylor with the results of that coordination effort.
5	n/a	Please include pro rata estimate for the signal at Wesley Miller Lane. This signal has not been installed, but is recommended by various other projects. The calculation of the Gateway projects pro-rata share towards cost of traffic signal installation at FM 973 and Wesley Miller Lane was included on Page 21 of v1.2 of the report (calculated as \$22,119).

PRELIMINARY DRAINAGE IMPACT ANALYSIS

TAYLOR GATEWAY

Taylor, Texas

SEPTEMBER 2024

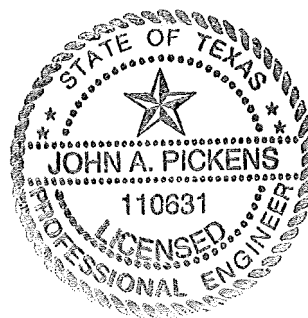
Project Owner/Developer:

GATEWAY OZ REAL ESTATE, LP
76 REINFELD DRIVE
PRINCETON, NEW JERSEY 08540

Prepared By:

KIMLEY-HORN AND ASSOCIATES, INC.
10814 JOLLYVILLE RD. BLDG. 4 STE. 200
AUSTIN, TX 78759
(512) 418-1771

KHA No. 067782815



A handwritten signature in black ink, appearing to read "John A. Pickens", written over the professional seal.

09/13/2024

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APPENDIX B	PRELIMINARY EXISTING DRAINAGE CALCULATIONS
APPENDIX C	PRELIMINARY PROPOSED DRAINAGE AREA MAP
APPENDIX D	PRELIMINARY PROPOSED DRAINAGE CALCULATIONS

PRELIMINARY DRAINAGE IMPACT ANALYSIS FOR TAYLOR GATEWAY

1.0 PROJECT OVERVIEW

The Taylor Gateway site is located on the southeast corner of Carlos G Parker Boulevard SW and FM 973. The site consists of 38.10 acres located in the Taylor City Limits. The site is proposing a mixed-use development with 2 Detention Ponds.

2.0 PRELIMINARY EXISTING DRAINAGE ANALYSIS

Approximately 165 acres (Areas A1 & A2 on the Existing Drainage Area Map) of offsite drainage that enters the site from the southwest through 2 sets of triples 24" culverts, approximately 105 acres (Area B on the Existing Drainage Area Map) of offsite drainage entering the site from the south discharging from the western Taylor High School Detention Pond, approximately 65 acres (a portion Area C on the Existing Drainage Area Map) of offsite drainage entering the site from the south discharging from the eastern Taylor High School Detention Pond, approximately 32 acres (Area D on the Existing Drainage Area Map) of offsite drainage entering the site from the north under Carlos G Parker Boulevard, and approximately 38 acres (a portion Area C and Area E on the Existing Drainage Area Map) of onsite drainage that all flow to the northeast and leave the site by way of an existing drainage channel located on the northeast portion of the property. See the Existing Drainage Area Map located in **Appendix A**

All runoff calculations were analyzed using the Soil Conservation Services (SCS) Method as documented in the Technical Release 55 utilizing ATLAS 14 storm data. Pond Pack V8i was used to calculate the runoff. The existing drainage calculations are shown below in **Appendix B**.

3.0 PRELIMINARY PROPOSED DRAINAGE ANALYSIS

Proposed Drainage Areas A1 & A2 are planned to be intercepted on the southwest corner of the property (adjacent to FM 973) by 2-5' x 8' box culverts, piped through the site (bypassing the detention ponds), and discharge to the existing ditch located on the northeast portion of the property. See the Proposed Drainage Area Map located in **Appendix C**

Proposed Drainage Area B is planned to be intercepted downstream of Taylor High School's wester detention pond, by 2-5' x 6' box culverts, piped through the site adjacent to the 2-5' x 8' box culverts (bypassing the detention ponds), and discharge to the existing ditch located on the northeast portion of the property. See the Proposed Drainage Area Map located in **Appendix C**

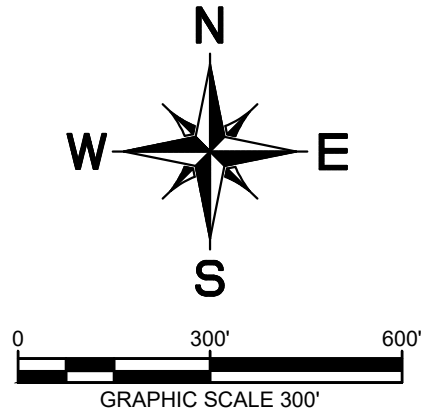
Proposed Drainage Areas C is planned to be intercepted and routed proposed Detention Pond A. The proposed Detention Pond A then discharge to the existing drainage channel located on the northeast portion of the property. See the Proposed Drainage Area Map located in **Appendix C**

Proposed Drainage Areas D, & E are planned to be intercepted and routed to Detention Pond B. The proposed detention pond then discharge to the existing drainage channel located on the northeast portion of the property. See the Proposed Drainage Area Map located in **Appendix C**

All runoff calculations were analyzed using the Soil Conservation Services (SCS) Method as documented in the Technical Release 55. Pond Pack V8i was used to calculate the runoff and design the pond volume and outlet structure. The existing and proposed runoff calculations are shown below in **Appendix D**.

APPENDIX A

PRELIMINARY EXISTING DRAINAGE AREA MAP



PRELIMINARY EXISTING DRAINAGE PEAK FLOW SUMMARY				
AREA	2-YR Q	10-YR Q	25-YR Q	100-YR Q
DA A1	114.13 CFS	344.29 CFS	458.07 CFS	649.43 CFS
DA A2	27.71 CFS	87.58 CFS	115.96 CFS	163.26 CFS
POA A	137.84 CFS	413.45 CFS	550.18 CFS	780.49 CFS
DA B	93.10 CFS	280.93 CFS	373.90 CFS	530.04 CFS
POA B	228.61 CFS	691.80 CFS	921.77 CFS	1310.21 CFS
DA C	63.09 CFS	190.22 CFS	253.42 CFS	359.95 CFS
DA D	33.87 CFS	107.11 CFS	141.73 CFS	199.85 CFS
POA C	33.87 CFS	107.11 CFS	141.73 CFS	199.85 CFS
DA E	41.25 CFS	135.22 CFS	178.71 CFS	251.49 CFS
POA D	336.72 CFS	966.89 CFS	1,280.80 CFS	1,811.95 CFS



**GATEWAY TRACT
EXISTING DRAINAGE AREA MAP**
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 9/13/2024

Kimley»Horn

TEXAS REGISTERED ENGINEERING FIRM F-928					
Scale	Drawn by	Checked by	Date	KHA Project No.	Sheet No.
1" = 300'	RRJ	JAP	SEPTEMBER 2024	069405306	OF 3

APPENDIX B

PRELIMINARY EXISTING DRAINAGE CALCULAITONS

Project Summary	
Title	Taylor Gateway
Engineer	John Pickens, P.E.
Company	Kimley-Horn & Associates, Inc.
Date	9/13/2024

Notes

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)
EX AREA C	EX 2	2	460,866.000	756.000	63.09
EX AREA C	EX 10	10	1,258,905.000	756.000	190.22
EX AREA C	EX 25	25	1,771,491.000	756.000	253.42
EX AREA C	EX 100	100	2,759,037.000	756.000	359.95
EX AREA B	EX 2	2	621,937.000	753.000	93.10
EX AREA B	EX 10	10	1,700,726.000	750.000	280.93
EX AREA B	EX 25	25	2,392,974.000	750.000	373.90
EX AREA B	EX 100	100	3,726,557.000	750.000	530.04
EX AREA A1	EX 2	2	800,993.000	753.000	114.31
EX AREA A1	EX 10	10	2,181,713.000	753.000	344.29
EX AREA A1	EX 25	25	3,067,288.000	753.000	458.07
EX AREA A1	EX 100	100	4,772,632.000	753.000	649.43
EX AREA A2	EX 2	2	153,458.000	741.000	27.71
EX AREA A2	EX 10	10	420,586.000	741.000	87.58
EX AREA A2	EX 25	25	591,665.000	741.000	115.96
EX AREA A2	EX 100	100	921,177.000	741.000	163.26
EX AREA E	EX 2	2	189,674.000	732.000	41.25
EX AREA E	EX 10	10	520,812.000	735.000	135.22
EX AREA E	EX 25	25	732,543.000	732.000	178.71
EX AREA E	EX 100	100	1,140,280.000	732.000	251.49
EX AREA D	EX 2	2	186,197.000	741.000	33.87
EX AREA D	EX 10	10	510,356.000	741.000	107.11
EX AREA D	EX 25	25	717,945.000	741.000	141.73
EX AREA D	EX 100	100	1,117,770.000	738.000	199.85

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)
POA D	EX 2	2	2,418,675.000	753.000	336.72
POA D	EX 10	10	6,595,384.000	750.000	966.89
POA D	EX 25	25	9,272,589.000	750.000	1,280.80
POA D	EX 100	100	14,433,741.000	750.000	1,811.95
POA A	EX 2	2	954,451.000	753.000	137.84
POA A	EX 10	10	2,602,299.000	750.000	413.45
POA A	EX 25	25	3,658,953.000	750.000	550.18
POA A	EX 100	100	5,693,810.000	747.000	780.49
POA B	EX 2	2	1,576,271.000	753.000	228.61
POA B	EX 10	10	4,301,264.000	750.000	691.80
POA B	EX 25	25	6,049,697.000	750.000	921.77
POA B	EX 100	100	9,417,348.000	750.000	1,310.21
POA C	EX 2	2	186,197.000	741.000	33.87
POA C	EX 10	10	510,356.000	741.000	107.11
POA C	EX 25	25	717,945.000	741.000	141.73

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)
POA C	EX 100	100	1,117,770.000	738.000	199.85

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Time-Depth Curve: 10 Yr Storm

Label	10 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	10 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0008	0.0017	0.0025	0.0034
5.000	0.0042	0.0051	0.0059	0.0068	0.0076
10.000	0.0085	0.0093	0.0102	0.0110	0.0119
15.000	0.0127	0.0136	0.0145	0.0153	0.0162
20.000	0.0170	0.0179	0.0188	0.0196	0.0205
25.000	0.0214	0.0222	0.0231	0.0240	0.0248
30.000	0.0257	0.0266	0.0275	0.0283	0.0292
35.000	0.0301	0.0310	0.0318	0.0327	0.0336
40.000	0.0345	0.0354	0.0362	0.0371	0.0380
45.000	0.0389	0.0398	0.0407	0.0416	0.0425
50.000	0.0434	0.0443	0.0452	0.0461	0.0470
55.000	0.0479	0.0488	0.0497	0.0506	0.0515
60.000	0.0524	0.0533	0.0542	0.0551	0.0560
65.000	0.0569	0.0578	0.0587	0.0596	0.0605
70.000	0.0615	0.0624	0.0633	0.0642	0.0651
75.000	0.0661	0.0670	0.0679	0.0688	0.0697
80.000	0.0707	0.0716	0.0725	0.0735	0.0744
85.000	0.0753	0.0763	0.0772	0.0781	0.0791
90.000	0.0800	0.0810	0.0819	0.0828	0.0838
95.000	0.0847	0.0857	0.0866	0.0876	0.0885
100.000	0.0895	0.0904	0.0914	0.0923	0.0933
105.000	0.0942	0.0952	0.0962	0.0971	0.0981
110.000	0.0990	0.1000	0.1010	0.1019	0.1029
115.000	0.1039	0.1049	0.1058	0.1068	0.1078
120.000	0.1088	0.1097	0.1107	0.1117	0.1127
125.000	0.1137	0.1146	0.1156	0.1166	0.1176
130.000	0.1186	0.1196	0.1206	0.1216	0.1226
135.000	0.1236	0.1246	0.1256	0.1266	0.1276
140.000	0.1286	0.1296	0.1306	0.1316	0.1326
145.000	0.1336	0.1346	0.1356	0.1367	0.1377
150.000	0.1387	0.1397	0.1407	0.1418	0.1428
155.000	0.1438	0.1448	0.1459	0.1469	0.1479
160.000	0.1490	0.1500	0.1510	0.1521	0.1531
165.000	0.1542	0.1552	0.1562	0.1573	0.1583
170.000	0.1594	0.1604	0.1615	0.1625	0.1636

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.1647	0.1657	0.1668	0.1678	0.1689
180.000	0.1700	0.1710	0.1721	0.1732	0.1742
185.000	0.1753	0.1764	0.1775	0.1785	0.1796
190.000	0.1807	0.1818	0.1829	0.1840	0.1850
195.000	0.1861	0.1872	0.1883	0.1894	0.1905
200.000	0.1916	0.1927	0.1938	0.1949	0.1960
205.000	0.1971	0.1982	0.1993	0.2005	0.2016
210.000	0.2027	0.2038	0.2049	0.2060	0.2072
215.000	0.2083	0.2094	0.2105	0.2117	0.2128
220.000	0.2139	0.2151	0.2162	0.2174	0.2185
225.000	0.2196	0.2208	0.2219	0.2231	0.2242
230.000	0.2254	0.2265	0.2277	0.2289	0.2300
235.000	0.2312	0.2323	0.2335	0.2347	0.2358
240.000	0.2370	0.2382	0.2394	0.2405	0.2417
245.000	0.2429	0.2441	0.2453	0.2465	0.2477
250.000	0.2488	0.2500	0.2512	0.2524	0.2536
255.000	0.2548	0.2560	0.2573	0.2585	0.2597
260.000	0.2609	0.2621	0.2633	0.2645	0.2658
265.000	0.2670	0.2682	0.2694	0.2707	0.2719
270.000	0.2731	0.2744	0.2756	0.2769	0.2781
275.000	0.2794	0.2806	0.2819	0.2831	0.2844
280.000	0.2856	0.2869	0.2881	0.2894	0.2907
285.000	0.2920	0.2932	0.2945	0.2958	0.2971
290.000	0.2983	0.2996	0.3009	0.3022	0.3035
295.000	0.3048	0.3061	0.3074	0.3087	0.3100
300.000	0.3113	0.3126	0.3139	0.3152	0.3165
305.000	0.3179	0.3192	0.3205	0.3218	0.3232
310.000	0.3245	0.3258	0.3272	0.3285	0.3299
315.000	0.3312	0.3326	0.3339	0.3353	0.3366
320.000	0.3380	0.3393	0.3407	0.3421	0.3435
325.000	0.3448	0.3462	0.3476	0.3490	0.3504
330.000	0.3517	0.3531	0.3545	0.3559	0.3573
335.000	0.3587	0.3601	0.3615	0.3630	0.3644
340.000	0.3658	0.3672	0.3686	0.3701	0.3715
345.000	0.3729	0.3744	0.3758	0.3772	0.3787
350.000	0.3801	0.3816	0.3831	0.3845	0.3860
355.000	0.3874	0.3889	0.3904	0.3919	0.3933
360.000	0.3948	0.3965	0.3982	0.3999	0.4016
365.000	0.4033	0.4050	0.4067	0.4084	0.4101
370.000	0.4118	0.4135	0.4152	0.4169	0.4187
375.000	0.4204	0.4221	0.4239	0.4256	0.4274
380.000	0.4291	0.4309	0.4326	0.4344	0.4362
385.000	0.4379	0.4397	0.4415	0.4433	0.4451

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.4469	0.4487	0.4505	0.4523	0.4541
395.000	0.4559	0.4577	0.4595	0.4614	0.4632
400.000	0.4650	0.4669	0.4687	0.4706	0.4724
405.000	0.4743	0.4762	0.4780	0.4799	0.4818
410.000	0.4837	0.4856	0.4875	0.4894	0.4913
415.000	0.4932	0.4951	0.4970	0.4989	0.5009
420.000	0.5028	0.5047	0.5067	0.5086	0.5106
425.000	0.5125	0.5145	0.5165	0.5185	0.5204
430.000	0.5224	0.5244	0.5264	0.5284	0.5304
435.000	0.5324	0.5345	0.5365	0.5385	0.5406
440.000	0.5426	0.5446	0.5467	0.5488	0.5508
445.000	0.5529	0.5550	0.5571	0.5591	0.5612
450.000	0.5633	0.5655	0.5676	0.5697	0.5718
455.000	0.5740	0.5761	0.5782	0.5804	0.5826
460.000	0.5847	0.5869	0.5891	0.5913	0.5935
465.000	0.5957	0.5979	0.6001	0.6023	0.6045
470.000	0.6068	0.6090	0.6113	0.6135	0.6158
475.000	0.6180	0.6203	0.6226	0.6249	0.6272
480.000	0.6295	0.6318	0.6342	0.6365	0.6388
485.000	0.6412	0.6435	0.6459	0.6483	0.6506
490.000	0.6530	0.6554	0.6578	0.6602	0.6627
495.000	0.6651	0.6675	0.6700	0.6724	0.6749
500.000	0.6774	0.6799	0.6823	0.6848	0.6874
505.000	0.6899	0.6924	0.6949	0.6975	0.7001
510.000	0.7026	0.7052	0.7078	0.7104	0.7130
515.000	0.7156	0.7182	0.7209	0.7235	0.7262
520.000	0.7289	0.7315	0.7342	0.7369	0.7396
525.000	0.7424	0.7451	0.7478	0.7506	0.7534
530.000	0.7561	0.7589	0.7617	0.7645	0.7674
535.000	0.7702	0.7730	0.7759	0.7788	0.7817
540.000	0.7846	0.7883	0.7920	0.7958	0.7996
545.000	0.8034	0.8072	0.8110	0.8149	0.8187
550.000	0.8226	0.8265	0.8304	0.8343	0.8383
555.000	0.8422	0.8462	0.8502	0.8542	0.8582
560.000	0.8623	0.8664	0.8705	0.8746	0.8787
565.000	0.8828	0.8870	0.8912	0.8954	0.8996
570.000	0.9039	0.9082	0.9125	0.9168	0.9211
575.000	0.9254	0.9298	0.9342	0.9386	0.9431
580.000	0.9476	0.9520	0.9566	0.9611	0.9657
585.000	0.9702	0.9749	0.9795	0.9842	0.9888
590.000	0.9936	0.9983	1.0031	1.0079	1.0127
595.000	1.0176	1.0224	1.0273	1.0323	1.0373
600.000	1.0423	1.0473	1.0523	1.0574	1.0626

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	1.0677	1.0729	1.0781	1.0834	1.0887
610.000	1.0940	1.0994	1.1048	1.1102	1.1157
615.000	1.1212	1.1268	1.1324	1.1380	1.1437
620.000	1.1494	1.1551	1.1609	1.1668	1.1727
625.000	1.1786	1.1846	1.1906	1.1967	1.2028
630.000	1.2089	1.2164	1.2239	1.2315	1.2391
635.000	1.2468	1.2546	1.2624	1.2703	1.2783
640.000	1.2863	1.2944	1.3025	1.3108	1.3191
645.000	1.3274	1.3359	1.3444	1.3530	1.3617
650.000	1.3705	1.3794	1.3883	1.3974	1.4065
655.000	1.4157	1.4250	1.4345	1.4440	1.4536
660.000	1.4634	1.4744	1.4855	1.4968	1.5081
665.000	1.5197	1.5313	1.5431	1.5550	1.5671
670.000	1.5794	1.5918	1.6043	1.6170	1.6299
675.000	1.6430	1.6563	1.6698	1.6834	1.6973
680.000	1.7114	1.7257	1.7403	1.7551	1.7702
685.000	1.7855	1.8012	1.8171	1.8333	1.8499
690.000	1.8668	1.8856	1.9049	1.9245	1.9446
695.000	1.9652	1.9863	2.0079	2.0301	2.0529
700.000	2.0764	2.1006	2.1256	2.1514	2.1782
705.000	2.2061	2.2438	2.2828	2.3234	2.3656
710.000	2.4098	2.4561	2.5050	2.5569	2.6193
715.000	2.6862	2.7718	2.8643	2.9666	3.1226
720.000	3.2786	3.4346	3.5906	3.7466	3.8435
725.000	3.9323	4.0018	4.0664	4.1269	4.1772
730.000	4.2248	4.2700	4.3131	4.3545	4.3943
735.000	4.4327	4.4610	4.4883	4.5146	4.5400
740.000	4.5646	4.5885	4.6116	4.6341	4.6560
745.000	4.6774	4.6982	4.7185	4.7384	4.7578
750.000	4.7768	4.7939	4.8107	4.8271	4.8431
755.000	4.8589	4.8744	4.8896	4.9045	4.9192
760.000	4.9337	4.9479	4.9619	4.9756	4.9892
765.000	5.0026	5.0158	5.0288	5.0416	5.0542
770.000	5.0667	5.0790	5.0912	5.1032	5.1150
775.000	5.1268	5.1383	5.1498	5.1611	5.1723
780.000	5.1834	5.1932	5.2029	5.2124	5.2219
785.000	5.2313	5.2406	5.2497	5.2588	5.2678
790.000	5.2767	5.2856	5.2943	5.3029	5.3115
795.000	5.3200	5.3284	5.3368	5.3450	5.3532
800.000	5.3613	5.3694	5.3774	5.3853	5.3931
805.000	5.4009	5.4087	5.4163	5.4239	5.4315
810.000	5.4390	5.4452	5.4513	5.4574	5.4635
815.000	5.4695	5.4754	5.4813	5.4872	5.4930

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	5.4988	5.5045	5.5102	5.5159	5.5215
825.000	5.5271	5.5326	5.5381	5.5435	5.5490
830.000	5.5543	5.5597	5.5650	5.5703	5.5755
835.000	5.5807	5.5859	5.5910	5.5962	5.6012
840.000	5.6063	5.6113	5.6163	5.6213	5.6262
845.000	5.6311	5.6360	5.6408	5.6456	5.6504
850.000	5.6551	5.6599	5.6646	5.6693	5.6739
855.000	5.6785	5.6831	5.6877	5.6923	5.6968
860.000	5.7013	5.7058	5.7102	5.7147	5.7191
865.000	5.7235	5.7278	5.7322	5.7365	5.7408
870.000	5.7451	5.7493	5.7536	5.7578	5.7620
875.000	5.7662	5.7703	5.7745	5.7786	5.7827
880.000	5.7868	5.7908	5.7949	5.7989	5.8029
885.000	5.8069	5.8109	5.8148	5.8187	5.8227
890.000	5.8266	5.8304	5.8343	5.8382	5.8420
895.000	5.8458	5.8496	5.8534	5.8572	5.8609
900.000	5.8647	5.8676	5.8705	5.8733	5.8762
905.000	5.8791	5.8819	5.8847	5.8875	5.8904
910.000	5.8931	5.8959	5.8987	5.9015	5.9042
915.000	5.9069	5.9097	5.9124	5.9151	5.9178
920.000	5.9205	5.9231	5.9258	5.9284	5.9311
925.000	5.9337	5.9363	5.9389	5.9415	5.9441
930.000	5.9467	5.9493	5.9518	5.9544	5.9569
935.000	5.9595	5.9620	5.9645	5.9670	5.9695
940.000	5.9720	5.9745	5.9769	5.9794	5.9818
945.000	5.9843	5.9867	5.9891	5.9916	5.9940
950.000	5.9964	5.9988	6.0011	6.0035	6.0059
955.000	6.0083	6.0106	6.0129	6.0153	6.0176
960.000	6.0199	6.0222	6.0245	6.0268	6.0291
965.000	6.0314	6.0337	6.0360	6.0382	6.0405
970.000	6.0427	6.0450	6.0472	6.0494	6.0516
975.000	6.0538	6.0560	6.0582	6.0604	6.0626
980.000	6.0648	6.0670	6.0691	6.0713	6.0734
985.000	6.0756	6.0777	6.0798	6.0820	6.0841
990.000	6.0862	6.0883	6.0904	6.0925	6.0946
995.000	6.0967	6.0987	6.1008	6.1029	6.1049
1,000.000	6.1070	6.1090	6.1111	6.1131	6.1151
1,005.000	6.1171	6.1192	6.1212	6.1232	6.1252
1,010.000	6.1272	6.1292	6.1312	6.1331	6.1351
1,015.000	6.1371	6.1390	6.1410	6.1429	6.1449
1,020.000	6.1468	6.1488	6.1507	6.1526	6.1546
1,025.000	6.1565	6.1584	6.1603	6.1622	6.1641
1,030.000	6.1660	6.1679	6.1698	6.1716	6.1735

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	6.1754	6.1772	6.1791	6.1809	6.1828
1,040.000	6.1846	6.1865	6.1883	6.1902	6.1920
1,045.000	6.1938	6.1956	6.1974	6.1992	6.2010
1,050.000	6.2028	6.2046	6.2064	6.2082	6.2100
1,055.000	6.2118	6.2136	6.2153	6.2171	6.2189
1,060.000	6.2206	6.2224	6.2241	6.2259	6.2276
1,065.000	6.2293	6.2311	6.2328	6.2345	6.2363
1,070.000	6.2380	6.2397	6.2414	6.2431	6.2448
1,075.000	6.2465	6.2482	6.2499	6.2516	6.2533
1,080.000	6.2549	6.2564	6.2579	6.2594	6.2608
1,085.000	6.2623	6.2638	6.2652	6.2667	6.2682
1,090.000	6.2696	6.2711	6.2725	6.2740	6.2754
1,095.000	6.2768	6.2783	6.2797	6.2811	6.2826
1,100.000	6.2840	6.2854	6.2868	6.2882	6.2896
1,105.000	6.2911	6.2925	6.2939	6.2953	6.2967
1,110.000	6.2980	6.2994	6.3008	6.3022	6.3036
1,115.000	6.3050	6.3063	6.3077	6.3091	6.3105
1,120.000	6.3118	6.3132	6.3145	6.3159	6.3172
1,125.000	6.3186	6.3199	6.3213	6.3226	6.3240
1,130.000	6.3253	6.3266	6.3280	6.3293	6.3306
1,135.000	6.3320	6.3333	6.3346	6.3359	6.3372
1,140.000	6.3385	6.3398	6.3412	6.3425	6.3438
1,145.000	6.3451	6.3464	6.3476	6.3489	6.3502
1,150.000	6.3515	6.3528	6.3541	6.3554	6.3566
1,155.000	6.3579	6.3592	6.3604	6.3617	6.3630
1,160.000	6.3642	6.3655	6.3668	6.3680	6.3693
1,165.000	6.3705	6.3718	6.3730	6.3743	6.3755
1,170.000	6.3767	6.3780	6.3792	6.3804	6.3817
1,175.000	6.3829	6.3841	6.3853	6.3866	6.3878
1,180.000	6.3890	6.3902	6.3914	6.3926	6.3938
1,185.000	6.3951	6.3963	6.3975	6.3987	6.3999
1,190.000	6.4011	6.4022	6.4034	6.4046	6.4058
1,195.000	6.4070	6.4082	6.4094	6.4105	6.4117
1,200.000	6.4129	6.4141	6.4152	6.4164	6.4176
1,205.000	6.4188	6.4199	6.4211	6.4222	6.4234
1,210.000	6.4246	6.4257	6.4269	6.4280	6.4292
1,215.000	6.4303	6.4314	6.4326	6.4337	6.4349
1,220.000	6.4360	6.4371	6.4383	6.4394	6.4405
1,225.000	6.4417	6.4428	6.4439	6.4450	6.4461
1,230.000	6.4473	6.4484	6.4495	6.4506	6.4517
1,235.000	6.4528	6.4539	6.4550	6.4561	6.4572
1,240.000	6.4583	6.4594	6.4605	6.4616	6.4627
1,245.000	6.4638	6.4649	6.4660	6.4671	6.4682

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	6.4693	6.4703	6.4714	6.4725	6.4736
1,255.000	6.4747	6.4757	6.4768	6.4779	6.4789
1,260.000	6.4800	6.4811	6.4821	6.4832	6.4843
1,265.000	6.4853	6.4864	6.4874	6.4885	6.4895
1,270.000	6.4906	6.4916	6.4927	6.4937	6.4948
1,275.000	6.4958	6.4969	6.4979	6.4990	6.5000
1,280.000	6.5010	6.5021	6.5031	6.5041	6.5052
1,285.000	6.5062	6.5072	6.5082	6.5093	6.5103
1,290.000	6.5113	6.5123	6.5133	6.5144	6.5154
1,295.000	6.5164	6.5174	6.5184	6.5194	6.5204
1,300.000	6.5214	6.5225	6.5235	6.5245	6.5255
1,305.000	6.5265	6.5275	6.5285	6.5295	6.5304
1,310.000	6.5314	6.5324	6.5334	6.5344	6.5354
1,315.000	6.5364	6.5374	6.5383	6.5393	6.5403
1,320.000	6.5413	6.5423	6.5432	6.5442	6.5452
1,325.000	6.5462	6.5471	6.5481	6.5491	6.5500
1,330.000	6.5510	6.5520	6.5529	6.5539	6.5549
1,335.000	6.5558	6.5568	6.5577	6.5587	6.5596
1,340.000	6.5606	6.5615	6.5625	6.5634	6.5644
1,345.000	6.5653	6.5663	6.5672	6.5682	6.5691
1,350.000	6.5700	6.5710	6.5719	6.5729	6.5738
1,355.000	6.5747	6.5757	6.5766	6.5775	6.5785
1,360.000	6.5794	6.5803	6.5813	6.5822	6.5831
1,365.000	6.5840	6.5849	6.5859	6.5868	6.5877
1,370.000	6.5886	6.5895	6.5904	6.5914	6.5923
1,375.000	6.5932	6.5941	6.5950	6.5959	6.5968
1,380.000	6.5977	6.5986	6.5995	6.6004	6.6013
1,385.000	6.6022	6.6031	6.6040	6.6049	6.6058
1,390.000	6.6067	6.6076	6.6085	6.6094	6.6103
1,395.000	6.6112	6.6121	6.6129	6.6138	6.6147
1,400.000	6.6156	6.6165	6.6174	6.6182	6.6191
1,405.000	6.6200	6.6209	6.6218	6.6226	6.6235
1,410.000	6.6244	6.6253	6.6261	6.6270	6.6279
1,415.000	6.6287	6.6296	6.6305	6.6313	6.6322
1,420.000	6.6331	6.6339	6.6348	6.6356	6.6365
1,425.000	6.6374	6.6382	6.6391	6.6399	6.6408
1,430.000	6.6416	6.6425	6.6433	6.6442	6.6450
1,435.000	6.6459	6.6467	6.6476	6.6484	6.6493
1,440.000	6.6501	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Time-Depth Curve: 100 Yr Storm	
Label	100 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	100 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0014	0.0028	0.0042	0.0057
5.000	0.0071	0.0085	0.0099	0.0113	0.0128
10.000	0.0142	0.0156	0.0170	0.0185	0.0199
15.000	0.0213	0.0228	0.0242	0.0257	0.0271
20.000	0.0285	0.0300	0.0314	0.0329	0.0343
25.000	0.0358	0.0372	0.0387	0.0401	0.0416
30.000	0.0431	0.0445	0.0460	0.0475	0.0489
35.000	0.0504	0.0519	0.0533	0.0548	0.0563
40.000	0.0578	0.0592	0.0607	0.0622	0.0637
45.000	0.0652	0.0667	0.0682	0.0697	0.0711
50.000	0.0726	0.0741	0.0756	0.0771	0.0786
55.000	0.0802	0.0817	0.0832	0.0847	0.0862
60.000	0.0877	0.0892	0.0908	0.0923	0.0938
65.000	0.0953	0.0969	0.0984	0.0999	0.1014
70.000	0.1030	0.1045	0.1061	0.1076	0.1091
75.000	0.1107	0.1122	0.1138	0.1153	0.1169
80.000	0.1184	0.1200	0.1215	0.1231	0.1247
85.000	0.1262	0.1278	0.1294	0.1309	0.1325
90.000	0.1341	0.1357	0.1372	0.1388	0.1404
95.000	0.1420	0.1436	0.1452	0.1468	0.1484
100.000	0.1500	0.1516	0.1532	0.1548	0.1564
105.000	0.1580	0.1596	0.1612	0.1628	0.1644
110.000	0.1660	0.1677	0.1693	0.1709	0.1725
115.000	0.1742	0.1758	0.1774	0.1791	0.1807
120.000	0.1823	0.1840	0.1856	0.1873	0.1889
125.000	0.1906	0.1922	0.1939	0.1956	0.1972
130.000	0.1989	0.2005	0.2022	0.2039	0.2056
135.000	0.2072	0.2089	0.2106	0.2123	0.2140
140.000	0.2157	0.2173	0.2190	0.2207	0.2224
145.000	0.2241	0.2258	0.2275	0.2292	0.2309
150.000	0.2327	0.2344	0.2361	0.2378	0.2395
155.000	0.2412	0.2430	0.2447	0.2464	0.2482
160.000	0.2499	0.2517	0.2534	0.2551	0.2569
165.000	0.2586	0.2604	0.2622	0.2639	0.2657
170.000	0.2674	0.2692	0.2710	0.2727	0.2745

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.2763	0.2781	0.2799	0.2816	0.2834
180.000	0.2852	0.2870	0.2888	0.2906	0.2924
185.000	0.2942	0.2960	0.2978	0.2996	0.3015
190.000	0.3033	0.3051	0.3069	0.3088	0.3106
195.000	0.3124	0.3143	0.3161	0.3179	0.3198
200.000	0.3216	0.3235	0.3253	0.3272	0.3291
205.000	0.3309	0.3328	0.3347	0.3365	0.3384
210.000	0.3403	0.3422	0.3440	0.3459	0.3478
215.000	0.3497	0.3516	0.3535	0.3554	0.3573
220.000	0.3592	0.3611	0.3631	0.3650	0.3669
225.000	0.3688	0.3708	0.3727	0.3746	0.3766
230.000	0.3785	0.3805	0.3824	0.3844	0.3863
235.000	0.3883	0.3902	0.3922	0.3942	0.3962
240.000	0.3981	0.4001	0.4021	0.4041	0.4061
245.000	0.4081	0.4101	0.4121	0.4141	0.4161
250.000	0.4181	0.4201	0.4221	0.4241	0.4262
255.000	0.4282	0.4302	0.4323	0.4343	0.4363
260.000	0.4384	0.4404	0.4425	0.4445	0.4466
265.000	0.4487	0.4507	0.4528	0.4549	0.4570
270.000	0.4591	0.4611	0.4632	0.4653	0.4674
275.000	0.4695	0.4716	0.4738	0.4759	0.4780
280.000	0.4801	0.4822	0.4844	0.4865	0.4887
285.000	0.4908	0.4929	0.4951	0.4973	0.4994
290.000	0.5016	0.5038	0.5059	0.5081	0.5103
295.000	0.5125	0.5147	0.5169	0.5191	0.5213
300.000	0.5235	0.5257	0.5279	0.5301	0.5323
305.000	0.5346	0.5368	0.5390	0.5413	0.5435
310.000	0.5458	0.5480	0.5503	0.5526	0.5548
315.000	0.5571	0.5594	0.5617	0.5640	0.5663
320.000	0.5686	0.5709	0.5732	0.5755	0.5778
325.000	0.5801	0.5825	0.5848	0.5871	0.5895
330.000	0.5918	0.5942	0.5965	0.5989	0.6013
335.000	0.6036	0.6060	0.6084	0.6108	0.6132
340.000	0.6156	0.6180	0.6204	0.6228	0.6252
345.000	0.6277	0.6301	0.6325	0.6350	0.6374
350.000	0.6399	0.6423	0.6448	0.6473	0.6497
355.000	0.6522	0.6547	0.6572	0.6597	0.6622
360.000	0.6647	0.6679	0.6711	0.6744	0.6776
365.000	0.6808	0.6841	0.6873	0.6906	0.6939
370.000	0.6971	0.7004	0.7037	0.7070	0.7103
375.000	0.7136	0.7170	0.7203	0.7236	0.7270
380.000	0.7303	0.7337	0.7370	0.7404	0.7438
385.000	0.7472	0.7506	0.7540	0.7574	0.7608

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.7643	0.7677	0.7712	0.7746	0.7781
395.000	0.7816	0.7850	0.7885	0.7920	0.7955
400.000	0.7991	0.8026	0.8061	0.8097	0.8132
405.000	0.8168	0.8203	0.8239	0.8275	0.8311
410.000	0.8347	0.8383	0.8419	0.8456	0.8492
415.000	0.8529	0.8565	0.8602	0.8639	0.8676
420.000	0.8713	0.8750	0.8787	0.8824	0.8862
425.000	0.8899	0.8937	0.8974	0.9012	0.9050
430.000	0.9088	0.9126	0.9164	0.9203	0.9241
435.000	0.9280	0.9318	0.9357	0.9396	0.9435
440.000	0.9474	0.9513	0.9552	0.9592	0.9631
445.000	0.9671	0.9710	0.9750	0.9790	0.9830
450.000	0.9870	0.9911	0.9951	0.9992	1.0032
455.000	1.0073	1.0114	1.0155	1.0196	1.0237
460.000	1.0279	1.0320	1.0362	1.0404	1.0446
465.000	1.0488	1.0530	1.0572	1.0615	1.0657
470.000	1.0700	1.0743	1.0786	1.0829	1.0872
475.000	1.0915	1.0959	1.1003	1.1046	1.1090
480.000	1.1134	1.1179	1.1223	1.1267	1.1312
485.000	1.1357	1.1402	1.1447	1.1492	1.1538
490.000	1.1583	1.1629	1.1675	1.1721	1.1767
495.000	1.1814	1.1860	1.1907	1.1954	1.2001
500.000	1.2048	1.2095	1.2143	1.2191	1.2238
505.000	1.2287	1.2335	1.2383	1.2432	1.2481
510.000	1.2530	1.2579	1.2628	1.2677	1.2727
515.000	1.2777	1.2827	1.2877	1.2928	1.2979
520.000	1.3030	1.3081	1.3132	1.3183	1.3235
525.000	1.3287	1.3339	1.3391	1.3444	1.3497
530.000	1.3550	1.3603	1.3656	1.3710	1.3764
535.000	1.3818	1.3872	1.3927	1.3981	1.4036
540.000	1.4092	1.4174	1.4257	1.4340	1.4424
545.000	1.4508	1.4592	1.4677	1.4762	1.4847
550.000	1.4932	1.5018	1.5105	1.5191	1.5278
555.000	1.5365	1.5453	1.5541	1.5630	1.5718
560.000	1.5808	1.5897	1.5987	1.6078	1.6168
565.000	1.6259	1.6351	1.6443	1.6535	1.6628
570.000	1.6721	1.6815	1.6909	1.7003	1.7098
575.000	1.7194	1.7289	1.7386	1.7482	1.7580
580.000	1.7677	1.7776	1.7874	1.7973	1.8073
585.000	1.8173	1.8274	1.8375	1.8476	1.8579
590.000	1.8681	1.8785	1.8888	1.8993	1.9098
595.000	1.9203	1.9309	1.9416	1.9523	1.9631
600.000	1.9740	1.9849	1.9958	2.0000	2.0180

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	2.0292	2.0404	2.0517	2.0631	2.0745
610.000	2.0860	2.0976	2.1092	2.1210	2.1328
615.000	2.1446	2.1566	2.1686	2.1807	2.1929
620.000	2.2052	2.2176	2.2300	2.2426	2.2552
625.000	2.2679	2.2807	2.2936	2.3066	2.3197
630.000	2.3329	2.3502	2.3676	2.3851	2.4027
635.000	2.4205	2.4383	2.4563	2.4745	2.4927
640.000	2.5111	2.5296	2.5483	2.5671	2.5861
645.000	2.6052	2.6244	2.6438	2.6634	2.6831
650.000	2.7030	2.7230	2.7433	2.7637	2.7842
655.000	2.8050	2.8259	2.8471	2.8684	2.8900
660.000	2.9117	2.9355	2.9596	2.9838	3.0083
665.000	3.0331	3.0580	3.0833	3.1088	3.1346
670.000	3.1607	3.1870	3.2137	3.2406	3.2679
675.000	3.2955	3.3235	3.3518	3.3804	3.4095
680.000	3.4389	3.4688	3.4991	3.5298	3.5610
685.000	3.5926	3.6248	3.6575	3.6907	3.7245
690.000	3.7589	3.7897	3.8212	3.8534	3.8862
695.000	3.9199	3.9544	3.9897	4.0260	4.0633
700.000	4.1017	4.1412	4.1820	4.2242	4.2680
705.000	4.3133	4.3752	4.4392	4.5056	4.5746
710.000	4.6468	4.7224	4.8021	4.8867	4.9854
715.000	5.0913	5.2264	5.3727	5.5350	5.7870
720.000	6.0390	6.2910	6.5430	6.7950	6.9485
725.000	7.0888	7.1990	7.3010	7.3967	7.4787
730.000	7.5563	7.6302	7.7007	7.7684	7.8336
735.000	7.8964	7.9427	7.9873	8.0302	8.0717
740.000	8.1119	8.1508	8.1886	8.2254	8.2612
745.000	8.2961	8.3302	8.3635	8.3960	8.4278
750.000	8.4589	8.4936	8.5278	8.5613	8.5942
755.000	8.6267	8.6586	8.6900	8.7209	8.7514
760.000	8.7815	8.8111	8.8403	8.8692	8.8977
765.000	8.9258	8.9536	8.9810	9.0082	9.0350
770.000	9.0615	9.0877	9.1136	9.1393	9.1646
775.000	9.1898	9.2146	9.2392	9.2636	9.2878
780.000	9.3117	9.3336	9.3552	9.3766	9.3979
785.000	9.4189	9.4398	9.4604	9.4809	9.5012
790.000	9.5214	9.5414	9.5612	9.5808	9.6003
795.000	9.6196	9.6388	9.6578	9.6767	9.6954
800.000	9.7140	9.7325	9.7508	9.7690	9.7871
805.000	9.8050	9.8228	9.8405	9.8581	9.8755
810.000	9.8929	9.9061	9.9193	9.9323	9.9452
815.000	9.9581	9.9709	9.9835	9.9961	10.0086

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	10.0210	10.0333	10.0456	10.0577	10.0698
825.000	10.0818	10.0937	10.1055	10.1173	10.1290
830.000	10.1406	10.1522	10.1636	10.1750	10.1864
835.000	10.1976	10.2088	10.2200	10.2310	10.2420
840.000	10.2530	10.2638	10.2747	10.2854	10.2961
845.000	10.3068	10.3173	10.3279	10.3383	10.3487
850.000	10.3591	10.3694	10.3796	10.3898	10.4000
855.000	10.4101	10.4201	10.4301	10.4400	10.4499
860.000	10.4598	10.4696	10.4793	10.4890	10.4986
865.000	10.5083	10.5178	10.5273	10.5368	10.5462
870.000	10.5556	10.5650	10.5743	10.5835	10.5927
875.000	10.6019	10.6110	10.6201	10.6292	10.6382
880.000	10.6472	10.6561	10.6650	10.6739	10.6827
885.000	10.6915	10.7002	10.7090	10.7176	10.7263
890.000	10.7349	10.7435	10.7520	10.7605	10.7690
895.000	10.7774	10.7859	10.7942	10.8026	10.8109
900.000	10.8192	10.8247	10.8302	10.8357	10.8412
905.000	10.8466	10.8520	10.8574	10.8628	10.8681
910.000	10.8735	10.8788	10.8841	10.8893	10.8946
915.000	10.8998	10.9050	10.9102	10.9153	10.9205
920.000	10.9256	10.9307	10.9358	10.9408	10.9459
925.000	10.9509	10.9559	10.9609	10.9658	10.9708
930.000	10.9757	10.9806	10.9855	10.9903	10.9952
935.000	11.0000	11.0048	11.0096	11.0144	11.0192
940.000	11.0239	11.0287	11.0334	11.0381	11.0427
945.000	11.0474	11.0520	11.0567	11.0613	11.0659
950.000	11.0705	11.0750	11.0796	11.0841	11.0886
955.000	11.0931	11.0976	11.1021	11.1066	11.1110
960.000	11.1154	11.1198	11.1242	11.1286	11.1330
965.000	11.1374	11.1417	11.1460	11.1504	11.1547
970.000	11.1589	11.1632	11.1675	11.1717	11.1760
975.000	11.1802	11.1844	11.1886	11.1928	11.1970
980.000	11.2011	11.2053	11.2094	11.2135	11.2176
985.000	11.2217	11.2258	11.2299	11.2339	11.2380
990.000	11.2420	11.2460	11.2501	11.2541	11.2580
995.000	11.2620	11.2660	11.2700	11.2739	11.2778
1,000.000	11.2817	11.2857	11.2896	11.2934	11.2973
1,005.000	11.3012	11.3051	11.3089	11.3127	11.3166
1,010.000	11.3204	11.3242	11.3280	11.3317	11.3355
1,015.000	11.3393	11.3430	11.3468	11.3505	11.3542
1,020.000	11.3579	11.3616	11.3653	11.3690	11.3727
1,025.000	11.3763	11.3800	11.3837	11.3873	11.3909
1,030.000	11.3945	11.3982	11.4018	11.4053	11.4089

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	11.4125	11.4161	11.4196	11.4232	11.4267
1,040.000	11.4302	11.4338	11.4373	11.4408	11.4443
1,045.000	11.4477	11.4512	11.4547	11.4582	11.4616
1,050.000	11.4650	11.4685	11.4719	11.4753	11.4787
1,055.000	11.4821	11.4855	11.4889	11.4923	11.4957
1,060.000	11.4990	11.5024	11.5058	11.5091	11.5124
1,065.000	11.5158	11.5191	11.5224	11.5257	11.5290
1,070.000	11.5323	11.5356	11.5388	11.5427	11.5454
1,075.000	11.5486	11.5518	11.5551	11.5583	11.5615
1,080.000	11.5648	11.5673	11.5698	11.5723	11.5748
1,085.000	11.5773	11.5797	11.5822	11.5847	11.5872
1,090.000	11.5896	11.5921	11.5945	11.5970	11.5994
1,095.000	11.6019	11.6043	11.6067	11.6091	11.6115
1,100.000	11.6139	11.6163	11.6187	11.6211	11.6235
1,105.000	11.6259	11.6283	11.6307	11.6330	11.6354
1,110.000	11.6377	11.6401	11.6424	11.6448	11.6471
1,115.000	11.6495	11.6518	11.6541	11.6564	11.6587
1,120.000	11.6610	11.6633	11.6656	11.6679	11.6702
1,125.000	11.6725	11.6748	11.6771	11.6793	11.6816
1,130.000	11.6838	11.6861	11.6883	11.6906	11.6928
1,135.000	11.6951	11.6973	11.6995	11.7018	11.7040
1,140.000	11.7062	11.7084	11.7106	11.7128	11.7150
1,145.000	11.7172	11.7194	11.7216	11.7237	11.7259
1,150.000	11.7281	11.7303	11.7324	11.7346	11.7367
1,155.000	11.7389	11.7410	11.7432	11.7453	11.7474
1,160.000	11.7496	11.7517	11.7538	11.7559	11.7581
1,165.000	11.7602	11.7623	11.7644	11.7665	11.7686
1,170.000	11.7707	11.7727	11.7748	11.7769	11.7790
1,175.000	11.7811	11.7831	11.7852	11.7872	11.7893
1,180.000	11.7914	11.7934	11.7955	11.7975	11.7995
1,185.000	11.8016	11.8036	11.8056	11.8076	11.8097
1,190.000	11.8117	11.8137	11.8157	11.8177	11.8197
1,195.000	11.8217	11.8237	11.8257	11.8277	11.8297
1,200.000	11.8316	11.8336	11.8356	11.8376	11.8395
1,205.000	11.8415	11.8435	11.8454	11.8474	11.8493
1,210.000	11.8513	11.8532	11.8551	11.8571	11.8590
1,215.000	11.8609	11.8629	11.8648	11.8667	11.8686
1,220.000	11.8705	11.8725	11.8744	11.8763	11.8782
1,225.000	11.8801	11.8820	11.8839	11.8857	11.8876
1,230.000	11.8895	11.8914	11.8933	11.8951	11.8970
1,235.000	11.8989	11.9007	11.9026	11.9045	11.9063
1,240.000	11.9082	11.9100	11.9119	11.9137	11.9156
1,245.000	11.9174	11.9192	11.9211	11.9229	11.9247

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	11.9265	11.9284	11.9302	11.9320	11.9338
1,255.000	11.9356	11.9374	11.9392	11.9410	11.9428
1,260.000	11.9446	11.9464	11.9482	11.9500	11.9518
1,265.000	11.9536	11.9553	11.9571	11.9589	11.9607
1,270.000	11.9624	11.9642	11.9660	11.9677	11.9695
1,275.000	11.9712	11.9730	11.9747	11.9765	11.9782
1,280.000	11.9800	11.9817	11.9834	11.9852	11.9869
1,285.000	11.9886	11.9904	11.9921	11.9938	11.9955
1,290.000	11.9972	11.9989	12.0007	12.0024	12.0041
1,295.000	12.0058	12.0075	12.0092	12.0109	12.0126
1,300.000	12.0143	12.0159	12.0176	12.0193	12.0210
1,305.000	12.0227	12.0244	12.0260	12.0277	12.0294
1,310.000	12.0310	12.0327	12.0344	12.0360	12.0377
1,315.000	12.0393	12.0410	12.0426	12.0443	12.0459
1,320.000	12.0476	12.0492	12.0509	12.0525	12.0541
1,325.000	12.0558	12.0574	12.0590	12.0606	12.0623
1,330.000	12.0639	12.0655	12.0671	12.0687	12.0703
1,335.000	12.0720	12.0736	12.0752	12.0768	12.0784
1,340.000	12.0800	12.0816	12.0832	12.0848	12.0864
1,345.000	12.0879	12.0895	12.0911	12.0927	12.0943
1,350.000	12.0959	12.0974	12.0990	12.1006	12.1021
1,355.000	12.1037	12.1053	12.1068	12.1084	12.1100
1,360.000	12.1115	12.1131	12.1146	12.1162	12.1177
1,365.000	12.1193	12.1208	12.1224	12.1239	12.1255
1,370.000	12.1270	12.1285	12.1301	12.1316	12.1331
1,375.000	12.1347	12.1362	12.1377	12.1392	12.1407
1,380.000	12.1423	12.1438	12.1453	12.1468	12.1483
1,385.000	12.1498	12.1513	12.1528	12.1543	12.1558
1,390.000	12.1573	12.1588	12.1603	12.1618	12.1633
1,395.000	12.1648	12.1663	12.1678	12.1693	12.1707
1,400.000	12.1722	12.1737	12.1752	12.1767	12.1781
1,405.000	12.1796	12.1811	12.1825	12.1840	12.1855
1,410.000	12.1869	12.1884	12.1899	12.1913	12.1928
1,415.000	12.1942	12.1957	12.1971	12.1986	12.2000
1,420.000	12.2015	12.2029	12.2044	12.2058	12.2072
1,425.000	12.2087	12.2101	12.2115	12.2130	12.2144
1,430.000	12.2158	12.2173	12.2187	12.2201	12.2215
1,435.000	12.2230	12.2244	12.2258	12.2272	12.2286
1,440.000	12.2300	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time-Depth Curve: 2 Yr Storm

Label	2 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	2 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0000	0.0000	0.0000	0.0000
5.000	0.0000	0.0000	0.0000	0.0000	0.0000
10.000	0.0000	0.0000	0.0000	0.0000	0.0000
15.000	0.0000	0.0000	0.0000	0.0000	0.0000
20.000	0.0000	0.0000	0.0000	0.0000	0.0000
25.000	0.0000	0.0000	0.0000	0.0000	0.0000
30.000	0.0000	0.0000	0.0000	0.0000	0.0000
35.000	0.0000	0.0000	0.0000	0.0000	0.0000
40.000	0.0000	0.0000	0.0000	0.0000	0.0000
45.000	0.0000	0.0000	0.0000	0.0000	0.0000
50.000	0.0000	0.0000	0.0000	0.0000	0.0000
55.000	0.0000	0.0000	0.0000	0.0000	0.0000
60.000	0.0000	0.0000	0.0000	0.0000	0.0000
65.000	0.0000	0.0000	0.0000	0.0000	0.0000
70.000	0.0000	0.0000	0.0000	0.0000	0.0000
75.000	0.0000	0.0000	0.0000	0.0000	0.0000
80.000	0.0000	0.0000	0.0000	0.0000	0.0000
85.000	0.0000	0.0200	0.0400	0.0600	0.0800
90.000	0.1000	0.1000	0.1000	0.1000	0.1000
95.000	0.1000	0.1000	0.1000	0.1000	0.1000
100.000	0.1000	0.1000	0.1000	0.1000	0.1000
105.000	0.1000	0.1000	0.1000	0.1000	0.1000
110.000	0.1000	0.1000	0.1000	0.1000	0.1000
115.000	0.1000	0.1000	0.1000	0.1000	0.1000
120.000	0.1000	0.1000	0.1000	0.1000	0.1000
125.000	0.1000	0.1000	0.1000	0.1000	0.1000
130.000	0.1000	0.1000	0.1000	0.1000	0.1000
135.000	0.1000	0.1000	0.1000	0.1000	0.1000
140.000	0.1000	0.1000	0.1000	0.1000	0.1000
145.000	0.1000	0.1000	0.1000	0.1000	0.1000
150.000	0.1000	0.1000	0.1000	0.1000	0.1000
155.000	0.1000	0.1000	0.1000	0.1000	0.1000
160.000	0.1000	0.1000	0.1000	0.1000	0.1000
165.000	0.1000	0.1000	0.1000	0.1000	0.1000
170.000	0.1000	0.1000	0.1000	0.1000	0.1000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.1000	0.1000	0.1000	0.1000	0.1000
180.000	0.1000	0.1000	0.1000	0.1000	0.1000
185.000	0.1000	0.1000	0.1000	0.1000	0.1000
190.000	0.1000	0.1000	0.1000	0.1000	0.1000
195.000	0.1000	0.1000	0.1000	0.1000	0.1000
200.000	0.1000	0.1000	0.1000	0.1000	0.1000
205.000	0.1000	0.1000	0.1000	0.1000	0.1000
210.000	0.1000	0.1000	0.1000	0.1000	0.1000
215.000	0.1000	0.1000	0.1000	0.1000	0.1000
220.000	0.1000	0.1000	0.1000	0.1000	0.1000
225.000	0.1000	0.1000	0.1000	0.1000	0.1000
230.000	0.1000	0.1000	0.1000	0.1000	0.1000
235.000	0.1000	0.1000	0.1000	0.1000	0.1000
240.000	0.1000	0.1200	0.1400	0.1600	0.1800
245.000	0.2000	0.2000	0.2000	0.2000	0.2000
250.000	0.2000	0.2000	0.2000	0.2000	0.2000
255.000	0.2000	0.2000	0.2000	0.2000	0.2000
260.000	0.2000	0.2000	0.2000	0.2000	0.2000
265.000	0.2000	0.2000	0.2000	0.2000	0.2000
270.000	0.2000	0.2000	0.2000	0.2000	0.2000
275.000	0.2000	0.2000	0.2000	0.2000	0.2000
280.000	0.2000	0.2000	0.2000	0.2000	0.2000
285.000	0.2000	0.2000	0.2000	0.2000	0.2000
290.000	0.2000	0.2000	0.2000	0.2000	0.2000
295.000	0.2000	0.2000	0.2000	0.2000	0.2000
300.000	0.2000	0.2000	0.2000	0.2000	0.2000
305.000	0.2000	0.2000	0.2000	0.2000	0.2000
310.000	0.2000	0.2000	0.2000	0.2000	0.2000
315.000	0.2000	0.2000	0.2000	0.2000	0.2000
320.000	0.2000	0.2000	0.2000	0.2000	0.2000
325.000	0.2000	0.2000	0.2000	0.2000	0.2000
330.000	0.2000	0.2000	0.2000	0.2000	0.2000
335.000	0.2000	0.2000	0.2000	0.2000	0.2000
340.000	0.2000	0.2000	0.2000	0.2000	0.2000
345.000	0.2000	0.2000	0.2000	0.2000	0.2000
350.000	0.2000	0.2000	0.2000	0.2000	0.2000
355.000	0.2000	0.2000	0.2000	0.2000	0.2000
360.000	0.2000	0.2200	0.2400	0.2600	0.2800
365.000	0.3000	0.3000	0.3000	0.3000	0.3000
370.000	0.3000	0.3000	0.3000	0.3000	0.3000
375.000	0.3000	0.3000	0.3000	0.3000	0.3000
380.000	0.3000	0.3000	0.3000	0.3000	0.3000
385.000	0.3000	0.3000	0.3000	0.3000	0.3000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.3000	0.3000	0.3000	0.3000	0.3000
395.000	0.3000	0.3000	0.3000	0.3000	0.3000
400.000	0.3000	0.3000	0.3000	0.3000	0.3000
405.000	0.3000	0.3000	0.3000	0.3000	0.3000
410.000	0.3000	0.3000	0.3000	0.3000	0.3000
415.000	0.3000	0.3000	0.3000	0.3000	0.3000
420.000	0.3000	0.3000	0.3000	0.3000	0.3000
425.000	0.3000	0.3000	0.3000	0.3000	0.3000
430.000	0.3000	0.3000	0.3000	0.3000	0.3000
435.000	0.3000	0.3000	0.3000	0.3000	0.3000
440.000	0.3000	0.3000	0.3000	0.3000	0.3000
445.000	0.3000	0.3000	0.3000	0.3000	0.3000
450.000	0.3000	0.3200	0.3400	0.3600	0.3800
455.000	0.4000	0.4000	0.4000	0.4000	0.4000
460.000	0.4000	0.4000	0.4000	0.4000	0.4000
465.000	0.4000	0.4000	0.4000	0.4000	0.4000
470.000	0.4000	0.4000	0.4000	0.4000	0.4000
475.000	0.4000	0.4000	0.4000	0.4000	0.4000
480.000	0.4000	0.4000	0.4000	0.4000	0.4000
485.000	0.4000	0.4000	0.4000	0.4000	0.4000
490.000	0.4000	0.4000	0.4000	0.4000	0.4000
495.000	0.4000	0.4000	0.4000	0.4000	0.4000
500.000	0.4000	0.4000	0.4000	0.4000	0.4000
505.000	0.4000	0.4000	0.4000	0.4000	0.4000
510.000	0.4000	0.4200	0.4400	0.4600	0.4800
515.000	0.5000	0.5000	0.5000	0.5000	0.5000
520.000	0.5000	0.5000	0.5000	0.5000	0.5000
525.000	0.5000	0.5000	0.5000	0.5000	0.5000
530.000	0.5000	0.5000	0.5000	0.5000	0.5000
535.000	0.5000	0.5000	0.5000	0.5000	0.5000
540.000	0.5000	0.5000	0.5000	0.5000	0.5000
545.000	0.5000	0.5000	0.5000	0.5000	0.5000
550.000	0.5000	0.5000	0.5000	0.5000	0.5000
555.000	0.5000	0.5000	0.5000	0.5000	0.5000
560.000	0.5000	0.5200	0.5400	0.5600	0.5800
565.000	0.6000	0.6000	0.6000	0.6000	0.6000
570.000	0.6000	0.6000	0.6000	0.6000	0.6000
575.000	0.6000	0.6000	0.6000	0.6000	0.6000
580.000	0.6000	0.6000	0.6000	0.6000	0.6000
585.000	0.6000	0.6000	0.6000	0.6000	0.6000
590.000	0.6000	0.6000	0.6000	0.6000	0.6000
595.000	0.6000	0.6200	0.6400	0.6600	0.6800
600.000	0.7000	0.7000	0.7000	0.7000	0.7000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	0.7000	0.7000	0.7000	0.7000	0.7000
610.000	0.7000	0.7000	0.7000	0.7000	0.7000
615.000	0.7000	0.7000	0.7000	0.7000	0.7000
620.000	0.7000	0.7000	0.7000	0.7000	0.7000
625.000	0.7000	0.7000	0.7000	0.7000	0.7000
630.000	0.7000	0.7200	0.7400	0.7600	0.7800
635.000	0.8000	0.8000	0.8000	0.8000	0.8000
640.000	0.8000	0.8000	0.8000	0.8000	0.8000
645.000	0.8000	0.8000	0.8000	0.8000	0.8000
650.000	0.8000	0.8000	0.8000	0.8000	0.8000
655.000	0.8000	0.8200	0.8400	0.8600	0.8800
660.000	0.9000	0.9000	0.9000	0.9000	0.9000
665.000	0.9000	0.9000	0.9000	0.9000	0.9000
670.000	0.9000	0.9000	0.9000	0.9000	0.9000
675.000	0.9000	0.9200	0.9400	0.9600	0.9800
680.000	1.0000	1.0000	1.0000	1.0000	1.0000
685.000	1.0000	1.0000	1.0000	1.0000	1.0000
690.000	1.0000	1.0200	1.0400	1.0600	1.0800
695.000	1.1000	1.1000	1.1000	1.1000	1.1000
700.000	1.1000	1.1200	1.1400	1.1600	1.1800
705.000	1.2000	1.2200	1.2400	1.2600	1.2800
710.000	1.3000	1.3400	1.3800	1.4200	1.4600
715.000	1.5000	1.5400	1.5800	1.6200	1.6600
720.000	1.7000	1.7600	1.8200	1.8800	1.9400
725.000	2.0000	2.0200	2.0400	2.0600	2.0800
730.000	2.1000	2.1200	2.1400	2.1600	2.1800
735.000	2.2000	2.2200	2.2400	2.2600	2.2800
740.000	2.3000	2.3200	2.3400	2.3600	2.3800
745.000	2.4000	2.4000	2.4000	2.4000	2.4000
750.000	2.4000	2.4000	2.4000	2.4000	2.4000
755.000	2.4000	2.4200	2.4400	2.4600	2.4800
760.000	2.5000	2.5000	2.5000	2.5000	2.5000
765.000	2.5000	2.5000	2.5000	2.5000	2.5000
770.000	2.5000	2.5200	2.5400	2.5600	2.5800
775.000	2.6000	2.6000	2.6000	2.6000	2.6000
780.000	2.6000	2.6000	2.6000	2.6000	2.6000
785.000	2.6000	2.6000	2.6000	2.6000	2.6000
790.000	2.6000	2.6000	2.6000	2.6000	2.6000
795.000	2.6000	2.6200	2.6400	2.6600	2.6800
800.000	2.7000	2.7000	2.7000	2.7000	2.7000
805.000	2.7000	2.7000	2.7000	2.7000	2.7000
810.000	2.7000	2.7000	2.7000	2.7000	2.7000
815.000	2.7000	2.7000	2.7000	2.7000	2.7000

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	2.7000	2.7000	2.7000	2.7000	2.7000
825.000	2.7000	2.7200	2.7400	2.7600	2.7800
830.000	2.8000	2.8000	2.8000	2.8000	2.8000
835.000	2.8000	2.8000	2.8000	2.8000	2.8000
840.000	2.8000	2.8000	2.8000	2.8000	2.8000
845.000	2.8000	2.8000	2.8000	2.8000	2.8000
850.000	2.8000	2.8000	2.8000	2.8000	2.8000
855.000	2.8000	2.8000	2.8000	2.8000	2.8000
860.000	2.8000	2.8200	2.8400	2.8600	2.8800
865.000	2.9000	2.9000	2.9000	2.9000	2.9000
870.000	2.9000	2.9000	2.9000	2.9000	2.9000
875.000	2.9000	2.9000	2.9000	2.9000	2.9000
880.000	2.9000	2.9000	2.9000	2.9000	2.9000
885.000	2.9000	2.9000	2.9000	2.9000	2.9000
890.000	2.9000	2.9000	2.9000	2.9000	2.9000
895.000	2.9000	2.9000	2.9000	2.9000	2.9000
900.000	2.9000	2.9000	2.9000	2.9000	2.9000
905.000	2.9000	2.9200	2.9400	2.9600	2.9800
910.000	3.0000	3.0000	3.0000	3.0000	3.0000
915.000	3.0000	3.0000	3.0000	3.0000	3.0000
920.000	3.0000	3.0000	3.0000	3.0000	3.0000
925.000	3.0000	3.0000	3.0000	3.0000	3.0000
930.000	3.0000	3.0000	3.0000	3.0000	3.0000
935.000	3.0000	3.0000	3.0000	3.0000	3.0000
940.000	3.0000	3.0000	3.0000	3.0000	3.0000
945.000	3.0000	3.0000	3.0000	3.0000	3.0000
950.000	3.0000	3.0000	3.0000	3.0000	3.0000
955.000	3.0000	3.0000	3.0000	3.0000	3.0000
960.000	3.0000	3.0200	3.0400	3.0600	3.0800
965.000	3.1000	3.1000	3.1000	3.1000	3.1000
970.000	3.1000	3.1000	3.1000	3.1000	3.1000
975.000	3.1000	3.1000	3.1000	3.1000	3.1000
980.000	3.1000	3.1000	3.1000	3.1000	3.1000
985.000	3.1000	3.1000	3.1000	3.1000	3.1000
990.000	3.1000	3.1000	3.1000	3.1000	3.1000
995.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,000.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,005.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,010.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,015.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,020.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,025.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,030.000	3.1000	3.1000	3.1000	3.1000	3.1000

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	3.1000	3.1200	3.1400	3.1600	3.1800
1,040.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,045.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,050.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,055.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,060.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,065.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,070.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,075.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,080.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,085.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,090.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,095.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,100.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,105.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,110.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,115.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,120.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,125.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,130.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,135.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,140.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,145.000	3.2000	3.2200	3.2400	3.2600	3.2800
1,150.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,155.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,160.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,165.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,170.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,175.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,180.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,185.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,190.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,195.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,200.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,205.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,210.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,215.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,220.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,225.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,230.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,235.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,240.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,245.000	3.3000	3.3000	3.3000	3.3000	3.3000

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,255.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,260.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,265.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,270.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,275.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,280.000	3.3000	3.3200	3.3400	3.3600	3.3800
1,285.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,290.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,295.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,300.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,305.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,310.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,315.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,320.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,325.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,330.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,335.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,340.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,345.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,350.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,355.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,360.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,365.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,370.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,375.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,380.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,385.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,390.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,395.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,400.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,405.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,410.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,415.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,420.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,425.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,430.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,435.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,440.000	3.4000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time-Depth Curve: 25 Yr Storm

Label	25 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0011	0.0021	0.0032	0.0042
5.000	0.0053	0.0064	0.0074	0.0085	0.0095
10.000	0.0106	0.0117	0.0127	0.0138	0.0149
15.000	0.0160	0.0170	0.0181	0.0192	0.0203
20.000	0.0213	0.0224	0.0235	0.0246	0.0257
25.000	0.0267	0.0278	0.0289	0.0300	0.0311
30.000	0.0322	0.0333	0.0344	0.0355	0.0366
35.000	0.0377	0.0388	0.0399	0.0410	0.0421
40.000	0.0432	0.0443	0.0454	0.0465	0.0476
45.000	0.0487	0.0498	0.0509	0.0520	0.0531
50.000	0.0543	0.0554	0.0565	0.0576	0.0587
55.000	0.0599	0.0610	0.0621	0.0633	0.0644
60.000	0.0655	0.0666	0.0678	0.0689	0.0701
65.000	0.0712	0.0723	0.0735	0.0746	0.0758
70.000	0.0769	0.0781	0.0792	0.0804	0.0815
75.000	0.0827	0.0838	0.0850	0.0861	0.0873
80.000	0.0884	0.0896	0.0908	0.0919	0.0931
85.000	0.0943	0.0954	0.0966	0.0978	0.0989
90.000	0.1001	0.1013	0.1025	0.1037	0.1048
95.000	0.1060	0.1072	0.1084	0.1096	0.1108
100.000	0.1120	0.1132	0.1143	0.1155	0.1167
105.000	0.1179	0.1191	0.1203	0.1215	0.1228
110.000	0.1240	0.1252	0.1264	0.1276	0.1288
115.000	0.1300	0.1312	0.1325	0.1337	0.1349
120.000	0.1361	0.1373	0.1386	0.1398	0.1410
125.000	0.1423	0.1435	0.1447	0.1460	0.1472
130.000	0.1484	0.1497	0.1509	0.1522	0.1534
135.000	0.1547	0.1559	0.1572	0.1584	0.1597
140.000	0.1609	0.1622	0.1635	0.1647	0.1660
145.000	0.1673	0.1685	0.1698	0.1711	0.1723
150.000	0.1736	0.1749	0.1762	0.1775	0.1787
155.000	0.1800	0.1813	0.1826	0.1839	0.1852
160.000	0.1865	0.1878	0.1891	0.1904	0.1917
165.000	0.1930	0.1943	0.1956	0.1969	0.1982
170.000	0.1995	0.2008	0.2022	0.2035	0.2048

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.2061	0.2075	0.2088	0.2101	0.2114
180.000	0.2128	0.2141	0.2155	0.2168	0.2181
185.000	0.2195	0.2208	0.2222	0.2235	0.2249
190.000	0.2262	0.2276	0.2289	0.2303	0.2317
195.000	0.2330	0.2344	0.2358	0.2371	0.2385
200.000	0.2399	0.2413	0.2427	0.2440	0.2454
205.000	0.2468	0.2482	0.2496	0.2510	0.2524
210.000	0.2538	0.2552	0.2566	0.2580	0.2594
215.000	0.2608	0.2622	0.2636	0.2650	0.2665
220.000	0.2679	0.2693	0.2707	0.2722	0.2736
225.000	0.2750	0.2765	0.2779	0.2793	0.2808
230.000	0.2822	0.2837	0.2851	0.2866	0.2880
235.000	0.2895	0.2909	0.2924	0.2939	0.2953
240.000	0.2968	0.2983	0.2997	0.3012	0.3027
245.000	0.3042	0.3057	0.3071	0.3086	0.3101
250.000	0.3116	0.3131	0.3146	0.3161	0.3176
255.000	0.3191	0.3207	0.3222	0.3237	0.3252
260.000	0.3267	0.3282	0.3298	0.3313	0.3328
265.000	0.3344	0.3359	0.3375	0.3390	0.3405
270.000	0.3421	0.3436	0.3452	0.3468	0.3483
275.000	0.3499	0.3515	0.3530	0.3546	0.3562
280.000	0.3578	0.3593	0.3609	0.3625	0.3641
285.000	0.3657	0.3673	0.3689	0.3705	0.3721
290.000	0.3737	0.3753	0.3769	0.3785	0.3802
295.000	0.3818	0.3834	0.3850	0.3867	0.3883
300.000	0.3900	0.3916	0.3932	0.3949	0.3965
305.000	0.3982	0.3999	0.4015	0.4032	0.4049
310.000	0.4065	0.4082	0.4099	0.4116	0.4133
315.000	0.4150	0.4166	0.4183	0.4200	0.4217
320.000	0.4235	0.4252	0.4269	0.4286	0.4303
325.000	0.4320	0.4338	0.4355	0.4372	0.4390
330.000	0.4407	0.4425	0.4442	0.4460	0.4477
335.000	0.4495	0.4513	0.4530	0.4548	0.4566
340.000	0.4584	0.4601	0.4619	0.4637	0.4655
345.000	0.4673	0.4691	0.4709	0.4727	0.4746
350.000	0.4764	0.4782	0.4800	0.4819	0.4837
355.000	0.4855	0.4874	0.4892	0.4911	0.4929
360.000	0.4948	0.4970	0.4992	0.5015	0.5037
365.000	0.5059	0.5082	0.5104	0.5127	0.5149
370.000	0.5172	0.5195	0.5217	0.5240	0.5263
375.000	0.5286	0.5309	0.5332	0.5355	0.5378
380.000	0.5401	0.5424	0.5448	0.5471	0.5494
385.000	0.5518	0.5541	0.5565	0.5588	0.5612

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.5636	0.5660	0.5683	0.5707	0.5731
395.000	0.5755	0.5779	0.5803	0.5828	0.5852
400.000	0.5876	0.5900	0.5925	0.5949	0.5974
405.000	0.5998	0.6023	0.6048	0.6073	0.6097
410.000	0.6122	0.6147	0.6172	0.6197	0.6223
415.000	0.6248	0.6273	0.6299	0.6324	0.6349
420.000	0.6375	0.6401	0.6426	0.6452	0.6478
425.000	0.6504	0.6530	0.6556	0.6582	0.6608
430.000	0.6634	0.6661	0.6687	0.6714	0.6740
435.000	0.6767	0.6793	0.6820	0.6847	0.6874
440.000	0.6901	0.6928	0.6955	0.6982	0.7010
445.000	0.7037	0.7065	0.7092	0.7120	0.7147
450.000	0.7175	0.7203	0.7231	0.7259	0.7287
455.000	0.7315	0.7344	0.7372	0.7400	0.7429
460.000	0.7457	0.7486	0.7515	0.7544	0.7573
465.000	0.7602	0.7631	0.7660	0.7690	0.7719
470.000	0.7748	0.7778	0.7808	0.7838	0.7867
475.000	0.7897	0.7928	0.7958	0.7988	0.8018
480.000	0.8049	0.8080	0.8110	0.8141	0.8172
485.000	0.8203	0.8234	0.8265	0.8297	0.8328
490.000	0.8359	0.8391	0.8423	0.8455	0.8487
495.000	0.8519	0.8551	0.8583	0.8616	0.8648
500.000	0.8681	0.8713	0.8746	0.8779	0.8812
505.000	0.8846	0.8879	0.8913	0.8946	0.8980
510.000	0.9014	0.9048	0.9082	0.9116	0.9150
515.000	0.9185	0.9220	0.9254	0.9289	0.9324
520.000	0.9359	0.9395	0.9430	0.9466	0.9502
525.000	0.9537	0.9573	0.9610	0.9646	0.9682
530.000	0.9719	0.9756	0.9793	0.9830	0.9867
535.000	0.9904	0.9942	0.9980	1.0018	1.0056
540.000	1.0094	1.0146	1.0198	1.0250	1.0302
545.000	1.0354	1.0407	1.0460	1.0513	1.0567
550.000	1.0620	1.0674	1.0728	1.0783	1.0837
555.000	1.0892	1.0947	1.1002	1.1058	1.1113
560.000	1.1169	1.1226	1.1282	1.1339	1.1396
565.000	1.1453	1.1511	1.1569	1.1627	1.1685
570.000	1.1744	1.1803	1.1862	1.1921	1.1981
575.000	1.2041	1.2102	1.2162	1.2223	1.2285
580.000	1.2346	1.2408	1.2470	1.2533	1.2596
585.000	1.2659	1.2722	1.2786	1.2851	1.2915
590.000	1.2980	1.3045	1.3111	1.3177	1.3243
595.000	1.3310	1.3377	1.3445	1.3513	1.3581
600.000	1.3650	1.3719	1.3789	1.3859	1.3929

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	1.4000	1.4071	1.4143	1.4215	1.4288
610.000	1.4361	1.4435	1.4509	1.4584	1.4659
615.000	1.4734	1.4810	1.4887	1.4964	1.5042
620.000	1.5120	1.5199	1.5278	1.5358	1.5439
625.000	1.5520	1.5602	1.5684	1.5768	1.5851
630.000	1.5936	1.6042	1.6149	1.6256	1.6365
635.000	1.6474	1.6584	1.6695	1.6807	1.6919
640.000	1.7033	1.7147	1.7263	1.7379	1.7497
645.000	1.7615	1.7734	1.7855	1.7976	1.8099
650.000	1.8223	1.8347	1.8473	1.8601	1.8729
655.000	1.8859	1.8990	1.9122	1.9256	1.9391
660.000	1.9527	1.9679	1.9831	1.9986	2.0142
665.000	2.0299	2.0459	2.0620	2.0783	2.0948
670.000	2.1115	2.1284	2.1456	2.1629	2.1805
675.000	2.1983	2.2163	2.2346	2.2532	2.2720
680.000	2.2911	2.3105	2.3302	2.3503	2.3706
685.000	2.3913	2.4124	2.4339	2.4557	2.4780
690.000	2.5007	2.5241	2.5479	2.5723	2.5972
695.000	2.6227	2.6488	2.6756	2.7031	2.7314
700.000	2.7605	2.7905	2.8214	2.8534	2.8866
705.000	2.9210	2.9678	3.0163	3.0666	3.1189
710.000	3.1736	3.2309	3.2914	3.3556	3.4314
715.000	3.5127	3.6166	3.7290	3.8535	4.0455
720.000	4.2375	4.4295	4.6215	4.8135	4.9313
725.000	5.0391	5.1237	5.2021	5.2756	5.3379
730.000	5.3967	5.4527	5.5062	5.5574	5.6068
735.000	5.6544	5.6895	5.7233	5.7559	5.7873
740.000	5.8178	5.8473	5.8760	5.9039	5.9310
745.000	5.9574	5.9832	6.0084	6.0331	6.0572
750.000	6.0808	6.1037	6.1262	6.1483	6.1699
755.000	6.1912	6.2121	6.2326	6.2528	6.2727
760.000	6.2922	6.3115	6.3305	6.3491	6.3676
765.000	6.3857	6.4037	6.4214	6.4388	6.4560
770.000	6.4731	6.4899	6.5065	6.5229	6.5391
775.000	6.5551	6.5710	6.5867	6.6022	6.6176
780.000	6.6327	6.6465	6.6601	6.6735	6.6868
785.000	6.7000	6.7130	6.7259	6.7387	6.7514
790.000	6.7639	6.7763	6.7886	6.8009	6.8130
795.000	6.8249	6.8368	6.8486	6.8603	6.8719
800.000	6.8834	6.8948	6.9061	6.9173	6.9285
805.000	6.9395	6.9505	6.9614	6.9722	6.9829
810.000	6.9935	7.0020	7.0104	7.0188	7.0270
815.000	7.0352	7.0434	7.0515	7.0595	7.0675

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	7.0754	7.0832	7.0910	7.0988	7.1065
825.000	7.1141	7.1217	7.1293	7.1367	7.1442
830.000	7.1516	7.1589	7.1662	7.1734	7.1806
835.000	7.1878	7.1949	7.2020	7.2090	7.2160
840.000	7.2229	7.2298	7.2367	7.2435	7.2502
845.000	7.2570	7.2637	7.2703	7.2770	7.2835
850.000	7.2901	7.2966	7.3031	7.3095	7.3159
855.000	7.3223	7.3286	7.3349	7.3412	7.3474
860.000	7.3537	7.3598	7.3660	7.3721	7.3782
865.000	7.3842	7.3902	7.3962	7.4022	7.4081
870.000	7.4140	7.4199	7.4258	7.4316	7.4374
875.000	7.4431	7.4489	7.4546	7.4603	7.4659
880.000	7.4716	7.4772	7.4828	7.4883	7.4939
885.000	7.4994	7.5049	7.5103	7.5158	7.5212
890.000	7.5266	7.5320	7.5373	7.5427	7.5480
895.000	7.5533	7.5585	7.5638	7.5690	7.5742
900.000	7.5794	7.5832	7.5870	7.5908	7.5946
905.000	7.5983	7.6021	7.6058	7.6095	7.6132
910.000	7.6169	7.6206	7.6243	7.6279	7.6315
915.000	7.6351	7.6387	7.6423	7.6459	7.6494
920.000	7.6530	7.6565	7.6600	7.6635	7.6670
925.000	7.6705	7.6739	7.6774	7.6808	7.6842
930.000	7.6876	7.6910	7.6944	7.6977	7.7011
935.000	7.7044	7.7078	7.7111	7.7144	7.7177
940.000	7.7210	7.7242	7.7275	7.7307	7.7340
945.000	7.7372	7.7404	7.7436	7.7468	7.7500
950.000	7.7531	7.7563	7.7595	7.7626	7.7657
955.000	7.7688	7.7719	7.7750	7.7781	7.7812
960.000	7.7842	7.7873	7.7903	7.7933	7.7964
965.000	7.7994	7.8024	7.8054	7.8083	7.8113
970.000	7.8143	7.8172	7.8202	7.8231	7.8261
975.000	7.8290	7.8319	7.8348	7.8377	7.8406
980.000	7.8434	7.8463	7.8492	7.8520	7.8549
985.000	7.8577	7.8605	7.8633	7.8661	7.8689
990.000	7.8717	7.8745	7.8773	7.8800	7.8828
995.000	7.8855	7.8883	7.8910	7.8937	7.8965
1,000.000	7.8992	7.9019	7.9046	7.9073	7.9099
1,005.000	7.9126	7.9153	7.9179	7.9206	7.9232
1,010.000	7.9259	7.9285	7.9311	7.9337	7.9363
1,015.000	7.9389	7.9415	7.9441	7.9467	7.9493
1,020.000	7.9518	7.9544	7.9570	7.9595	7.9620
1,025.000	7.9646	7.9671	7.9696	7.9721	7.9746
1,030.000	7.9771	7.9796	7.9821	7.9846	7.9871

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	7.9896	7.9920	7.9945	7.9969	7.9994
1,040.000	8.0018	8.0042	8.0067	8.0091	8.0115
1,045.000	8.0139	8.0163	8.0187	8.0211	8.0235
1,050.000	8.0259	8.0282	8.0306	8.0330	8.0353
1,055.000	8.0377	8.0400	8.0424	8.0447	8.0470
1,060.000	8.0494	8.0517	8.0540	8.0563	8.0586
1,065.000	8.0609	8.0632	8.0655	8.0678	8.0700
1,070.000	8.0723	8.0746	8.0768	8.0791	8.0813
1,075.000	8.0836	8.0858	8.0881	8.0903	8.0925
1,080.000	8.0948	8.0966	8.0985	8.1003	8.1022
1,085.000	8.1040	8.1059	8.1077	8.1095	8.1114
1,090.000	8.1132	8.1150	8.1168	8.1187	8.1205
1,095.000	8.1223	8.1241	8.1259	8.1277	8.1295
1,100.000	8.1312	8.1330	8.1348	8.1366	8.1383
1,105.000	8.1401	8.1419	8.1436	8.1454	8.1471
1,110.000	8.1489	8.1506	8.1524	8.1541	8.1558
1,115.000	8.1576	8.1593	8.1610	8.1627	8.1645
1,120.000	8.1662	8.1679	8.1696	8.1713	8.1730
1,125.000	8.1747	8.1764	8.1781	8.1797	8.1814
1,130.000	8.1831	8.1848	8.1864	8.1881	8.1898
1,135.000	8.1914	8.1931	8.1947	8.1964	8.1980
1,140.000	8.1997	8.2013	8.2030	8.2046	8.2062
1,145.000	8.2079	8.2095	8.2111	8.2127	8.2144
1,150.000	8.2160	8.2176	8.2192	8.2208	8.2224
1,155.000	8.2240	8.2256	8.2272	8.2288	8.2303
1,160.000	8.2319	8.2335	8.2351	8.2367	8.2382
1,165.000	8.2398	8.2414	8.2429	8.2445	8.2460
1,170.000	8.2476	8.2491	8.2507	8.2522	8.2538
1,175.000	8.2553	8.2569	8.2584	8.2599	8.2615
1,180.000	8.2630	8.2645	8.2660	8.2675	8.2691
1,185.000	8.2706	8.2721	8.2736	8.2751	8.2766
1,190.000	8.2781	8.2796	8.2811	8.2826	8.2841
1,195.000	8.2856	8.2870	8.2885	8.2900	8.2915
1,200.000	8.2929	8.2944	8.2959	8.2974	8.2988
1,205.000	8.3003	8.3017	8.3032	8.3046	8.3061
1,210.000	8.3075	8.3090	8.3104	8.3119	8.3133
1,215.000	8.3148	8.3162	8.3176	8.3190	8.3205
1,220.000	8.3219	8.3233	8.3247	8.3262	8.3276
1,225.000	8.3290	8.3304	8.3318	8.3332	8.3346
1,230.000	8.3360	8.3374	8.3388	8.3402	8.3416
1,235.000	8.3430	8.3444	8.3458	8.3471	8.3485
1,240.000	8.3499	8.3513	8.3527	8.3540	8.3554
1,245.000	8.3568	8.3581	8.3595	8.3609	8.3622

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	8.3636	8.3649	8.3663	8.3676	8.3690
1,255.000	8.3703	8.3717	8.3730	8.3744	8.3757
1,260.000	8.3770	8.3784	8.3797	8.3810	8.3824
1,265.000	8.3837	8.3850	8.3863	8.3877	8.3890
1,270.000	8.3903	8.3916	8.3929	8.3942	8.3955
1,275.000	8.3968	8.3982	8.3995	8.4008	8.4021
1,280.000	8.4034	8.4047	8.4059	8.4072	8.4085
1,285.000	8.4098	8.4111	8.4124	8.4137	8.4149
1,290.000	8.4162	8.4175	8.4188	8.4201	8.4213
1,295.000	8.4226	8.4239	8.4251	8.4264	8.4277
1,300.000	8.4289	8.4302	8.4314	8.4327	8.4339
1,305.000	8.4352	8.4364	8.4377	8.4389	8.4402
1,310.000	8.4414	8.4427	8.4439	8.4451	8.4464
1,315.000	8.4476	8.4488	8.4501	8.4513	8.4525
1,320.000	8.4538	8.4550	8.4562	8.4574	8.4586
1,325.000	8.4599	8.4611	8.4623	8.4635	8.4647
1,330.000	8.4659	8.4671	8.4683	8.4695	8.4707
1,335.000	8.4719	8.4731	8.4743	8.4755	8.4767
1,340.000	8.4779	8.4791	8.4803	8.4815	8.4827
1,345.000	8.4839	8.4851	8.4862	8.4874	8.4886
1,350.000	8.4898	8.4910	8.4921	8.4933	8.4945
1,355.000	8.4956	8.4968	8.4980	8.4991	8.5003
1,360.000	8.5015	8.5026	8.5038	8.5049	8.5061
1,365.000	8.5073	8.5084	8.5096	8.5107	8.5119
1,370.000	8.5130	8.5142	8.5153	8.5164	8.5176
1,375.000	8.5187	8.5199	8.5210	8.5221	8.5233
1,380.000	8.5244	8.5255	8.5267	8.5278	8.5289
1,385.000	8.5300	8.5312	8.5323	8.5334	8.5345
1,390.000	8.5356	8.5368	8.5379	8.5390	8.5401
1,395.000	8.5412	8.5423	8.5434	8.5446	8.5457
1,400.000	8.5468	8.5479	8.5490	8.5501	8.5512
1,405.000	8.5523	8.5534	8.5545	8.5556	8.5567
1,410.000	8.5577	8.5588	8.5599	8.5610	8.5621
1,415.000	8.5632	8.5643	8.5654	8.5664	8.5675
1,420.000	8.5686	8.5697	8.5708	8.5718	8.5729
1,425.000	8.5740	8.5750	8.5761	8.5772	8.5783
1,430.000	8.5793	8.5804	8.5815	8.5825	8.5836
1,435.000	8.5846	8.5857	8.5868	8.5878	8.5889
1,440.000	8.5899	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	49.459 min

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min

Time of Concentration (Composite)

Time of Concentration (Composite)	49.459 min
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Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min

Time of Concentration (Composite)

Time of Concentration (Composite)	49.459 min
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Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	49.459 min

Subsection: Time of Concentration Calculations
Label: EX AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
 Label: EX AREA A2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min

Time of Concentration (Composite)

Time of Concentration (Composite)	30.302 min
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Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
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Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
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Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,065.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	7.304 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	53.064 min

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,065.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	7.304 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	53.064 min

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	2,065.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	7.304 min

Time of Concentration (Composite)

Time of Concentration (Composite)	53.064 min
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Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,065.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	7.304 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	53.064 min

Subsection: Time of Concentration Calculations
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min

Segment #3: TR-55 Channel Flow

Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min

Time of Concentration (Composite)

Time of Concentration (Composite)	29.791 min
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Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min

Segment #3: TR-55 Channel Flow

Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min

Time of Concentration (Composite)

Time of Concentration (Composite)	29.791 min
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Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.015 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.13 ft/s
Segment Time of Concentration	12.693 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	490.00 ft
Is Paved?	False
Slope	0.015 ft/ft
Average Velocity	1.98 ft/s
Segment Time of Concentration	4.133 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	750.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	2.653 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	19.479 min

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.015 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.13 ft/s
Segment Time of Concentration	12.693 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	490.00 ft
Is Paved?	False
Slope	0.015 ft/ft
Average Velocity	1.98 ft/s
Segment Time of Concentration	4.133 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	750.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	2.653 min

Time of Concentration (Composite)

Time of Concentration (Composite)	19.479 min
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Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.015 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.13 ft/s
Segment Time of Concentration	12.693 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	490.00 ft
Is Paved?	False
Slope	0.015 ft/ft
Average Velocity	1.98 ft/s
Segment Time of Concentration	4.133 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	750.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	2.653 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	19.479 min

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.015 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.13 ft/s
Segment Time of Concentration	12.693 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	490.00 ft
Is Paved?	False
Slope	0.015 ft/ft
Average Velocity	1.98 ft/s
Segment Time of Concentration	4.133 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	750.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	2.653 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	19.479 min

Subsection: Time of Concentration Calculations
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Runoff CN-Area
Label: EX AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	25.970	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	25.970	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	25.970	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	25.970	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	25.970	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	25.970	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	25.970	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	25.970	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	77.990	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	77.990	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	77.990	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	77.990	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	77.990	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	77.990	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	77.990	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	77.990	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	32.100	0.0	0.0	81.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	32.100	(N/A)	(N/A)	81.000

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	32.100	0.0	0.0	81.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	32.100	(N/A)	(N/A)	81.000

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	32.100	0.0	0.0	81.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	32.100	(N/A)	(N/A)	81.000

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	32.100	0.0	0.0	81.000
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	32.100	(N/A)	(N/A)	81.000

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method (Computational Notes)

Definition of Terms

At	Total area (acres): $A_t = A_i + A_p$
Ai	Impervious area (acres)
Ap	Pervious area (acres)
CNi	Runoff curve number for impervious area
CNp	Runoff curve number for pervious area
fLoss	f loss constant infiltration (depth/time)
gKs	Saturated Hydraulic Conductivity (depth/time)
Md	Volumetric Moisture Deficit
Psi	Capillary Suction (length)
hK	Horton Infiltration Decay Rate (time^{-1})
fo	Initial Infiltration Rate (depth/time)
fc	Ultimate(capacity)Infiltration Rate (depth/time)
Ia	Initial Abstraction (length)
dt	Computational increment (duration of unit excess rainfall) Default dt is smallest value of $0.1333T_c$, r_{tm} , and t_h (Smallest dt is then adjusted to match up with T_p)
UDdt	User specified override computational main time increment (only used if UDdt is $\Rightarrow .1333T_c$)
D(t)	Point on distribution curve (fraction of P) for time step t
K	$2 / (1 + (T_r/T_p))$: default $K = 0.75$: (for $T_r/T_p = 1.67$)
Ks	Hydrograph shape factor = Unit Conversions * K : $= ((1\text{hr}/3600\text{sec}) * (1\text{ft}/12\text{in}) * ((5280\text{ft})^2/\text{sq.mi})) * K$ Default $K_s = 645.333 * 0.75 = 484$
Lag	Lag time from center of excess runoff (dt) to T_p : $\text{Lag} = 0.6T_c$
P	Total precipitation depth, inches
Pa(t)	Accumulated rainfall at time step t
Pi(t)	Incremental rainfall at time step t
qp	Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi. $= (K_s * A * Q) / T_p$ (where $Q = 1\text{in. runoff}$, $A = \text{sq.mi.}$)
Qu(t)	Unit hydrograph ordinate (cfs) at time step t
Q(t)	Final hydrograph ordinate (cfs) at time step t
Rai(t)	Accumulated runoff (inches) at time step t for impervious area
Rap(t)	Accumulated runoff (inches) at time step t for pervious area
Rii(t)	Incremental runoff (inches) at time step t for impervious area
Rip(t)	Incremental runoff (inches) at time step t for pervious area
R(t)	Incremental weighted total runoff (inches)
Rtm	Time increment for rainfall table
Si	S for impervious area: $S_i = (1000/CN_i) - 10$
Sp	S for pervious area: $S_p = (1000/CN_p) - 10$
t	Time step (row) number
Tc	Time of concentration
Tb	Time (hrs) of entire unit hydrograph: $T_b = T_p + T_r$
Tp	Time (hrs) to peak of a unit hydrograph: $T_p = (dt/2) + \text{Lag}$
Tr	Time (hrs) of receding limb of unit hydrograph: $T_r = \text{ratio of } T_p$

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method

Computational Notes

Precipitation

Column (1)	Time for time step t
Column (2)	$D(t)$ = Point on distribution curve for time step t
Column (3)	$P_i(t) = P_a(t) - P_a(t-1)$: Col.(4) - Preceding Col.(4)
Column (4)	$P_a(t) = D(t) \times P$: Col.(2) x P

Pervious Area Runoff (using SCS Runoff CN Method)

Column (5)	$Rap(t)$ = Accumulated pervious runoff for time step t If $(P_a(t) \leq 0.2Sp)$ then use: $Rap(t) = 0.0$ If $(P_a(t) > 0.2Sp)$ then use: $Rap(t) = (Col.(4) - 0.2Sp) \times 2 / (Col.(4) + 0.8Sp)$
Column (6)	$Rip(t)$ = Incremental pervious runoff for time step t $Rip(t) = Rap(t) - Rap(t-1)$ $Rip(t) = Col.(5)$ for current row - $Col.(5)$ for preceding row.

Impervious Area Runoff

Column (7 & 8)...	Did not specify to use impervious areas.
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Incremental Weighted Runoff

Column (9)	$R(t) = (A_p/A_t) \times Rip(t) + (A_i/A_t) \times Rii(t)$ $R(t) = (A_p/A_t) \times Col.(6) + (A_i/A_t) \times Col.(8)$
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SCS Unit Hydrograph Method

Column (10)	$Q(t)$ is computed with the SCS unit hydrograph method using $R(t)$ and $Qu(t)$.
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Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	114.34 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	114.31 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6401 in
Runoff Volume (Pervious)	801,008.304 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	800,993.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
507.000	0.00	0.00	0.00	0.01	0.01
522.000	0.02	0.04	0.05	0.06	0.08
537.000	0.09	0.10	0.10	0.10	0.10
552.000	0.09	0.09	0.08	0.09	0.14
567.000	0.19	0.31	0.43	0.61	0.81
582.000	0.98	1.13	1.24	1.27	1.31
597.000	1.39	1.46	1.62	1.78	2.04
612.000	2.33	2.59	2.81	2.96	2.98
627.000	2.99	2.97	2.94	3.01	3.08
642.000	3.27	3.49	3.76	4.07	4.38
657.000	4.70	5.02	5.34	5.67	6.13
672.000	6.59	7.05	7.51	7.90	8.25
687.000	8.71	9.34	9.99	10.72	11.45
702.000	12.35	13.25	14.89	16.71	19.71
717.000	23.50	28.54	35.46	42.95	52.70
732.000	62.45	73.41	84.38	93.43	102.03
747.000	107.88	111.94	114.31	114.25	113.33
762.000	109.22	105.09	98.78	92.47	86.48
777.000	80.58	75.12	69.95	65.10	60.72
792.000	56.51	52.87	49.24	46.11	42.98
807.000	40.66	38.51	36.70	35.08	33.31
822.000	31.33	29.43	27.82	26.23	25.33
837.000	24.42	24.24	24.19	24.00	23.72
852.000	23.14	22.19	21.24	20.33	19.43
867.000	18.95	18.47	18.49	18.60	18.89
882.000	19.28	19.39	19.17	18.78	17.89
897.000	16.98	15.63	14.28	13.24	12.25
912.000	11.87	11.81	12.37	13.66	14.90
927.000	15.99	17.03	16.98	16.94	16.25
942.000	15.45	14.37	13.15	11.74	10.13
957.000	8.72	7.86	7.07	7.33	7.59
972.000	8.79	10.13	11.62	13.17	14.34
987.000	15.08	15.54	15.27	14.95	14.05
1,002.000	13.14	11.85	10.50	9.16	7.82

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,017.000	6.67	5.71	4.85	4.22	3.62
1,032.000	3.20	2.79	3.08	3.45	4.40
1,047.000	5.61	7.26	9.36	11.30	12.87
1,062.000	14.31	14.62	14.92	14.45	13.89
1,077.000	12.98	11.91	10.64	9.18	7.82
1,092.000	6.70	5.62	4.89	4.17	3.65
1,107.000	3.16	2.74	2.34	2.00	1.73
1,122.000	1.48	1.28	1.08	0.94	0.80
1,137.000	0.70	0.60	0.75	1.01	1.64
1,152.000	2.61	3.86	5.68	7.52	9.58
1,167.000	11.63	12.83	13.96	14.27	14.27
1,182.000	13.88	13.14	12.23	10.99	9.73
1,197.000	8.41	7.10	6.13	5.19	4.49
1,212.000	3.87	3.35	2.91	2.50	2.16
1,227.000	1.84	1.60	1.37	1.19	1.02
1,242.000	0.88	0.76	0.65	0.57	0.49
1,257.000	0.42	0.36	0.31	0.27	0.23
1,272.000	0.20	0.17	0.15	0.51	1.16
1,287.000	2.09	3.49	5.04	7.39	9.74
1,302.000	11.57	13.38	14.16	14.63	14.57
1,317.000	14.09	13.42	12.41	11.32	9.84
1,332.000	8.35	7.19	6.05	5.21	4.47
1,347.000	3.87	3.38	2.92	2.51	2.13
1,362.000	1.86	1.59	1.39	1.19	1.03
1,377.000	0.88	0.76	0.66	0.56	0.49
1,392.000	0.42	0.37	0.31	0.27	0.23
1,407.000	0.20	0.17	0.15	0.13	0.11
1,422.000	0.10	0.09	0.07	0.06	0.05
1,437.000	0.03	0.02	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	346.07 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	344.29 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4990 in
Runoff Volume (Pervious)	2,197,220.614 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,181,713.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA A1
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
402.000	0.00	0.00	0.00	0.01	0.01
417.000	0.02	0.03	0.05	0.07	0.09
432.000	0.12	0.16	0.20	0.24	0.30
447.000	0.35	0.41	0.47	0.54	0.61
462.000	0.68	0.75	0.83	0.91	0.99
477.000	1.07	1.16	1.25	1.34	1.43
492.000	1.52	1.62	1.72	1.82	1.93
507.000	2.03	2.14	2.25	2.36	2.48
522.000	2.60	2.72	2.84	2.97	3.10
537.000	3.24	3.37	3.52	3.68	3.86
552.000	4.05	4.26	4.50	4.75	5.05
567.000	5.34	5.66	5.98	6.32	6.66
582.000	7.00	7.36	7.71	8.07	8.43
597.000	8.80	9.16	9.55	9.93	10.34
612.000	10.75	11.17	11.61	12.05	12.52
627.000	13.00	13.53	14.05	14.67	15.29
642.000	16.02	16.79	17.67	18.62	19.63
657.000	20.75	21.90	23.19	24.47	25.97
672.000	27.47	29.18	30.96	32.92	35.01
687.000	37.25	39.72	42.29	45.30	48.30
702.000	52.15	56.01	61.24	66.82	75.01
717.000	84.93	98.51	117.57	138.57	167.32
732.000	196.08	229.62	263.17	289.95	315.10
747.000	330.81	340.40	344.29	339.91	333.42
762.000	318.99	304.50	284.72	264.95	246.31
777.000	227.93	211.66	196.71	182.98	170.93
792.000	159.32	149.26	139.23	131.02	122.81
807.000	115.76	108.97	102.90	97.27	92.02
822.000	87.30	82.73	78.66	74.61	71.09
837.000	67.57	64.48	61.47	58.77	56.24
852.000	53.90	51.80	49.79	48.07	46.36
867.000	44.91	43.46	42.18	40.93	39.80
882.000	38.75	37.79	36.96	36.15	35.44
897.000	34.73	34.09	33.44	32.80	32.16

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
912.000	31.48	30.78	30.04	29.26	28.48
927.000	27.68	26.88	26.12	25.36	24.67
942.000	23.99	23.38	22.81	22.28	21.82
957.000	21.37	20.98	20.59	20.25	19.92
972.000	19.62	19.33	19.06	18.80	18.55
987.000	18.32	18.10	17.89	17.68	17.48
1,002.000	17.29	17.11	16.93	16.76	16.59
1,017.000	16.43	16.27	16.12	15.97	15.82
1,032.000	15.68	15.54	15.40	15.27	15.14
1,047.000	15.01	14.89	14.77	14.65	14.53
1,062.000	14.42	14.30	14.19	14.09	13.98
1,077.000	13.88	13.77	13.66	13.55	13.43
1,092.000	13.30	13.16	13.00	12.83	12.66
1,107.000	12.48	12.30	12.12	11.94	11.78
1,122.000	11.62	11.47	11.33	11.20	11.08
1,137.000	10.97	10.86	10.76	10.67	10.58
1,152.000	10.49	10.41	10.33	10.26	10.19
1,167.000	10.12	10.05	9.99	9.93	9.86
1,182.000	9.81	9.75	9.69	9.63	9.58
1,197.000	9.52	9.47	9.41	9.36	9.31
1,212.000	9.26	9.21	9.16	9.11	9.06
1,227.000	9.01	8.96	8.92	8.87	8.82
1,242.000	8.78	8.73	8.69	8.65	8.60
1,257.000	8.56	8.52	8.48	8.44	8.40
1,272.000	8.36	8.32	8.28	8.25	8.21
1,287.000	8.17	8.13	8.10	8.06	8.02
1,302.000	7.99	7.95	7.92	7.88	7.85
1,317.000	7.82	7.78	7.75	7.72	7.68
1,332.000	7.65	7.62	7.58	7.55	7.52
1,347.000	7.49	7.45	7.42	7.39	7.36
1,362.000	7.33	7.30	7.28	7.25	7.22
1,377.000	7.19	7.16	7.13	7.11	7.08
1,392.000	7.05	7.03	7.00	6.97	6.95
1,407.000	6.92	6.90	6.87	6.85	6.82
1,422.000	6.80	6.77	6.75	6.73	6.70
1,437.000	6.68	6.63	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	460.87 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	458.07 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3213 in
Runoff Volume (Pervious)	3,087,200.419 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	3,067,288.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
348.000	0.00	0.00	0.00	0.01	0.02
363.000	0.03	0.04	0.06	0.09	0.12
378.000	0.16	0.21	0.26	0.32	0.38
393.000	0.46	0.53	0.62	0.70	0.79
408.000	0.88	0.98	1.08	1.18	1.29
423.000	1.39	1.50	1.61	1.73	1.84
438.000	1.96	2.08	2.20	2.33	2.45
453.000	2.58	2.71	2.84	2.98	3.11
468.000	3.25	3.40	3.54	3.69	3.83
483.000	3.99	4.14	4.30	4.46	4.62
498.000	4.79	4.95	5.13	5.30	5.48
513.000	5.66	5.85	6.04	6.23	6.43
528.000	6.63	6.84	7.05	7.27	7.49
543.000	7.75	8.01	8.32	8.66	9.03
558.000	9.49	9.96	10.51	11.06	11.67
573.000	12.28	12.91	13.54	14.18	14.82
588.000	15.45	16.08	16.71	17.34	17.97
603.000	18.61	19.26	19.93	20.61	21.30
618.000	22.02	22.75	23.51	24.28	25.13
633.000	25.98	26.99	28.01	29.25	30.54
648.000	32.04	33.67	35.42	37.34	39.31
663.000	41.50	43.68	46.14	48.61	51.35
678.000	54.18	57.24	60.46	63.87	67.56
693.000	71.37	75.68	79.99	85.25	90.52
708.000	97.45	104.80	115.48	128.40	146.05
723.000	170.80	198.05	235.19	272.34	315.32
738.000	358.29	392.14	423.77	443.02	454.23
753.000	458.07	451.23	441.71	422.18	402.56
768.000	376.60	350.65	326.52	302.81	281.96
783.000	262.89	245.40	230.13	215.40	202.63
798.000	189.90	179.43	168.96	159.93	151.21
813.000	143.38	136.07	129.23	123.00	116.93
828.000	111.42	105.93	101.03	96.14	91.78
843.000	87.53	83.69	80.08	76.74	73.73

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
858.000	70.85	68.40	65.96	63.90	61.84
873.000	60.03	58.27	56.69	55.21	53.86
888.000	52.68	51.54	50.54	49.54	48.62
903.000	47.71	46.78	45.86	44.86	43.82
918.000	42.71	41.53	40.33	39.10	37.87
933.000	36.70	35.53	34.47	33.42	32.49
948.000	31.62	30.82	30.12	29.45	28.87
963.000	28.29	27.80	27.31	26.88	26.46
978.000	26.07	25.70	25.35	25.03	24.71
993.000	24.41	24.12	23.85	23.58	23.33
1,008.000	23.08	22.84	22.61	22.39	22.17
1,023.000	21.96	21.75	21.55	21.36	21.17
1,038.000	20.98	20.80	20.62	20.45	20.28
1,053.000	20.12	19.96	19.80	19.64	19.49
1,068.000	19.34	19.19	19.05	18.90	18.76
1,083.000	18.60	18.44	18.27	18.06	17.86
1,098.000	17.60	17.34	17.05	16.76	16.46
1,113.000	16.17	15.89	15.61	15.35	15.12
1,128.000	14.88	14.69	14.50	14.33	14.17
1,143.000	14.03	13.89	13.76	13.64	13.52
1,158.000	13.41	13.30	13.21	13.11	13.02
1,173.000	12.92	12.84	12.75	12.67	12.59
1,188.000	12.52	12.44	12.37	12.29	12.22
1,203.000	12.15	12.08	12.01	11.94	11.88
1,218.000	11.81	11.75	11.68	11.62	11.56
1,233.000	11.50	11.44	11.38	11.32	11.26
1,248.000	11.21	11.15	11.09	11.04	10.98
1,263.000	10.93	10.88	10.82	10.77	10.72
1,278.000	10.67	10.62	10.57	10.52	10.48
1,293.000	10.43	10.38	10.34	10.29	10.25
1,308.000	10.20	10.15	10.11	10.06	10.02
1,323.000	9.97	9.93	9.88	9.84	9.80
1,338.000	9.76	9.72	9.67	9.63	9.59
1,353.000	9.56	9.52	9.48	9.44	9.40
1,368.000	9.37	9.33	9.29	9.26	9.22
1,383.000	9.18	9.15	9.11	9.07	9.04
1,398.000	9.00	8.97	8.94	8.90	8.87
1,413.000	8.84	8.80	8.77	8.73	8.70
1,428.000	8.67	8.64	8.60	8.57	8.51

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	653.99 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	649.43 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8283 in
Runoff Volume (Pervious)	4,799,931.587 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	4,772,632.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
276.000	0.00	0.00	0.00	0.01	0.02
291.000	0.03	0.05	0.07	0.10	0.13
306.000	0.17	0.22	0.27	0.33	0.40
321.000	0.47	0.54	0.62	0.70	0.79
336.000	0.87	0.97	1.06	1.15	1.25
351.000	1.35	1.45	1.55	1.66	1.77
366.000	1.89	2.01	2.15	2.29	2.46
381.000	2.63	2.81	3.01	3.21	3.42
396.000	3.63	3.85	4.06	4.28	4.50
411.000	4.72	4.94	5.16	5.38	5.60
426.000	5.82	6.04	6.27	6.49	6.72
441.000	6.95	7.18	7.42	7.66	7.89
456.000	8.14	8.38	8.63	8.88	9.14
471.000	9.40	9.66	9.92	10.19	10.47
486.000	10.75	11.03	11.32	11.61	11.90
501.000	12.20	12.51	12.82	13.14	13.46
516.000	13.79	14.13	14.47	14.82	15.17
531.000	15.54	15.91	16.30	16.69	17.16
546.000	17.66	18.27	18.97	19.78	20.78
561.000	21.83	23.09	24.35	25.73	27.12
576.000	28.52	29.93	31.33	32.71	34.06
591.000	35.36	36.65	37.91	39.17	40.42
606.000	41.67	42.94	44.22	45.52	46.84
621.000	48.18	49.57	50.98	52.52	54.06
636.000	55.94	57.83	60.17	62.64	65.52
651.000	68.70	72.09	75.82	79.63	83.79
666.000	87.94	92.48	97.03	101.95	106.97
681.000	112.29	117.82	123.56	129.62	135.79
696.000	142.40	149.01	156.36	163.73	172.84
711.000	182.40	196.33	213.19	236.54	269.62
726.000	306.12	356.14	406.18	463.90	521.63
741.000	566.45	608.15	632.67	646.07	649.43
756.000	638.26	623.58	595.71	567.75	532.17
771.000	496.59	464.24	432.62	405.13	380.21

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA A1
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
786.000	357.43	337.66	318.57	301.94	285.36
801.000	271.49	257.61	245.47	233.69	222.97
816.000	212.88	203.32	194.47	185.77	177.61
831.000	169.47	161.98	154.48	147.68	141.02
846.000	134.95	129.22	123.90	119.10	114.52
861.000	110.62	106.74	103.49	100.24	97.40
876.000	94.63	92.15	89.83	87.71	85.85
891.000	84.05	82.46	80.88	79.41	77.95
906.000	76.42	74.88	73.15	71.33	69.35
921.000	67.20	65.01	62.72	60.44	58.25
936.000	56.06	54.09	52.15	50.43	48.83
951.000	47.38	46.12	44.93	43.91	42.91
966.000	42.07	41.23	40.51	39.80	39.16
981.000	38.56	37.99	37.46	36.94	36.47
996.000	36.01	35.59	35.16	34.77	34.39
1,011.000	34.02	33.67	33.32	32.99	32.67
1,026.000	32.36	32.05	31.76	31.46	31.18
1,041.000	30.91	30.64	30.38	30.13	29.88
1,056.000	29.64	29.40	29.17	28.94	28.72
1,071.000	28.50	28.29	28.07	27.85	27.62
1,086.000	27.36	27.08	26.74	26.39	25.94
1,101.000	25.49	24.97	24.44	23.90	23.37
1,116.000	22.86	22.37	21.90	21.48	21.07
1,131.000	20.74	20.41	20.13	19.86	19.63
1,146.000	19.40	19.19	19.00	18.81	18.64
1,161.000	18.48	18.33	18.18	18.04	17.90
1,176.000	17.77	17.65	17.53	17.41	17.30
1,191.000	17.19	17.08	16.97	16.87	16.77
1,206.000	16.66	16.57	16.47	16.38	16.28
1,221.000	16.19	16.10	16.02	15.93	15.84
1,236.000	15.76	15.68	15.60	15.52	15.44
1,251.000	15.36	15.28	15.21	15.13	15.05
1,266.000	14.98	14.91	14.83	14.76	14.69
1,281.000	14.62	14.55	14.48	14.42	14.35
1,296.000	14.28	14.22	14.16	14.09	14.03
1,311.000	13.96	13.90	13.84	13.77	13.71
1,326.000	13.65	13.59	13.53	13.47	13.41
1,341.000	13.35	13.29	13.23	13.18	13.12
1,356.000	13.07	13.01	12.96	12.91	12.85
1,371.000	12.80	12.75	12.70	12.65	12.60
1,386.000	12.55	12.50	12.45	12.40	12.35
1,401.000	12.30	12.25	12.21	12.16	12.12
1,416.000	12.07	12.03	11.98	11.94	11.89

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA A1
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,431.000	11.85	11.81	11.76	11.68	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	743.401 min
Flow (Peak, Computed)	27.76 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	27.71 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	25.970 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	153,468.931 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	153,458.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	58.26 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
513.000	0.00	0.00	0.00	0.01	0.01
528.000	0.02	0.02	0.02	0.02	0.02
543.000	0.02	0.01	0.01	0.01	0.01
558.000	0.01	0.01	0.04	0.09	0.15
573.000	0.25	0.32	0.36	0.37	0.35
588.000	0.31	0.26	0.20	0.19	0.23
603.000	0.32	0.47	0.64	0.75	0.81
618.000	0.80	0.73	0.63	0.52	0.40
633.000	0.39	0.45	0.60	0.84	1.05
648.000	1.19	1.23	1.17	1.11	1.08
663.000	1.11	1.30	1.52	1.68	1.75
678.000	1.76	1.81	1.92	2.09	2.28
693.000	2.47	2.66	2.87	3.23	3.62
708.000	4.10	4.76	5.97	7.62	9.74
723.000	12.47	15.86	19.38	22.77	25.56
738.000	27.06	27.71	27.60	26.80	25.37
753.000	23.53	21.39	19.13	17.04	15.20
768.000	13.65	12.31	11.18	10.32	9.73
783.000	9.42	8.88	8.14	7.25	6.34
798.000	5.63	5.13	4.89	5.23	5.52
813.000	5.63	5.42	4.91	4.27	3.61
828.000	3.02	2.92	3.23	3.83	4.52
843.000	4.81	4.82	4.55	4.05	3.38
858.000	2.71	2.17	2.05	2.34	2.94
873.000	3.77	4.44	4.68	4.62	4.30
888.000	3.74	3.06	2.39	1.81	1.43
903.000	1.13	0.98	1.05	1.53	2.32
918.000	3.25	4.12	4.45	4.46	4.21
933.000	3.74	3.06	2.41	1.84	1.43
948.000	1.14	0.90	0.70	0.55	0.51
963.000	0.69	1.11	1.91	2.90	3.79
978.000	4.40	4.46	4.24	3.80	3.21
993.000	2.52	1.95	1.50	1.17	0.93
1,008.000	0.73	0.57	0.44	0.35	0.27

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA A2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,023.000	0.21	0.17	0.13	0.11	0.13
1,038.000	0.34	0.88	1.69	2.70	3.70
1,053.000	4.24	4.43	4.29	3.87	3.25
1,068.000	2.60	1.99	1.55	1.22	0.96
1,083.000	0.75	0.59	0.47	0.37	0.28
1,098.000	0.22	0.18	0.14	0.11	0.09
1,113.000	0.07	0.05	0.04	0.03	0.03
1,128.000	0.02	0.01	0.01	0.00	0.00
1,143.000	0.00	0.14	0.50	1.10	2.05
1,158.000	3.10	3.93	4.44	4.45	4.16
1,173.000	3.67	3.04	2.37	1.84	1.43
1,188.000	1.11	0.89	0.70	0.54	0.42
1,203.000	0.34	0.26	0.21	0.16	0.13
1,218.000	0.10	0.08	0.06	0.05	0.04
1,233.000	0.03	0.02	0.02	0.01	0.00
1,248.000	0.00	0.00	0.00	0.00	0.00
1,263.000	0.00	0.00	0.00	0.00	0.00
1,278.000	0.02	0.10	0.49	1.17	2.09
1,293.000	3.23	4.07	4.46	4.50	4.24
1,308.000	3.70	3.03	2.36	1.79	1.42
1,323.000	1.12	0.88	0.68	0.54	0.43
1,338.000	0.33	0.26	0.21	0.16	0.13
1,353.000	0.10	0.08	0.06	0.05	0.04
1,368.000	0.03	0.02	0.02	0.01	0.00
1,383.000	0.00	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	88.24 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	87.58 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	25.970 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	422,389.331 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	420,586.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	58.26 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.00	0.01	0.01
423.000	0.02	0.02	0.03	0.04	0.05
438.000	0.07	0.08	0.09	0.11	0.12
453.000	0.14	0.15	0.17	0.18	0.20
468.000	0.22	0.24	0.25	0.27	0.29
483.000	0.31	0.33	0.35	0.37	0.39
498.000	0.41	0.43	0.45	0.48	0.50
513.000	0.52	0.55	0.57	0.60	0.62
528.000	0.65	0.68	0.70	0.73	0.76
543.000	0.80	0.84	0.88	0.94	1.00
558.000	1.07	1.15	1.22	1.30	1.37
573.000	1.44	1.51	1.58	1.65	1.72
588.000	1.79	1.87	1.94	2.02	2.10
603.000	2.18	2.26	2.35	2.43	2.53
618.000	2.62	2.72	2.83	2.94	3.05
633.000	3.18	3.33	3.50	3.70	3.94
648.000	4.20	4.46	4.74	5.03	5.32
663.000	5.62	5.94	6.30	6.70	7.14
678.000	7.62	8.14	8.69	9.26	9.89
693.000	10.57	11.34	12.19	13.24	14.48
708.000	15.98	17.81	20.47	24.30	29.70
723.000	37.42	48.28	60.25	72.14	81.78
738.000	86.41	87.58	85.56	80.27	73.05
753.000	65.58	58.37	52.09	46.65	41.83
768.000	37.60	34.10	31.10	28.48	26.20
783.000	24.33	22.68	21.20	19.84	18.64
798.000	17.55	16.56	15.66	14.89	14.18
813.000	13.54	12.93	12.35	11.79	11.25
828.000	10.73	10.25	9.80	9.39	9.03
843.000	8.73	8.45	8.20	7.98	7.78
858.000	7.60	7.43	7.27	7.13	6.99
873.000	6.86	6.74	6.62	6.51	6.40
888.000	6.30	6.21	6.11	6.02	5.93
903.000	5.84	5.73	5.60	5.43	5.25

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	5.06	4.86	4.69	4.52	4.38
933.000	4.26	4.16	4.07	3.99	3.92
948.000	3.85	3.79	3.74	3.69	3.64
963.000	3.60	3.55	3.51	3.47	3.43
978.000	3.40	3.36	3.33	3.30	3.26
993.000	3.23	3.20	3.17	3.14	3.12
1,008.000	3.09	3.06	3.03	3.01	2.98
1,023.000	2.95	2.93	2.90	2.88	2.86
1,038.000	2.84	2.81	2.79	2.77	2.75
1,053.000	2.73	2.71	2.69	2.67	2.65
1,068.000	2.63	2.61	2.59	2.57	2.55
1,083.000	2.53	2.51	2.48	2.44	2.40
1,098.000	2.35	2.30	2.26	2.22	2.19
1,113.000	2.16	2.13	2.11	2.09	2.07
1,128.000	2.05	2.03	2.02	2.00	1.99
1,143.000	1.98	1.96	1.95	1.94	1.93
1,158.000	1.91	1.90	1.89	1.88	1.87
1,173.000	1.86	1.85	1.84	1.83	1.82
1,188.000	1.81	1.80	1.79	1.78	1.77
1,203.000	1.76	1.75	1.74	1.73	1.73
1,218.000	1.72	1.71	1.70	1.69	1.68
1,233.000	1.67	1.66	1.66	1.65	1.64
1,248.000	1.63	1.62	1.62	1.61	1.60
1,263.000	1.60	1.59	1.58	1.57	1.57
1,278.000	1.56	1.55	1.54	1.54	1.53
1,293.000	1.53	1.52	1.51	1.51	1.50
1,308.000	1.49	1.49	1.48	1.47	1.47
1,323.000	1.46	1.45	1.45	1.44	1.44
1,338.000	1.43	1.42	1.42	1.41	1.41
1,353.000	1.40	1.40	1.39	1.38	1.38
1,368.000	1.37	1.37	1.36	1.36	1.35
1,383.000	1.35	1.34	1.34	1.33	1.33
1,398.000	1.32	1.32	1.31	1.31	1.31
1,413.000	1.30	1.30	1.29	1.29	1.28
1,428.000	1.28	1.27	1.27	1.26	1.25

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	117.06 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	115.96 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	25.970 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	593,981.958 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	591,665.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	58.26 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
354.000	0.00	0.00	0.00	0.01	0.01
369.000	0.02	0.03	0.04	0.06	0.07
384.000	0.09	0.11	0.12	0.14	0.16
399.000	0.18	0.20	0.22	0.24	0.26
414.000	0.29	0.31	0.33	0.35	0.38
429.000	0.40	0.42	0.45	0.47	0.50
444.000	0.52	0.55	0.58	0.60	0.63
459.000	0.66	0.69	0.71	0.74	0.77
474.000	0.80	0.83	0.86	0.89	0.93
489.000	0.96	0.99	1.03	1.06	1.10
504.000	1.13	1.17	1.21	1.24	1.28
519.000	1.32	1.37	1.41	1.45	1.49
534.000	1.54	1.58	1.63	1.69	1.76
549.000	1.85	1.96	2.09	2.23	2.37
564.000	2.52	2.66	2.80	2.93	3.06
579.000	3.18	3.30	3.42	3.54	3.66
594.000	3.78	3.91	4.04	4.17	4.30
609.000	4.44	4.59	4.74	4.89	5.05
624.000	5.22	5.39	5.57	5.78	6.03
639.000	6.32	6.68	7.10	7.55	8.03
654.000	8.52	9.01	9.50	9.99	10.51
669.000	11.08	11.69	12.36	13.09	13.86
684.000	14.67	15.52	16.43	17.41	18.46
699.000	19.60	20.93	22.49	24.36	26.68
714.000	30.11	35.09	42.15	52.23	66.34
729.000	81.82	97.09	109.29	114.88	115.96
744.000	112.90	105.65	95.95	86.01	76.50
759.000	68.31	61.32	55.19	49.86	45.50
774.000	41.79	38.54	35.71	33.37	31.29
789.000	29.41	27.69	26.16	24.76	23.49
804.000	22.33	21.33	20.40	19.54	18.72
819.000	17.91	17.11	16.31	15.52	14.79
834.000	14.11	13.49	12.95	12.49	12.08
849.000	11.72	11.39	11.10	10.83	10.59

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
864.000	10.36	10.15	9.96	9.77	9.60
879.000	9.44	9.28	9.13	8.99	8.86
894.000	8.72	8.60	8.47	8.33	8.17
909.000	7.98	7.72	7.43	7.13	6.82
924.000	6.54	6.29	6.07	5.88	5.73
939.000	5.59	5.47	5.37	5.27	5.19
954.000	5.11	5.04	4.97	4.91	4.85
969.000	4.79	4.73	4.68	4.63	4.58
984.000	4.54	4.49	4.45	4.40	4.36
999.000	4.32	4.28	4.24	4.20	4.17
1,014.000	4.13	4.10	4.06	4.03	3.99
1,029.000	3.96	3.93	3.90	3.86	3.83
1,044.000	3.80	3.77	3.75	3.72	3.69
1,059.000	3.66	3.63	3.61	3.58	3.56
1,074.000	3.53	3.51	3.48	3.45	3.41
1,089.000	3.36	3.30	3.23	3.15	3.07
1,104.000	3.00	2.93	2.88	2.83	2.78
1,119.000	2.75	2.72	2.69	2.66	2.64
1,134.000	2.62	2.60	2.58	2.56	2.54
1,149.000	2.52	2.50	2.49	2.47	2.46
1,164.000	2.45	2.43	2.42	2.40	2.39
1,179.000	2.38	2.36	2.35	2.34	2.32
1,194.000	2.31	2.30	2.29	2.27	2.26
1,209.000	2.25	2.24	2.22	2.21	2.20
1,224.000	2.19	2.18	2.17	2.16	2.15
1,239.000	2.14	2.13	2.12	2.11	2.10
1,254.000	2.09	2.08	2.07	2.06	2.05
1,269.000	2.04	2.03	2.02	2.01	2.00
1,284.000	1.99	1.98	1.97	1.96	1.95
1,299.000	1.95	1.94	1.93	1.92	1.91
1,314.000	1.90	1.90	1.89	1.88	1.87
1,329.000	1.86	1.86	1.85	1.84	1.83
1,344.000	1.82	1.82	1.81	1.80	1.80
1,359.000	1.79	1.78	1.78	1.77	1.76
1,374.000	1.76	1.75	1.74	1.73	1.73
1,389.000	1.72	1.71	1.71	1.70	1.70
1,404.000	1.69	1.68	1.68	1.67	1.66
1,419.000	1.66	1.65	1.65	1.64	1.63
1,434.000	1.63	1.62	1.61	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	165.13 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	163.26 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	25.970 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	924,349.992 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	921,177.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	58.26 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	25.970 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.01	0.01	0.02
297.000	0.02	0.04	0.05	0.06	0.08
312.000	0.09	0.11	0.12	0.14	0.16
327.000	0.18	0.20	0.22	0.24	0.26
342.000	0.28	0.30	0.32	0.34	0.36
357.000	0.38	0.40	0.43	0.45	0.49
372.000	0.52	0.56	0.61	0.66	0.70
387.000	0.75	0.80	0.84	0.88	0.93
402.000	0.97	1.01	1.05	1.10	1.14
417.000	1.18	1.22	1.27	1.31	1.36
432.000	1.40	1.45	1.49	1.54	1.58
447.000	1.63	1.68	1.73	1.78	1.83
462.000	1.88	1.93	1.98	2.03	2.09
477.000	2.14	2.20	2.25	2.31	2.37
492.000	2.43	2.49	2.55	2.61	2.68
507.000	2.74	2.81	2.88	2.95	3.02
522.000	3.09	3.16	3.24	3.31	3.39
537.000	3.47	3.56	3.67	3.81	4.00
552.000	4.27	4.57	4.91	5.27	5.62
567.000	5.95	6.26	6.55	6.81	7.06
582.000	7.31	7.55	7.78	8.01	8.25
597.000	8.48	8.72	8.96	9.20	9.46
612.000	9.72	9.98	10.26	10.54	10.84
627.000	11.14	11.46	11.84	12.30	12.86
642.000	13.58	14.43	15.35	16.30	17.26
657.000	18.20	19.13	20.04	20.96	21.95
672.000	23.01	24.14	25.36	26.63	27.95
687.000	29.33	30.76	32.23	33.72	35.23
702.000	36.77	38.54	40.69	43.43	47.80
717.000	54.39	63.92	77.72	97.08	118.24
732.000	138.99	155.33	162.42	163.26	158.41
747.000	147.82	133.97	119.94	106.67	95.45
762.000	86.12	78.11	71.31	65.88	61.30
777.000	57.29	53.78	50.81	48.13	45.66

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA A2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
792.000	43.37	41.30	39.39	37.62	36.00
807.000	34.57	33.25	31.99	30.77	29.51
822.000	28.21	26.89	25.57	24.32	23.15
837.000	22.09	21.16	20.39	19.70	19.09
852.000	18.55	18.08	17.64	17.24	16.88
867.000	16.55	16.23	15.94	15.67	15.41
882.000	15.16	14.92	14.70	14.48	14.28
897.000	14.08	13.88	13.65	13.36	13.01
912.000	12.53	11.98	11.39	10.80	10.26
927.000	9.78	9.35	9.00	8.72	8.47
942.000	8.26	8.08	7.92	7.77	7.64
957.000	7.52	7.42	7.32	7.22	7.13
972.000	7.05	6.97	6.89	6.82	6.75
987.000	6.68	6.61	6.55	6.49	6.42
1,002.000	6.37	6.31	6.25	6.20	6.14
1,017.000	6.09	6.04	5.99	5.93	5.88
1,032.000	5.84	5.79	5.74	5.70	5.65
1,047.000	5.61	5.57	5.52	5.48	5.44
1,062.000	5.40	5.36	5.32	5.28	5.25
1,077.000	5.21	5.17	5.12	5.06	4.97
1,092.000	4.86	4.72	4.58	4.43	4.30
1,107.000	4.17	4.07	3.98	3.91	3.84
1,122.000	3.79	3.74	3.70	3.66	3.63
1,137.000	3.60	3.57	3.54	3.52	3.49
1,152.000	3.47	3.44	3.42	3.40	3.38
1,167.000	3.36	3.34	3.32	3.30	3.28
1,182.000	3.26	3.24	3.22	3.20	3.19
1,197.000	3.17	3.15	3.13	3.12	3.10
1,212.000	3.08	3.07	3.05	3.04	3.02
1,227.000	3.01	2.99	2.97	2.96	2.94
1,242.000	2.93	2.91	2.90	2.89	2.87
1,257.000	2.86	2.84	2.83	2.82	2.80
1,272.000	2.79	2.78	2.77	2.75	2.74
1,287.000	2.73	2.71	2.70	2.69	2.68
1,302.000	2.67	2.65	2.64	2.63	2.62
1,317.000	2.61	2.60	2.58	2.57	2.56
1,332.000	2.55	2.54	2.53	2.52	2.51
1,347.000	2.50	2.49	2.48	2.47	2.46
1,362.000	2.45	2.44	2.43	2.42	2.41
1,377.000	2.40	2.39	2.38	2.37	2.36
1,392.000	2.35	2.34	2.33	2.33	2.32
1,407.000	2.31	2.30	2.29	2.28	2.28
1,422.000	2.27	2.26	2.25	2.24	2.23

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,437.000	2.23	2.21	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	93.54 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	93.10 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	621,971.696 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	621,937.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
510.000	0.00	0.00	0.01	0.01	0.02
525.000	0.03	0.04	0.05	0.06	0.06
540.000	0.06	0.06	0.06	0.05	0.05
555.000	0.04	0.04	0.07	0.10	0.18
570.000	0.27	0.43	0.58	0.73	0.87
585.000	0.94	1.01	1.00	0.98	1.01
600.000	1.06	1.17	1.29	1.54	1.82
615.000	2.06	2.29	2.36	2.39	2.34
630.000	2.25	2.23	2.22	2.30	2.41
645.000	2.68	3.02	3.30	3.56	3.78
660.000	3.98	4.19	4.43	4.76	5.17
675.000	5.56	5.94	6.29	6.62	7.07
690.000	7.62	8.20	8.82	9.43	10.04
705.000	10.95	12.20	14.02	16.53	19.90
720.000	24.41	30.08	37.40	45.42	54.53
735.000	63.46	72.09	79.75	85.56	90.14
750.000	92.14	93.10	91.63	89.32	84.88
765.000	80.14	74.52	69.05	64.10	59.28
780.000	54.96	50.83	47.50	44.25	41.31
795.000	38.40	35.65	32.98	30.85	28.83
810.000	27.86	26.88	25.75	24.59	22.99
825.000	21.40	20.20	19.02	18.42	17.82
840.000	17.86	17.90	17.98	18.05	17.48
855.000	16.87	15.94	14.97	14.28	13.61
870.000	13.35	13.14	13.71	14.39	14.88
885.000	15.34	15.13	14.78	13.94	12.97
900.000	11.76	10.47	9.50	8.63	8.52
915.000	8.67	9.67	11.01	12.21	13.36
930.000	13.79	13.87	13.48	12.84	11.92
945.000	10.85	9.57	8.14	6.89	5.78
960.000	5.17	4.95	5.22	5.91	7.06
975.000	8.66	10.17	11.59	12.52	12.89
990.000	12.89	12.43	11.76	10.81	9.70
1,005.000	8.35	7.09	5.97	4.99	4.25

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,020.000	3.59	3.10	2.64	2.25	1.98
1,035.000	1.97	2.16	2.86	3.76	5.26
1,050.000	6.83	8.65	10.30	11.35	12.20
1,065.000	12.20	12.10	11.42	10.68	9.56
1,080.000	8.42	7.19	6.00	5.11	4.25
1,095.000	3.66	3.09	2.66	2.24	1.92
1,110.000	1.60	1.38	1.16	1.00	0.84
1,125.000	0.72	0.60	0.52	0.44	0.38
1,140.000	0.32	0.42	0.55	1.24	2.00
1,155.000	3.40	4.91	6.83	8.84	10.28
1,170.000	11.61	12.02	12.21	11.81	11.23
1,185.000	10.30	9.25	8.04	6.77	5.73
1,200.000	4.78	4.06	3.45	2.96	2.53
1,215.000	2.16	1.82	1.55	1.32	1.13
1,230.000	0.96	0.81	0.69	0.59	0.50
1,245.000	0.43	0.36	0.31	0.26	0.22
1,260.000	0.19	0.16	0.14	0.12	0.10
1,275.000	0.09	0.08	0.32	0.93	1.80
1,290.000	3.09	4.70	6.87	8.87	10.55
1,305.000	11.83	12.27	12.44	12.01	11.43
1,320.000	10.50	9.46	8.09	6.79	5.74
1,335.000	4.76	4.07	3.43	2.98	2.54
1,350.000	2.17	1.81	1.56	1.32	1.14
1,365.000	0.95	0.82	0.68	0.59	0.50
1,380.000	0.43	0.36	0.31	0.26	0.23
1,395.000	0.19	0.16	0.14	0.12	0.10
1,410.000	0.09	0.08	0.06	0.05	0.04
1,425.000	0.03	0.01	0.00	0.00	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	282.49 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	280.93 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	1,711,839.703 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,700,726.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
405.000	0.00	0.00	0.00	0.01	0.01
420.000	0.02	0.03	0.04	0.06	0.09
435.000	0.11	0.15	0.18	0.22	0.27
450.000	0.31	0.36	0.42	0.47	0.53
465.000	0.59	0.65	0.71	0.77	0.84
480.000	0.91	0.98	1.05	1.12	1.20
495.000	1.28	1.35	1.43	1.52	1.60
510.000	1.69	1.77	1.86	1.96	2.05
525.000	2.15	2.24	2.34	2.45	2.55
540.000	2.66	2.78	2.91	3.05	3.20
555.000	3.39	3.58	3.81	4.03	4.29
570.000	4.54	4.81	5.08	5.35	5.63
585.000	5.90	6.18	6.46	6.74	7.03
600.000	7.33	7.63	7.93	8.25	8.58
615.000	8.92	9.26	9.62	9.99	10.39
630.000	10.79	11.24	11.70	12.24	12.81
645.000	13.48	14.19	15.00	15.85	16.78
660.000	17.74	18.79	19.88	21.08	22.34
675.000	23.72	25.20	26.81	28.52	30.39
690.000	32.39	34.60	36.99	39.67	42.65
705.000	46.17	50.29	55.44	61.85	70.95
720.000	83.61	99.72	120.80	144.81	173.41
735.000	201.44	228.48	251.62	267.34	278.70
750.000	280.93	279.95	271.51	261.04	245.31
765.000	229.18	211.91	195.19	180.25	165.95
780.000	153.98	142.42	132.59	123.06	115.00
795.000	107.14	100.49	93.99	88.47	83.05
810.000	78.49	73.99	70.17	66.39	63.12
825.000	59.86	56.98	54.11	51.60	49.08
840.000	46.91	44.74	42.89	41.05	39.48
855.000	37.92	36.60	35.29	34.17	33.06
870.000	32.15	31.26	30.51	29.78	29.14
885.000	28.51	27.95	27.41	26.91	26.42
900.000	25.95	25.49	25.02	24.54	24.01

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
915.000	23.46	22.86	22.24	21.61	20.97
930.000	20.35	19.74	19.17	18.63	18.14
945.000	17.68	17.28	16.91	16.57	16.26
960.000	15.98	15.71	15.46	15.23	15.01
975.000	14.80	14.61	14.42	14.25	14.08
990.000	13.91	13.76	13.61	13.46	13.32
1,005.000	13.19	13.06	12.93	12.80	12.68
1,020.000	12.56	12.45	12.33	12.22	12.11
1,035.000	12.01	11.91	11.81	11.71	11.62
1,050.000	11.52	11.43	11.34	11.25	11.16
1,065.000	11.08	10.99	10.91	10.83	10.75
1,080.000	10.67	10.58	10.50	10.40	10.30
1,095.000	10.17	10.05	9.90	9.76	9.61
1,110.000	9.46	9.32	9.18	9.05	8.92
1,125.000	8.82	8.71	8.62	8.54	8.46
1,140.000	8.38	8.31	8.24	8.18	8.12
1,155.000	8.06	8.01	7.95	7.90	7.85
1,170.000	7.80	7.75	7.70	7.65	7.60
1,185.000	7.56	7.51	7.47	7.42	7.38
1,200.000	7.34	7.30	7.26	7.22	7.19
1,215.000	7.15	7.11	7.07	7.04	7.00
1,230.000	6.96	6.93	6.89	6.86	6.82
1,245.000	6.79	6.75	6.72	6.68	6.65
1,260.000	6.62	6.59	6.56	6.53	6.50
1,275.000	6.47	6.44	6.41	6.38	6.35
1,290.000	6.32	6.29	6.26	6.23	6.21
1,305.000	6.18	6.15	6.13	6.10	6.08
1,320.000	6.05	6.02	6.00	5.97	5.95
1,335.000	5.92	5.90	5.87	5.85	5.82
1,350.000	5.80	5.78	5.75	5.73	5.71
1,365.000	5.68	5.66	5.64	5.62	5.59
1,380.000	5.57	5.55	5.53	5.51	5.49
1,395.000	5.46	5.44	5.42	5.41	5.39
1,410.000	5.37	5.35	5.33	5.31	5.29
1,425.000	5.27	5.25	5.23	5.21	5.19
1,440.000	5.16	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	375.16 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	373.90 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	2,407,262.266 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,392,974.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
351.000	0.00	0.00	0.00	0.01	0.02
366.000	0.03	0.04	0.06	0.08	0.12
381.000	0.15	0.20	0.24	0.30	0.35
396.000	0.42	0.48	0.55	0.62	0.70
411.000	0.77	0.85	0.93	1.02	1.10
426.000	1.19	1.27	1.36	1.46	1.55
441.000	1.64	1.74	1.84	1.94	2.04
456.000	2.14	2.25	2.35	2.46	2.57
471.000	2.68	2.80	2.91	3.03	3.15
486.000	3.27	3.40	3.52	3.65	3.78
501.000	3.91	4.05	4.19	4.33	4.47
516.000	4.62	4.77	4.93	5.08	5.24
531.000	5.41	5.57	5.75	5.92	6.12
546.000	6.33	6.59	6.85	7.20	7.56
561.000	7.99	8.43	8.92	9.40	9.91
576.000	10.41	10.92	11.43	11.92	12.42
591.000	12.90	13.39	13.88	14.38	14.89
606.000	15.40	15.93	16.46	17.01	17.57
621.000	18.16	18.76	19.39	20.04	20.77
636.000	21.53	22.43	23.38	24.53	25.76
651.000	27.15	28.64	30.24	31.90	33.68
666.000	35.52	37.49	39.54	41.75	44.07
681.000	46.56	49.19	52.01	55.00	58.22
696.000	61.66	65.42	69.50	74.22	79.64
711.000	86.36	94.67	106.51	122.98	143.91
726.000	171.27	202.34	239.24	275.25	309.70
741.000	338.98	358.38	372.11	373.90	371.58
756.000	359.66	345.20	324.16	302.74	280.19
771.000	258.44	239.24	220.90	205.67	190.98
786.000	178.54	166.47	156.27	146.32	137.85
801.000	129.55	122.48	115.53	109.66	103.85
816.000	98.86	93.91	89.53	85.16	81.19
831.000	77.24	73.69	70.14	67.03	63.92
846.000	61.25	58.59	56.33	54.08	52.19

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
861.000	50.31	48.71	47.14	45.84	44.58
876.000	43.51	42.47	41.56	40.67	39.88
891.000	39.10	38.40	37.71	37.04	36.39
906.000	35.70	35.01	34.22	33.40	32.49
921.000	31.54	30.56	29.57	28.61	27.67
936.000	26.79	25.95	25.20	24.51	23.90
951.000	23.34	22.84	22.38	21.95	21.56
966.000	21.20	20.86	20.55	20.25	19.97
981.000	19.71	19.45	19.21	18.98	18.77
996.000	18.56	18.35	18.16	17.97	17.79
1,011.000	17.62	17.44	17.28	17.12	16.96
1,026.000	16.81	16.66	16.51	16.37	16.23
1,041.000	16.09	15.96	15.83	15.70	15.58
1,056.000	15.45	15.33	15.21	15.10	14.98
1,071.000	14.87	14.76	14.65	14.54	14.42
1,086.000	14.30	14.15	14.00	13.80	13.60
1,101.000	13.36	13.13	12.88	12.64	12.41
1,116.000	12.17	11.97	11.76	11.60	11.43
1,131.000	11.29	11.16	11.04	10.92	10.82
1,146.000	10.72	10.62	10.53	10.45	10.37
1,161.000	10.30	10.22	10.15	10.08	10.01
1,176.000	9.95	9.89	9.82	9.76	9.70
1,191.000	9.65	9.59	9.54	9.48	9.43
1,206.000	9.38	9.33	9.28	9.23	9.18
1,221.000	9.13	9.08	9.03	8.98	8.94
1,236.000	8.89	8.85	8.80	8.75	8.71
1,251.000	8.67	8.62	8.58	8.54	8.50
1,266.000	8.46	8.42	8.38	8.34	8.30
1,281.000	8.26	8.22	8.18	8.15	8.11
1,296.000	8.07	8.04	8.00	7.96	7.93
1,311.000	7.89	7.86	7.82	7.79	7.75
1,326.000	7.72	7.69	7.65	7.62	7.59
1,341.000	7.56	7.53	7.50	7.47	7.43
1,356.000	7.41	7.38	7.35	7.32	7.29
1,371.000	7.26	7.23	7.21	7.18	7.15
1,386.000	7.12	7.09	7.06	7.03	7.01
1,401.000	6.98	6.95	6.92	6.90	6.87
1,416.000	6.85	6.82	6.80	6.77	6.75
1,431.000	6.72	6.70	6.68	6.63	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	530.55 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	530.04 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	3,746,162.365 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	3,726,557.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
279.000	0.00	0.00	0.00	0.01	0.02
294.000	0.03	0.04	0.07	0.09	0.13
309.000	0.16	0.21	0.25	0.31	0.36
324.000	0.42	0.48	0.55	0.62	0.69
339.000	0.76	0.83	0.91	0.99	1.06
354.000	1.14	1.22	1.31	1.40	1.49
369.000	1.59	1.70	1.82	1.95	2.10
384.000	2.25	2.41	2.57	2.74	2.91
399.000	3.08	3.25	3.42	3.60	3.77
414.000	3.94	4.11	4.28	4.45	4.62
429.000	4.80	4.97	5.15	5.33	5.51
444.000	5.69	5.87	6.06	6.25	6.44
459.000	6.63	6.83	7.02	7.22	7.43
474.000	7.64	7.85	8.06	8.28	8.49
489.000	8.72	8.94	9.18	9.41	9.65
504.000	9.89	10.14	10.39	10.65	10.91
519.000	11.17	11.44	11.72	12.00	12.29
534.000	12.58	12.89	13.19	13.57	13.96
549.000	14.49	15.03	15.82	16.60	17.61
564.000	18.63	19.74	20.86	22.01	23.15
579.000	24.27	25.39	26.43	27.47	28.47
594.000	29.46	30.43	31.40	32.38	33.36
609.000	34.35	35.35	36.38	37.42	38.49
624.000	39.57	40.72	41.88	43.21	44.61
639.000	46.29	48.10	50.31	52.70	55.44
654.000	58.35	61.47	64.69	68.08	71.56
669.000	75.19	78.94	82.87	86.94	91.24
684.000	95.71	100.42	105.32	110.42	115.72
699.000	121.23	126.96	133.28	140.26	148.88
714.000	159.47	174.92	196.77	224.77	261.64
729.000	303.56	353.47	402.00	448.16	487.05
744.000	512.11	529.32	530.04	525.19	507.39
759.000	486.28	456.64	426.73	396.02	366.55
774.000	341.11	316.89	297.13	278.08	262.05

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
789.000	246.50	233.28	220.36	209.17	198.20
804.000	188.68	179.32	171.27	163.31	156.31
819.000	149.35	142.96	136.59	130.58	124.58
834.000	119.03	113.48	108.52	103.56	99.26
849.000	94.97	91.32	87.71	84.68	81.68
864.000	79.15	76.66	74.59	72.59	70.88
879.000	69.23	67.78	66.37	65.10	63.87
894.000	62.74	61.64	60.58	59.51	58.37
909.000	57.20	55.81	54.36	52.69	50.94
924.000	49.12	47.26	45.46	43.68	42.04
939.000	40.47	39.08	37.79	36.68	35.67
954.000	34.78	33.97	33.24	32.57	31.96
969.000	31.39	30.87	30.38	29.92	29.49
984.000	29.09	28.71	28.35	28.01	27.68
999.000	27.37	27.07	26.78	26.50	26.23
1,014.000	25.97	25.72	25.47	25.23	24.99
1,029.000	24.77	24.54	24.33	24.12	23.92
1,044.000	23.72	23.52	23.33	23.15	22.96
1,059.000	22.78	22.60	22.43	22.26	22.09
1,074.000	21.93	21.77	21.61	21.43	21.24
1,089.000	21.00	20.74	20.39	20.03	19.60
1,104.000	19.17	18.72	18.27	17.85	17.43
1,119.000	17.06	16.70	16.41	16.12	15.88
1,134.000	15.65	15.46	15.26	15.10	14.93
1,149.000	14.79	14.65	14.52	14.40	14.28
1,164.000	14.17	14.06	13.96	13.86	13.77
1,179.000	13.67	13.58	13.50	13.41	13.33
1,194.000	13.25	13.17	13.09	13.02	12.94
1,209.000	12.87	12.79	12.72	12.65	12.58
1,224.000	12.51	12.45	12.38	12.32	12.25
1,239.000	12.19	12.13	12.07	12.00	11.94
1,254.000	11.88	11.82	11.76	11.70	11.65
1,269.000	11.59	11.53	11.48	11.43	11.37
1,284.000	11.32	11.27	11.22	11.17	11.11
1,299.000	11.06	11.01	10.96	10.91	10.86
1,314.000	10.81	10.76	10.71	10.66	10.62
1,329.000	10.57	10.52	10.48	10.43	10.39
1,344.000	10.34	10.30	10.26	10.21	10.17
1,359.000	10.13	10.09	10.05	10.01	9.97
1,374.000	9.93	9.89	9.85	9.81	9.77
1,389.000	9.73	9.69	9.65	9.62	9.58
1,404.000	9.54	9.50	9.47	9.43	9.40
1,419.000	9.36	9.33	9.29	9.26	9.23

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,434.000	9.19	9.16	9.10	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres
Computational Time Increment	7.075 min
Time to Peak (Computed)	757.047 min
Flow (Peak, Computed)	63.54 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	63.09 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	77.990 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	460,879.550 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	460,866.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	53.064 min
Computational Time Increment	7.075 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	99.92 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	35.376 min
Unit receding limb, Tr	141.504 min
Total unit time, Tb	176.880 min

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA C
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
510.000	0.00	0.00	0.00	0.01	0.01
525.000	0.02	0.02	0.03	0.03	0.04
540.000	0.04	0.04	0.04	0.04	0.04
555.000	0.03	0.03	0.05	0.07	0.11
570.000	0.16	0.22	0.31	0.40	0.49
585.000	0.56	0.62	0.64	0.66	0.69
600.000	0.73	0.79	0.87	0.96	1.11
615.000	1.25	1.37	1.48	1.54	1.55
630.000	1.56	1.59	1.61	1.67	1.75
645.000	1.86	2.02	2.18	2.37	2.55
660.000	2.73	2.90	3.08	3.29	3.51
675.000	3.78	4.06	4.30	4.51	4.75
690.000	5.11	5.46	5.87	6.28	6.79
705.000	7.38	8.06	9.22	10.37	12.59
720.000	15.06	18.29	22.47	26.75	32.19
735.000	37.63	43.04	48.43	53.19	56.87
750.000	60.53	61.81	63.09	62.85	61.79
765.000	60.24	57.49	54.75	51.63	48.50
780.000	45.50	42.61	39.80	37.30	34.81
795.000	32.70	30.67	28.79	27.07	25.43
810.000	24.25	23.08	21.98	20.91	19.79
825.000	18.62	17.46	16.61	15.75	15.25
840.000	14.88	14.54	14.25	13.97	13.48
855.000	13.00	12.51	12.02	11.58	11.29
870.000	11.01	10.98	10.98	11.03	11.13
885.000	11.17	11.00	10.84	10.41	9.93
900.000	9.37	8.73	8.11	7.64	7.16
915.000	7.04	7.03	7.25	7.82	8.38
930.000	8.81	9.25	9.32	9.23	9.02
945.000	8.57	8.12	7.47	6.82	6.17
960.000	5.53	5.02	4.90	4.79	5.23
975.000	5.74	6.38	7.15	7.84	8.22
990.000	8.60	8.57	8.45	8.17	7.72
1,005.000	7.24	6.57	5.90	5.21	4.51

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA C
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,020.000	3.88	3.37	2.86	2.52	2.19
1,035.000	2.14	2.23	2.45	2.97	3.48
1,050.000	4.50	5.53	6.43	7.23	7.88
1,065.000	8.04	8.20	7.98	7.69	7.28
1,080.000	6.74	6.17	5.42	4.67	4.05
1,095.000	3.47	2.98	2.61	2.24	1.99
1,110.000	1.74	1.52	1.32	1.14	1.00
1,125.000	0.86	0.75	0.65	0.56	0.48
1,140.000	0.44	0.47	0.50	0.86	1.25
1,155.000	1.87	2.65	3.51	4.60	5.69
1,170.000	6.47	7.21	7.63	7.74	7.79
1,185.000	7.49	7.18	6.67	6.11	5.48
1,200.000	4.78	4.09	3.56	3.03	2.64
1,215.000	2.30	1.99	1.76	1.53	1.34
1,230.000	1.15	1.01	0.88	0.76	0.67
1,245.000	0.58	0.50	0.44	0.38	0.33
1,260.000	0.29	0.25	0.22	0.19	0.17
1,275.000	0.17	0.19	0.26	0.63	1.00
1,290.000	1.67	2.43	3.35	4.49	5.62
1,305.000	6.45	7.27	7.64	7.82	7.84
1,320.000	7.56	7.29	6.75	6.22	5.57
1,335.000	4.84	4.16	3.61	3.06	2.68
1,350.000	2.33	2.03	1.79	1.56	1.36
1,365.000	1.17	1.03	0.89	0.78	0.68
1,380.000	0.59	0.51	0.44	0.39	0.34
1,395.000	0.29	0.26	0.22	0.20	0.17
1,410.000	0.15	0.13	0.11	0.10	0.09
1,425.000	0.08	0.07	0.06	0.05	0.04
1,440.000	0.04	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres
Computational Time Increment	7.075 min
Time to Peak (Computed)	757.047 min
Flow (Peak, Computed)	190.78 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	190.22 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	77.990 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	1,268,469.154 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,258,905.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	53.064 min
Computational Time Increment	7.075 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	99.92 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	35.376 min
Unit receding limb, Tr	141.504 min
Total unit time, Tb	176.880 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.00	0.01	0.01
423.000	0.02	0.02	0.03	0.05	0.06
438.000	0.08	0.10	0.13	0.15	0.19
453.000	0.22	0.25	0.29	0.33	0.37
468.000	0.41	0.45	0.49	0.54	0.59
483.000	0.64	0.69	0.74	0.79	0.84
498.000	0.90	0.96	1.02	1.07	1.13
513.000	1.20	1.26	1.33	1.39	1.46
528.000	1.53	1.60	1.68	1.75	1.83
543.000	1.91	2.00	2.10	2.20	2.32
558.000	2.45	2.59	2.74	2.90	3.07
573.000	3.24	3.43	3.62	3.81	4.01
588.000	4.21	4.41	4.61	4.82	5.03
603.000	5.25	5.47	5.69	5.92	6.16
618.000	6.41	6.66	6.92	7.19	7.47
633.000	7.78	8.10	8.46	8.84	9.26
648.000	9.74	10.23	10.82	11.40	12.06
663.000	12.75	13.47	14.28	15.09	16.05
678.000	17.01	18.08	19.23	20.43	21.80
693.000	23.17	24.83	26.53	28.53	30.81
708.000	33.28	36.76	40.23	46.54	53.55
723.000	62.80	74.87	87.33	104.19	121.05
738.000	137.50	153.79	167.66	177.35	186.96
753.000	188.59	190.22	187.69	182.93	176.88
768.000	167.70	158.52	148.42	138.26	128.91
783.000	120.16	111.80	104.82	97.84	91.89
798.000	86.12	80.85	76.09	71.45	67.53
813.000	63.62	60.22	56.97	53.93	51.19
828.000	48.48	46.15	43.82	41.73	39.75
843.000	37.87	36.20	34.52	33.13	31.73
858.000	30.50	29.37	28.29	27.36	26.43
873.000	25.65	24.87	24.15	23.48	22.83
888.000	22.24	21.66	21.17	20.70	20.26
903.000	19.85	19.45	19.05	18.66	18.24

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA C
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	17.81	17.37	16.92	16.46	16.01
933.000	15.55	15.12	14.70	14.30	13.93
948.000	13.56	13.25	12.95	12.68	12.43
963.000	12.19	11.98	11.77	11.58	11.40
978.000	11.23	11.07	10.92	10.77	10.63
993.000	10.50	10.38	10.26	10.14	10.03
1,008.000	9.92	9.82	9.72	9.62	9.52
1,023.000	9.43	9.34	9.25	9.17	9.08
1,038.000	9.00	8.92	8.85	8.77	8.69
1,053.000	8.62	8.55	8.48	8.41	8.35
1,068.000	8.28	8.22	8.16	8.09	8.03
1,083.000	7.97	7.90	7.84	7.76	7.68
1,098.000	7.60	7.50	7.41	7.31	7.20
1,113.000	7.10	7.00	6.91	6.81	6.72
1,128.000	6.64	6.56	6.49	6.42	6.36
1,143.000	6.30	6.24	6.19	6.14	6.09
1,158.000	6.04	5.99	5.95	5.91	5.87
1,173.000	5.83	5.79	5.75	5.71	5.68
1,188.000	5.64	5.61	5.58	5.54	5.51
1,203.000	5.48	5.45	5.42	5.39	5.36
1,218.000	5.33	5.30	5.27	5.24	5.22
1,233.000	5.19	5.16	5.13	5.11	5.08
1,248.000	5.05	5.03	5.00	4.98	4.95
1,263.000	4.93	4.91	4.88	4.86	4.84
1,278.000	4.82	4.79	4.77	4.75	4.73
1,293.000	4.71	4.69	4.67	4.65	4.63
1,308.000	4.61	4.59	4.57	4.55	4.52
1,323.000	4.50	4.49	4.47	4.45	4.43
1,338.000	4.41	4.39	4.37	4.35	4.33
1,353.000	4.32	4.30	4.28	4.26	4.25
1,368.000	4.23	4.21	4.19	4.18	4.16
1,383.000	4.15	4.13	4.11	4.10	4.08
1,398.000	4.07	4.05	4.04	4.02	4.01
1,413.000	3.99	3.98	3.96	3.95	3.93
1,428.000	3.92	3.90	3.89	3.87	3.85

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres
Computational Time Increment	7.075 min
Time to Peak (Computed)	757.047 min
Flow (Peak, Computed)	253.93 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	253.42 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	77.990 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	1,783,775.448 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,771,491.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	53.064 min
Computational Time Increment	7.075 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	99.92 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	35.376 min
Unit receding limb, Tr	141.504 min
Total unit time, Tb	176.880 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
354.000	0.00	0.00	0.00	0.01	0.01
369.000	0.02	0.03	0.04	0.06	0.08
384.000	0.11	0.14	0.17	0.21	0.24
399.000	0.29	0.33	0.38	0.43	0.48
414.000	0.54	0.59	0.65	0.71	0.77
429.000	0.83	0.89	0.96	1.02	1.09
444.000	1.16	1.23	1.30	1.37	1.45
459.000	1.52	1.60	1.68	1.75	1.83
474.000	1.91	2.00	2.08	2.17	2.26
489.000	2.34	2.44	2.53	2.62	2.72
504.000	2.81	2.91	3.01	3.12	3.22
519.000	3.33	3.44	3.55	3.67	3.78
534.000	3.90	4.02	4.16	4.29	4.44
549.000	4.61	4.79	5.01	5.24	5.50
564.000	5.78	6.08	6.41	6.74	7.08
579.000	7.43	7.79	8.15	8.51	8.87
594.000	9.23	9.59	9.96	10.32	10.70
609.000	11.07	11.46	11.85	12.26	12.67
624.000	13.10	13.54	13.99	14.50	15.02
639.000	15.62	16.25	16.96	17.79	18.61
654.000	19.63	20.64	21.76	22.93	24.16
669.000	25.50	26.84	28.39	29.94	31.63
684.000	33.41	35.26	37.32	39.38	41.78
699.000	44.22	47.01	50.13	53.48	58.07
714.000	62.66	70.91	80.07	92.11	107.78
729.000	123.95	145.70	167.44	188.45	209.20
744.000	226.71	238.66	250.51	251.97	253.42
759.000	249.56	242.85	234.53	222.32	210.11
774.000	196.98	183.77	171.71	160.49	149.78
789.000	140.89	132.00	124.44	117.09	110.38
804.000	104.30	98.36	93.32	88.29	83.87
819.000	79.63	75.65	72.01	68.41	65.24
834.000	62.07	59.19	56.43	53.81	51.45
849.000	49.09	47.11	45.13	43.38	41.76

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
864.000	40.22	38.91	37.59	36.48	35.38
879.000	34.37	33.42	32.50	31.69	30.87
894.000	30.18	29.52	28.90	28.33	27.75
909.000	27.18	26.61	25.99	25.35	24.69
924.000	24.00	23.30	22.61	21.91	21.25
939.000	20.61	19.99	19.42	18.86	18.39
954.000	17.93	17.52	17.15	16.79	16.48
969.000	16.17	15.90	15.64	15.39	15.16
984.000	14.94	14.74	14.54	14.36	14.18
999.000	14.01	13.85	13.69	13.54	13.39
1,014.000	13.25	13.12	12.98	12.86	12.73
1,029.000	12.61	12.50	12.38	12.27	12.16
1,044.000	12.06	11.95	11.85	11.75	11.66
1,059.000	11.56	11.47	11.38	11.29	11.20
1,074.000	11.11	11.03	10.94	10.86	10.76
1,089.000	10.66	10.55	10.43	10.30	10.15
1,104.000	10.00	9.84	9.67	9.51	9.35
1,119.000	9.19	9.04	8.89	8.76	8.63
1,134.000	8.52	8.41	8.31	8.23	8.14
1,149.000	8.06	7.98	7.91	7.85	7.78
1,164.000	7.72	7.66	7.60	7.55	7.50
1,179.000	7.45	7.39	7.35	7.30	7.25
1,194.000	7.21	7.16	7.12	7.08	7.04
1,209.000	7.00	6.96	6.92	6.88	6.84
1,224.000	6.81	6.77	6.73	6.70	6.66
1,239.000	6.63	6.59	6.56	6.52	6.49
1,254.000	6.46	6.43	6.39	6.36	6.33
1,269.000	6.30	6.27	6.24	6.21	6.18
1,284.000	6.15	6.12	6.10	6.07	6.04
1,299.000	6.01	5.99	5.96	5.93	5.90
1,314.000	5.88	5.85	5.83	5.80	5.77
1,329.000	5.75	5.72	5.70	5.67	5.65
1,344.000	5.63	5.60	5.58	5.56	5.53
1,359.000	5.51	5.49	5.47	5.45	5.43
1,374.000	5.40	5.38	5.36	5.34	5.32
1,389.000	5.30	5.28	5.25	5.23	5.21
1,404.000	5.19	5.17	5.16	5.14	5.12
1,419.000	5.10	5.08	5.06	5.04	5.02
1,434.000	5.00	4.98	4.95	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres
Computational Time Increment	7.075 min
Time to Peak (Computed)	757.047 min
Flow (Peak, Computed)	360.28 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	359.95 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	77.990 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	2,775,897.414 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,759,037.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	53.064 min
Computational Time Increment	7.075 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	99.92 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	35.376 min
Unit receding limb, Tr	141.504 min
Total unit time, Tb	176.880 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	53.064 min
Area (User Defined)	77.990 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
279.000	0.00	0.00	0.00	0.01	0.01
294.000	0.02	0.02	0.04	0.05	0.07
309.000	0.09	0.12	0.15	0.18	0.21
324.000	0.25	0.29	0.33	0.38	0.43
339.000	0.48	0.53	0.58	0.63	0.69
354.000	0.74	0.80	0.86	0.92	0.99
369.000	1.06	1.13	1.21	1.30	1.39
384.000	1.49	1.60	1.71	1.82	1.94
399.000	2.06	2.18	2.30	2.43	2.55
414.000	2.68	2.80	2.93	3.05	3.18
429.000	3.31	3.43	3.56	3.69	3.82
444.000	3.96	4.09	4.23	4.36	4.50
459.000	4.64	4.78	4.93	5.07	5.22
474.000	5.37	5.52	5.67	5.83	5.99
489.000	6.15	6.31	6.48	6.65	6.82
504.000	6.99	7.17	7.35	7.54	7.72
519.000	7.91	8.11	8.31	8.51	8.71
534.000	8.92	9.14	9.38	9.63	9.92
549.000	10.25	10.60	11.08	11.57	12.16
564.000	12.80	13.47	14.21	14.95	15.73
579.000	16.51	17.29	18.08	18.86	19.61
594.000	20.37	21.10	21.83	22.55	23.28
609.000	24.01	24.75	25.49	26.25	27.02
624.000	27.80	28.61	29.43	30.38	31.32
639.000	32.45	33.64	34.99	36.58	38.18
654.000	40.15	42.11	44.27	46.53	48.86
669.000	51.36	53.87	56.65	59.45	62.42
684.000	65.51	68.67	72.07	75.48	79.17
699.000	82.91	86.98	91.33	95.96	102.00
714.000	108.05	119.00	131.17	147.27	168.31
729.000	190.05	219.34	248.64	276.73	304.38
744.000	327.50	342.84	358.04	358.99	359.95
759.000	353.78	343.80	331.77	314.79	297.81
774.000	280.08	262.30	246.24	231.44	217.34

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA C
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
789.000	205.76	194.19	184.32	174.74	165.91
804.000	157.85	149.95	143.13	136.30	130.21
819.000	124.33	118.74	113.54	108.37	103.66
834.000	98.95	94.59	90.37	86.34	82.65
849.000	78.96	75.84	72.71	69.95	67.38
864.000	64.95	62.88	60.80	59.05	57.32
879.000	55.74	54.26	52.82	51.55	50.27
894.000	49.20	48.16	47.18	46.26	45.33
909.000	44.38	43.42	42.34	41.23	40.05
924.000	38.78	37.51	36.22	34.93	33.71
939.000	32.52	31.37	30.33	29.29	28.45
954.000	27.61	26.88	26.22	25.59	25.05
969.000	24.52	24.06	23.61	23.20	22.82
984.000	22.45	22.12	21.80	21.50	21.22
999.000	20.94	20.69	20.44	20.21	19.98
1,014.000	19.76	19.55	19.35	19.15	18.96
1,029.000	18.78	18.60	18.42	18.25	18.09
1,044.000	17.93	17.77	17.62	17.47	17.32
1,059.000	17.18	17.04	16.91	16.77	16.64
1,074.000	16.52	16.39	16.26	16.13	15.97
1,089.000	15.82	15.63	15.43	15.20	14.94
1,104.000	14.68	14.39	14.09	13.80	13.51
1,119.000	13.23	12.96	12.69	12.47	12.24
1,134.000	12.05	11.87	11.70	11.55	11.40
1,149.000	11.28	11.16	11.04	10.93	10.83
1,164.000	10.74	10.64	10.56	10.47	10.39
1,179.000	10.32	10.24	10.17	10.10	10.03
1,194.000	9.97	9.90	9.84	9.78	9.72
1,209.000	9.66	9.61	9.55	9.49	9.44
1,224.000	9.39	9.34	9.28	9.23	9.18
1,239.000	9.13	9.09	9.04	8.99	8.94
1,254.000	8.90	8.85	8.81	8.76	8.72
1,269.000	8.68	8.63	8.59	8.55	8.51
1,284.000	8.47	8.43	8.39	8.35	8.31
1,299.000	8.27	8.24	8.20	8.16	8.12
1,314.000	8.09	8.05	8.01	7.98	7.94
1,329.000	7.91	7.87	7.84	7.80	7.77
1,344.000	7.74	7.70	7.67	7.64	7.61
1,359.000	7.57	7.54	7.51	7.48	7.45
1,374.000	7.42	7.39	7.36	7.33	7.30
1,389.000	7.27	7.24	7.21	7.18	7.15
1,404.000	7.13	7.10	7.07	7.04	7.02
1,419.000	6.99	6.97	6.94	6.91	6.89

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,434.000	6.86	6.83	6.79	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	742.793 min
Flow (Peak, Computed)	33.95 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	33.87 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	186,207.393 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	186,197.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
513.000	0.00	0.00	0.01	0.01	0.02
528.000	0.03	0.03	0.03	0.03	0.02
543.000	0.02	0.01	0.01	0.01	0.01
558.000	0.01	0.01	0.04	0.10	0.19
573.000	0.30	0.40	0.45	0.45	0.43
588.000	0.38	0.31	0.25	0.23	0.28
603.000	0.41	0.62	0.82	0.98	1.01
618.000	0.97	0.88	0.75	0.59	0.48
633.000	0.46	0.55	0.76	1.06	1.32
648.000	1.49	1.50	1.41	1.33	1.29
663.000	1.34	1.60	1.87	2.08	2.13
678.000	2.15	2.20	2.33	2.55	2.79
693.000	3.02	3.25	3.51	3.95	4.42
708.000	5.01	5.82	7.34	9.40	12.07
723.000	15.42	19.66	24.03	28.11	31.55
738.000	33.30	33.87	33.57	32.55	30.73
753.000	28.37	25.71	22.92	20.27	18.10
768.000	16.23	14.60	13.26	12.31	11.71
783.000	11.35	10.76	9.81	8.72	7.59
798.000	6.64	6.01	5.86	6.14	6.64
813.000	6.74	6.51	5.98	5.23	4.38
828.000	3.81	3.65	4.03	4.94	5.66
843.000	6.06	5.99	5.48	4.79	3.96
858.000	3.04	2.51	2.44	2.80	3.58
873.000	4.64	5.39	5.75	5.66	5.16
888.000	4.44	3.62	2.77	2.14	1.67
903.000	1.33	1.15	1.37	2.01	3.00
918.000	4.20	5.15	5.50	5.45	5.06
933.000	4.40	3.54	2.75	2.10	1.63
948.000	1.30	1.02	0.78	0.61	0.57
963.000	0.82	1.40	2.37	3.68	4.75
978.000	5.42	5.54	5.17	4.57	3.80
993.000	2.92	2.26	1.75	1.36	1.07
1,008.000	0.83	0.65	0.51	0.39	0.31

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,023.000	0.24	0.19	0.15	0.12	0.20
1,038.000	0.51	1.14	2.22	3.52	4.65
1,053.000	5.40	5.48	5.16	4.59	3.83
1,068.000	2.97	2.30	1.77	1.38	1.09
1,083.000	0.85	0.67	0.52	0.40	0.32
1,098.000	0.25	0.19	0.15	0.12	0.09
1,113.000	0.07	0.06	0.05	0.04	0.03
1,128.000	0.02	0.01	0.00	0.00	0.00
1,143.000	0.00	0.17	0.58	1.30	2.56
1,158.000	3.85	4.91	5.52	5.47	5.09
1,173.000	4.46	3.63	2.82	2.18	1.69
1,188.000	1.32	1.04	0.81	0.63	0.49
1,203.000	0.38	0.30	0.24	0.18	0.14
1,218.000	0.11	0.09	0.07	0.05	0.04
1,233.000	0.03	0.03	0.02	0.01	0.00
1,248.000	0.00	0.00	0.00	0.00	0.00
1,263.000	0.00	0.00	0.00	0.00	0.00
1,278.000	0.00	0.17	0.58	1.31	2.58
1,293.000	3.88	4.96	5.58	5.52	5.14
1,308.000	4.50	3.67	2.85	2.20	1.70
1,323.000	1.33	1.05	0.82	0.64	0.50
1,338.000	0.39	0.31	0.24	0.19	0.15
1,353.000	0.11	0.09	0.07	0.06	0.04
1,368.000	0.03	0.03	0.02	0.01	0.00
1,383.000	0.00	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	107.95 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	107.11 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	512,494.718 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	510,356.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.00	0.01	0.01
423.000	0.02	0.03	0.04	0.05	0.07
438.000	0.08	0.10	0.11	0.13	0.15
453.000	0.17	0.19	0.21	0.23	0.25
468.000	0.27	0.29	0.31	0.33	0.36
483.000	0.38	0.40	0.43	0.45	0.48
498.000	0.50	0.53	0.55	0.58	0.61
513.000	0.64	0.67	0.70	0.73	0.76
528.000	0.79	0.83	0.86	0.89	0.93
543.000	0.97	1.02	1.08	1.14	1.23
558.000	1.31	1.41	1.50	1.59	1.68
573.000	1.76	1.85	1.93	2.02	2.10
588.000	2.19	2.28	2.37	2.46	2.55
603.000	2.65	2.75	2.86	2.97	3.08
618.000	3.20	3.32	3.45	3.58	3.72
633.000	3.88	4.06	4.27	4.53	4.81
648.000	5.12	5.45	5.79	6.14	6.49
663.000	6.85	7.25	7.69	8.18	8.72
678.000	9.32	9.95	10.62	11.32	12.09
693.000	12.93	13.86	14.91	16.20	17.73
708.000	19.58	21.83	25.11	29.85	36.65
723.000	46.11	59.68	74.72	89.13	101.07
738.000	106.47	107.11	103.87	97.32	88.00
753.000	78.74	70.08	62.31	55.71	49.99
768.000	45.00	40.69	37.09	34.03	31.38
783.000	29.08	27.12	25.39	23.80	22.34
798.000	21.04	19.88	18.83	17.88	17.04
813.000	16.28	15.56	14.88	14.21	13.56
828.000	12.93	12.34	11.79	11.31	10.89
843.000	10.52	10.19	9.90	9.64	9.40
858.000	9.18	8.98	8.79	8.61	8.45
873.000	8.29	8.15	8.01	7.88	7.75
888.000	7.63	7.51	7.40	7.29	7.18
903.000	7.07	6.93	6.77	6.57	6.34

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA D
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	6.11	5.87	5.66	5.46	5.29
933.000	5.14	5.02	4.92	4.82	4.74
948.000	4.66	4.59	4.53	4.46	4.41
963.000	4.35	4.30	4.25	4.20	4.16
978.000	4.12	4.07	4.03	3.99	3.96
993.000	3.92	3.88	3.84	3.81	3.77
1,008.000	3.74	3.70	3.67	3.64	3.61
1,023.000	3.58	3.55	3.52	3.49	3.47
1,038.000	3.44	3.41	3.38	3.36	3.33
1,053.000	3.30	3.28	3.26	3.23	3.21
1,068.000	3.19	3.16	3.14	3.12	3.10
1,083.000	3.07	3.04	3.00	2.96	2.90
1,098.000	2.85	2.79	2.74	2.69	2.65
1,113.000	2.61	2.58	2.55	2.53	2.50
1,128.000	2.48	2.46	2.44	2.43	2.41
1,143.000	2.40	2.38	2.37	2.35	2.34
1,158.000	2.32	2.31	2.29	2.28	2.27
1,173.000	2.26	2.24	2.23	2.22	2.20
1,188.000	2.19	2.18	2.17	2.16	2.15
1,203.000	2.13	2.12	2.11	2.10	2.09
1,218.000	2.08	2.07	2.06	2.05	2.04
1,233.000	2.03	2.02	2.01	2.00	1.99
1,248.000	1.98	1.97	1.96	1.95	1.94
1,263.000	1.93	1.93	1.92	1.91	1.90
1,278.000	1.89	1.88	1.87	1.86	1.86
1,293.000	1.85	1.84	1.83	1.82	1.82
1,308.000	1.81	1.80	1.80	1.79	1.78
1,323.000	1.77	1.76	1.76	1.75	1.74
1,338.000	1.73	1.73	1.72	1.71	1.71
1,353.000	1.70	1.69	1.69	1.68	1.67
1,368.000	1.67	1.66	1.65	1.65	1.64
1,383.000	1.64	1.63	1.62	1.62	1.61
1,398.000	1.60	1.60	1.59	1.59	1.58
1,413.000	1.58	1.57	1.57	1.56	1.55
1,428.000	1.55	1.54	1.54	1.53	1.52

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	143.23 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	141.73 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	720,692.010 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	717,945.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
351.000	0.00	0.00	0.00	0.01	0.01
366.000	0.02	0.03	0.04	0.05	0.07
381.000	0.09	0.11	0.13	0.15	0.17
396.000	0.20	0.22	0.25	0.27	0.30
411.000	0.32	0.35	0.38	0.41	0.43
426.000	0.46	0.49	0.52	0.55	0.58
441.000	0.61	0.64	0.67	0.70	0.73
456.000	0.77	0.80	0.84	0.87	0.91
471.000	0.94	0.98	1.01	1.05	1.09
486.000	1.13	1.17	1.21	1.25	1.29
501.000	1.34	1.38	1.42	1.47	1.52
516.000	1.56	1.61	1.66	1.71	1.77
531.000	1.82	1.87	1.93	1.99	2.06
546.000	2.14	2.25	2.38	2.55	2.72
561.000	2.90	3.08	3.26	3.42	3.58
576.000	3.73	3.88	4.03	4.17	4.32
591.000	4.46	4.61	4.77	4.92	5.08
606.000	5.24	5.41	5.59	5.77	5.96
621.000	6.15	6.35	6.57	6.79	7.05
636.000	7.35	7.71	8.16	8.67	9.22
651.000	9.81	10.40	11.00	11.59	12.19
666.000	12.83	13.52	14.27	15.09	15.99
681.000	16.93	17.92	18.96	20.07	21.26
696.000	22.55	23.95	25.58	27.49	29.80
711.000	32.64	36.88	43.04	51.93	64.30
726.000	81.94	101.38	119.87	134.98	141.46
741.000	141.73	137.01	128.02	115.52	103.23
756.000	91.82	81.70	73.22	65.97	59.71
771.000	54.35	49.90	46.11	42.82	39.94
786.000	37.46	35.26	33.24	31.39	29.72
801.000	28.23	26.87	25.64	24.53	23.52
816.000	22.54	21.59	20.62	19.65	18.71
831.000	17.81	16.98	16.25	15.61	15.05
846.000	14.56	14.14	13.76	13.41	13.08

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
861.000	12.80	12.53	12.28	12.04	11.82
876.000	11.61	11.42	11.23	11.05	10.88
891.000	10.72	10.56	10.41	10.26	10.09
906.000	9.89	9.64	9.33	8.98	8.60
921.000	8.24	7.90	7.59	7.32	7.10
936.000	6.92	6.76	6.61	6.49	6.38
951.000	6.28	6.18	6.09	6.01	5.94
966.000	5.87	5.80	5.73	5.67	5.61
981.000	5.55	5.49	5.44	5.39	5.33
996.000	5.28	5.24	5.19	5.14	5.10
1,011.000	5.05	5.01	4.96	4.92	4.88
1,026.000	4.84	4.80	4.76	4.72	4.68
1,041.000	4.65	4.61	4.58	4.54	4.51
1,056.000	4.47	4.44	4.41	4.37	4.34
1,071.000	4.31	4.28	4.25	4.22	4.18
1,086.000	4.13	4.08	4.00	3.91	3.81
1,101.000	3.72	3.63	3.55	3.48	3.42
1,116.000	3.37	3.33	3.29	3.26	3.22
1,131.000	3.20	3.17	3.14	3.12	3.10
1,146.000	3.08	3.06	3.04	3.02	3.00
1,161.000	2.98	2.96	2.95	2.93	2.91
1,176.000	2.90	2.88	2.86	2.85	2.83
1,191.000	2.82	2.80	2.79	2.77	2.76
1,206.000	2.74	2.73	2.71	2.70	2.68
1,221.000	2.67	2.66	2.65	2.63	2.62
1,236.000	2.60	2.59	2.58	2.56	2.55
1,251.000	2.54	2.53	2.52	2.50	2.49
1,266.000	2.48	2.47	2.46	2.45	2.43
1,281.000	2.42	2.41	2.40	2.39	2.38
1,296.000	2.37	2.36	2.35	2.34	2.33
1,311.000	2.32	2.31	2.30	2.29	2.28
1,326.000	2.27	2.26	2.25	2.24	2.23
1,341.000	2.22	2.21	2.20	2.20	2.19
1,356.000	2.18	2.17	2.16	2.15	2.15
1,371.000	2.14	2.13	2.12	2.11	2.10
1,386.000	2.10	2.09	2.08	2.07	2.06
1,401.000	2.06	2.05	2.04	2.03	2.03
1,416.000	2.02	2.01	2.00	2.00	1.99
1,431.000	1.98	1.97	1.97	1.95	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	202.08 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	738.000 min
Flow (Peak Interpolated Output)	199.85 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	1,121,535.165 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,117,770.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.01	0.01	0.02
297.000	0.03	0.04	0.06	0.08	0.09
312.000	0.11	0.13	0.15	0.18	0.20
327.000	0.22	0.24	0.27	0.29	0.31
342.000	0.34	0.36	0.39	0.41	0.44
357.000	0.47	0.49	0.52	0.55	0.59
372.000	0.64	0.69	0.74	0.80	0.86
387.000	0.92	0.97	1.03	1.08	1.13
402.000	1.18	1.23	1.29	1.34	1.39
417.000	1.44	1.49	1.55	1.60	1.65
432.000	1.71	1.76	1.82	1.87	1.93
447.000	1.99	2.04	2.10	2.16	2.22
462.000	2.29	2.35	2.41	2.48	2.54
477.000	2.61	2.68	2.74	2.81	2.89
492.000	2.96	3.03	3.11	3.18	3.26
507.000	3.34	3.42	3.50	3.59	3.67
522.000	3.76	3.85	3.94	4.03	4.13
537.000	4.23	4.33	4.46	4.64	4.87
552.000	5.19	5.58	6.01	6.44	6.88
567.000	7.28	7.65	8.00	8.32	8.62
582.000	8.91	9.20	9.48	9.76	10.04
597.000	10.33	10.62	10.90	11.20	11.50
612.000	11.81	12.14	12.48	12.84	13.20
627.000	13.58	13.99	14.45	15.00	15.71
642.000	16.61	17.63	18.74	19.91	21.08
657.000	22.22	23.34	24.44	25.57	26.77
672.000	28.06	29.46	30.95	32.50	34.12
687.000	35.80	37.55	39.33	41.14	42.97
702.000	44.86	47.01	49.65	53.02	58.41
717.000	66.58	78.63	95.56	119.77	146.35
732.000	171.45	191.70	199.85	199.44	192.14
747.000	179.02	161.20	143.89	128.00	114.11
762.000	102.83	93.44	85.49	78.82	73.35
777.000	68.70	64.60	60.96	57.76	54.86

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA D
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
792.000	52.16	49.65	47.35	45.28	43.37
807.000	41.62	40.04	38.56	37.09	35.60
822.000	34.05	32.42	30.82	29.28	27.85
837.000	26.60	25.51	24.56	23.74	23.04
852.000	22.40	21.83	21.30	20.84	20.41
867.000	20.00	19.63	19.28	18.95	18.64
882.000	18.34	18.06	17.79	17.53	17.28
897.000	17.05	16.80	16.52	16.18	15.72
912.000	15.13	14.46	13.74	13.02	12.37
927.000	11.78	11.27	10.86	10.52	10.23
942.000	9.98	9.76	9.57	9.40	9.24
957.000	9.10	8.98	8.86	8.74	8.64
972.000	8.54	8.44	8.35	8.26	8.17
987.000	8.09	8.01	7.93	7.86	7.79
1,002.000	7.72	7.65	7.58	7.51	7.44
1,017.000	7.38	7.31	7.25	7.19	7.13
1,032.000	7.07	7.01	6.96	6.90	6.85
1,047.000	6.80	6.75	6.69	6.64	6.59
1,062.000	6.54	6.50	6.45	6.40	6.36
1,077.000	6.32	6.27	6.22	6.14	6.03
1,092.000	5.89	5.71	5.53	5.35	5.18
1,107.000	5.04	4.91	4.81	4.72	4.65
1,122.000	4.59	4.53	4.48	4.44	4.40
1,137.000	4.36	4.32	4.29	4.26	4.23
1,152.000	4.20	4.17	4.15	4.12	4.09
1,167.000	4.07	4.05	4.02	4.00	3.98
1,182.000	3.95	3.93	3.91	3.89	3.86
1,197.000	3.84	3.82	3.80	3.78	3.76
1,212.000	3.74	3.72	3.70	3.68	3.66
1,227.000	3.64	3.62	3.61	3.59	3.57
1,242.000	3.55	3.53	3.52	3.50	3.48
1,257.000	3.47	3.45	3.43	3.42	3.40
1,272.000	3.38	3.37	3.35	3.34	3.32
1,287.000	3.31	3.29	3.28	3.26	3.25
1,302.000	3.23	3.22	3.20	3.19	3.17
1,317.000	3.16	3.15	3.13	3.12	3.11
1,332.000	3.09	3.08	3.07	3.06	3.04
1,347.000	3.03	3.02	3.01	2.99	2.98
1,362.000	2.97	2.96	2.94	2.93	2.92
1,377.000	2.91	2.90	2.89	2.88	2.86
1,392.000	2.85	2.84	2.83	2.82	2.81
1,407.000	2.80	2.79	2.78	2.77	2.76
1,422.000	2.75	2.74	2.73	2.72	2.71

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,437.000	2.70	2.68	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres
Computational Time Increment	2.597 min
Time to Peak (Computed)	732.417 min
Flow (Peak, Computed)	41.62 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	732.000 min
Flow (Peak Interpolated Output)	41.25 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	32.100 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	189,693.981 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	189,674.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	19.479 min
Computational Time Increment	2.597 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	112.03 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	12.986 min
Unit receding limb, Tr	51.944 min
Total unit time, Tb	64.931 min

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
513.000	0.00	0.01	0.02	0.03	0.04
528.000	0.04	0.04	0.03	0.02	0.01
543.000	0.01	0.01	0.00	0.00	0.00
558.000	0.00	0.01	0.08	0.26	0.50
573.000	0.67	0.69	0.60	0.45	0.30
588.000	0.21	0.14	0.10	0.12	0.34
603.000	0.81	1.30	1.50	1.41	1.13
618.000	0.78	0.52	0.36	0.25	0.20
633.000	0.31	0.75	1.47	2.05	2.18
648.000	1.94	1.50	1.03	0.80	1.08
663.000	1.83	2.63	2.94	2.72	2.19
678.000	1.77	2.04	2.88	3.59	3.71
693.000	3.56	3.61	4.17	4.91	5.64
708.000	6.72	8.43	10.96	14.77	19.70
723.000	25.44	31.82	37.79	41.25	41.23
738.000	38.75	35.43	32.42	30.09	27.55
753.000	23.81	19.16	15.28	13.18	12.56
768.000	11.99	10.74	9.52	9.33	9.99
783.000	10.23	9.35	7.58	5.45	3.71
798.000	3.20	4.26	6.47	8.25	8.46
813.000	7.39	5.53	3.73	2.52	1.84
828.000	1.95	3.33	5.82	7.78	8.15
843.000	7.20	5.50	3.75	2.52	1.75
858.000	1.20	1.01	1.76	3.88	6.73
873.000	8.26	8.04	6.72	4.86	3.26
888.000	2.21	1.51	1.04	0.71	0.49
903.000	0.34	0.43	1.35	3.64	6.39
918.000	8.01	7.97	6.72	4.88	3.28
933.000	2.25	1.56	1.07	0.73	0.50
948.000	0.34	0.24	0.16	0.11	0.18
963.000	0.85	2.67	5.52	7.72	8.22
978.000	7.31	5.59	3.82	2.56	1.78
993.000	1.23	0.84	0.58	0.39	0.27
1,008.000	0.19	0.13	0.09	0.06	0.04

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,023.000	0.02	0.00	0.00	0.00	0.11
1,038.000	0.84	2.73	5.63	7.87	8.33
1,053.000	7.38	5.65	3.88	2.61	1.80
1,068.000	1.24	0.85	0.58	0.40	0.27
1,083.000	0.19	0.13	0.09	0.06	0.04
1,098.000	0.02	0.01	0.00	0.00	0.00
1,113.000	0.00	0.00	0.00	0.00	0.00
1,128.000	0.00	0.00	0.00	0.00	0.00
1,143.000	0.01	0.21	1.28	3.62	6.57
1,158.000	8.39	8.38	7.02	5.11	3.44
1,173.000	2.34	1.61	1.10	0.75	0.52
1,188.000	0.36	0.25	0.17	0.12	0.08
1,203.000	0.06	0.04	0.02	0.00	0.00
1,218.000	0.00	0.00	0.00	0.00	0.00
1,233.000	0.00	0.00	0.00	0.00	0.00
1,248.000	0.00	0.00	0.00	0.00	0.00
1,263.000	0.00	0.00	0.00	0.00	0.00
1,278.000	0.00	0.21	1.30	3.66	6.62
1,293.000	8.45	8.46	7.09	5.16	3.47
1,308.000	2.36	1.63	1.11	0.76	0.52
1,323.000	0.36	0.25	0.17	0.12	0.08
1,338.000	0.06	0.04	0.02	0.00	0.00

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres
Computational Time Increment	2.597 min
Time to Peak (Computed)	732.417 min
Flow (Peak, Computed)	136.35 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	735.000 min
Flow (Peak Interpolated Output)	135.22 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	32.100 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	522,090.758 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	520,812.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	19.479 min
Computational Time Increment	2.597 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	112.03 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	12.986 min
Unit receding limb, Tr	51.944 min
Total unit time, Tb	64.931 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.01	0.02	0.03
423.000	0.04	0.06	0.07	0.09	0.11
438.000	0.12	0.14	0.16	0.18	0.20
453.000	0.22	0.24	0.26	0.29	0.31
468.000	0.33	0.35	0.38	0.40	0.42
483.000	0.45	0.47	0.50	0.53	0.55
498.000	0.58	0.61	0.64	0.67	0.70
513.000	0.73	0.76	0.79	0.83	0.86
528.000	0.90	0.93	0.97	1.01	1.05
543.000	1.09	1.16	1.25	1.36	1.48
558.000	1.59	1.68	1.78	1.86	1.95
573.000	2.03	2.12	2.20	2.29	2.38
588.000	2.47	2.56	2.66	2.76	2.87
603.000	2.97	3.09	3.20	3.32	3.45
618.000	3.58	3.72	3.86	4.01	4.18
633.000	4.37	4.61	4.93	5.31	5.71
648.000	6.10	6.47	6.83	7.19	7.55
663.000	7.96	8.45	9.03	9.70	10.40
678.000	11.13	11.87	12.65	13.47	14.37
693.000	15.39	16.62	18.14	19.94	22.06
708.000	24.75	28.45	33.66	41.31	53.88
723.000	73.53	98.61	121.84	134.95	135.22
738.000	124.40	108.77	92.78	78.75	67.06
753.000	57.53	50.05	44.10	39.24	35.28
768.000	32.07	29.48	27.37	25.65	24.17
783.000	22.85	21.64	20.47	19.35	18.33
798.000	17.45	16.70	16.06	15.50	15.00
813.000	14.52	13.97	13.32	12.63	11.97
828.000	11.39	10.91	10.53	10.21	9.93
843.000	9.68	9.46	9.25	9.07	8.89
858.000	8.73	8.57	8.42	8.28	8.14
873.000	8.01	7.89	7.77	7.65	7.54
888.000	7.43	7.33	7.23	7.13	7.03
903.000	6.90	6.72	6.44	6.12	5.81

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	5.54	5.33	5.17	5.04	4.94
933.000	4.85	4.77	4.70	4.64	4.58
948.000	4.52	4.47	4.42	4.37	4.33
963.000	4.28	4.23	4.19	4.15	4.11
978.000	4.07	4.03	3.99	3.95	3.92
993.000	3.88	3.85	3.82	3.78	3.75
1,008.000	3.72	3.68	3.65	3.62	3.59
1,023.000	3.56	3.53	3.51	3.48	3.45
1,038.000	3.43	3.40	3.37	3.35	3.32
1,053.000	3.30	3.27	3.25	3.23	3.20
1,068.000	3.18	3.16	3.14	3.12	3.09
1,083.000	3.07	3.02	2.95	2.88	2.80
1,098.000	2.74	2.69	2.64	2.61	2.58
1,113.000	2.56	2.54	2.52	2.50	2.48
1,128.000	2.47	2.45	2.44	2.42	2.41
1,143.000	2.40	2.38	2.37	2.35	2.34
1,158.000	2.33	2.31	2.30	2.29	2.27
1,173.000	2.26	2.25	2.24	2.22	2.21
1,188.000	2.20	2.19	2.18	2.17	2.15
1,203.000	2.14	2.13	2.12	2.11	2.10
1,218.000	2.09	2.08	2.07	2.06	2.05
1,233.000	2.04	2.03	2.02	2.01	2.00
1,248.000	1.99	1.98	1.97	1.97	1.96
1,263.000	1.95	1.94	1.93	1.92	1.91
1,278.000	1.90	1.89	1.89	1.88	1.87
1,293.000	1.86	1.85	1.84	1.84	1.83
1,308.000	1.82	1.82	1.81	1.80	1.79
1,323.000	1.78	1.77	1.77	1.76	1.75
1,338.000	1.75	1.74	1.73	1.73	1.72
1,353.000	1.71	1.71	1.70	1.69	1.69
1,368.000	1.68	1.67	1.67	1.66	1.65
1,383.000	1.65	1.64	1.63	1.63	1.62
1,398.000	1.62	1.61	1.61	1.60	1.60
1,413.000	1.59	1.59	1.58	1.57	1.57
1,428.000	1.56	1.56	1.55	1.55	1.53

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres
Computational Time Increment	2.597 min
Time to Peak (Computed)	732.417 min
Flow (Peak, Computed)	180.41 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	732.000 min
Flow (Peak Interpolated Output)	178.71 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	32.100 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	734,186.401 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	732,543.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	19.479 min
Computational Time Increment	2.597 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	112.03 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	12.986 min
Unit receding limb, Tr	51.944 min
Total unit time, Tb	64.931 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
351.000	0.00	0.00	0.01	0.01	0.02
366.000	0.04	0.05	0.07	0.09	0.12
381.000	0.14	0.17	0.19	0.22	0.24
396.000	0.27	0.29	0.32	0.35	0.37
411.000	0.40	0.43	0.46	0.49	0.52
426.000	0.55	0.58	0.61	0.64	0.67
441.000	0.70	0.74	0.77	0.80	0.84
456.000	0.87	0.91	0.94	0.98	1.02
471.000	1.06	1.10	1.14	1.18	1.22
486.000	1.26	1.30	1.34	1.39	1.43
501.000	1.48	1.53	1.57	1.62	1.67
516.000	1.73	1.78	1.83	1.89	1.94
531.000	2.00	2.06	2.12	2.18	2.27
546.000	2.40	2.59	2.81	3.05	3.27
561.000	3.46	3.63	3.79	3.93	4.08
576.000	4.22	4.36	4.51	4.65	4.80
591.000	4.96	5.12	5.28	5.45	5.62
606.000	5.80	5.99	6.18	6.38	6.59
621.000	6.80	7.03	7.27	7.52	7.84
636.000	8.27	8.85	9.55	10.27	10.97
651.000	11.61	12.22	12.80	13.39	14.04
666.000	14.78	15.66	16.65	17.69	18.76
681.000	19.85	20.97	22.16	23.45	24.87
696.000	26.46	28.30	30.41	32.95	36.29
711.000	41.09	48.00	58.11	74.61	100.24
726.000	132.74	162.49	178.71	178.13	163.26
741.000	142.36	121.21	102.75	87.39	74.92
756.000	65.32	57.94	52.08	47.38	43.57
771.000	40.45	37.87	35.72	33.84	32.15
786.000	30.58	29.09	27.65	26.34	25.19
801.000	24.20	23.35	22.60	21.93	21.24
816.000	20.43	19.42	18.33	17.28	16.37
831.000	15.63	15.05	14.56	14.15	13.79
846.000	13.47	13.17	12.90	12.65	12.42

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
861.000	12.20	11.99	11.79	11.60	11.42
876.000	11.24	11.08	10.92	10.76	10.61
891.000	10.46	10.32	10.19	10.05	9.86
906.000	9.57	9.14	8.62	8.12	7.70
921.000	7.36	7.11	6.92	6.76	6.63
936.000	6.52	6.41	6.32	6.24	6.17
951.000	6.09	6.02	5.96	5.89	5.83
966.000	5.77	5.71	5.65	5.60	5.54
981.000	5.49	5.44	5.39	5.34	5.29
996.000	5.24	5.20	5.15	5.11	5.06
1,011.000	5.02	4.98	4.94	4.89	4.85
1,026.000	4.81	4.78	4.74	4.70	4.67
1,041.000	4.63	4.60	4.56	4.53	4.49
1,056.000	4.46	4.43	4.40	4.37	4.33
1,071.000	4.30	4.27	4.24	4.21	4.17
1,086.000	4.10	3.98	3.85	3.72	3.61
1,101.000	3.52	3.45	3.40	3.36	3.32
1,116.000	3.29	3.26	3.24	3.22	3.20
1,131.000	3.18	3.15	3.13	3.11	3.09
1,146.000	3.07	3.06	3.04	3.02	3.01
1,161.000	2.99	2.97	2.95	2.94	2.92
1,176.000	2.90	2.89	2.87	2.86	2.84
1,191.000	2.83	2.81	2.80	2.78	2.77
1,206.000	2.75	2.74	2.72	2.71	2.70
1,221.000	2.68	2.67	2.66	2.64	2.63
1,236.000	2.62	2.60	2.59	2.58	2.57
1,251.000	2.55	2.54	2.53	2.52	2.50
1,266.000	2.49	2.48	2.47	2.46	2.45
1,281.000	2.44	2.43	2.42	2.41	2.40
1,296.000	2.39	2.38	2.37	2.36	2.34
1,311.000	2.33	2.32	2.31	2.31	2.30
1,326.000	2.29	2.28	2.27	2.26	2.24
1,341.000	2.24	2.23	2.22	2.22	2.21
1,356.000	2.20	2.19	2.18	2.17	2.16
1,371.000	2.15	2.15	2.14	2.13	2.12
1,386.000	2.11	2.10	2.09	2.09	2.08
1,401.000	2.07	2.07	2.06	2.05	2.04
1,416.000	2.03	2.03	2.02	2.01	2.01
1,431.000	2.00	1.99	1.99	1.97	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres
Computational Time Increment	2.597 min
Time to Peak (Computed)	732.417 min
Flow (Peak, Computed)	253.67 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	732.000 min
Flow (Peak Interpolated Output)	251.49 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	32.100 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	1,142,534.980 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,140,280.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	19.479 min
Computational Time Increment	2.597 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	112.03 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	12.986 min
Unit receding limb, Tr	51.944 min
Total unit time, Tb	64.931 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	19.479 min
Area (User Defined)	32.100 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
279.000	0.00	0.00	0.01	0.02	0.03
294.000	0.04	0.06	0.08	0.10	0.12
309.000	0.14	0.17	0.19	0.21	0.24
324.000	0.26	0.29	0.31	0.34	0.36
339.000	0.39	0.41	0.44	0.47	0.49
354.000	0.52	0.55	0.58	0.61	0.65
369.000	0.71	0.78	0.85	0.91	0.98
384.000	1.03	1.09	1.14	1.19	1.25
399.000	1.30	1.35	1.40	1.45	1.51
414.000	1.56	1.61	1.67	1.72	1.78
429.000	1.83	1.89	1.95	2.00	2.06
444.000	2.12	2.18	2.24	2.31	2.37
459.000	2.43	2.50	2.57	2.63	2.70
474.000	2.77	2.84	2.91	2.98	3.06
489.000	3.13	3.21	3.29	3.37	3.45
504.000	3.54	3.62	3.71	3.80	3.89
519.000	3.98	4.07	4.17	4.27	4.37
534.000	4.47	4.58	4.69	4.85	5.15
549.000	5.63	6.20	6.79	7.33	7.79
564.000	8.16	8.49	8.80	9.08	9.36
579.000	9.63	9.91	10.18	10.46	10.74
594.000	11.02	11.32	11.62	11.90	12.18
609.000	12.50	12.89	13.29	13.69	14.09
624.000	14.47	14.87	15.31	15.88	16.73
639.000	17.93	19.38	20.87	22.27	23.52
654.000	24.65	25.71	26.76	27.86	29.12
669.000	30.60	32.23	33.94	35.67	37.41
684.000	39.19	41.06	43.05	45.07	47.01
699.000	48.81	50.74	53.25	57.10	63.27
714.000	72.68	86.69	109.58	145.02	189.75
729.000	230.20	251.49	249.42	227.75	198.02
744.000	168.26	142.39	120.92	103.69	90.86
759.000	81.64	74.81	69.56	65.32	61.72
774.000	58.60	55.88	53.42	51.15	49.01

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
789.000	46.91	44.86	42.97	41.30	39.85
804.000	38.59	37.47	36.44	35.37	34.01
819.000	32.26	30.31	28.44	26.81	25.51
834.000	24.51	23.69	23.00	22.40	21.88
849.000	21.41	20.98	20.58	20.22	19.87
864.000	19.54	19.23	18.93	18.64	18.37
879.000	18.11	17.85	17.61	17.37	17.14
894.000	16.92	16.71	16.48	16.16	15.61
909.000	14.77	13.75	12.76	11.92	11.28
924.000	10.80	10.44	10.16	9.93	9.74
939.000	9.57	9.43	9.30	9.18	9.07
954.000	8.96	8.86	8.76	8.67	8.58
969.000	8.50	8.41	8.33	8.25	8.17
984.000	8.09	8.02	7.94	7.87	7.80
999.000	7.73	7.66	7.59	7.53	7.46
1,014.000	7.40	7.34	7.27	7.21	7.15
1,029.000	7.10	7.04	6.99	6.94	6.88
1,044.000	6.83	6.78	6.73	6.68	6.62
1,059.000	6.57	6.53	6.48	6.44	6.40
1,074.000	6.36	6.32	6.27	6.19	6.05
1,089.000	5.84	5.58	5.33	5.11	4.95
1,104.000	4.83	4.74	4.67	4.62	4.57
1,119.000	4.53	4.49	4.45	4.42	4.39
1,134.000	4.36	4.33	4.30	4.28	4.25
1,149.000	4.22	4.20	4.17	4.15	4.12
1,164.000	4.10	4.08	4.05	4.03	4.01
1,179.000	3.99	3.96	3.94	3.92	3.90
1,194.000	3.88	3.86	3.83	3.81	3.79
1,209.000	3.77	3.76	3.74	3.72	3.70
1,224.000	3.68	3.66	3.64	3.62	3.60
1,239.000	3.58	3.56	3.55	3.53	3.52
1,254.000	3.50	3.48	3.47	3.45	3.43
1,269.000	3.42	3.40	3.39	3.37	3.36
1,284.000	3.34	3.33	3.31	3.30	3.28
1,299.000	3.27	3.25	3.24	3.22	3.21
1,314.000	3.20	3.18	3.17	3.15	3.14
1,329.000	3.13	3.11	3.10	3.09	3.08
1,344.000	3.07	3.05	3.04	3.02	3.01
1,359.000	3.00	2.99	2.98	2.97	2.95
1,374.000	2.94	2.93	2.92	2.91	2.90
1,389.000	2.89	2.87	2.86	2.85	2.84
1,404.000	2.84	2.83	2.81	2.80	2.79
1,419.000	2.78	2.77	2.76	2.75	2.74

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,434.000	2.73	2.72	2.70	(N/A)	(N/A)

Subsection: Channel Equations
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{2/3}) * (S^{1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

**Solution to Manning's Open Channel Flow Equation
 (Computed values are based on normal depth.)**

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.497 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	61.42 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.497 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	61.42 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	137.84 ft ³ /s	Time to Peak (In)	753.000 min
Flow (Peak Out)	136.25 ft ³ /s	Time to Peak (Out)	756.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	954,451.096 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	954,334.200 ft ³
Volume (Retained)	111.785 ft ³
Volume (Unrouted)	-5.111 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Peak Discharge	136.25 ft ³ /s
Time to Peak	756.000 min
Hydrograph Volume	954,334.036 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
519.000	0.00	0.00	0.00	0.01	0.01
534.000	0.01	0.01	0.02	0.02	0.05
549.000	0.08	0.10	0.10	0.09	0.09
564.000	0.12	0.18	0.30	0.46	0.67
579.000	0.90	1.12	1.30	1.43	1.50
594.000	1.51	1.53	1.59	1.73	1.95
609.000	2.26	2.64	3.00	3.31	3.52
624.000	3.60	3.58	3.50	3.41	3.40
639.000	3.50	3.74	4.10	4.49	4.87
654.000	5.20	5.49	5.77	6.08	6.46
669.000	6.97	7.56	8.15	8.68	9.17
684.000	9.64	10.15	10.79	11.54	12.37
699.000	13.27	14.28	15.46	16.95	18.93
714.000	21.74	25.76	31.16	38.38	47.43
729.000	58.31	70.60	83.58	96.64	108.51
744.000	118.70	126.83	132.36	135.51	136.25
759.000	134.92	131.56	126.56	120.40	113.27
774.000	105.99	98.92	92.25	86.12	80.41
789.000	74.98	69.75	64.76	60.09	55.84
804.000	52.05	48.93	46.53	44.50	42.64
819.000	40.66	38.39	35.93	33.52	31.39
834.000	29.86	28.98	28.69	28.80	28.87
849.000	28.67	28.10	27.06	25.67	24.19
864.000	22.86	21.97	21.60	21.74	22.29
879.000	22.91	23.41	23.64	23.44	22.80
894.000	21.75	20.41	18.91	17.30	15.80
909.000	14.57	13.83	13.79	14.44	15.81
924.000	17.47	18.94	20.09	20.63	20.47
939.000	19.78	18.69	17.40	15.98	14.46
954.000	12.82	11.15	9.74	8.73	8.35
969.000	8.72	9.85	11.64	13.65	15.57
984.000	17.10	18.08	18.52	18.37	17.75
999.000	16.83	15.68	14.39	12.95	11.46
1,014.000	9.97	8.57	7.32	6.24	5.32
1,029.000	4.57	3.94	3.44	3.28	3.64
1,044.000	4.59	6.17	8.26	10.69	13.09
1,059.000	15.11	16.66	17.48	17.61	17.23
1,074.000	16.45	15.46	14.30	12.99	11.54
1,089.000	10.02	8.60	7.32	6.23	5.32

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,104.000	4.56	3.94	3.40	2.92	2.50
1,119.000	2.15	1.84	1.58	1.35	1.15
1,134.000	0.99	0.84	0.73	0.70	0.85
1,149.000	1.33	2.29	3.81	5.94	8.46
1,164.000	11.04	13.46	15.32	16.52	17.10
1,179.000	17.03	16.52	15.69	14.64	13.41
1,194.000	12.05	10.63	9.19	7.86	6.71
1,209.000	5.72	4.90	4.21	3.63	3.12
1,224.000	2.69	2.30	1.98	1.70	1.46
1,239.000	1.26	1.08	0.93	0.79	0.68
1,254.000	0.59	0.51	0.44	0.38	0.33
1,269.000	0.28	0.24	0.21	0.19	0.31
1,284.000	0.81	1.80	3.38	5.52	8.15
1,299.000	10.98	13.49	15.52	16.86	17.40
1,314.000	17.34	16.77	15.92	14.87	13.66
1,329.000	12.27	10.72	9.23	7.88	6.70
1,344.000	5.72	4.89	4.21	3.64	3.14
1,359.000	2.69	2.30	1.98	1.71	1.47
1,374.000	1.26	1.08	0.93	0.79	0.68
1,389.000	0.59	0.51	0.44	0.38	0.33
1,404.000	0.28	0.24	0.21	0.18	0.16
1,419.000	0.14	0.12	0.10	0.09	0.08
1,434.000	0.06	0.05	0.03	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.213 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	189.10 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.213 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	189.10 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	413.45 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	411.54 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	2,602,299.147 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	2,600,538.365 ft ³
Volume (Retained)	763.654 ft ³
Volume (Unrouted)	-997.128 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Peak Discharge	411.54 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	2,600,538.211 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
414.000	0.00	0.00	0.00	0.00	0.01
429.000	0.01	0.01	0.02	0.12	0.23
444.000	0.30	0.38	0.45	0.52	0.60
459.000	0.68	0.76	0.85	0.94	1.03
474.000	1.13	1.23	1.33	1.43	1.54
489.000	1.65	1.76	1.88	1.99	2.11
504.000	2.24	2.36	2.49	2.62	2.75
519.000	2.89	3.03	3.17	3.32	3.47
534.000	3.62	3.78	3.95	4.12	4.30
549.000	4.50	4.72	4.96	5.24	5.54
564.000	5.87	6.22	6.59	6.97	7.37
579.000	7.77	8.18	8.60	9.02	9.44
594.000	9.87	10.31	10.75	11.20	11.66
609.000	12.13	12.61	13.11	13.63	14.16
624.000	14.71	15.27	15.87	16.50	17.17
639.000	17.91	18.72	19.63	20.64	21.75
654.000	22.94	24.23	25.60	27.05	28.59
669.000	30.25	32.04	33.98	36.07	38.33
684.000	40.78	43.39	46.21	49.24	52.57
699.000	56.19	60.27	64.90	70.31	76.72
714.000	84.78	95.39	109.48	128.88	154.27
729.000	199.26	240.73	282.59	323.44	357.65
744.000	384.46	402.72	410.86	411.54	405.17
759.000	393.85	378.26	359.27	338.00	314.70
774.000	292.12	270.62	250.59	232.56	216.18
789.000	201.44	187.97	175.66	164.30	154.17
804.000	147.44	139.66	131.96	124.61	117.79
819.000	111.45	105.58	100.12	95.03	90.26
834.000	85.80	81.65	77.78	74.20	70.88
849.000	67.83	65.01	62.43	60.05	57.87
864.000	55.88	54.04	52.35	50.77	49.31
879.000	47.94	46.67	45.50	44.43	43.45
894.000	42.55	41.71	40.91	40.15	39.40
909.000	38.64	37.85	37.02	36.14	35.22
924.000	34.27	33.31	32.36	31.43	30.53
939.000	29.68	28.88	28.13	27.43	26.79
954.000	26.20	25.66	25.16	24.70	24.28
969.000	23.88	23.51	23.16	22.83	22.51
984.000	22.22	21.93	21.66	21.41	21.16

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
999.000	20.92	20.69	20.47	20.26	20.06
1,014.000	19.86	19.66	19.47	19.28	19.10
1,029.000	18.93	18.76	18.59	18.42	18.27
1,044.000	18.11	17.96	17.81	17.66	17.52
1,059.000	17.38	17.24	17.11	16.97	16.85
1,074.000	16.72	16.59	16.47	16.34	16.21
1,089.000	16.07	15.92	15.75	15.57	15.37
1,104.000	15.16	14.95	14.73	14.52	14.31
1,119.000	14.11	13.92	13.74	13.57	13.41
1,134.000	13.26	13.13	13.00	12.87	12.76
1,149.000	12.65	12.55	12.45	12.35	12.26
1,164.000	12.18	12.09	12.01	11.94	11.86
1,179.000	11.79	11.72	11.65	11.58	11.51
1,194.000	11.44	11.38	11.31	11.25	11.18
1,209.000	11.12	11.06	11.00	10.94	10.88
1,224.000	10.82	10.77	10.71	10.65	10.60
1,239.000	10.54	10.49	10.44	10.38	10.33
1,254.000	10.28	10.23	10.18	10.13	10.08
1,269.000	10.03	9.99	9.94	9.89	9.85
1,284.000	9.80	9.76	9.72	9.67	9.63
1,299.000	9.59	9.54	9.50	9.46	9.42
1,314.000	9.38	9.33	9.29	9.25	9.21
1,329.000	9.17	9.14	9.10	9.06	9.02
1,344.000	8.98	8.94	8.90	8.87	8.83
1,359.000	8.79	8.76	8.72	8.69	8.66
1,374.000	8.62	8.59	8.55	8.52	8.49
1,389.000	8.46	8.42	8.39	8.36	8.33
1,404.000	8.30	8.27	8.24	8.21	8.18
1,419.000	8.15	8.12	8.09	8.06	8.03
1,434.000	8.00	7.97	7.94	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.932 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	247.15 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.932 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	247.15 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	550.18 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	547.87 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	3,658,953.230 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	3,656,723.092 ft ³
Volume (Retained)	949.550 ft ³
Volume (Unrouted)	-1,280.588 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Peak Discharge	547.87 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	3,656,722.982 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
357.000	0.00	0.00	0.00	0.00	0.01
372.000	0.01	0.01	0.02	0.11	0.23
387.000	0.32	0.41	0.50	0.59	0.68
402.000	0.78	0.89	1.00	1.11	1.23
417.000	1.35	1.47	1.60	1.73	1.86
432.000	1.99	2.13	2.27	2.41	2.56
447.000	2.70	2.85	3.01	3.16	3.32
462.000	3.48	3.64	3.80	3.97	4.14
477.000	4.31	4.49	4.67	4.85	5.04
492.000	5.23	5.42	5.62	5.82	6.02
507.000	6.23	6.44	6.66	6.88	7.10
522.000	7.33	7.56	7.80	8.05	8.30
537.000	8.55	8.82	9.10	9.40	9.74
552.000	10.13	10.58	11.08	11.65	12.27
567.000	12.94	13.64	14.37	15.10	15.85
582.000	16.61	17.36	18.12	18.87	19.62
597.000	20.38	21.13	21.89	22.67	23.45
612.000	24.26	25.08	25.92	26.79	27.68
627.000	28.60	29.56	30.57	31.67	32.88
642.000	34.24	35.77	37.49	39.40	41.46
657.000	43.68	46.04	48.52	51.13	53.89
672.000	56.83	59.96	63.30	66.85	70.63
687.000	74.63	78.89	83.41	88.27	93.46
702.000	99.15	105.46	112.68	121.11	131.63
717.000	145.47	167.59	200.02	237.28	282.83
732.000	334.33	387.79	439.90	483.17	516.61
747.000	538.83	547.87	547.24	537.58	521.63
762.000	500.42	475.01	446.95	416.55	387.28
777.000	359.61	333.95	310.94	290.08	271.35
792.000	254.26	238.63	224.22	211.07	199.16
807.000	188.17	178.17	168.87	160.33	153.02
822.000	147.34	140.90	134.37	128.01	121.92
837.000	116.17	110.74	105.69	100.98	96.63
852.000	92.60	88.90	85.50	82.38	79.53
867.000	76.91	74.50	72.27	70.20	68.27
882.000	66.48	64.83	63.32	61.94	60.66
897.000	59.48	58.35	57.27	56.20	55.11
912.000	53.96	52.72	51.41	50.01	48.57
927.000	47.10	45.63	44.20	42.82	41.52

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
942.000	40.28	39.13	38.07	37.09	36.20
957.000	35.39	34.65	33.96	33.33	32.75
972.000	32.21	31.71	31.23	30.79	30.37
987.000	29.97	29.59	29.22	28.88	28.54
1,002.000	28.23	27.92	27.62	27.34	27.06
1,017.000	26.79	26.53	26.28	26.03	25.79
1,032.000	25.55	25.32	25.10	24.88	24.67
1,047.000	24.46	24.26	24.06	23.87	23.68
1,062.000	23.49	23.31	23.13	22.95	22.78
1,077.000	22.60	22.43	22.26	22.07	21.87
1,092.000	21.64	21.39	21.10	20.79	20.45
1,107.000	20.10	19.75	19.40	19.06	18.73
1,122.000	18.42	18.13	17.85	17.60	17.37
1,137.000	17.16	16.97	16.79	16.62	16.46
1,152.000	16.31	16.17	16.03	15.91	15.79
1,167.000	15.67	15.56	15.45	15.35	15.24
1,182.000	15.15	15.05	14.96	14.87	14.78
1,197.000	14.69	14.60	14.52	14.44	14.35
1,212.000	14.27	14.19	14.11	14.04	13.96
1,227.000	13.89	13.81	13.74	13.67	13.60
1,242.000	13.53	13.46	13.39	13.33	13.26
1,257.000	13.19	13.13	13.06	13.00	12.93
1,272.000	12.87	12.81	12.75	12.69	12.63
1,287.000	12.57	12.52	12.46	12.40	12.35
1,302.000	12.29	12.24	12.18	12.13	12.08
1,317.000	12.02	11.97	11.91	11.86	11.81
1,332.000	11.76	11.70	11.65	11.60	11.56
1,347.000	11.51	11.46	11.41	11.37	11.32
1,362.000	11.28	11.23	11.19	11.14	11.10
1,377.000	11.05	11.01	10.97	10.92	10.88
1,392.000	10.84	10.79	10.75	10.71	10.67
1,407.000	10.63	10.59	10.55	10.51	10.47
1,422.000	10.43	10.39	10.35	10.31	10.28
1,437.000	10.24	10.19	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.676 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	343.38 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.676 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	343.38 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	780.49 ft ³ /s	Time to Peak (In)	747.000 min
Flow (Peak Out)	780.16 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	5,693,809.707 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	5,690,791.034 ft ³
Volume (Retained)	1,261.863 ft ³
Volume (Unrouted)	-1,756.809 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Peak Discharge	780.16 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	5,690,790.942 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
285.000	0.00	0.00	0.00	0.00	0.01
300.000	0.01	0.02	0.02	0.15	0.26
315.000	0.35	0.43	0.51	0.60	0.69
330.000	0.79	0.89	0.99	1.10	1.21
345.000	1.32	1.43	1.55	1.67	1.79
360.000	1.92	2.05	2.18	2.33	2.48
375.000	2.65	2.84	3.04	3.26	3.49
390.000	3.73	3.97	4.22	4.48	4.74
405.000	5.00	5.26	5.52	5.78	6.04
420.000	6.30	6.56	6.83	7.09	7.36
435.000	7.63	7.90	8.17	8.45	8.73
450.000	9.01	9.29	9.58	9.87	10.16
465.000	10.46	10.77	11.07	11.38	11.70
480.000	12.02	12.34	12.67	13.01	13.35
495.000	13.70	14.05	14.40	14.77	15.14
510.000	15.51	15.90	16.29	16.68	17.09
525.000	17.50	17.92	18.35	18.79	19.24
540.000	19.71	20.21	20.78	21.44	22.23
555.000	23.18	24.29	25.56	26.97	28.50
570.000	30.10	31.75	33.41	35.07	36.73
585.000	38.37	39.99	41.57	43.12	44.64
600.000	46.15	47.65	49.14	50.65	52.17
615.000	53.71	55.27	56.87	58.50	60.17
630.000	61.91	63.74	65.75	67.99	70.54
645.000	73.48	76.80	80.50	84.53	88.85
660.000	93.41	98.17	103.12	108.28	113.69
675.000	119.35	125.31	131.54	138.06	144.87
690.000	151.97	161.30	170.48	178.75	187.22
705.000	196.16	206.17	217.70	232.44	252.41
720.000	279.37	317.35	366.75	428.18	497.76
735.000	569.77	639.59	705.85	747.54	773.16
750.000	780.16	774.78	757.39	732.33	700.87
765.000	664.36	627.24	589.49	550.70	514.05
780.000	480.25	450.16	422.98	398.64	376.46
795.000	356.10	337.25	319.92	304.09	289.37
810.000	275.83	263.14	251.34	240.17	229.58
825.000	219.42	209.61	200.12	191.03	182.35
840.000	174.13	166.45	159.27	153.22	148.49
855.000	143.25	138.07	133.20	128.70	124.55

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
870.000	120.75	117.22	113.96	110.92	108.11
885.000	105.51	103.14	100.97	98.96	97.08
900.000	95.30	93.57	91.83	90.02	88.07
915.000	85.91	83.56	81.03	78.38	75.66
930.000	72.93	70.27	67.70	65.27	62.98
945.000	60.85	58.90	57.12	55.51	54.06
960.000	52.75	51.56	50.47	49.48	48.56
975.000	47.72	46.93	46.20	45.51	44.86
990.000	44.25	43.67	43.12	42.60	42.10
1,005.000	41.62	41.16	40.72	40.30	39.88
1,020.000	39.48	39.10	38.72	38.35	38.00
1,035.000	37.65	37.31	36.98	36.66	36.35
1,050.000	36.04	35.75	35.45	35.17	34.89
1,065.000	34.61	34.35	34.08	33.83	33.57
1,080.000	33.32	33.05	32.77	32.45	32.07
1,095.000	31.64	31.14	30.58	29.98	29.35
1,110.000	28.71	28.08	27.46	26.87	26.32
1,125.000	25.80	25.32	24.88	24.48	24.12
1,140.000	23.80	23.50	23.22	22.97	22.73
1,155.000	22.51	22.30	22.10	21.91	21.73
1,170.000	21.56	21.40	21.25	21.10	20.95
1,185.000	20.81	20.68	20.54	20.41	20.29
1,200.000	20.16	20.04	19.92	19.80	19.69
1,215.000	19.57	19.46	19.35	19.25	19.14
1,230.000	19.04	18.94	18.84	18.74	18.64
1,245.000	18.54	18.45	18.35	18.26	18.17
1,260.000	18.08	17.99	17.90	17.81	17.72
1,275.000	17.64	17.55	17.47	17.39	17.30
1,290.000	17.22	17.14	17.06	16.99	16.91
1,305.000	16.83	16.76	16.68	16.61	16.53
1,320.000	16.46	16.38	16.31	16.23	16.16
1,335.000	16.09	16.02	15.95	15.88	15.81
1,350.000	15.74	15.68	15.61	15.54	15.48
1,365.000	15.42	15.35	15.29	15.23	15.17
1,380.000	15.11	15.05	14.99	14.93	14.87
1,395.000	14.81	14.75	14.69	14.64	14.58
1,410.000	14.53	14.47	14.42	14.36	14.31
1,425.000	14.26	14.20	14.15	14.10	14.05
1,440.000	13.98	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.866 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	101.60 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.866 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	101.60 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	228.61 ft ³ /s	Time to Peak (In)	753.000 min
Flow (Peak Out)	228.17 ft ³ /s	Time to Peak (Out)	756.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	1,576,271.367 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	1,576,136.777 ft ³
Volume (Retained)	127.694 ft ³
Volume (Unrouted)	-6.897 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Peak Discharge	228.17 ft ³ /s
Time to Peak	756.000 min
Hydrograph Volume	1,576,136.628 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
519.000	0.00	0.00	0.00	0.00	0.01
534.000	0.01	0.01	0.01	0.02	0.02
549.000	0.02	0.03	0.08	0.12	0.13
564.000	0.17	0.25	0.40	0.63	0.93
579.000	1.28	1.65	1.97	2.23	2.39
594.000	2.46	2.50	2.57	2.71	2.96
609.000	3.35	3.89	4.49	5.07	5.53
624.000	5.81	5.91	5.86	5.74	5.67
639.000	5.70	5.89	6.28	6.87	7.54
654.000	8.18	8.75	9.27	9.78	10.33
669.000	11.01	11.86	12.80	13.74	14.64
684.000	15.48	16.35	17.36	18.55	19.88
699.000	21.30	22.83	24.58	26.79	29.73
714.000	33.79	39.44	47.16	57.42	70.69
729.000	86.98	105.93	126.75	148.25	169.13
744.000	194.71	210.03	220.34	226.33	228.17
759.000	226.14	220.56	211.90	201.21	189.08
774.000	176.68	166.29	156.91	146.67	136.84
789.000	127.68	119.02	110.79	102.96	95.59
804.000	88.87	82.97	78.21	74.54	71.33
819.000	68.21	64.83	61.05	57.24	53.67
834.000	50.68	48.51	47.25	46.82	46.79
849.000	46.78	46.35	45.25	43.54	41.36
864.000	39.14	37.22	35.87	35.21	35.37
879.000	36.25	37.31	38.22	38.60	38.24
894.000	37.13	35.34	33.05	30.40	27.73
909.000	25.33	23.56	22.76	23.12	24.74
924.000	27.16	29.80	32.07	33.54	34.01
939.000	33.51	32.20	30.29	27.95	25.27
954.000	22.43	19.59	17.05	15.15	14.10
969.000	14.10	15.26	17.57	20.67	24.00
984.000	27.03	29.29	30.60	30.95	30.40
999.000	29.14	27.33	25.07	22.53	19.91
1,014.000	17.35	14.96	12.82	10.96	9.36
1,029.000	8.00	6.87	6.04	5.62	5.86
1,044.000	6.94	8.98	11.93	15.58	19.57
1,059.000	23.27	26.30	28.35	29.28	29.21
1,074.000	28.30	26.76	24.77	22.48	19.98
1,089.000	17.46	15.06	12.89	11.00	9.38

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,104.000	8.01	6.85	5.87	5.03	4.31
1,119.000	3.68	3.15	2.69	2.30	1.96
1,134.000	1.68	1.43	1.23	1.13	1.22
1,149.000	1.74	2.91	4.87	7.74	11.42
1,164.000	15.68	19.92	23.66	26.47	28.17
1,179.000	28.79	28.44	27.35	25.69	23.61
1,194.000	21.21	18.71	16.26	13.97	11.95
1,209.000	10.20	8.72	7.46	6.39	5.47
1,224.000	4.69	4.02	3.44	2.95	2.53
1,239.000	2.16	1.85	1.59	1.36	1.16
1,254.000	1.00	0.86	0.73	0.63	0.54
1,269.000	0.47	0.40	0.35	0.30	0.40
1,284.000	0.94	2.13	4.14	7.05	10.90
1,299.000	15.41	19.92	23.91	26.90	28.68
1,314.000	29.31	28.94	27.82	26.13	24.00
1,329.000	21.52	18.93	16.37	14.03	11.97
1,344.000	10.21	8.73	7.47	6.40	5.48
1,359.000	4.70	4.03	3.45	2.95	2.53
1,374.000	2.17	1.86	1.59	1.36	1.16
1,389.000	1.00	0.86	0.74	0.63	0.54
1,404.000	0.47	0.40	0.35	0.30	0.26
1,419.000	0.22	0.19	0.16	0.13	0.10
1,434.000	0.08	0.06	0.05	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.287 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	314.63 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.287 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	314.63 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	691.80 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	691.96 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	4,301,263.925 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	4,298,891.853 ft ³
Volume (Retained)	821.545 ft ³
Volume (Unrouted)	-1,550.527 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Peak Discharge	691.96 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	4,298,891.699 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
417.000	0.00	0.00	0.00	0.00	0.01
432.000	0.01	0.01	0.02	0.10	0.35
447.000	0.51	0.64	0.77	0.89	1.02
462.000	1.16	1.30	1.45	1.60	1.75
477.000	1.92	2.08	2.25	2.42	2.60
492.000	2.79	2.97	3.16	3.36	3.56
507.000	3.76	3.97	4.19	4.41	4.63
522.000	4.86	5.09	5.33	5.58	5.83
537.000	6.09	6.35	6.63	6.92	7.24
552.000	7.58	7.96	8.39	8.87	9.39
567.000	9.95	10.55	11.18	11.82	12.48
582.000	13.16	13.85	14.54	15.24	15.94
597.000	16.66	17.38	18.12	18.87	19.64
612.000	20.43	21.25	22.08	22.95	23.84
627.000	24.76	25.73	26.74	27.82	28.99
642.000	30.27	31.69	33.26	35.01	36.91
657.000	38.98	41.18	43.54	46.04	48.71
672.000	51.57	54.66	57.99	61.59	65.48
687.000	69.67	74.17	79.04	84.33	90.11
702.000	96.52	103.75	112.10	121.96	134.00
717.000	149.53	170.37	209.30	250.29	307.42
732.000	376.50	446.45	515.38	578.23	629.24
747.000	669.94	688.09	691.96	683.39	664.22
762.000	637.00	605.64	570.50	531.37	492.59
777.000	455.85	421.85	390.94	362.93	337.59
792.000	314.60	293.68	274.52	257.13	242.58
807.000	229.79	217.06	204.98	193.71	183.29
822.000	173.63	166.34	159.19	151.59	144.20
837.000	137.20	130.65	124.55	118.89	113.67
852.000	108.85	104.41	100.33	96.58	93.15
867.000	89.99	87.10	84.45	82.01	79.77
882.000	77.69	75.77	73.99	72.36	70.84
897.000	69.42	68.09	66.82	65.59	64.35
912.000	63.09	61.77	60.37	58.90	57.36
927.000	55.79	54.20	52.64	51.12	49.66
942.000	48.28	46.99	45.80	44.70	43.69
957.000	42.76	41.91	41.13	40.40	39.72
972.000	39.09	38.50	37.95	37.42	36.92
987.000	36.45	36.00	35.57	35.15	34.76

Subsection: Channel Routing (Hydrograph output)
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,002.000	34.37	34.01	33.65	33.30	32.97
1,017.000	32.64	32.33	32.02	31.72	31.42
1,032.000	31.14	30.86	30.59	30.32	30.06
1,047.000	29.81	29.56	29.32	29.08	28.85
1,062.000	28.62	28.39	28.17	27.96	27.75
1,077.000	27.54	27.33	27.12	26.91	26.69
1,092.000	26.45	26.19	25.90	25.59	25.25
1,107.000	24.90	24.53	24.17	23.82	23.48
1,122.000	23.15	22.84	22.55	22.28	22.03
1,137.000	21.80	21.58	21.37	21.18	21.00
1,152.000	20.83	20.66	20.51	20.36	20.21
1,167.000	20.07	19.94	19.81	19.68	19.55
1,182.000	19.43	19.31	19.20	19.08	18.97
1,197.000	18.86	18.75	18.65	18.54	18.44
1,212.000	18.34	18.24	18.14	18.05	17.95
1,227.000	17.86	17.76	17.67	17.58	17.49
1,242.000	17.40	17.31	17.22	17.13	17.04
1,257.000	16.96	16.87	16.79	16.71	16.64
1,272.000	16.56	16.48	16.40	16.33	16.25
1,287.000	16.18	16.10	16.03	15.96	15.89
1,302.000	15.82	15.75	15.68	15.61	15.54
1,317.000	15.47	15.41	15.34	15.28	15.21
1,332.000	15.14	15.08	15.02	14.95	14.89
1,347.000	14.83	14.76	14.70	14.64	14.58
1,362.000	14.52	14.46	14.41	14.35	14.29
1,377.000	14.24	14.18	14.12	14.07	14.01
1,392.000	13.96	13.91	13.85	13.80	13.75
1,407.000	13.70	13.65	13.60	13.55	13.50
1,422.000	13.46	13.41	13.36	13.31	13.26
1,437.000	13.21	13.16	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.129 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	411.30 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.129 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	411.30 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	921.77 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	920.49 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	6,049,696.676 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	6,046,685.347 ft ³
Volume (Retained)	1,019.419 ft ³
Volume (Unrouted)	-1,991.910 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Peak Discharge	920.49 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	6,046,685.094 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
363.000	0.00	0.00	0.00	0.01	0.01
378.000	0.01	0.02	0.10	0.37	0.56
393.000	0.71	0.86	1.01	1.18	1.35
408.000	1.52	1.71	1.90	2.09	2.29
423.000	2.50	2.71	2.93	3.15	3.37
438.000	3.60	3.83	4.07	4.31	4.56
453.000	4.81	5.06	5.32	5.58	5.84
468.000	6.11	6.39	6.67	6.95	7.24
483.000	7.54	7.84	8.14	8.45	8.77
498.000	9.09	9.42	9.75	10.09	10.44
513.000	10.79	11.15	11.52	11.90	12.28
528.000	12.67	13.07	13.48	13.90	14.33
543.000	14.78	15.26	15.79	16.39	17.07
558.000	17.86	18.74	19.73	20.80	21.94
573.000	23.13	24.35	25.59	26.84	28.10
588.000	29.35	30.59	31.84	33.08	34.33
603.000	35.58	36.85	38.14	39.46	40.80
618.000	42.18	43.59	45.04	46.54	48.10
633.000	49.73	51.48	53.40	55.51	57.89
648.000	60.57	63.56	66.85	70.41	74.22
663.000	78.26	82.51	87.00	91.75	96.78
678.000	102.14	107.83	113.90	120.34	127.20
693.000	134.49	142.29	150.65	159.71	169.69
708.000	185.06	199.36	216.26	238.31	270.08
723.000	315.54	374.04	444.22	526.28	614.97
738.000	713.28	792.33	853.57	896.47	918.13
753.000	920.49	906.92	879.82	842.67	797.47
768.000	748.44	696.85	646.66	602.48	561.60
783.000	522.25	486.61	454.43	425.27	398.74
798.000	374.42	352.18	331.90	313.37	296.40
813.000	280.86	266.50	253.55	242.32	231.86
828.000	221.21	210.80	200.80	191.31	182.37
843.000	174.01	167.45	161.43	154.97	148.75
858.000	142.94	137.59	132.67	128.17	124.05
873.000	120.28	116.83	113.65	110.71	107.99
888.000	105.48	103.16	101.01	99.02	97.13
903.000	95.34	93.57	91.80	89.97	88.02
918.000	85.94	83.72	81.39	78.98	76.54
933.000	74.13	71.79	69.55	67.43	65.45

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
948.000	63.62	61.95	60.42	59.02	57.75
963.000	56.59	55.51	54.52	53.60	52.74
978.000	51.94	51.19	50.47	49.80	49.16
993.000	48.55	47.97	47.42	46.88	46.37
1,008.000	45.87	45.40	44.93	44.49	44.05
1,023.000	43.63	43.22	42.82	42.43	42.05
1,038.000	41.68	41.32	40.96	40.62	40.28
1,053.000	39.95	39.62	39.31	38.99	38.69
1,068.000	38.39	38.09	37.80	37.52	37.24
1,083.000	36.95	36.66	36.34	35.99	35.59
1,098.000	35.15	34.65	34.11	33.54	32.95
1,113.000	32.36	31.78	31.22	30.68	30.18
1,128.000	29.71	29.28	28.89	28.53	28.19
1,143.000	27.88	27.60	27.33	27.08	26.84
1,158.000	26.61	26.40	26.20	26.00	25.81
1,173.000	25.63	25.46	25.29	25.12	24.96
1,188.000	24.81	24.66	24.51	24.36	24.22
1,203.000	24.08	23.94	23.81	23.67	23.54
1,218.000	23.41	23.28	23.16	23.04	22.91
1,233.000	22.79	22.67	22.56	22.44	22.32
1,248.000	22.21	22.10	21.99	21.88	21.77
1,263.000	21.66	21.55	21.45	21.34	21.24
1,278.000	21.14	21.04	20.94	20.85	20.75
1,293.000	20.66	20.56	20.47	20.38	20.29
1,308.000	20.20	20.11	20.02	19.93	19.84
1,323.000	19.75	19.66	19.57	19.49	19.41
1,338.000	19.32	19.24	19.16	19.08	19.00
1,353.000	18.92	18.84	18.77	18.69	18.62
1,368.000	18.55	18.47	18.40	18.33	18.26
1,383.000	18.19	18.11	18.04	17.97	17.90
1,398.000	17.83	17.76	17.69	17.62	17.55
1,413.000	17.49	17.42	17.36	17.29	17.23
1,428.000	17.16	17.10	17.04	16.97	16.90

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	1.984 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	571.30 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	1.984 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	571.30 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	1,310.21 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	1,308.21 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	9,417,347.994 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	9,413,263.395 ft ³
Volume (Retained)	1,351.637 ft ³
Volume (Unrouted)	-2,732.962 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Peak Discharge	1,308.21 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	9,413,263.308 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
288.000	0.00	0.00	0.00	0.00	0.01
303.000	0.01	0.01	0.02	0.16	0.42
318.000	0.59	0.74	0.88	1.03	1.19
333.000	1.35	1.51	1.69	1.87	2.05
348.000	2.24	2.43	2.63	2.83	3.03
363.000	3.24	3.46	3.69	3.94	4.21
378.000	4.50	4.82	5.17	5.53	5.92
393.000	6.32	6.73	7.16	7.58	8.01
408.000	8.44	8.87	9.31	9.74	10.17
423.000	10.60	11.04	11.47	11.91	12.36
438.000	12.80	13.25	13.70	14.16	14.63
453.000	15.09	15.57	16.05	16.53	17.02
468.000	17.52	18.02	18.53	19.05	19.58
483.000	20.11	20.65	21.20	21.76	22.33
498.000	22.90	23.49	24.09	24.69	25.31
513.000	25.94	26.58	27.23	27.90	28.57
528.000	29.26	29.97	30.69	31.42	32.18
543.000	32.98	33.87	34.88	36.07	37.49
558.000	39.19	41.15	43.39	45.83	48.44
573.000	51.14	53.92	56.70	59.48	62.23
588.000	64.92	67.56	70.15	72.69	75.19
603.000	77.67	80.15	82.64	85.14	87.67
618.000	90.24	92.85	95.52	98.25	101.07
633.000	104.04	107.23	110.74	114.69	119.20
648.000	124.34	130.13	136.54	143.49	150.89
663.000	158.68	166.81	177.15	187.66	197.07
678.000	206.87	217.13	227.87	239.10	250.84
693.000	264.01	278.42	292.56	306.55	321.26
708.000	337.33	355.81	378.39	408.49	450.17
723.000	506.99	582.41	683.35	801.11	921.32
738.000	1,039.41	1,149.30	1,233.12	1,285.33	1,308.21
753.000	1,304.81	1,280.38	1,238.25	1,183.60	1,119.03
768.000	1,051.67	984.13	918.57	856.29	799.52
783.000	748.65	702.90	661.72	624.39	594.19
798.000	562.63	533.37	506.46	481.66	458.76
813.000	437.57	417.80	399.29	381.70	364.91
828.000	348.72	333.04	317.95	303.49	289.75
843.000	276.82	264.72	253.77	244.35	235.71
858.000	227.17	219.12	211.65	204.79	198.51

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
873.000	192.79	187.52	182.68	178.19	174.04
888.000	170.31	167.71	164.64	161.58	158.62
903.000	155.76	152.92	150.01	146.93	143.58
918.000	139.91	135.91	131.65	127.20	122.68
933.000	118.19	113.81	109.62	105.67	102.00
948.000	98.63	95.56	92.78	90.28	88.02
963.000	85.98	84.11	82.41	80.85	79.41
978.000	78.07	76.83	75.67	74.57	73.55
993.000	72.57	71.65	70.77	69.94	69.14
1,008.000	68.37	67.64	66.93	66.24	65.57
1,023.000	64.93	64.30	63.69	63.10	62.52
1,038.000	61.96	61.41	60.88	60.36	59.85
1,053.000	59.35	58.87	58.40	57.93	57.47
1,068.000	57.03	56.59	56.16	55.74	55.32
1,083.000	54.90	54.45	53.95	53.38	52.72
1,098.000	51.95	51.07	50.10	49.07	48.00
1,113.000	46.93	45.88	44.87	43.91	43.01
1,128.000	42.19	41.43	40.75	40.13	39.57
1,143.000	39.06	38.59	38.15	37.75	37.37
1,158.000	37.02	36.69	36.37	36.07	35.79
1,173.000	35.52	35.26	35.00	34.76	34.53
1,188.000	34.30	34.08	33.86	33.65	33.45
1,203.000	33.24	33.05	32.85	32.66	32.47
1,218.000	32.29	32.10	31.93	31.75	31.58
1,233.000	31.41	31.24	31.08	30.92	30.76
1,248.000	30.60	30.44	30.29	30.14	29.98
1,263.000	29.83	29.69	29.54	29.39	29.25
1,278.000	29.11	28.97	28.83	28.70	28.57
1,293.000	28.43	28.30	28.17	28.04	27.92
1,308.000	27.79	27.66	27.54	27.41	27.29
1,323.000	27.16	27.04	26.92	26.80	26.68
1,338.000	26.56	26.45	26.33	26.22	26.11
1,353.000	26.00	25.89	25.78	25.67	25.56
1,368.000	25.46	25.36	25.25	25.15	25.05
1,383.000	24.95	24.85	24.75	24.66	24.56
1,398.000	24.46	24.36	24.27	24.17	24.08
1,413.000	23.99	23.90	23.81	23.72	23.63
1,428.000	23.55	23.46	23.37	23.29	23.19

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	6.100 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	14.25 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.00	0.00	0.000	0.000	0.00	0.00	0.00
587.01	0.00	13.000	0.031	0.00	0.00	0.15
587.04	0.02	55.000	0.033	0.00	0.02	0.62
587.08	0.06	113.000	0.035	0.00	0.06	1.32
587.12	0.11	176.000	0.037	0.00	0.11	2.07
587.16	0.19	243.000	0.040	0.00	0.19	2.89
587.20	0.28	315.000	0.042	0.00	0.28	3.77
587.24	0.38	390.000	0.045	0.00	0.38	4.71
587.28	0.50	470.000	0.047	0.00	0.50	5.72
587.32	0.63	554.000	0.049	0.00	0.63	6.79
587.36	0.79	642.000	0.052	0.00	0.79	7.92
587.40	0.95	734.000	0.054	0.00	0.95	9.11
587.44	1.14	831.000	0.057	0.00	1.14	10.36
587.48	1.33	931.000	0.059	0.00	1.33	11.68
587.52	1.55	1,036.000	0.061	0.00	1.55	13.07
587.56	1.79	1,145.000	0.064	0.00	1.79	14.51
587.60	2.04	1,259.000	0.066	0.00	2.04	16.02
587.64	2.31	1,376.000	0.069	0.00	2.31	17.60

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.68	2.60	1,498.000	0.071	0.00	2.60	19.24
587.72	2.90	1,624.000	0.073	0.00	2.90	20.94
587.76	3.23	1,754.000	0.076	0.00	3.23	22.71
587.80	3.57	1,888.000	0.078	0.00	3.57	24.55
587.84	3.94	2,026.000	0.081	0.00	3.94	26.45
587.88	4.32	2,169.000	0.083	0.00	4.32	28.42
587.92	4.73	2,316.000	0.085	0.00	4.73	30.46
587.96	5.16	2,467.000	0.088	0.00	5.16	32.57
588.00	5.61	2,622.000	0.090	0.00	5.61	34.74
588.04	6.08	2,781.000	0.093	0.00	6.08	36.98
588.08	6.57	2,945.000	0.095	0.00	6.57	39.29
588.12	7.08	3,113.000	0.098	0.00	7.08	41.67
588.16	7.62	3,285.000	0.100	0.00	7.62	44.12
588.20	8.18	3,461.000	0.102	0.00	8.18	46.64
588.24	8.77	3,641.000	0.105	0.00	8.77	49.23
588.28	9.37	3,826.000	0.107	0.00	9.37	51.88
588.32	10.01	4,015.000	0.110	0.00	10.01	54.61
588.36	10.66	4,208.000	0.112	0.00	10.66	57.41
588.40	11.34	4,405.000	0.114	0.00	11.34	60.29
588.44	12.05	4,606.000	0.117	0.00	12.05	63.23
588.48	12.78	4,812.000	0.119	0.00	12.78	66.25
588.52	13.54	5,022.000	0.122	0.00	13.54	69.34
588.56	14.32	5,236.000	0.124	0.00	14.32	72.50
588.60	15.13	5,454.000	0.126	0.00	15.13	75.73
588.64	15.97	5,676.000	0.129	0.00	15.97	79.04
588.68	16.84	5,903.000	0.131	0.00	16.84	82.42
588.72	17.73	6,133.000	0.134	0.00	17.73	85.88
588.76	18.65	6,368.000	0.136	0.00	18.65	89.41
588.80	19.60	6,607.000	0.138	0.00	19.60	93.01
588.84	20.57	6,851.000	0.141	0.00	20.57	96.69
588.88	21.58	7,098.000	0.143	0.00	21.58	100.45
588.92	22.62	7,350.000	0.146	0.00	22.62	104.28
588.96	23.68	7,606.000	0.148	0.00	23.68	108.19
589.00	24.77	7,866.000	0.150	0.00	24.77	112.17

Subsection: Channel Routing Summary
 Label: R3
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	6.100 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	14.25 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	(N/A) ft ³ /s	Time to Peak (In)	(N/A) min
Flow (Peak Out)	(N/A) ft ³ /s	Time to Peak (Out)	(N/A) min

Mass Balance (ft³)

Volume (Initial)	(N/A) ft ³
Volume (Total Inflow)	(N/A) ft ³
Volume (Total Infiltration)	(N/A) ft ³
Volume (Total Outlet Outflow)	(N/A) ft ³
Volume (Retained)	(N/A) ft ³
Volume (Unrouted)	(N/A) ft ³
Error (Mass Balance)	(N/A) %

Subsection: Channel Routing (Hydrograph output)
 Label: R3
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Peak Discharge	24.77 ft ³ /s
Time to Peak	735.000 min
Hydrograph Volume	191,998.896 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
525.000	0.00	0.00	0.00	0.00	0.01
540.000	0.01	0.01	0.01	0.01	0.01
555.000	0.01	0.01	0.01	0.01	0.01
570.000	0.03	0.05	0.09	0.15	0.20
585.000	0.25	0.28	0.30	0.29	0.28
600.000	0.28	0.29	0.34	0.43	0.54
615.000	0.66	0.75	0.79	0.80	0.77
630.000	0.70	0.64	0.61	0.62	0.70
645.000	0.83	0.99	1.14	1.24	1.28
660.000	1.28	1.29	1.35	1.47	1.63
675.000	1.79	1.90	1.99	2.09	2.21
690.000	2.37	2.56	2.76	2.99	3.26
705.000	3.61	4.04	4.58	5.39	6.65
720.000	8.45	10.90	14.12	18.05	22.31
735.000	24.77	24.77	24.77	24.77	24.77
750.000	24.77	24.77	24.77	24.77	24.77
765.000	24.77	24.77	24.77	24.77	24.77
780.000	24.77	24.77	21.66	15.71	12.50
795.000	10.42	8.89	7.74	6.95	6.54
810.000	6.48	6.57	6.59	6.44	6.09
825.000	5.55	4.95	4.46	4.22	4.32
840.000	4.71	5.18	5.53	5.61	5.42
855.000	4.99	4.39	3.76	3.27	3.03
870.000	3.09	3.47	4.06	4.66	5.08
885.000	5.22	5.05	4.64	4.07	3.45
900.000	2.88	2.39	1.99	1.75	1.73
915.000	1.99	2.54	3.32	4.09	4.64
930.000	4.89	4.82	4.48	3.96	3.37
945.000	2.82	2.33	1.93	1.59	1.31
960.000	1.09	0.98	1.02	1.27	1.83
975.000	2.65	3.55	4.30	4.72	4.78
990.000	4.54	4.08	3.51	2.95	2.45
1,005.000	2.02	1.67	1.38	1.13	0.94
1,020.000	0.77	0.64	0.53	0.44	0.37
1,035.000	0.32	0.33	0.44	0.76	1.38
1,050.000	2.27	3.26	4.10	4.58	4.70
1,065.000	4.50	4.07	3.52	2.96	2.47
1,080.000	2.04	1.69	1.39	1.15	0.95
1,095.000	0.78	0.65	0.54	0.45	0.38

Subsection: Channel Routing (Hydrograph output)
 Label: R3
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,110.000	0.32	0.27	0.23	0.19	0.16
1,125.000	0.14	0.12	0.10	0.09	0.08
1,140.000	0.06	0.05	0.06	0.11	0.26
1,155.000	0.66	1.40	2.39	3.41	4.22
1,170.000	4.64	4.69	4.43	3.96	3.40
1,185.000	2.86	2.37	1.96	1.62	1.34
1,200.000	1.11	0.91	0.75	0.63	0.52
1,215.000	0.43	0.36	0.31	0.26	0.22
1,230.000	0.18	0.16	0.14	0.12	0.10
1,245.000	0.09	0.07	0.06	0.05	0.05
1,260.000	0.04	0.04	0.03	0.03	0.03
1,275.000	0.02	0.02	0.03	0.07	0.22
1,290.000	0.62	1.37	2.38	3.43	4.25
1,305.000	4.68	4.74	4.48	4.00	3.43
1,320.000	2.88	2.40	1.98	1.64	1.35
1,335.000	1.11	0.92	0.76	0.63	0.52
1,350.000	0.44	0.37	0.31	0.26	0.22
1,365.000	0.19	0.16	0.14	0.12	0.10
1,380.000	0.09	0.07	0.06	0.05	0.05
1,395.000	0.04	0.04	0.03	0.03	0.03
1,410.000	0.02	0.02	0.02	0.02	0.02
1,425.000	0.01	0.01	0.01	0.01	0.01
1,440.000	0.01	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	43.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1
Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.00	0.00	0.000	0.000	0.00	0.00	0.00
587.01	0.00	13.000	0.031	0.00	0.00	0.15
587.04	0.02	55.000	0.033	0.00	0.02	0.62
587.08	0.06	113.000	0.035	0.00	0.06	1.32
587.12	0.11	176.000	0.037	0.00	0.11	2.07
587.16	0.19	243.000	0.040	0.00	0.19	2.89
587.20	0.28	315.000	0.042	0.00	0.28	3.77
587.24	0.38	390.000	0.045	0.00	0.38	4.71
587.28	0.50	470.000	0.047	0.00	0.50	5.72
587.32	0.63	554.000	0.049	0.00	0.63	6.79
587.36	0.79	642.000	0.052	0.00	0.79	7.92
587.40	0.95	734.000	0.054	0.00	0.95	9.11
587.44	1.14	831.000	0.057	0.00	1.14	10.36
587.48	1.33	931.000	0.059	0.00	1.33	11.68
587.52	1.55	1,036.000	0.061	0.00	1.55	13.07
587.56	1.79	1,145.000	0.064	0.00	1.79	14.51
587.60	2.04	1,259.000	0.066	0.00	2.04	16.02
587.64	2.31	1,376.000	0.069	0.00	2.31	17.60

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.68	2.60	1,498.000	0.071	0.00	2.60	19.24
587.72	2.90	1,624.000	0.073	0.00	2.90	20.94
587.76	3.23	1,754.000	0.076	0.00	3.23	22.71
587.80	3.57	1,888.000	0.078	0.00	3.57	24.55
587.84	3.94	2,026.000	0.081	0.00	3.94	26.45
587.88	4.32	2,169.000	0.083	0.00	4.32	28.42
587.92	4.73	2,316.000	0.085	0.00	4.73	30.46
587.96	5.16	2,467.000	0.088	0.00	5.16	32.57
588.00	5.61	2,622.000	0.090	0.00	5.61	34.74
588.04	6.08	2,781.000	0.093	0.00	6.08	36.98
588.08	6.57	2,945.000	0.095	0.00	6.57	39.29
588.12	7.08	3,113.000	0.098	0.00	7.08	41.67
588.16	7.62	3,285.000	0.100	0.00	7.62	44.12
588.20	8.18	3,461.000	0.102	0.00	8.18	46.64
588.24	8.77	3,641.000	0.105	0.00	8.77	49.23
588.28	9.37	3,826.000	0.107	0.00	9.37	51.88
588.32	10.01	4,015.000	0.110	0.00	10.01	54.61
588.36	10.66	4,208.000	0.112	0.00	10.66	57.41
588.40	11.34	4,405.000	0.114	0.00	11.34	60.29
588.44	12.05	4,606.000	0.117	0.00	12.05	63.23
588.48	12.78	4,812.000	0.119	0.00	12.78	66.25
588.52	13.54	5,022.000	0.122	0.00	13.54	69.34
588.56	14.32	5,236.000	0.124	0.00	14.32	72.50
588.60	15.13	5,454.000	0.126	0.00	15.13	75.73
588.64	15.97	5,676.000	0.129	0.00	15.97	79.04
588.68	16.84	5,903.000	0.131	0.00	16.84	82.42
588.72	17.73	6,133.000	0.134	0.00	17.73	85.88
588.76	18.65	6,368.000	0.136	0.00	18.65	89.41
588.80	19.60	6,607.000	0.138	0.00	19.60	93.01
588.84	20.57	6,851.000	0.141	0.00	20.57	96.69
588.88	21.58	7,098.000	0.143	0.00	21.58	100.45
588.92	22.62	7,350.000	0.146	0.00	22.62	104.28
588.96	23.68	7,606.000	0.148	0.00	23.68	108.19
589.00	24.77	7,866.000	0.150	0.00	24.77	112.17

Subsection: Channel Routing Summary
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	43.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	(N/A) ft ³ /s	Time to Peak (In)	(N/A) min
Flow (Peak Out)	(N/A) ft ³ /s	Time to Peak (Out)	(N/A) min

Mass Balance (ft³)

Volume (Initial)	(N/A) ft ³
Volume (Total Inflow)	(N/A) ft ³
Volume (Total Infiltration)	(N/A) ft ³
Volume (Total Outlet Outflow)	(N/A) ft ³
Volume (Retained)	(N/A) ft ³
Volume (Unrouted)	(N/A) ft ³
Error (Mass Balance)	(N/A) %

Subsection: Channel Routing (Hydrograph output)
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Peak Discharge	24.77 ft ³ /s
Time to Peak	861.000 min
Hydrograph Volume	516,774.540 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
423.000	0.00	0.00	0.00	0.01	0.01
438.000	0.01	0.02	0.03	0.04	0.05
453.000	0.07	0.08	0.10	0.12	0.14
468.000	0.16	0.18	0.20	0.23	0.25
483.000	0.27	0.30	0.33	0.35	0.38
498.000	0.40	0.43	0.46	0.48	0.51
513.000	0.54	0.57	0.60	0.63	0.66
528.000	0.69	0.72	0.75	0.79	0.82
543.000	0.86	0.90	0.94	0.99	1.05
558.000	1.11	1.19	1.27	1.35	1.44
573.000	1.53	1.62	1.70	1.79	1.88
588.000	1.97	2.06	2.15	2.24	2.33
603.000	2.43	2.52	2.62	2.73	2.84
618.000	2.95	3.06	3.18	3.30	3.44
633.000	3.57	3.73	3.89	4.09	4.32
648.000	4.58	4.86	5.17	5.50	5.84
663.000	6.19	6.56	6.95	7.38	7.85
678.000	8.37	8.94	9.56	10.21	10.92
693.000	11.68	12.51	13.43	14.48	15.73
708.000	17.23	19.04	21.39	24.70	24.77
723.000	24.77	24.77	24.77	24.77	24.77
738.000	24.77	24.77	24.77	24.77	24.77
753.000	24.77	24.77	24.77	24.77	24.77
768.000	24.77	24.77	24.77	24.77	24.77
783.000	24.77	24.77	24.77	24.77	24.77
798.000	24.77	24.77	24.77	24.77	24.77
813.000	24.77	24.77	24.77	24.77	24.77
828.000	24.77	24.77	24.77	24.77	24.77
843.000	24.77	24.77	24.77	24.77	24.77
858.000	24.77	24.77	24.77	24.77	24.77
873.000	24.77	24.77	24.77	24.77	24.77
888.000	24.77	24.77	24.77	24.77	24.77
903.000	24.77	24.77	24.77	24.77	24.77
918.000	24.77	24.77	24.77	24.77	24.77
933.000	24.77	24.77	24.77	24.77	24.77
948.000	24.77	24.77	24.77	24.77	24.77
963.000	24.77	24.77	24.77	24.77	22.72
978.000	13.09	8.83	6.71	5.57	4.92
993.000	4.52	4.27	4.11	4.00	3.92

Subsection: Channel Routing (Hydrograph output)
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,008.000	3.86	3.80	3.76	3.72	3.68
1,023.000	3.65	3.62	3.59	3.56	3.53
1,038.000	3.50	3.47	3.44	3.42	3.39
1,053.000	3.36	3.34	3.31	3.28	3.26
1,068.000	3.24	3.21	3.19	3.17	3.15
1,083.000	3.12	3.10	3.07	3.04	3.00
1,098.000	2.95	2.90	2.85	2.80	2.75
1,113.000	2.71	2.67	2.63	2.60	2.57
1,128.000	2.54	2.52	2.50	2.47	2.46
1,143.000	2.44	2.42	2.40	2.39	2.37
1,158.000	2.36	2.34	2.33	2.31	2.30
1,173.000	2.29	2.27	2.26	2.25	2.24
1,188.000	2.22	2.21	2.20	2.19	2.17
1,203.000	2.16	2.15	2.14	2.13	2.12
1,218.000	2.11	2.10	2.09	2.08	2.06
1,233.000	2.05	2.04	2.03	2.02	2.01
1,248.000	2.00	1.99	1.98	1.97	1.96
1,263.000	1.96	1.95	1.94	1.93	1.92
1,278.000	1.91	1.90	1.89	1.89	1.88
1,293.000	1.87	1.86	1.85	1.84	1.84
1,308.000	1.83	1.82	1.81	1.81	1.80
1,323.000	1.79	1.78	1.78	1.77	1.76
1,338.000	1.75	1.75	1.74	1.73	1.72
1,353.000	1.72	1.71	1.70	1.70	1.69
1,368.000	1.68	1.68	1.67	1.66	1.66
1,383.000	1.65	1.65	1.64	1.63	1.63
1,398.000	1.62	1.61	1.61	1.60	1.60
1,413.000	1.59	1.59	1.58	1.57	1.57
1,428.000	1.56	1.56	1.55	1.55	1.54

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	56.37 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.00	0.00	0.000	0.000	0.00	0.00	0.00
587.01	0.00	13.000	0.031	0.00	0.00	0.15
587.04	0.02	55.000	0.033	0.00	0.02	0.62
587.08	0.06	113.000	0.035	0.00	0.06	1.32
587.12	0.11	176.000	0.037	0.00	0.11	2.07
587.16	0.19	243.000	0.040	0.00	0.19	2.89
587.20	0.28	315.000	0.042	0.00	0.28	3.77
587.24	0.38	390.000	0.045	0.00	0.38	4.71
587.28	0.50	470.000	0.047	0.00	0.50	5.72
587.32	0.63	554.000	0.049	0.00	0.63	6.79
587.36	0.79	642.000	0.052	0.00	0.79	7.92
587.40	0.95	734.000	0.054	0.00	0.95	9.11
587.44	1.14	831.000	0.057	0.00	1.14	10.36
587.48	1.33	931.000	0.059	0.00	1.33	11.68
587.52	1.55	1,036.000	0.061	0.00	1.55	13.07
587.56	1.79	1,145.000	0.064	0.00	1.79	14.51
587.60	2.04	1,259.000	0.066	0.00	2.04	16.02
587.64	2.31	1,376.000	0.069	0.00	2.31	17.60

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.68	2.60	1,498.000	0.071	0.00	2.60	19.24
587.72	2.90	1,624.000	0.073	0.00	2.90	20.94
587.76	3.23	1,754.000	0.076	0.00	3.23	22.71
587.80	3.57	1,888.000	0.078	0.00	3.57	24.55
587.84	3.94	2,026.000	0.081	0.00	3.94	26.45
587.88	4.32	2,169.000	0.083	0.00	4.32	28.42
587.92	4.73	2,316.000	0.085	0.00	4.73	30.46
587.96	5.16	2,467.000	0.088	0.00	5.16	32.57
588.00	5.61	2,622.000	0.090	0.00	5.61	34.74
588.04	6.08	2,781.000	0.093	0.00	6.08	36.98
588.08	6.57	2,945.000	0.095	0.00	6.57	39.29
588.12	7.08	3,113.000	0.098	0.00	7.08	41.67
588.16	7.62	3,285.000	0.100	0.00	7.62	44.12
588.20	8.18	3,461.000	0.102	0.00	8.18	46.64
588.24	8.77	3,641.000	0.105	0.00	8.77	49.23
588.28	9.37	3,826.000	0.107	0.00	9.37	51.88
588.32	10.01	4,015.000	0.110	0.00	10.01	54.61
588.36	10.66	4,208.000	0.112	0.00	10.66	57.41
588.40	11.34	4,405.000	0.114	0.00	11.34	60.29
588.44	12.05	4,606.000	0.117	0.00	12.05	63.23
588.48	12.78	4,812.000	0.119	0.00	12.78	66.25
588.52	13.54	5,022.000	0.122	0.00	13.54	69.34
588.56	14.32	5,236.000	0.124	0.00	14.32	72.50
588.60	15.13	5,454.000	0.126	0.00	15.13	75.73
588.64	15.97	5,676.000	0.129	0.00	15.97	79.04
588.68	16.84	5,903.000	0.131	0.00	16.84	82.42
588.72	17.73	6,133.000	0.134	0.00	17.73	85.88
588.76	18.65	6,368.000	0.136	0.00	18.65	89.41
588.80	19.60	6,607.000	0.138	0.00	19.60	93.01
588.84	20.57	6,851.000	0.141	0.00	20.57	96.69
588.88	21.58	7,098.000	0.143	0.00	21.58	100.45
588.92	22.62	7,350.000	0.146	0.00	22.62	104.28
588.96	23.68	7,606.000	0.148	0.00	23.68	108.19
589.00	24.77	7,866.000	0.150	0.00	24.77	112.17

Subsection: Channel Routing Summary
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	56.37 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	(N/A) ft ³ /s	Time to Peak (In)	(N/A) min
Flow (Peak Out)	(N/A) ft ³ /s	Time to Peak (Out)	(N/A) min

Mass Balance (ft³)

Volume (Initial)	(N/A) ft ³
Volume (Total Inflow)	(N/A) ft ³
Volume (Total Infiltration)	(N/A) ft ³
Volume (Total Outlet Outflow)	(N/A) ft ³
Volume (Retained)	(N/A) ft ³
Volume (Unrouted)	(N/A) ft ³
Error (Mass Balance)	(N/A) %

Subsection: Channel Routing (Hydrograph output)
 Label: R3
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Peak Discharge	24.77 ft ³ /s
Time to Peak	948.000 min
Hydrograph Volume	721,869.485 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
366.000	0.00	0.00	0.00	0.01	0.01
381.000	0.01	0.02	0.03	0.04	0.06
396.000	0.08	0.10	0.12	0.14	0.17
411.000	0.20	0.22	0.25	0.28	0.31
426.000	0.34	0.37	0.40	0.43	0.46
441.000	0.50	0.53	0.56	0.59	0.62
456.000	0.66	0.69	0.73	0.76	0.79
471.000	0.83	0.87	0.90	0.94	0.98
486.000	1.02	1.05	1.09	1.13	1.18
501.000	1.22	1.26	1.30	1.35	1.39
516.000	1.44	1.49	1.53	1.58	1.63
531.000	1.69	1.74	1.79	1.85	1.91
546.000	1.97	2.05	2.14	2.25	2.38
561.000	2.53	2.70	2.87	3.04	3.21
576.000	3.38	3.54	3.70	3.85	4.00
591.000	4.15	4.30	4.46	4.61	4.77
606.000	4.93	5.09	5.26	5.43	5.61
621.000	5.80	5.99	6.19	6.40	6.62
636.000	6.87	7.16	7.50	7.91	8.37
651.000	8.89	9.45	10.03	10.62	11.22
666.000	11.84	12.48	13.17	13.91	14.72
681.000	15.60	16.53	17.51	18.55	19.67
696.000	20.86	22.14	23.57	24.77	24.77
711.000	24.77	24.77	24.77	24.77	24.77
726.000	24.77	24.77	24.77	24.77	24.77
741.000	24.77	24.77	24.77	24.77	24.77
756.000	24.77	24.77	24.77	24.77	24.77
771.000	24.77	24.77	24.77	24.77	24.77
786.000	24.77	24.77	24.77	24.77	24.77
801.000	24.77	24.77	24.77	24.77	24.77
816.000	24.77	24.77	24.77	24.77	24.77
831.000	24.77	24.77	24.77	24.77	24.77
846.000	24.77	24.77	24.77	24.77	24.77
861.000	24.77	24.77	24.77	24.77	24.77
876.000	24.77	24.77	24.77	24.77	24.77
891.000	24.77	24.77	24.77	24.77	24.77
906.000	24.77	24.77	24.77	24.77	24.77
921.000	24.77	24.77	24.77	24.77	24.77
936.000	24.77	24.77	24.77	24.77	24.77

Subsection: Channel Routing (Hydrograph output)
 Label: R3
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
951.000	24.77	24.77	24.77	24.77	24.77
966.000	24.77	24.77	24.77	24.77	24.77
981.000	24.77	24.77	24.77	24.77	24.77
996.000	24.77	24.77	24.77	24.77	24.77
1,011.000	24.77	24.77	24.77	24.77	24.77
1,026.000	24.77	24.77	24.77	24.77	24.77
1,041.000	24.77	24.77	24.77	24.77	24.77
1,056.000	24.77	24.77	24.77	24.77	24.77
1,071.000	24.77	24.77	24.77	24.77	24.77
1,086.000	24.77	24.77	19.85	11.80	8.11
1,101.000	6.21	5.15	4.52	4.12	3.86
1,116.000	3.68	3.55	3.46	3.39	3.33
1,131.000	3.29	3.25	3.21	3.18	3.16
1,146.000	3.13	3.11	3.08	3.06	3.04
1,161.000	3.02	3.01	2.99	2.97	2.95
1,176.000	2.93	2.92	2.90	2.88	2.87
1,191.000	2.85	2.84	2.82	2.81	2.79
1,206.000	2.78	2.76	2.75	2.73	2.72
1,221.000	2.70	2.69	2.68	2.66	2.65
1,236.000	2.64	2.62	2.61	2.59	2.58
1,251.000	2.57	2.56	2.54	2.53	2.52
1,266.000	2.51	2.50	2.48	2.47	2.46
1,281.000	2.45	2.44	2.43	2.42	2.41
1,296.000	2.40	2.39	2.38	2.37	2.35
1,311.000	2.34	2.33	2.32	2.31	2.30
1,326.000	2.29	2.28	2.27	2.26	2.25
1,341.000	2.24	2.24	2.23	2.22	2.21
1,356.000	2.20	2.19	2.18	2.18	2.17
1,371.000	2.16	2.15	2.14	2.13	2.12
1,386.000	2.12	2.11	2.10	2.09	2.08
1,401.000	2.08	2.07	2.06	2.05	2.04
1,416.000	2.04	2.03	2.02	2.01	2.01
1,431.000	2.00	1.99	1.99	1.98	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	76.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.00	0.00	0.000	0.000	0.00	0.00	0.00
587.01	0.00	13.000	0.031	0.00	0.00	0.15
587.04	0.02	55.000	0.033	0.00	0.02	0.62
587.08	0.06	113.000	0.035	0.00	0.06	1.32
587.12	0.11	176.000	0.037	0.00	0.11	2.07
587.16	0.19	243.000	0.040	0.00	0.19	2.89
587.20	0.28	315.000	0.042	0.00	0.28	3.77
587.24	0.38	390.000	0.045	0.00	0.38	4.71
587.28	0.50	470.000	0.047	0.00	0.50	5.72
587.32	0.63	554.000	0.049	0.00	0.63	6.79
587.36	0.79	642.000	0.052	0.00	0.79	7.92
587.40	0.95	734.000	0.054	0.00	0.95	9.11
587.44	1.14	831.000	0.057	0.00	1.14	10.36
587.48	1.33	931.000	0.059	0.00	1.33	11.68
587.52	1.55	1,036.000	0.061	0.00	1.55	13.07
587.56	1.79	1,145.000	0.064	0.00	1.79	14.51
587.60	2.04	1,259.000	0.066	0.00	2.04	16.02
587.64	2.31	1,376.000	0.069	0.00	2.31	17.60

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R3
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 437.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.68	2.60	1,498.000	0.071	0.00	2.60	19.24
587.72	2.90	1,624.000	0.073	0.00	2.90	20.94
587.76	3.23	1,754.000	0.076	0.00	3.23	22.71
587.80	3.57	1,888.000	0.078	0.00	3.57	24.55
587.84	3.94	2,026.000	0.081	0.00	3.94	26.45
587.88	4.32	2,169.000	0.083	0.00	4.32	28.42
587.92	4.73	2,316.000	0.085	0.00	4.73	30.46
587.96	5.16	2,467.000	0.088	0.00	5.16	32.57
588.00	5.61	2,622.000	0.090	0.00	5.61	34.74
588.04	6.08	2,781.000	0.093	0.00	6.08	36.98
588.08	6.57	2,945.000	0.095	0.00	6.57	39.29
588.12	7.08	3,113.000	0.098	0.00	7.08	41.67
588.16	7.62	3,285.000	0.100	0.00	7.62	44.12
588.20	8.18	3,461.000	0.102	0.00	8.18	46.64
588.24	8.77	3,641.000	0.105	0.00	8.77	49.23
588.28	9.37	3,826.000	0.107	0.00	9.37	51.88
588.32	10.01	4,015.000	0.110	0.00	10.01	54.61
588.36	10.66	4,208.000	0.112	0.00	10.66	57.41
588.40	11.34	4,405.000	0.114	0.00	11.34	60.29
588.44	12.05	4,606.000	0.117	0.00	12.05	63.23
588.48	12.78	4,812.000	0.119	0.00	12.78	66.25
588.52	13.54	5,022.000	0.122	0.00	13.54	69.34
588.56	14.32	5,236.000	0.124	0.00	14.32	72.50
588.60	15.13	5,454.000	0.126	0.00	15.13	75.73
588.64	15.97	5,676.000	0.129	0.00	15.97	79.04
588.68	16.84	5,903.000	0.131	0.00	16.84	82.42
588.72	17.73	6,133.000	0.134	0.00	17.73	85.88
588.76	18.65	6,368.000	0.136	0.00	18.65	89.41
588.80	19.60	6,607.000	0.138	0.00	19.60	93.01
588.84	20.57	6,851.000	0.141	0.00	20.57	96.69
588.88	21.58	7,098.000	0.143	0.00	21.58	100.45
588.92	22.62	7,350.000	0.146	0.00	22.62	104.28
588.96	23.68	7,606.000	0.148	0.00	23.68	108.19
589.00	24.77	7,866.000	0.150	0.00	24.77	112.17

Subsection: Channel Routing Summary
 Label: R3
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	437.00 ft
Travel Time (Channel)	5.353 min
Number of Sections	1
Length (Section)	437.00 ft
Flow (Weighted)	76.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	589.00 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	(N/A) ft ³ /s	Time to Peak (In)	(N/A) min
Flow (Peak Out)	(N/A) ft ³ /s	Time to Peak (Out)	(N/A) min

Mass Balance (ft³)

Volume (Initial)	(N/A) ft ³
Volume (Total Inflow)	(N/A) ft ³
Volume (Total Infiltration)	(N/A) ft ³
Volume (Total Outlet Outflow)	(N/A) ft ³
Volume (Retained)	(N/A) ft ³
Volume (Unrouted)	(N/A) ft ³
Error (Mass Balance)	(N/A) %

Subsection: Channel Routing (Hydrograph output)
 Label: R3
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Peak Discharge	24.77 ft ³ /s
Time to Peak	927.000 min
Hydrograph Volume	1,121,160.111 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
294.000	0.00	0.00	0.00	0.01	0.01
309.000	0.02	0.02	0.04	0.05	0.06
324.000	0.08	0.10	0.12	0.14	0.17
339.000	0.19	0.22	0.25	0.27	0.30
354.000	0.33	0.36	0.38	0.41	0.44
369.000	0.47	0.51	0.55	0.59	0.64
384.000	0.69	0.74	0.80	0.85	0.91
399.000	0.97	1.02	1.08	1.13	1.18
414.000	1.24	1.29	1.35	1.40	1.45
429.000	1.51	1.56	1.62	1.67	1.73
444.000	1.78	1.84	1.90	1.96	2.02
459.000	2.08	2.14	2.20	2.26	2.32
474.000	2.39	2.45	2.52	2.59	2.66
489.000	2.73	2.80	2.87	2.94	3.02
504.000	3.09	3.17	3.25	3.33	3.41
519.000	3.49	3.58	3.66	3.75	3.84
534.000	3.93	4.03	4.13	4.23	4.36
549.000	4.52	4.72	4.99	5.32	5.70
564.000	6.10	6.52	6.93	7.32	7.69
579.000	8.03	8.36	8.67	8.98	9.27
594.000	9.56	9.85	10.14	10.43	10.72
609.000	11.02	11.32	11.63	11.96	12.30
624.000	12.65	13.01	13.39	13.80	14.26
639.000	14.81	15.49	16.32	17.28	18.34
654.000	19.47	20.63	21.78	22.92	24.06
669.000	24.77	24.77	24.77	24.77	24.77
684.000	24.77	24.77	24.77	24.77	24.77
699.000	24.77	24.77	24.77	24.77	24.77
714.000	24.77	24.77	24.77	24.77	24.77
729.000	24.77	24.77	24.77	24.77	24.77
744.000	24.77	24.77	24.77	24.77	24.77
759.000	24.77	24.77	24.77	24.77	24.77
774.000	24.77	24.77	24.77	24.77	24.77
789.000	24.77	24.77	24.77	24.77	24.77
804.000	24.77	24.77	24.77	24.77	24.77
819.000	24.77	24.77	24.77	24.77	24.77
834.000	24.77	24.77	24.77	24.77	24.77
849.000	24.77	24.77	24.77	24.77	24.77
864.000	24.77	24.77	24.77	24.77	24.77

Subsection: Channel Routing (Hydrograph output)
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
879.000	24.77	24.77	24.77	24.77	24.77
894.000	24.77	24.77	24.77	24.77	24.77
909.000	24.77	24.77	24.77	24.77	24.77
924.000	24.77	24.77	24.77	24.77	24.77
939.000	24.77	24.77	24.77	24.77	24.77
954.000	24.77	24.77	24.77	24.77	24.77
969.000	24.77	24.77	24.77	24.77	24.77
984.000	24.77	24.77	24.77	24.77	24.77
999.000	24.77	24.77	24.77	24.77	24.77
1,014.000	24.77	24.77	24.77	24.77	24.77
1,029.000	24.77	24.77	24.77	24.77	24.77
1,044.000	24.77	24.77	24.77	24.77	24.77
1,059.000	24.77	24.77	24.77	24.77	24.77
1,074.000	24.77	24.77	24.77	24.77	24.77
1,089.000	24.77	24.77	24.77	24.77	24.77
1,104.000	24.77	24.77	24.77	24.77	24.77
1,119.000	24.77	24.77	24.77	24.77	24.77
1,134.000	24.77	24.77	24.77	24.77	24.77
1,149.000	24.77	24.77	24.77	24.77	24.77
1,164.000	24.77	24.77	24.77	24.77	24.77
1,179.000	24.77	24.77	24.77	24.77	24.77
1,194.000	24.77	24.77	24.77	24.77	24.77
1,209.000	24.77	24.77	24.77	24.77	24.77
1,224.000	24.77	24.77	24.77	24.77	24.77
1,239.000	24.77	24.77	24.77	24.77	24.77
1,254.000	24.77	24.77	24.77	24.77	24.77
1,269.000	24.77	24.77	24.77	24.77	24.77
1,284.000	24.77	24.77	24.77	24.77	24.77
1,299.000	24.77	24.77	24.77	24.77	24.77
1,314.000	24.77	24.77	24.77	24.77	24.77
1,329.000	24.77	23.89	13.05	8.35	6.04
1,344.000	4.81	4.10	3.69	3.43	3.27
1,359.000	3.16	3.09	3.04	3.01	2.98
1,374.000	2.96	2.95	2.93	2.92	2.90
1,389.000	2.89	2.88	2.87	2.86	2.85
1,404.000	2.84	2.82	2.81	2.80	2.79
1,419.000	2.78	2.77	2.76	2.75	2.74
1,434.000	2.73	2.72	2.71	(N/A)	(N/A)

Subsection: Addition Summary
Label: POA A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA A1
<Catchment to Outflow Node>	EX AREA A2

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA A1	800,993.168	753.000	114.31
Flow (From)	EX AREA A2	153,457.928	741.000	27.71
Flow (In)	POA A	954,451.096	753.000	137.84

Subsection: Addition Summary
Label: POA A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA A1
<Catchment to Outflow Node>	EX AREA A2

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA A1	2,181,713.455	753.000	344.29
Flow (From)	EX AREA A2	420,585.692	741.000	87.58
Flow (In)	POA A	2,602,299.147	750.000	413.45

Subsection: Addition Summary
Label: POA A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA A1
<Catchment to Outflow Node>	EX AREA A2

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA A1	3,067,287.859	753.000	458.07
Flow (From)	EX AREA A2	591,665.370	741.000	115.96
Flow (In)	POA A	3,658,953.230	750.000	550.18

Subsection: Addition Summary
Label: POA A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA A1
<Catchment to Outflow Node>	EX AREA A2

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA A1	4,772,632.436	753.000	649.43
Flow (From)	EX AREA A2	921,177.270	741.000	163.26
Flow (In)	POA A	5,693,809.707	747.000	780.49

Subsection: Addition Summary
Label: POA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	EX AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA B	621,937.167	753.000	93.10
Flow (From)	R-1	954,334.200	756.000	136.25
Flow (In)	POA B	1,576,271.367	753.000	228.61

Subsection: Addition Summary
Label: POA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	EX AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA B	1,700,725.560	750.000	280.93
Flow (From)	R-1	2,600,538.365	753.000	411.54
Flow (In)	POA B	4,301,263.925	750.000	691.80

Subsection: Addition Summary
Label: POA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	EX AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA B	2,392,973.584	750.000	373.90
Flow (From)	R-1	3,656,723.092	750.000	547.87
Flow (In)	POA B	6,049,696.676	750.000	921.77

Subsection: Addition Summary
Label: POA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	EX AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA B	3,726,556.960	750.000	530.04
Flow (From)	R-1	5,690,791.034	750.000	780.16
Flow (In)	POA B	9,417,347.994	750.000	1,310.21

Subsection: Addition Summary
Label: POA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA C'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	EX AREA D	186,197.006	741.000	33.87
Flow (In)	POA C	186,197.006	741.000	33.87

Subsection: Addition Summary
Label: POA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA C'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	EX AREA D	510,356.033	741.000	107.11
Flow (In)	POA C	510,356.033	741.000	107.11

Subsection: Addition Summary
Label: POA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA C'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA D	717,944.967	741.000	141.73
Flow (In)	POA C	717,944.967	741.000	141.73

Subsection: Addition Summary
Label: POA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA C'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA D	1,117,769.907	738.000	199.85
Flow (In)	POA C	1,117,769.907	738.000	199.85

Subsection: Addition Summary
Label: POA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA C
<Catchment to Outflow Node>	EX AREA E
R2	POA B
R3	POA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA C	460,865.704	756.000	63.09
Flow (From)	EX AREA E	189,673.770	732.000	41.25
Flow (From)	R2	1,576,136.777	756.000	228.17
Flow (From)	R3	191,999.071	735.000	24.77
Flow (In)	POA D	2,418,675.322	753.000	336.72

Subsection: Addition Summary
Label: POA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA C
<Catchment to Outflow Node>	EX AREA E
R2	POA B
R3	POA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA C	1,258,905.485	756.000	190.22
Flow (From)	EX AREA E	520,812.165	735.000	135.22
Flow (From)	R2	4,298,891.853	753.000	691.96
Flow (From)	R3	516,774.769	720.000	24.77
Flow (In)	POA D	6,595,384.272	750.000	966.89

Subsection: Addition Summary
Label: POA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA C
<Catchment to Outflow Node>	EX AREA E
R2	POA B
R3	POA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA C	1,771,491.463	756.000	253.42
Flow (From)	EX AREA E	732,542.833	732.000	178.71
Flow (From)	R2	6,046,685.347	753.000	920.49
Flow (From)	R3	721,869.677	705.000	24.77
Flow (In)	POA D	9,272,589.320	750.000	1,280.80

Subsection: Addition Summary
Label: POA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA C
<Catchment to Outflow Node>	EX AREA E
R2	POA B
R3	POA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA C	2,759,037.263	756.000	359.95
Flow (From)	EX AREA E	1,140,280.129	732.000	251.49
Flow (From)	R2	9,413,263.395	750.000	1,308.21
Flow (From)	R3	1,121,160.332	669.000	24.77
Flow (In)	POA D	14,433,741.11	750.000	1,811.95

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Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.00	595.00	595.00	595.00	595.00
390.000	595.00	595.00	595.00	595.00	595.00
405.000	595.00	595.00	595.00	595.00	595.00
420.000	595.00	595.00	595.00	595.00	595.00
435.000	595.00	595.00	595.00	595.00	595.00
450.000	595.00	595.00	595.00	595.00	595.00
465.000	595.00	595.00	595.00	595.00	595.00
480.000	595.00	595.00	595.00	595.00	595.00
495.000	595.00	595.00	595.00	595.00	595.00
510.000	595.00	595.00	595.00	595.00	595.00
525.000	595.00	595.00	595.00	595.00	595.01
540.000	595.01	595.01	595.01	595.01	595.01
555.000	595.01	595.01	595.01	595.01	595.01
570.000	595.01	595.01	595.02	595.02	595.02
585.000	595.02	595.03	595.03	595.03	595.03
600.000	595.03	595.03	595.03	595.03	595.04
615.000	595.04	595.05	595.05	595.05	595.05

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.05	595.05	595.05	595.05	595.05
645.000	595.05	595.06	595.06	595.07	595.07
660.000	595.07	595.08	595.08	595.08	595.09
675.000	595.10	595.10	595.11	595.11	595.12
690.000	595.13	595.13	595.14	595.15	595.16
705.000	595.18	595.19	595.21	595.24	595.29
720.000	595.34	595.42	595.52	595.64	595.77
735.000	595.91	596.05	596.17	596.28	596.37
750.000	596.43	596.46	596.47	596.46	596.42
765.000	596.37	596.30	596.23	596.15	596.07
780.000	596.00	595.93	595.87	595.81	595.76
795.000	595.70	595.65	595.61	595.57	595.53
810.000	595.51	595.49	595.47	595.45	595.42
825.000	595.40	595.37	595.35	595.33	595.32
840.000	595.32	595.32	595.32	595.32	595.31
855.000	595.30	595.29	595.27	595.26	595.25
870.000	595.24	595.24	595.25	595.26	595.26
885.000	595.26	595.26	595.25	595.24	595.23
900.000	595.21	595.20	595.18	595.17	595.16
915.000	595.16	595.16	595.18	595.20	595.21
930.000	595.23	595.23	595.23	595.22	595.21
945.000	595.20	595.18	595.16	595.15	595.13
960.000	595.11	595.10	595.10	595.10	595.12
975.000	595.13	595.16	595.18	595.19	595.20
990.000	595.21	595.21	595.20	595.19	595.18
1,005.000	595.16	595.15	595.13	595.12	595.10
1,020.000	595.09	595.08	595.07	595.06	595.05
1,035.000	595.05	595.04	595.05	595.06	595.08
1,050.000	595.10	595.12	595.15	595.17	595.19
1,065.000	595.20	595.20	595.19	595.19	595.18
1,080.000	595.16	595.15	595.13	595.12	595.10
1,095.000	595.09	595.08	595.07	595.06	595.05
1,110.000	595.05	595.04	595.04	595.03	595.03
1,125.000	595.03	595.02	595.02	595.02	595.02
1,140.000	595.02	595.02	595.02	595.02	595.03
1,155.000	595.05	595.07	595.10	595.13	595.15
1,170.000	595.17	595.19	595.19	595.19	595.19
1,185.000	595.18	595.17	595.15	595.14	595.12
1,200.000	595.11	595.09	595.08	595.07	595.06
1,215.000	595.05	595.05	595.04	595.04	595.03
1,230.000	595.03	595.03	595.03	595.02	595.02
1,245.000	595.02	595.02	595.02	595.02	595.02

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.01	595.01	595.01	595.01	595.01
1,275.000	595.01	595.01	595.01	595.02	595.03
1,290.000	595.05	595.07	595.10	595.13	595.15
1,305.000	595.18	595.19	595.20	595.20	595.19
1,320.000	595.18	595.17	595.16	595.14	595.12
1,335.000	595.11	595.09	595.08	595.07	595.06
1,350.000	595.05	595.05	595.04	595.04	595.03
1,365.000	595.03	595.03	595.03	595.02	595.02
1,380.000	595.02	595.02	595.02	595.02	595.02
1,395.000	595.01	595.01	595.01	595.01	595.01
1,410.000	595.01	595.01	595.01	595.01	595.01
1,425.000	595.01	595.01	595.01	595.01	595.01
1,440.000	595.01	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.00	595.00	595.00	595.00	595.00
390.000	595.00	595.00	595.00	595.00	595.00
405.000	595.00	595.00	595.00	595.00	595.00
420.000	595.00	595.00	595.00	595.00	595.01
435.000	595.01	595.01	595.01	595.01	595.01
450.000	595.01	595.02	595.02	595.02	595.02
465.000	595.02	595.02	595.02	595.02	595.02
480.000	595.02	595.03	595.03	595.03	595.03
495.000	595.03	595.03	595.03	595.03	595.04
510.000	595.04	595.04	595.04	595.04	595.04
525.000	595.04	595.05	595.05	595.05	595.05
540.000	595.05	595.05	595.06	595.06	595.06
555.000	595.06	595.07	595.07	595.07	595.08
570.000	595.08	595.08	595.09	595.09	595.10
585.000	595.10	595.11	595.11	595.12	595.12
600.000	595.13	595.13	595.13	595.14	595.15
615.000	595.15	595.16	595.16	595.17	595.17

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.18	595.19	595.19	595.20	595.21
645.000	595.22	595.23	595.24	595.26	595.27
660.000	595.28	595.30	595.32	595.33	595.35
675.000	595.37	595.40	595.42	595.45	595.48
690.000	595.51	595.54	595.57	595.61	595.66
705.000	595.71	595.76	595.83	595.92	596.03
720.000	596.19	596.39	596.67	596.82	596.97
735.000	597.11	597.25	597.37	597.46	597.52
750.000	597.55	597.55	597.53	597.49	597.44
765.000	597.37	597.30	597.22	597.14	597.07
780.000	597.00	596.94	596.88	596.83	596.78
795.000	596.74	596.70	596.66	596.59	596.51
810.000	596.43	596.35	596.27	596.21	596.14
825.000	596.08	596.03	595.98	595.93	595.89
840.000	595.84	595.81	595.77	595.74	595.71
855.000	595.68	595.65	595.63	595.61	595.59
870.000	595.57	595.55	595.54	595.52	595.51
885.000	595.50	595.49	595.48	595.47	595.46
900.000	595.45	595.44	595.43	595.42	595.42
915.000	595.41	595.40	595.39	595.38	595.37
930.000	595.36	595.35	595.34	595.33	595.32
945.000	595.31	595.30	595.30	595.29	595.29
960.000	595.28	595.27	595.27	595.27	595.26
975.000	595.26	595.25	595.25	595.25	595.25
990.000	595.24	595.24	595.24	595.23	595.23
1,005.000	595.23	595.23	595.23	595.22	595.22
1,020.000	595.22	595.22	595.21	595.21	595.21
1,035.000	595.21	595.21	595.21	595.20	595.20
1,050.000	595.20	595.20	595.20	595.20	595.19
1,065.000	595.19	595.19	595.19	595.19	595.19
1,080.000	595.19	595.19	595.18	595.18	595.18
1,095.000	595.18	595.18	595.17	595.17	595.17
1,110.000	595.17	595.17	595.16	595.16	595.16
1,125.000	595.16	595.16	595.15	595.15	595.15
1,140.000	595.15	595.15	595.15	595.15	595.14
1,155.000	595.14	595.14	595.14	595.14	595.14
1,170.000	595.14	595.14	595.14	595.14	595.14
1,185.000	595.13	595.13	595.13	595.13	595.13
1,200.000	595.13	595.13	595.13	595.13	595.13
1,215.000	595.13	595.13	595.13	595.13	595.13
1,230.000	595.12	595.12	595.12	595.12	595.12
1,245.000	595.12	595.12	595.12	595.12	595.12

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.12	595.12	595.12	595.12	595.12
1,275.000	595.12	595.12	595.12	595.11	595.11
1,290.000	595.11	595.11	595.11	595.11	595.11
1,305.000	595.11	595.11	595.11	595.11	595.11
1,320.000	595.11	595.11	595.11	595.11	595.11
1,335.000	595.11	595.11	595.11	595.11	595.11
1,350.000	595.11	595.10	595.10	595.10	595.10
1,365.000	595.10	595.10	595.10	595.10	595.10
1,380.000	595.10	595.10	595.10	595.10	595.10
1,395.000	595.10	595.10	595.10	595.10	595.10
1,410.000	595.10	595.10	595.10	595.10	595.10
1,425.000	595.10	595.10	595.10	595.10	595.10
1,440.000	595.09	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.01	595.01	595.01	595.01	595.01
390.000	595.01	595.02	595.02	595.02	595.02
405.000	595.02	595.02	595.02	595.02	595.02
420.000	595.03	595.03	595.03	595.03	595.03
435.000	595.03	595.03	595.04	595.04	595.04
450.000	595.04	595.04	595.04	595.05	595.05
465.000	595.05	595.05	595.05	595.05	595.06
480.000	595.06	595.06	595.06	595.06	595.07
495.000	595.07	595.07	595.07	595.07	595.08
510.000	595.08	595.08	595.08	595.09	595.09
525.000	595.09	595.09	595.10	595.10	595.10
540.000	595.10	595.11	595.11	595.11	595.12
555.000	595.12	595.13	595.13	595.14	595.15
570.000	595.16	595.16	595.17	595.18	595.19
585.000	595.20	595.20	595.21	595.22	595.23
600.000	595.24	595.24	595.25	595.26	595.27
615.000	595.28	595.29	595.30	595.31	595.32

Subsection: Time vs. Elevation
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.33	595.34	595.35	595.36	595.38
645.000	595.39	595.41	595.43	595.45	595.48
660.000	595.50	595.53	595.56	595.59	595.62
675.000	595.65	595.69	595.73	595.77	595.81
690.000	595.86	595.91	595.96	596.01	596.07
705.000	596.14	596.22	596.31	596.42	596.57
720.000	596.71	596.83	596.95	597.11	597.29
735.000	597.47	597.65	597.80	597.92	597.99
750.000	598.02	598.02	597.99	597.93	597.86
765.000	597.77	597.68	597.57	597.47	597.38
780.000	597.29	597.21	597.14	597.07	597.01
795.000	596.96	596.91	596.86	596.82	596.79
810.000	596.75	596.72	596.69	596.65	596.59
825.000	596.52	596.45	596.38	596.32	596.26
840.000	596.20	596.14	596.09	596.05	596.00
855.000	595.96	595.93	595.89	595.86	595.84
870.000	595.81	595.79	595.76	595.74	595.72
885.000	595.71	595.69	595.67	595.66	595.65
900.000	595.64	595.62	595.61	595.60	595.59
915.000	595.58	595.56	595.55	595.53	595.52
930.000	595.50	595.48	595.47	595.46	595.44
945.000	595.43	595.42	595.41	595.40	595.39
960.000	595.38	595.37	595.37	595.36	595.36
975.000	595.35	595.35	595.34	595.34	595.33
990.000	595.33	595.32	595.32	595.32	595.31
1,005.000	595.31	595.31	595.30	595.30	595.30
1,020.000	595.29	595.29	595.29	595.29	595.28
1,035.000	595.28	595.28	595.28	595.27	595.27
1,050.000	595.27	595.27	595.27	595.26	595.26
1,065.000	595.26	595.26	595.26	595.25	595.25
1,080.000	595.25	595.25	595.25	595.24	595.24
1,095.000	595.24	595.24	595.23	595.23	595.23
1,110.000	595.22	595.22	595.21	595.21	595.21
1,125.000	595.20	595.20	595.20	595.20	595.19
1,140.000	595.19	595.19	595.19	595.19	595.18
1,155.000	595.18	595.18	595.18	595.18	595.18
1,170.000	595.18	595.18	595.17	595.17	595.17
1,185.000	595.17	595.17	595.17	595.17	595.17
1,200.000	595.17	595.17	595.16	595.16	595.16
1,215.000	595.16	595.16	595.16	595.16	595.16
1,230.000	595.16	595.16	595.16	595.16	595.16
1,245.000	595.15	595.15	595.15	595.15	595.15

Subsection: Time vs. Elevation
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.15	595.15	595.15	595.15	595.15
1,275.000	595.15	595.15	595.15	595.15	595.14
1,290.000	595.14	595.14	595.14	595.14	595.14
1,305.000	595.14	595.14	595.14	595.14	595.14
1,320.000	595.14	595.14	595.14	595.14	595.14
1,335.000	595.14	595.13	595.13	595.13	595.13
1,350.000	595.13	595.13	595.13	595.13	595.13
1,365.000	595.13	595.13	595.13	595.13	595.13
1,380.000	595.13	595.13	595.13	595.13	595.13
1,395.000	595.13	595.13	595.12	595.12	595.12
1,410.000	595.12	595.12	595.12	595.12	595.12
1,425.000	595.12	595.12	595.12	595.12	595.12
1,440.000	595.12	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.01	595.01	595.01	595.01
315.000	595.01	595.01	595.02	595.02	595.02
330.000	595.02	595.02	595.02	595.02	595.02
345.000	595.02	595.03	595.03	595.03	595.03
360.000	595.03	595.03	595.03	595.03	595.04
375.000	595.04	595.04	595.04	595.04	595.05
390.000	595.05	595.05	595.06	595.06	595.06
405.000	595.06	595.07	595.07	595.07	595.07
420.000	595.08	595.08	595.08	595.09	595.09
435.000	595.09	595.09	595.10	595.10	595.10
450.000	595.11	595.11	595.11	595.12	595.12
465.000	595.12	595.13	595.13	595.13	595.14
480.000	595.14	595.14	595.15	595.15	595.15
495.000	595.16	595.16	595.16	595.17	595.17
510.000	595.18	595.18	595.18	595.19	595.19
525.000	595.20	595.20	595.21	595.21	595.22
540.000	595.22	595.23	595.23	595.24	595.25
555.000	595.26	595.27	595.28	595.30	595.32
570.000	595.33	595.35	595.37	595.39	595.40
585.000	595.42	595.44	595.46	595.47	595.49
600.000	595.51	595.52	595.54	595.55	595.57
615.000	595.59	595.60	595.62	595.64	595.66

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.67	595.69	595.72	595.74	595.77
645.000	595.80	595.83	595.87	595.92	595.96
660.000	596.01	596.06	596.12	596.17	596.23
675.000	596.29	596.35	596.42	596.49	596.56
690.000	596.64	596.69	596.72	596.75	596.78
705.000	596.81	596.85	596.89	596.94	597.01
720.000	597.10	597.23	597.40	597.61	597.85
735.000	598.10	598.34	598.46	598.53	598.58
750.000	598.59	598.58	598.55	598.51	598.45
765.000	598.38	598.30	598.17	598.03	597.91
780.000	597.79	597.69	597.59	597.51	597.43
795.000	597.36	597.30	597.24	597.18	597.13
810.000	597.09	597.04	597.00	596.96	596.93
825.000	596.89	596.86	596.83	596.80	596.77
840.000	596.74	596.71	596.69	596.65	596.60
855.000	596.55	596.49	596.44	596.39	596.35
870.000	596.31	596.27	596.23	596.20	596.17
885.000	596.14	596.12	596.09	596.07	596.05
900.000	596.03	596.01	596.00	595.98	595.96
915.000	595.93	595.91	595.88	595.85	595.82
930.000	595.79	595.76	595.74	595.71	595.69
945.000	595.66	595.64	595.62	595.61	595.59
960.000	595.58	595.56	595.55	595.54	595.53
975.000	595.52	595.51	595.51	595.50	595.49
990.000	595.48	595.48	595.47	595.47	595.46
1,005.000	595.46	595.45	595.45	595.44	595.44
1,020.000	595.43	595.43	595.43	595.42	595.42
1,035.000	595.41	595.41	595.41	595.40	595.40
1,050.000	595.40	595.39	595.39	595.39	595.38
1,065.000	595.38	595.38	595.38	595.37	595.37
1,080.000	595.37	595.36	595.36	595.36	595.35
1,095.000	595.35	595.34	595.34	595.33	595.32
1,110.000	595.32	595.31	595.30	595.30	595.29
1,125.000	595.29	595.28	595.28	595.27	595.27
1,140.000	595.27	595.26	595.26	595.26	595.25
1,155.000	595.25	595.25	595.25	595.24	595.24
1,170.000	595.24	595.24	595.24	595.24	595.23
1,185.000	595.23	595.23	595.23	595.23	595.23
1,200.000	595.23	595.22	595.22	595.22	595.22
1,215.000	595.22	595.22	595.22	595.22	595.22
1,230.000	595.21	595.21	595.21	595.21	595.21
1,245.000	595.21	595.21	595.21	595.21	595.20

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.20	595.20	595.20	595.20	595.20
1,275.000	595.20	595.20	595.20	595.20	595.20
1,290.000	595.19	595.19	595.19	595.19	595.19
1,305.000	595.19	595.19	595.19	595.19	595.19
1,320.000	595.19	595.19	595.18	595.18	595.18
1,335.000	595.18	595.18	595.18	595.18	595.18
1,350.000	595.18	595.18	595.18	595.18	595.18
1,365.000	595.18	595.17	595.17	595.17	595.17
1,380.000	595.17	595.17	595.17	595.17	595.17
1,395.000	595.17	595.17	595.17	595.17	595.17
1,410.000	595.17	595.17	595.16	595.16	595.16
1,425.000	595.16	595.16	595.16	595.16	595.16
1,440.000	595.16	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.50	587.50	587.50
390.000	587.50	587.50	587.50	587.50	587.50
405.000	587.50	587.50	587.50	587.50	587.50
420.000	587.50	587.50	587.50	587.50	587.50
435.000	587.50	587.50	587.50	587.50	587.50
450.000	587.50	587.50	587.50	587.50	587.50
465.000	587.50	587.50	587.50	587.50	587.50
480.000	587.50	587.50	587.50	587.50	587.50
495.000	587.50	587.50	587.50	587.50	587.50
510.000	587.50	587.50	587.50	587.50	587.50
525.000	587.50	587.50	587.50	587.50	587.50
540.000	587.50	587.51	587.51	587.51	587.51
555.000	587.51	587.51	587.51	587.51	587.51
570.000	587.51	587.52	587.52	587.52	587.53
585.000	587.53	587.53	587.53	587.53	587.53
600.000	587.53	587.54	587.54	587.54	587.55
615.000	587.55	587.56	587.56	587.57	587.57

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.57	587.57	587.56	587.57	587.57
645.000	587.57	587.58	587.58	587.59	587.59
660.000	587.60	587.60	587.61	587.62	587.63
675.000	587.63	587.64	587.65	587.66	587.67
690.000	587.68	587.69	587.70	587.72	587.73
705.000	587.75	587.77	587.80	587.84	587.89
720.000	587.97	588.07	588.20	588.36	588.54
735.000	588.74	588.95	589.16	589.26	589.32
750.000	589.35	589.38	589.38	589.37	589.35
765.000	589.32	589.28	589.24	589.19	589.13
780.000	589.04	588.94	588.84	588.75	588.67
795.000	588.59	588.51	588.44	588.37	588.32
810.000	588.27	588.24	588.20	588.17	588.14
825.000	588.10	588.07	588.03	588.00	587.98
840.000	587.97	587.97	587.97	587.97	587.96
855.000	587.95	587.93	587.91	587.89	587.87
870.000	587.86	587.85	587.85	587.86	587.87
885.000	587.88	587.89	587.88	587.87	587.85
900.000	587.83	587.81	587.78	587.76	587.74
915.000	587.73	587.73	587.75	587.77	587.80
930.000	587.82	587.84	587.84	587.84	587.82
945.000	587.80	587.78	587.76	587.73	587.70
960.000	587.68	587.66	587.65	587.65	587.66
975.000	587.68	587.71	587.74	587.77	587.79
990.000	587.81	587.81	587.81	587.79	587.78
1,005.000	587.75	587.73	587.70	587.68	587.66
1,020.000	587.63	587.62	587.60	587.59	587.58
1,035.000	587.57	587.56	587.57	587.58	587.60
1,050.000	587.63	587.66	587.70	587.74	587.77
1,065.000	587.79	587.79	587.79	587.79	587.77
1,080.000	587.75	587.73	587.70	587.68	587.66
1,095.000	587.64	587.62	587.60	587.59	587.58
1,110.000	587.57	587.56	587.55	587.55	587.54
1,125.000	587.54	587.53	587.53	587.53	587.52
1,140.000	587.52	587.52	587.52	587.53	587.54
1,155.000	587.56	587.59	587.62	587.66	587.70
1,170.000	587.74	587.77	587.78	587.79	587.79
1,185.000	587.78	587.76	587.74	587.72	587.69
1,200.000	587.67	587.65	587.63	587.61	587.59
1,215.000	587.58	587.57	587.56	587.56	587.55
1,230.000	587.54	587.54	587.53	587.53	587.53
1,245.000	587.53	587.52	587.52	587.52	587.52

Subsection: Time vs. Elevation
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.52	587.52	587.52	587.51	587.51
1,275.000	587.51	587.51	587.51	587.52	587.53
1,290.000	587.55	587.58	587.62	587.66	587.70
1,305.000	587.74	587.77	587.79	587.80	587.79
1,320.000	587.78	587.76	587.74	587.72	587.69
1,335.000	587.67	587.65	587.63	587.61	587.59
1,350.000	587.58	587.57	587.56	587.56	587.55
1,365.000	587.54	587.54	587.53	587.53	587.53
1,380.000	587.53	587.52	587.52	587.52	587.52
1,395.000	587.52	587.52	587.52	587.51	587.51
1,410.000	587.51	587.51	587.51	587.51	587.51
1,425.000	587.51	587.51	587.51	587.51	587.51
1,440.000	587.51	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.50	587.50	587.50
390.000	587.50	587.50	587.50	587.50	587.50
405.000	587.50	587.50	587.50	587.50	587.50
420.000	587.50	587.50	587.50	587.50	587.50
435.000	587.50	587.51	587.51	587.51	587.51
450.000	587.52	587.52	587.52	587.52	587.52
465.000	587.52	587.52	587.53	587.53	587.53
480.000	587.53	587.53	587.53	587.54	587.54
495.000	587.54	587.54	587.54	587.54	587.55
510.000	587.55	587.55	587.55	587.55	587.56
525.000	587.56	587.56	587.56	587.57	587.57
540.000	587.57	587.57	587.58	587.58	587.58
555.000	587.59	587.59	587.60	587.60	587.61
570.000	587.61	587.62	587.62	587.63	587.64
585.000	587.64	587.65	587.66	587.66	587.67
600.000	587.68	587.69	587.69	587.70	587.71
615.000	587.72	587.72	587.73	587.74	587.75

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.76	587.77	587.78	587.79	587.80
645.000	587.82	587.83	587.85	587.87	587.89
660.000	587.91	587.93	587.96	587.98	588.01
675.000	588.04	588.07	588.11	588.15	588.19
690.000	588.23	588.28	588.33	588.39	588.45
705.000	588.52	588.60	588.70	588.81	588.97
720.000	589.17	589.31	589.46	589.67	589.93
735.000	590.19	590.44	590.67	590.85	590.94
750.000	590.98	590.99	590.97	590.93	590.87
765.000	590.77	590.64	590.50	590.36	590.22
780.000	590.10	589.98	589.88	589.79	589.70
795.000	589.62	589.55	589.49	589.44	589.39
810.000	589.34	589.30	589.26	589.22	589.18
825.000	589.13	589.06	588.99	588.91	588.85
840.000	588.78	588.72	588.67	588.62	588.57
855.000	588.53	588.49	588.45	588.42	588.39
870.000	588.36	588.33	588.31	588.29	588.27
885.000	588.25	588.23	588.21	588.20	588.19
900.000	588.17	588.16	588.15	588.14	588.12
915.000	588.11	588.10	588.08	588.07	588.05
930.000	588.04	588.02	588.01	587.99	587.98
945.000	587.97	587.96	587.94	587.93	587.93
960.000	587.92	587.91	587.90	587.90	587.89
975.000	587.88	587.88	587.87	587.87	587.86
990.000	587.86	587.86	587.85	587.85	587.84
1,005.000	587.84	587.84	587.83	587.83	587.83
1,020.000	587.82	587.82	587.82	587.82	587.81
1,035.000	587.81	587.81	587.80	587.80	587.80
1,050.000	587.80	587.80	587.79	587.79	587.79
1,065.000	587.79	587.78	587.78	587.78	587.78
1,080.000	587.78	587.77	587.77	587.77	587.77
1,095.000	587.76	587.76	587.76	587.76	587.75
1,110.000	587.75	587.75	587.74	587.74	587.74
1,125.000	587.73	587.73	587.73	587.72	587.72
1,140.000	587.72	587.72	587.72	587.71	587.71
1,155.000	587.71	587.71	587.71	587.71	587.71
1,170.000	587.70	587.70	587.70	587.70	587.70
1,185.000	587.70	587.70	587.70	587.69	587.69
1,200.000	587.69	587.69	587.69	587.69	587.69
1,215.000	587.69	587.69	587.69	587.68	587.68
1,230.000	587.68	587.68	587.68	587.68	587.68
1,245.000	587.68	587.68	587.68	587.68	587.67

Subsection: Time vs. Elevation
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.67	587.67	587.67	587.67	587.67
1,275.000	587.67	587.67	587.67	587.67	587.67
1,290.000	587.67	587.67	587.67	587.66	587.66
1,305.000	587.66	587.66	587.66	587.66	587.66
1,320.000	587.66	587.66	587.66	587.66	587.66
1,335.000	587.66	587.66	587.66	587.65	587.65
1,350.000	587.65	587.65	587.65	587.65	587.65
1,365.000	587.65	587.65	587.65	587.65	587.65
1,380.000	587.65	587.65	587.65	587.65	587.65
1,395.000	587.65	587.64	587.64	587.64	587.64
1,410.000	587.64	587.64	587.64	587.64	587.64
1,425.000	587.64	587.64	587.64	587.64	587.64
1,440.000	587.64	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.51	587.51	587.51
390.000	587.52	587.52	587.52	587.52	587.52
405.000	587.52	587.52	587.53	587.53	587.53
420.000	587.53	587.53	587.54	587.54	587.54
435.000	587.54	587.54	587.55	587.55	587.55
450.000	587.55	587.56	587.56	587.56	587.56
465.000	587.57	587.57	587.57	587.57	587.58
480.000	587.58	587.58	587.59	587.59	587.59
495.000	587.60	587.60	587.60	587.60	587.61
510.000	587.61	587.61	587.62	587.62	587.63
525.000	587.63	587.63	587.64	587.64	587.65
540.000	587.65	587.65	587.66	587.66	587.67
555.000	587.68	587.68	587.69	587.70	587.71
570.000	587.72	587.73	587.75	587.76	587.77
585.000	587.78	587.80	587.81	587.82	587.83
600.000	587.84	587.86	587.87	587.88	587.89
615.000	587.91	587.92	587.93	587.95	587.96

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.98	587.99	588.01	588.03	588.05
645.000	588.07	588.10	588.13	588.16	588.20
660.000	588.23	588.27	588.31	588.36	588.40
675.000	588.45	588.50	588.56	588.62	588.68
690.000	588.75	588.82	588.89	588.98	589.06
705.000	589.16	589.22	589.28	589.34	589.42
720.000	589.54	589.70	589.92	590.18	590.48
735.000	590.81	591.03	591.20	591.33	591.43
750.000	591.47	591.48	591.45	591.39	591.31
765.000	591.21	591.11	591.00	590.89	590.76
780.000	590.61	590.47	590.33	590.22	590.11
795.000	590.01	589.92	589.84	589.76	589.70
810.000	589.63	589.58	589.52	589.48	589.43
825.000	589.40	589.36	589.32	589.28	589.25
840.000	589.21	589.18	589.14	589.08	589.02
855.000	588.96	588.90	588.85	588.80	588.76
870.000	588.72	588.68	588.65	588.62	588.59
885.000	588.56	588.54	588.51	588.49	588.47
900.000	588.46	588.44	588.42	588.40	588.39
915.000	588.37	588.35	588.32	588.30	588.28
930.000	588.25	588.23	588.21	588.19	588.17
945.000	588.15	588.13	588.11	588.10	588.08
960.000	588.07	588.06	588.05	588.04	588.03
975.000	588.02	588.02	588.01	588.00	587.99
990.000	587.99	587.98	587.98	587.97	587.97
1,005.000	587.96	587.96	587.95	587.95	587.94
1,020.000	587.94	587.93	587.93	587.93	587.92
1,035.000	587.92	587.92	587.91	587.91	587.91
1,050.000	587.90	587.90	587.90	587.89	587.89
1,065.000	587.89	587.88	587.88	587.88	587.87
1,080.000	587.87	587.87	587.87	587.86	587.86
1,095.000	587.86	587.85	587.85	587.84	587.84
1,110.000	587.83	587.82	587.82	587.81	587.81
1,125.000	587.80	587.80	587.79	587.79	587.79
1,140.000	587.78	587.78	587.78	587.78	587.77
1,155.000	587.77	587.77	587.77	587.76	587.76
1,170.000	587.76	587.76	587.76	587.76	587.75
1,185.000	587.75	587.75	587.75	587.75	587.75
1,200.000	587.75	587.74	587.74	587.74	587.74
1,215.000	587.74	587.74	587.74	587.74	587.73
1,230.000	587.73	587.73	587.73	587.73	587.73
1,245.000	587.73	587.73	587.72	587.72	587.72

Subsection: Time vs. Elevation
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.72	587.72	587.72	587.72	587.72
1,275.000	587.72	587.72	587.71	587.71	587.71
1,290.000	587.71	587.71	587.71	587.71	587.71
1,305.000	587.71	587.71	587.71	587.70	587.70
1,320.000	587.70	587.70	587.70	587.70	587.70
1,335.000	587.70	587.70	587.70	587.70	587.70
1,350.000	587.69	587.69	587.69	587.69	587.69
1,365.000	587.69	587.69	587.69	587.69	587.69
1,380.000	587.69	587.69	587.69	587.69	587.68
1,395.000	587.68	587.68	587.68	587.68	587.68
1,410.000	587.68	587.68	587.68	587.68	587.68
1,425.000	587.68	587.68	587.68	587.68	587.67
1,440.000	587.67	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.51	587.51
315.000	587.51	587.52	587.52	587.52	587.52
330.000	587.52	587.52	587.52	587.53	587.53
345.000	587.53	587.53	587.53	587.54	587.54
360.000	587.54	587.54	587.54	587.55	587.55
375.000	587.55	587.55	587.56	587.56	587.56
390.000	587.57	587.57	587.58	587.58	587.58
405.000	587.59	587.59	587.60	587.60	587.60
420.000	587.61	587.61	587.62	587.62	587.63
435.000	587.63	587.63	587.64	587.64	587.65
450.000	587.65	587.66	587.66	587.67	587.67
465.000	587.68	587.68	587.69	587.69	587.70
480.000	587.70	587.71	587.71	587.72	587.72
495.000	587.73	587.73	587.74	587.74	587.75
510.000	587.76	587.76	587.77	587.77	587.78
525.000	587.79	587.79	587.80	587.81	587.82
540.000	587.82	587.83	587.84	587.85	587.86
555.000	587.87	587.89	587.91	587.93	587.96
570.000	587.98	588.01	588.03	588.06	588.09
585.000	588.12	588.14	588.17	588.19	588.22
600.000	588.24	588.27	588.29	588.31	588.34
615.000	588.36	588.39	588.41	588.44	588.47

Subsection: Time vs. Elevation
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	588.49	588.52	588.55	588.59	588.63
645.000	588.67	588.72	588.78	588.84	588.91
660.000	588.98	589.05	589.13	589.19	589.23
675.000	589.27	589.30	589.34	589.38	589.42
690.000	589.47	589.51	589.57	589.62	589.67
705.000	589.73	589.78	589.85	589.94	590.05
720.000	590.20	590.41	590.69	590.97	591.22
735.000	591.48	591.73	591.97	592.15	592.26
750.000	592.31	592.30	592.25	592.16	592.04
765.000	591.90	591.76	591.61	591.47	591.34
780.000	591.22	591.11	591.01	590.92	590.84
795.000	590.73	590.61	590.51	590.41	590.32
810.000	590.23	590.15	590.08	590.01	589.95
825.000	589.89	589.83	589.77	589.71	589.66
840.000	589.61	589.56	589.52	589.48	589.44
855.000	589.41	589.38	589.35	589.32	589.30
870.000	589.27	589.25	589.23	589.21	589.20
885.000	589.18	589.17	589.14	589.11	589.08
900.000	589.05	589.03	589.00	588.97	588.94
915.000	588.91	588.87	588.83	588.79	588.75
930.000	588.70	588.66	588.62	588.58	588.54
945.000	588.50	588.47	588.44	588.41	588.39
960.000	588.37	588.35	588.33	588.31	588.30
975.000	588.28	588.27	588.26	588.25	588.24
990.000	588.23	588.22	588.21	588.20	588.19
1,005.000	588.18	588.18	588.17	588.16	588.15
1,020.000	588.15	588.14	588.14	588.13	588.12
1,035.000	588.12	588.11	588.11	588.10	588.10
1,050.000	588.09	588.09	588.08	588.08	588.07
1,065.000	588.07	588.06	588.06	588.06	588.05
1,080.000	588.05	588.04	588.04	588.03	588.03
1,095.000	588.02	588.02	588.01	588.00	587.99
1,110.000	587.98	587.97	587.96	587.95	587.94
1,125.000	587.93	587.92	587.91	587.91	587.90
1,140.000	587.89	587.89	587.89	587.88	587.88
1,155.000	587.87	587.87	587.87	587.86	587.86
1,170.000	587.86	587.86	587.85	587.85	587.85
1,185.000	587.85	587.84	587.84	587.84	587.84
1,200.000	587.84	587.83	587.83	587.83	587.83
1,215.000	587.83	587.82	587.82	587.82	587.82
1,230.000	587.82	587.82	587.81	587.81	587.81
1,245.000	587.81	587.81	587.81	587.80	587.80

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.80	587.80	587.80	587.80	587.80
1,275.000	587.79	587.79	587.79	587.79	587.79
1,290.000	587.79	587.79	587.79	587.78	587.78
1,305.000	587.78	587.78	587.78	587.78	587.78
1,320.000	587.78	587.77	587.77	587.77	587.77
1,335.000	587.77	587.77	587.77	587.77	587.76
1,350.000	587.76	587.76	587.76	587.76	587.76
1,365.000	587.76	587.76	587.76	587.76	587.75
1,380.000	587.75	587.75	587.75	587.75	587.75
1,395.000	587.75	587.75	587.75	587.75	587.75
1,410.000	587.74	587.74	587.74	587.74	587.74
1,425.000	587.74	587.74	587.74	587.74	587.74
1,440.000	587.74	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.00	587.00	587.00	587.00	587.00
15.000	587.00	587.00	587.00	587.00	587.00
30.000	587.00	587.00	587.00	587.00	587.00
45.000	587.00	587.00	587.00	587.00	587.00
60.000	587.00	587.00	587.00	587.00	587.00
75.000	587.00	587.00	587.00	587.00	587.00
90.000	587.00	587.00	587.00	587.00	587.00
105.000	587.00	587.00	587.00	587.00	587.00
120.000	587.00	587.00	587.00	587.00	587.00
135.000	587.00	587.00	587.00	587.00	587.00
150.000	587.00	587.00	587.00	587.00	587.00
165.000	587.00	587.00	587.00	587.00	587.00
180.000	587.00	587.00	587.00	587.00	587.00
195.000	587.00	587.00	587.00	587.00	587.00
210.000	587.00	587.00	587.00	587.00	587.00
225.000	587.00	587.00	587.00	587.00	587.00
240.000	587.00	587.00	587.00	587.00	587.00
255.000	587.00	587.00	587.00	587.00	587.00
270.000	587.00	587.00	587.00	587.00	587.00
285.000	587.00	587.00	587.00	587.00	587.00
300.000	587.00	587.00	587.00	587.00	587.00
315.000	587.00	587.00	587.00	587.00	587.00
330.000	587.00	587.00	587.00	587.00	587.00
345.000	587.00	587.00	587.00	587.00	587.00
360.000	587.00	587.00	587.00	587.00	587.00
375.000	587.00	587.00	587.00	587.00	587.00
390.000	587.00	587.00	587.00	587.00	587.00
405.000	587.00	587.00	587.00	587.00	587.00
420.000	587.00	587.00	587.00	587.00	587.00
435.000	587.00	587.00	587.00	587.00	587.00
450.000	587.00	587.00	587.00	587.00	587.00
465.000	587.00	587.00	587.00	587.00	587.00
480.000	587.00	587.00	587.00	587.00	587.00
495.000	587.00	587.00	587.00	587.00	587.00
510.000	587.00	587.00	587.00	587.00	587.00
525.000	587.00	587.01	587.01	587.01	587.02
540.000	587.02	587.02	587.02	587.02	587.02
555.000	587.02	587.02	587.02	587.02	587.03
570.000	587.05	587.07	587.11	587.14	587.17
585.000	587.19	587.20	587.21	587.21	587.20
600.000	587.20	587.21	587.23	587.26	587.29
615.000	587.33	587.35	587.36	587.36	587.35

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.34	587.32	587.31	587.32	587.34
645.000	587.37	587.41	587.44	587.46	587.47
660.000	587.47	587.47	587.48	587.50	587.53
675.000	587.56	587.58	587.59	587.61	587.63
690.000	587.65	587.67	587.70	587.73	587.76
705.000	587.80	587.85	587.91	587.98	588.09
720.000	588.22	588.37	588.55	588.73	588.91
735.000	589.00	589.00	589.00	589.00	589.00
750.000	589.00	589.00	589.00	589.00	589.00
765.000	589.00	589.00	589.00	589.00	589.00
780.000	589.00	589.00	588.88	588.63	588.46
795.000	588.35	588.25	588.17	588.11	588.08
810.000	588.07	588.08	588.08	588.07	588.04
825.000	587.99	587.94	587.89	587.87	587.88
840.000	587.92	587.96	587.99	588.00	587.98
855.000	587.94	587.89	587.82	587.77	587.74
870.000	587.74	587.79	587.85	587.91	587.95
885.000	587.97	587.95	587.91	587.85	587.79
900.000	587.72	587.65	587.59	587.55	587.55
915.000	587.59	587.67	587.77	587.86	587.91
930.000	587.93	587.93	587.90	587.84	587.78
945.000	587.71	587.64	587.58	587.53	587.48
960.000	587.43	587.41	587.41	587.47	587.57
975.000	587.69	587.80	587.88	587.92	587.92
990.000	587.90	587.85	587.79	587.73	587.66
1,005.000	587.60	587.54	587.49	587.44	587.40
1,020.000	587.36	587.32	587.29	587.26	587.24
1,035.000	587.22	587.22	587.26	587.35	587.49
1,050.000	587.63	587.76	587.86	587.91	587.92
1,065.000	587.90	587.85	587.79	587.73	587.66
1,080.000	587.60	587.54	587.49	587.44	587.40
1,095.000	587.36	587.32	587.29	587.26	587.24
1,110.000	587.22	587.20	587.18	587.16	587.15
1,125.000	587.13	587.12	587.11	587.10	587.09
1,140.000	587.08	587.08	587.08	587.11	587.19
1,155.000	587.33	587.49	587.65	587.78	587.87
1,170.000	587.91	587.92	587.89	587.84	587.78
1,185.000	587.71	587.65	587.59	587.53	587.48
1,200.000	587.43	587.39	587.35	587.32	587.29
1,215.000	587.26	587.23	587.21	587.19	587.17
1,230.000	587.16	587.14	587.13	587.12	587.11
1,245.000	587.10	587.09	587.08	587.08	587.07

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.06	587.06	587.06	587.05	587.05
1,275.000	587.05	587.04	587.05	587.09	587.18
1,290.000	587.32	587.49	587.65	587.78	587.87
1,305.000	587.91	587.92	587.90	587.85	587.78
1,320.000	587.72	587.65	587.59	587.53	587.48
1,335.000	587.44	587.39	587.35	587.32	587.29
1,350.000	587.26	587.23	587.21	587.19	587.17
1,365.000	587.16	587.14	587.13	587.12	587.11
1,380.000	587.10	587.09	587.08	587.08	587.07
1,395.000	587.06	587.06	587.06	587.05	587.05
1,410.000	587.05	587.04	587.04	587.04	587.04
1,425.000	587.03	587.03	587.03	587.03	587.03
1,440.000	587.03	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.00	587.00	587.00	587.00	587.00
15.000	587.00	587.00	587.00	587.00	587.00
30.000	587.00	587.00	587.00	587.00	587.00
45.000	587.00	587.00	587.00	587.00	587.00
60.000	587.00	587.00	587.00	587.00	587.00
75.000	587.00	587.00	587.00	587.00	587.00
90.000	587.00	587.00	587.00	587.00	587.00
105.000	587.00	587.00	587.00	587.00	587.00
120.000	587.00	587.00	587.00	587.00	587.00
135.000	587.00	587.00	587.00	587.00	587.00
150.000	587.00	587.00	587.00	587.00	587.00
165.000	587.00	587.00	587.00	587.00	587.00
180.000	587.00	587.00	587.00	587.00	587.00
195.000	587.00	587.00	587.00	587.00	587.00
210.000	587.00	587.00	587.00	587.00	587.00
225.000	587.00	587.00	587.00	587.00	587.00
240.000	587.00	587.00	587.00	587.00	587.00
255.000	587.00	587.00	587.00	587.00	587.00
270.000	587.00	587.00	587.00	587.00	587.00
285.000	587.00	587.00	587.00	587.00	587.00
300.000	587.00	587.00	587.00	587.00	587.00
315.000	587.00	587.00	587.00	587.00	587.00
330.000	587.00	587.00	587.00	587.00	587.00
345.000	587.00	587.00	587.00	587.00	587.00
360.000	587.00	587.00	587.00	587.00	587.00
375.000	587.00	587.00	587.00	587.00	587.00
390.000	587.00	587.00	587.00	587.00	587.00
405.000	587.00	587.00	587.00	587.00	587.00
420.000	587.00	587.00	587.01	587.01	587.02
435.000	587.02	587.03	587.04	587.05	587.06
450.000	587.07	587.09	587.10	587.11	587.12
465.000	587.13	587.15	587.16	587.17	587.18
480.000	587.19	587.20	587.21	587.22	587.23
495.000	587.24	587.25	587.26	587.27	587.27
510.000	587.28	587.29	587.30	587.31	587.32
525.000	587.33	587.33	587.34	587.35	587.36
540.000	587.37	587.38	587.39	587.40	587.41
555.000	587.42	587.43	587.45	587.47	587.48
570.000	587.50	587.52	587.53	587.55	587.56
585.000	587.58	587.59	587.60	587.62	587.63
600.000	587.64	587.66	587.67	587.68	587.70
615.000	587.71	587.73	587.74	587.75	587.77

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.78	587.80	587.82	587.84	587.86
645.000	587.88	587.90	587.93	587.96	587.99
660.000	588.02	588.05	588.08	588.11	588.14
675.000	588.18	588.21	588.25	588.29	588.33
690.000	588.38	588.42	588.46	588.51	588.57
705.000	588.63	588.70	588.78	588.87	589.00
720.000	589.00	589.00	589.00	589.00	589.00
735.000	589.00	589.00	589.00	589.00	589.00
750.000	589.00	589.00	589.00	589.00	589.00
765.000	589.00	589.00	589.00	589.00	589.00
780.000	589.00	589.00	589.00	589.00	589.00
795.000	589.00	589.00	589.00	589.00	589.00
810.000	589.00	589.00	589.00	589.00	589.00
825.000	589.00	589.00	589.00	589.00	589.00
840.000	589.00	589.00	589.00	589.00	589.00
855.000	589.00	589.00	589.00	589.00	589.00
870.000	589.00	589.00	589.00	589.00	589.00
885.000	589.00	589.00	589.00	589.00	589.00
900.000	589.00	589.00	589.00	589.00	589.00
915.000	589.00	589.00	589.00	589.00	589.00
930.000	589.00	589.00	589.00	589.00	589.00
945.000	589.00	589.00	589.00	589.00	589.00
960.000	589.00	589.00	589.00	589.00	589.00
975.000	588.92	588.50	588.24	588.09	588.00
990.000	587.94	587.90	587.87	587.86	587.85
1,005.000	587.84	587.83	587.83	587.82	587.82
1,020.000	587.81	587.81	587.80	587.80	587.80
1,035.000	587.79	587.79	587.79	587.78	587.78
1,050.000	587.78	587.78	587.77	587.77	587.77
1,065.000	587.76	587.76	587.76	587.76	587.75
1,080.000	587.75	587.75	587.74	587.74	587.74
1,095.000	587.73	587.73	587.72	587.71	587.71
1,110.000	587.70	587.69	587.69	587.68	587.68
1,125.000	587.68	587.67	587.67	587.67	587.66
1,140.000	587.66	587.66	587.66	587.65	587.65
1,155.000	587.65	587.65	587.64	587.64	587.64
1,170.000	587.64	587.64	587.64	587.63	587.63
1,185.000	587.63	587.63	587.63	587.62	587.62
1,200.000	587.62	587.62	587.62	587.62	587.61
1,215.000	587.61	587.61	587.61	587.61	587.61
1,230.000	587.60	587.60	587.60	587.60	587.60
1,245.000	587.60	587.59	587.59	587.59	587.59

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.59	587.59	587.59	587.58	587.58
1,275.000	587.58	587.58	587.58	587.58	587.58
1,290.000	587.57	587.57	587.57	587.57	587.57
1,305.000	587.57	587.57	587.57	587.56	587.56
1,320.000	587.56	587.56	587.56	587.56	587.56
1,335.000	587.56	587.55	587.55	587.55	587.55
1,350.000	587.55	587.55	587.55	587.55	587.54
1,365.000	587.54	587.54	587.54	587.54	587.54
1,380.000	587.54	587.54	587.54	587.53	587.53
1,395.000	587.53	587.53	587.53	587.53	587.53
1,410.000	587.53	587.53	587.53	587.52	587.52
1,425.000	587.52	587.52	587.52	587.52	587.52
1,440.000	587.52	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.00	587.00	587.00	587.00	587.00
15.000	587.00	587.00	587.00	587.00	587.00
30.000	587.00	587.00	587.00	587.00	587.00
45.000	587.00	587.00	587.00	587.00	587.00
60.000	587.00	587.00	587.00	587.00	587.00
75.000	587.00	587.00	587.00	587.00	587.00
90.000	587.00	587.00	587.00	587.00	587.00
105.000	587.00	587.00	587.00	587.00	587.00
120.000	587.00	587.00	587.00	587.00	587.00
135.000	587.00	587.00	587.00	587.00	587.00
150.000	587.00	587.00	587.00	587.00	587.00
165.000	587.00	587.00	587.00	587.00	587.00
180.000	587.00	587.00	587.00	587.00	587.00
195.000	587.00	587.00	587.00	587.00	587.00
210.000	587.00	587.00	587.00	587.00	587.00
225.000	587.00	587.00	587.00	587.00	587.00
240.000	587.00	587.00	587.00	587.00	587.00
255.000	587.00	587.00	587.00	587.00	587.00
270.000	587.00	587.00	587.00	587.00	587.00
285.000	587.00	587.00	587.00	587.00	587.00
300.000	587.00	587.00	587.00	587.00	587.00
315.000	587.00	587.00	587.00	587.00	587.00
330.000	587.00	587.00	587.00	587.00	587.00
345.000	587.00	587.00	587.00	587.00	587.00
360.000	587.00	587.00	587.00	587.01	587.01
375.000	587.02	587.02	587.03	587.04	587.05
390.000	587.07	587.08	587.09	587.11	587.12
405.000	587.14	587.15	587.16	587.18	587.19
420.000	587.20	587.21	587.23	587.24	587.25
435.000	587.26	587.27	587.28	587.29	587.30
450.000	587.31	587.32	587.33	587.34	587.34
465.000	587.35	587.36	587.37	587.38	587.39
480.000	587.40	587.41	587.41	587.42	587.43
495.000	587.44	587.45	587.46	587.47	587.47
510.000	587.48	587.49	587.50	587.51	587.52
525.000	587.53	587.53	587.54	587.55	587.56
540.000	587.57	587.58	587.59	587.60	587.61
555.000	587.63	587.65	587.67	587.69	587.72
570.000	587.74	587.76	587.78	587.80	587.81
585.000	587.83	587.85	587.86	587.88	587.89
600.000	587.91	587.92	587.94	587.95	587.97
615.000	587.98	588.00	588.02	588.03	588.05

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	588.07	588.08	588.10	588.13	588.15
645.000	588.18	588.21	588.25	588.28	588.32
660.000	588.36	588.39	588.43	588.46	588.50
675.000	588.54	588.58	588.62	588.67	588.71
690.000	588.76	588.80	588.85	588.90	588.96
705.000	589.00	589.00	589.00	589.00	589.00
720.000	589.00	589.00	589.00	589.00	589.00
735.000	589.00	589.00	589.00	589.00	589.00
750.000	589.00	589.00	589.00	589.00	589.00
765.000	589.00	589.00	589.00	589.00	589.00
780.000	589.00	589.00	589.00	589.00	589.00
795.000	589.00	589.00	589.00	589.00	589.00
810.000	589.00	589.00	589.00	589.00	589.00
825.000	589.00	589.00	589.00	589.00	589.00
840.000	589.00	589.00	589.00	589.00	589.00
855.000	589.00	589.00	589.00	589.00	589.00
870.000	589.00	589.00	589.00	589.00	589.00
885.000	589.00	589.00	589.00	589.00	589.00
900.000	589.00	589.00	589.00	589.00	589.00
915.000	589.00	589.00	589.00	589.00	589.00
930.000	589.00	589.00	589.00	589.00	589.00
945.000	589.00	589.00	589.00	589.00	589.00
960.000	589.00	589.00	589.00	589.00	589.00
975.000	589.00	589.00	589.00	589.00	589.00
990.000	589.00	589.00	589.00	589.00	589.00
1,005.000	589.00	589.00	589.00	589.00	589.00
1,020.000	589.00	589.00	589.00	589.00	589.00
1,035.000	589.00	589.00	589.00	589.00	589.00
1,050.000	589.00	589.00	589.00	589.00	589.00
1,065.000	589.00	589.00	589.00	589.00	589.00
1,080.000	589.00	589.00	589.00	589.00	588.81
1,095.000	588.43	588.19	588.05	587.96	587.90
1,110.000	587.86	587.83	587.81	587.80	587.79
1,125.000	587.78	587.77	587.77	587.76	587.76
1,140.000	587.75	587.75	587.75	587.75	587.74
1,155.000	587.74	587.74	587.73	587.73	587.73
1,170.000	587.73	587.73	587.72	587.72	587.72
1,185.000	587.72	587.72	587.71	587.71	587.71
1,200.000	587.71	587.71	587.70	587.70	587.70
1,215.000	587.70	587.70	587.69	587.69	587.69
1,230.000	587.69	587.69	587.69	587.68	587.68
1,245.000	587.68	587.68	587.68	587.67	587.67

Subsection: Time vs. Elevation
 Label: R3
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.67	587.67	587.67	587.67	587.66
1,275.000	587.66	587.66	587.66	587.66	587.66
1,290.000	587.66	587.65	587.65	587.65	587.65
1,305.000	587.65	587.65	587.65	587.64	587.64
1,320.000	587.64	587.64	587.64	587.64	587.64
1,335.000	587.63	587.63	587.63	587.63	587.63
1,350.000	587.63	587.63	587.62	587.62	587.62
1,365.000	587.62	587.62	587.62	587.62	587.62
1,380.000	587.61	587.61	587.61	587.61	587.61
1,395.000	587.61	587.61	587.61	587.60	587.60
1,410.000	587.60	587.60	587.60	587.60	587.60
1,425.000	587.60	587.60	587.59	587.59	587.59
1,440.000	587.59	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.00	587.00	587.00	587.00	587.00
15.000	587.00	587.00	587.00	587.00	587.00
30.000	587.00	587.00	587.00	587.00	587.00
45.000	587.00	587.00	587.00	587.00	587.00
60.000	587.00	587.00	587.00	587.00	587.00
75.000	587.00	587.00	587.00	587.00	587.00
90.000	587.00	587.00	587.00	587.00	587.00
105.000	587.00	587.00	587.00	587.00	587.00
120.000	587.00	587.00	587.00	587.00	587.00
135.000	587.00	587.00	587.00	587.00	587.00
150.000	587.00	587.00	587.00	587.00	587.00
165.000	587.00	587.00	587.00	587.00	587.00
180.000	587.00	587.00	587.00	587.00	587.00
195.000	587.00	587.00	587.00	587.00	587.00
210.000	587.00	587.00	587.00	587.00	587.00
225.000	587.00	587.00	587.00	587.00	587.00
240.000	587.00	587.00	587.00	587.00	587.00
255.000	587.00	587.00	587.00	587.00	587.00
270.000	587.00	587.00	587.00	587.00	587.00
285.000	587.00	587.00	587.00	587.00	587.01
300.000	587.01	587.02	587.03	587.04	587.05
315.000	587.06	587.07	587.08	587.10	587.11
330.000	587.12	587.14	587.15	587.16	587.17
345.000	587.19	587.20	587.21	587.22	587.23
360.000	587.24	587.25	587.26	587.27	587.28
375.000	587.29	587.31	587.32	587.33	587.35
390.000	587.36	587.38	587.39	587.40	587.42
405.000	587.43	587.44	587.45	587.46	587.47
420.000	587.48	587.49	587.50	587.51	587.52
435.000	587.53	587.54	587.55	587.56	587.57
450.000	587.58	587.59	587.60	587.61	587.61
465.000	587.62	587.63	587.64	587.65	587.66
480.000	587.67	587.68	587.69	587.70	587.71
495.000	587.72	587.72	587.73	587.74	587.75
510.000	587.76	587.77	587.78	587.79	587.80
525.000	587.81	587.82	587.83	587.84	587.85
540.000	587.86	587.87	587.88	587.90	587.92
555.000	587.94	587.97	588.01	588.04	588.08
570.000	588.11	588.14	588.16	588.19	588.21
585.000	588.23	588.25	588.27	588.29	588.31
600.000	588.33	588.35	588.36	588.38	588.40
615.000	588.42	588.43	588.45	588.47	588.49

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	588.51	588.53	588.56	588.58	588.62
645.000	588.66	588.70	588.75	588.79	588.84
660.000	588.89	588.93	588.97	589.00	589.00
675.000	589.00	589.00	589.00	589.00	589.00
690.000	589.00	589.00	589.00	589.00	589.00
705.000	589.00	589.00	589.00	589.00	589.00
720.000	589.00	589.00	589.00	589.00	589.00
735.000	589.00	589.00	589.00	589.00	589.00
750.000	589.00	589.00	589.00	589.00	589.00
765.000	589.00	589.00	589.00	589.00	589.00
780.000	589.00	589.00	589.00	589.00	589.00
795.000	589.00	589.00	589.00	589.00	589.00
810.000	589.00	589.00	589.00	589.00	589.00
825.000	589.00	589.00	589.00	589.00	589.00
840.000	589.00	589.00	589.00	589.00	589.00
855.000	589.00	589.00	589.00	589.00	589.00
870.000	589.00	589.00	589.00	589.00	589.00
885.000	589.00	589.00	589.00	589.00	589.00
900.000	589.00	589.00	589.00	589.00	589.00
915.000	589.00	589.00	589.00	589.00	589.00
930.000	589.00	589.00	589.00	589.00	589.00
945.000	589.00	589.00	589.00	589.00	589.00
960.000	589.00	589.00	589.00	589.00	589.00
975.000	589.00	589.00	589.00	589.00	589.00
990.000	589.00	589.00	589.00	589.00	589.00
1,005.000	589.00	589.00	589.00	589.00	589.00
1,020.000	589.00	589.00	589.00	589.00	589.00
1,035.000	589.00	589.00	589.00	589.00	589.00
1,050.000	589.00	589.00	589.00	589.00	589.00
1,065.000	589.00	589.00	589.00	589.00	589.00
1,080.000	589.00	589.00	589.00	589.00	589.00
1,095.000	589.00	589.00	589.00	589.00	589.00
1,110.000	589.00	589.00	589.00	589.00	589.00
1,125.000	589.00	589.00	589.00	589.00	589.00
1,140.000	589.00	589.00	589.00	589.00	589.00
1,155.000	589.00	589.00	589.00	589.00	589.00
1,170.000	589.00	589.00	589.00	589.00	589.00
1,185.000	589.00	589.00	589.00	589.00	589.00
1,200.000	589.00	589.00	589.00	589.00	589.00
1,215.000	589.00	589.00	589.00	589.00	589.00
1,230.000	589.00	589.00	589.00	589.00	589.00
1,245.000	589.00	589.00	589.00	589.00	589.00

Subsection: Time vs. Elevation
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	589.00	589.00	589.00	589.00	589.00
1,275.000	589.00	589.00	589.00	589.00	589.00
1,290.000	589.00	589.00	589.00	589.00	589.00
1,305.000	589.00	589.00	589.00	589.00	589.00
1,320.000	589.00	589.00	589.00	589.00	588.97
1,335.000	588.49	588.21	588.04	587.93	587.86
1,350.000	587.81	587.78	587.76	587.75	587.74
1,365.000	587.74	587.73	587.73	587.73	587.73
1,380.000	587.72	587.72	587.72	587.72	587.72
1,395.000	587.72	587.71	587.71	587.71	587.71
1,410.000	587.71	587.71	587.71	587.70	587.70
1,425.000	587.70	587.70	587.70	587.70	587.70
1,440.000	587.70	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	0.000	0.000
465.000	0.000	0.000	0.000	0.000	0.000
480.000	0.000	0.000	0.000	0.000	0.000
495.000	0.000	0.000	0.000	0.000	0.000
510.000	0.000	0.000	2.000	4.000	9.000
525.000	16.000	26.000	39.000	54.000	71.000
540.000	88.000	105.000	119.000	127.000	130.000
555.000	130.000	129.000	129.000	134.000	148.000
570.000	171.000	206.000	249.000	297.000	343.000
585.000	380.000	407.000	421.000	424.000	428.000
600.000	441.000	469.000	516.000	580.000	659.000
615.000	735.000	798.000	842.000	859.000	854.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	837.000	819.000	818.000	839.000	888.000
645.000	962.000	1,044.000	1,123.000	1,192.000	1,252.000
660.000	1,310.000	1,374.000	1,454.000	1,560.000	1,683.000
675.000	1,805.000	1,916.000	2,018.000	2,115.000	2,221.000
690.000	2,355.000	2,511.000	2,684.000	2,870.000	3,081.000
705.000	3,325.000	3,636.000	4,048.000	4,632.000	5,469.000
720.000	6,593.000	8,094.000	9,978.000	12,242.000	14,797.000
735.000	17,499.000	20,215.000	22,684.000	24,805.000	26,495.000
750.000	27,647.000	28,302.000	28,457.000	28,180.000	27,481.000
765.000	26,440.000	25,158.000	23,676.000	22,161.000	20,690.000
780.000	19,301.000	18,027.000	16,840.000	15,710.000	14,620.000
795.000	13,582.000	12,612.000	11,727.000	10,938.000	10,289.000
810.000	9,790.000	9,369.000	8,980.000	8,568.000	8,097.000
825.000	7,585.000	7,083.000	6,640.000	6,322.000	6,139.000
840.000	6,079.000	6,102.000	6,116.000	6,075.000	5,957.000
855.000	5,740.000	5,450.000	5,143.000	4,866.000	4,681.000
870.000	4,603.000	4,633.000	4,747.000	4,875.000	4,980.000
885.000	5,029.000	4,986.000	4,852.000	4,634.000	4,356.000
900.000	4,044.000	3,708.000	3,397.000	3,140.000	2,986.000
915.000	2,978.000	3,115.000	3,398.000	3,744.000	4,050.000
930.000	4,290.000	4,401.000	4,368.000	4,225.000	3,998.000
945.000	3,729.000	3,434.000	3,118.000	2,776.000	2,429.000
960.000	2,137.000	1,926.000	1,847.000	1,925.000	2,160.000
975.000	2,531.000	2,949.000	3,349.000	3,668.000	3,872.000
990.000	3,964.000	3,932.000	3,803.000	3,612.000	3,373.000
1,005.000	3,104.000	2,804.000	2,494.000	2,185.000	1,892.000
1,020.000	1,633.000	1,407.000	1,217.000	1,060.000	929.000
1,035.000	826.000	792.000	866.000	1,064.000	1,393.000
1,050.000	1,829.000	2,334.000	2,833.000	3,254.000	3,575.000
1,065.000	3,747.000	3,773.000	3,694.000	3,532.000	3,327.000
1,080.000	3,085.000	2,812.000	2,510.000	2,194.000	1,900.000
1,095.000	1,633.000	1,405.000	1,217.000	1,059.000	930.000
1,110.000	817.000	718.000	631.000	556.000	492.000
1,125.000	438.000	391.000	350.000	315.000	285.000
1,140.000	261.000	254.000	287.000	386.000	586.000
1,155.000	902.000	1,346.000	1,870.000	2,408.000	2,911.000
1,170.000	3,297.000	3,547.000	3,667.000	3,652.000	3,546.000
1,185.000	3,373.000	3,156.000	2,901.000	2,616.000	2,321.000
1,200.000	2,021.000	1,745.000	1,506.000	1,300.000	1,130.000
1,215.000	986.000	864.000	759.000	668.000	589.000
1,230.000	522.000	464.000	415.000	372.000	335.000
1,245.000	303.000	275.000	252.000	232.000	215.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	201.000	188.000	177.000	168.000	160.000
1,275.000	153.000	149.000	174.000	277.000	485.000
1,290.000	814.000	1,259.000	1,805.000	2,394.000	2,917.000
1,305.000	3,338.000	3,617.000	3,731.000	3,717.000	3,600.000
1,320.000	3,422.000	3,204.000	2,952.000	2,663.000	2,341.000
1,335.000	2,031.000	1,750.000	1,504.000	1,299.000	1,128.000
1,350.000	986.000	867.000	762.000	669.000	589.000
1,365.000	522.000	465.000	416.000	372.000	335.000
1,380.000	303.000	275.000	252.000	232.000	215.000
1,395.000	201.000	188.000	178.000	168.000	160.000
1,410.000	153.000	147.000	142.000	138.000	134.000
1,425.000	131.000	128.000	125.000	123.000	120.000
1,440.000	117.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	1.000	3.000	7.000
420.000	12.000	21.000	33.000	51.000	75.000
435.000	105.000	135.000	157.000	173.000	188.000
450.000	203.000	218.000	234.000	251.000	268.000
465.000	286.000	305.000	325.000	345.000	365.000
480.000	386.000	408.000	430.000	453.000	476.000
495.000	500.000	524.000	549.000	575.000	601.000
510.000	628.000	655.000	683.000	711.000	740.000
525.000	770.000	801.000	832.000	864.000	897.000
540.000	931.000	966.000	1,004.000	1,045.000	1,091.000
555.000	1,143.000	1,200.000	1,263.000	1,331.000	1,404.000
570.000	1,481.000	1,561.000	1,643.000	1,726.000	1,811.000
585.000	1,898.000	1,986.000	2,074.000	2,163.000	2,254.000
600.000	2,346.000	2,439.000	2,535.000	2,633.000	2,734.000
615.000	2,838.000	2,945.000	3,055.000	3,169.000	3,287.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	3,411.000	3,542.000	3,682.000	3,836.000	4,005.000
645.000	4,194.000	4,403.000	4,634.000	4,883.000	5,151.000
660.000	5,435.000	5,738.000	6,059.000	6,404.000	6,776.000
675.000	7,178.000	7,615.000	8,085.000	8,593.000	9,137.000
690.000	9,723.000	10,355.000	11,046.000	11,799.000	12,648.000
705.000	13,613.000	14,738.000	16,072.000	17,747.000	19,956.000
720.000	22,887.000	26,923.000	32,205.000	37,685.000	42,707.000
735.000	47,774.000	52,720.000	56,861.000	60,108.000	62,318.000
750.000	63,304.000	63,386.000	62,615.000	61,244.000	59,357.000
765.000	57,057.000	54,482.000	51,662.000	48,928.000	46,325.000
780.000	43,900.000	41,718.000	39,734.000	37,949.000	36,319.000
795.000	34,828.000	33,453.000	32,184.000	30,784.000	29,166.000
810.000	27,563.000	26,035.000	24,616.000	23,296.000	22,075.000
825.000	20,939.000	19,880.000	18,887.000	17,960.000	17,097.000
840.000	16,291.000	15,547.000	14,856.000	14,222.000	13,635.000
855.000	13,098.000	12,603.000	12,150.000	11,735.000	11,353.000
870.000	11,001.000	10,673.000	10,368.000	10,083.000	9,820.000
885.000	9,576.000	9,353.000	9,150.000	8,962.000	8,787.000
900.000	8,621.000	8,463.000	8,306.000	8,149.000	7,985.000
915.000	7,811.000	7,629.000	7,437.000	7,240.000	7,040.000
930.000	6,842.000	6,648.000	6,462.000	6,285.000	6,118.000
945.000	5,962.000	5,817.000	5,683.000	5,561.000	5,448.000
960.000	5,345.000	5,249.000	5,160.000	5,078.000	5,000.000
975.000	4,927.000	4,858.000	4,794.000	4,732.000	4,673.000
990.000	4,617.000	4,563.000	4,512.000	4,463.000	4,415.000
1,005.000	4,369.000	4,325.000	4,282.000	4,241.000	4,200.000
1,020.000	4,160.000	4,122.000	4,084.000	4,047.000	4,012.000
1,035.000	3,977.000	3,943.000	3,910.000	3,877.000	3,846.000
1,050.000	3,815.000	3,784.000	3,754.000	3,725.000	3,696.000
1,065.000	3,668.000	3,641.000	3,614.000	3,588.000	3,562.000
1,080.000	3,536.000	3,510.000	3,483.000	3,454.000	3,422.000
1,095.000	3,387.000	3,349.000	3,308.000	3,264.000	3,220.000
1,110.000	3,174.000	3,130.000	3,087.000	3,046.000	3,006.000
1,125.000	2,968.000	2,933.000	2,900.000	2,869.000	2,840.000
1,140.000	2,814.000	2,788.000	2,765.000	2,742.000	2,720.000
1,155.000	2,700.000	2,680.000	2,661.000	2,643.000	2,626.000
1,170.000	2,609.000	2,593.000	2,577.000	2,562.000	2,547.000
1,185.000	2,533.000	2,518.000	2,504.000	2,490.000	2,476.000
1,200.000	2,463.000	2,450.000	2,436.000	2,423.000	2,411.000
1,215.000	2,398.000	2,386.000	2,374.000	2,362.000	2,350.000
1,230.000	2,338.000	2,326.000	2,315.000	2,303.000	2,292.000
1,245.000	2,281.000	2,270.000	2,259.000	2,248.000	2,238.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	2,227.000	2,217.000	2,207.000	2,197.000	2,188.000
1,275.000	2,178.000	2,168.000	2,159.000	2,149.000	2,140.000
1,290.000	2,131.000	2,122.000	2,113.000	2,104.000	2,095.000
1,305.000	2,086.000	2,077.000	2,069.000	2,060.000	2,052.000
1,320.000	2,043.000	2,035.000	2,027.000	2,018.000	2,010.000
1,335.000	2,002.000	1,994.000	1,986.000	1,978.000	1,970.000
1,350.000	1,962.000	1,954.000	1,947.000	1,939.000	1,932.000
1,365.000	1,925.000	1,918.000	1,911.000	1,904.000	1,897.000
1,380.000	1,890.000	1,883.000	1,876.000	1,869.000	1,862.000
1,395.000	1,855.000	1,849.000	1,842.000	1,836.000	1,829.000
1,410.000	1,823.000	1,817.000	1,811.000	1,805.000	1,799.000
1,425.000	1,793.000	1,787.000	1,781.000	1,774.000	1,768.000
1,440.000	1,761.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	1.000	3.000
360.000	5.000	10.000	18.000	29.000	46.000
375.000	70.000	101.000	133.000	158.000	177.000
390.000	195.000	213.000	232.000	252.000	273.000
405.000	294.000	317.000	341.000	365.000	390.000
420.000	416.000	442.000	469.000	496.000	524.000
435.000	553.000	582.000	611.000	642.000	672.000
450.000	703.000	735.000	767.000	800.000	833.000
465.000	867.000	901.000	936.000	971.000	1,007.000
480.000	1,044.000	1,082.000	1,120.000	1,158.000	1,198.000
495.000	1,238.000	1,279.000	1,320.000	1,363.000	1,406.000
510.000	1,450.000	1,495.000	1,540.000	1,587.000	1,635.000
525.000	1,683.000	1,733.000	1,784.000	1,836.000	1,889.000
540.000	1,944.000	2,003.000	2,066.000	2,137.000	2,218.000
555.000	2,310.000	2,416.000	2,534.000	2,663.000	2,802.000
570.000	2,948.000	3,099.000	3,252.000	3,408.000	3,564.000
585.000	3,722.000	3,879.000	4,036.000	4,192.000	4,349.000
600.000	4,506.000	4,665.000	4,826.000	4,989.000	5,157.000
615.000	5,328.000	5,503.000	5,683.000	5,868.000	6,059.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	6,259.000	6,469.000	6,698.000	6,950.000	7,232.000
645.000	7,552.000	7,910.000	8,306.000	8,736.000	9,198.000
660.000	9,687.000	10,204.000	10,747.000	11,321.000	11,933.000
675.000	12,584.000	13,279.000	14,018.000	14,805.000	15,637.000
690.000	16,523.000	17,464.000	18,474.000	19,553.000	20,738.000
705.000	22,050.000	23,552.000	25,307.000	27,496.000	30,374.000
720.000	33,851.000	37,777.000	42,288.000	47,804.000	54,038.000
735.000	60,511.000	66,819.000	72,058.000	76,107.000	78,797.000
750.000	79,891.000	79,815.000	78,645.000	76,715.000	74,147.000
765.000	71,070.000	67,673.000	63,993.000	60,449.000	57,099.000
780.000	53,992.000	51,206.000	48,680.000	46,413.000	44,344.000
795.000	42,452.000	40,707.000	39,115.000	37,674.000	36,343.000
810.000	35,132.000	34,007.000	32,973.000	31,945.000	30,764.000
825.000	29,423.000	28,066.000	26,741.000	25,475.000	24,278.000
840.000	23,149.000	22,098.000	21,119.000	20,214.000	19,376.000
855.000	18,606.000	17,897.000	17,249.000	16,655.000	16,110.000
870.000	15,610.000	15,145.000	14,714.000	14,312.000	13,941.000
885.000	13,596.000	13,283.000	12,996.000	12,731.000	12,484.000
900.000	12,250.000	12,025.000	11,802.000	11,575.000	11,336.000
915.000	11,079.000	10,805.000	10,515.000	10,214.000	9,908.000
930.000	9,603.000	9,305.000	9,019.000	8,747.000	8,491.000
945.000	8,251.000	8,030.000	7,826.000	7,641.000	7,472.000
960.000	7,318.000	7,176.000	7,045.000	6,924.000	6,811.000
975.000	6,706.000	6,608.000	6,515.000	6,427.000	6,344.000
990.000	6,265.000	6,189.000	6,117.000	6,048.000	5,982.000
1,005.000	5,918.000	5,857.000	5,797.000	5,740.000	5,684.000
1,020.000	5,629.000	5,576.000	5,525.000	5,475.000	5,426.000
1,035.000	5,378.000	5,332.000	5,287.000	5,243.000	5,199.000
1,050.000	5,157.000	5,116.000	5,075.000	5,036.000	4,997.000
1,065.000	4,958.000	4,921.000	4,884.000	4,848.000	4,812.000
1,080.000	4,777.000	4,741.000	4,702.000	4,660.000	4,613.000
1,095.000	4,560.000	4,500.000	4,434.000	4,365.000	4,292.000
1,110.000	4,218.000	4,145.000	4,074.000	4,006.000	3,942.000
1,125.000	3,881.000	3,824.000	3,772.000	3,724.000	3,680.000
1,140.000	3,640.000	3,602.000	3,567.000	3,534.000	3,503.000
1,155.000	3,473.000	3,446.000	3,419.000	3,394.000	3,370.000
1,170.000	3,347.000	3,324.000	3,302.000	3,281.000	3,261.000
1,185.000	3,241.000	3,222.000	3,203.000	3,184.000	3,166.000
1,200.000	3,148.000	3,130.000	3,113.000	3,096.000	3,079.000
1,215.000	3,062.000	3,046.000	3,030.000	3,014.000	2,999.000
1,230.000	2,984.000	2,969.000	2,954.000	2,939.000	2,925.000
1,245.000	2,910.000	2,896.000	2,882.000	2,868.000	2,854.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	2,840.000	2,827.000	2,813.000	2,800.000	2,787.000
1,275.000	2,775.000	2,762.000	2,750.000	2,737.000	2,725.000
1,290.000	2,714.000	2,702.000	2,690.000	2,679.000	2,667.000
1,305.000	2,656.000	2,644.000	2,633.000	2,622.000	2,611.000
1,320.000	2,599.000	2,588.000	2,577.000	2,566.000	2,555.000
1,335.000	2,545.000	2,534.000	2,524.000	2,514.000	2,504.000
1,350.000	2,494.000	2,484.000	2,475.000	2,465.000	2,456.000
1,365.000	2,447.000	2,437.000	2,428.000	2,419.000	2,410.000
1,380.000	2,401.000	2,392.000	2,383.000	2,374.000	2,364.000
1,395.000	2,356.000	2,347.000	2,338.000	2,330.000	2,321.000
1,410.000	2,313.000	2,305.000	2,297.000	2,288.000	2,280.000
1,425.000	2,272.000	2,264.000	2,256.000	2,248.000	2,240.000
1,440.000	2,230.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	3.000	6.000	11.000	19.000	32.000
300.000	51.000	76.000	110.000	141.000	164.000
315.000	182.000	199.000	216.000	234.000	253.000
330.000	273.000	294.000	316.000	338.000	361.000
345.000	384.000	408.000	432.000	457.000	483.000
360.000	509.000	535.000	564.000	594.000	626.000
375.000	662.000	701.000	743.000	788.000	835.000
390.000	885.000	936.000	988.000	1,041.000	1,095.000
405.000	1,149.000	1,203.000	1,258.000	1,312.000	1,366.000
420.000	1,421.000	1,475.000	1,530.000	1,585.000	1,641.000
435.000	1,697.000	1,753.000	1,810.000	1,867.000	1,925.000
450.000	1,984.000	2,043.000	2,103.000	2,163.000	2,224.000
465.000	2,287.000	2,349.000	2,413.000	2,478.000	2,544.000
480.000	2,610.000	2,678.000	2,747.000	2,816.000	2,887.000
495.000	2,959.000	3,032.000	3,106.000	3,182.000	3,259.000
510.000	3,337.000	3,417.000	3,498.000	3,581.000	3,665.000
525.000	3,751.000	3,838.000	3,928.000	4,019.000	4,113.000
540.000	4,210.000	4,315.000	4,433.000	4,570.000	4,735.000
555.000	4,931.000	5,163.000	5,428.000	5,722.000	6,039.000
570.000	6,372.000	6,715.000	7,060.000	7,407.000	7,751.000
585.000	8,093.000	8,429.000	8,758.000	9,081.000	9,398.000
600.000	9,711.000	10,023.000	10,334.000	10,647.000	10,964.000
615.000	11,284.000	11,609.000	11,941.000	12,280.000	12,627.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	12,989.000	13,370.000	13,789.000	14,256.000	14,786.000
645.000	15,398.000	16,087.000	16,858.000	17,697.000	18,596.000
660.000	19,543.000	20,533.000	21,563.000	22,637.000	23,763.000
675.000	24,941.000	26,179.000	27,475.000	28,833.000	30,248.000
690.000	31,727.000	33,090.000	34,202.000	35,202.000	36,228.000
705.000	37,310.000	38,523.000	39,917.000	41,702.000	44,120.000
720.000	47,384.000	51,982.000	57,963.000	65,401.000	73,825.000
735.000	82,543.000	90,996.000	97,145.000	101,004.000	103,375.000
750.000	104,024.000	103,525.000	101,916.000	99,596.000	96,684.000
765.000	93,305.000	89,501.000	84,930.000	80,234.000	75,796.000
780.000	71,704.000	68,061.000	64,771.000	61,824.000	59,139.000
795.000	56,674.000	54,392.000	52,294.000	50,377.000	48,595.000
810.000	46,955.000	45,419.000	43,991.000	42,639.000	41,357.000
825.000	40,127.000	38,939.000	37,790.000	36,689.000	35,638.000
840.000	34,643.000	33,713.000	32,844.000	31,986.000	31,002.000
855.000	29,913.000	28,835.000	27,822.000	26,885.000	26,022.000
870.000	25,231.000	24,497.000	23,818.000	23,186.000	22,602.000
885.000	22,061.000	21,568.000	21,116.000	20,698.000	20,307.000
900.000	19,936.000	19,577.000	19,215.000	18,838.000	18,432.000
915.000	17,982.000	17,494.000	16,968.000	16,416.000	15,851.000
930.000	15,283.000	14,729.000	14,195.000	13,688.000	13,213.000
945.000	12,770.000	12,364.000	11,993.000	11,659.000	11,357.000
960.000	11,084.000	10,837.000	10,611.000	10,404.000	10,213.000
975.000	10,037.000	9,873.000	9,721.000	9,577.000	9,443.000
990.000	9,315.000	9,195.000	9,081.000	8,972.000	8,868.000
1,005.000	8,768.000	8,673.000	8,582.000	8,493.000	8,407.000
1,020.000	8,324.000	8,244.000	8,165.000	8,089.000	8,015.000
1,035.000	7,942.000	7,872.000	7,804.000	7,737.000	7,672.000
1,050.000	7,608.000	7,546.000	7,486.000	7,426.000	7,368.000
1,065.000	7,311.000	7,255.000	7,201.000	7,147.000	7,094.000
1,080.000	7,041.000	6,987.000	6,927.000	6,860.000	6,782.000
1,095.000	6,693.000	6,589.000	6,473.000	6,348.000	6,216.000
1,110.000	6,082.000	5,951.000	5,823.000	5,701.000	5,585.000
1,125.000	5,477.000	5,377.000	5,286.000	5,203.000	5,128.000
1,140.000	5,060.000	4,998.000	4,941.000	4,888.000	4,839.000
1,155.000	4,792.000	4,749.000	4,708.000	4,669.000	4,632.000
1,170.000	4,596.000	4,563.000	4,530.000	4,499.000	4,469.000
1,185.000	4,440.000	4,411.000	4,384.000	4,357.000	4,330.000
1,200.000	4,304.000	4,279.000	4,254.000	4,229.000	4,205.000
1,215.000	4,182.000	4,159.000	4,136.000	4,114.000	4,092.000
1,230.000	4,071.000	4,049.000	4,028.000	4,008.000	3,987.000
1,245.000	3,967.000	3,948.000	3,928.000	3,909.000	3,890.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,871.000	3,852.000	3,834.000	3,815.000	3,797.000
1,275.000	3,779.000	3,762.000	3,744.000	3,727.000	3,710.000
1,290.000	3,693.000	3,676.000	3,660.000	3,644.000	3,628.000
1,305.000	3,612.000	3,596.000	3,580.000	3,565.000	3,549.000
1,320.000	3,533.000	3,518.000	3,502.000	3,487.000	3,472.000
1,335.000	3,457.000	3,442.000	3,428.000	3,413.000	3,399.000
1,350.000	3,385.000	3,371.000	3,357.000	3,344.000	3,330.000
1,365.000	3,317.000	3,304.000	3,291.000	3,279.000	3,266.000
1,380.000	3,253.000	3,241.000	3,228.000	3,216.000	3,204.000
1,395.000	3,191.000	3,179.000	3,167.000	3,155.000	3,143.000
1,410.000	3,132.000	3,121.000	3,109.000	3,098.000	3,087.000
1,425.000	3,076.000	3,065.000	3,054.000	3,043.000	3,032.000
1,440.000	3,019.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	0.000	0.000
465.000	0.000	0.000	0.000	0.000	0.000
480.000	0.000	0.000	0.000	0.000	0.000
495.000	0.000	0.000	0.000	0.000	0.000
510.000	0.000	0.000	1.000	3.000	6.000
525.000	11.000	18.000	27.000	37.000	48.000
540.000	60.000	72.000	86.000	104.000	124.000
555.000	140.000	146.000	149.000	156.000	170.000
570.000	195.000	234.000	286.000	345.000	408.000
585.000	463.000	506.000	534.000	547.000	553.000
600.000	565.000	589.000	631.000	698.000	790.000
615.000	893.000	992.000	1,070.000	1,118.000	1,135.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
630.000	1,126.000	1,107.000	1,093.000	1,099.000	1,132.000
645.000	1,199.000	1,300.000	1,413.000	1,522.000	1,621.000
660.000	1,709.000	1,796.000	1,890.000	2,006.000	2,150.000
675.000	2,311.000	2,472.000	2,626.000	2,770.000	2,918.000
690.000	3,090.000	3,292.000	3,519.000	3,763.000	4,023.000
705.000	4,322.000	4,699.000	5,201.000	5,894.000	6,858.000
720.000	8,176.000	9,927.000	12,193.000	14,974.000	18,209.000
735.000	21,762.000	25,432.000	28,996.000	31,577.000	33,060.000
750.000	34,058.000	34,637.000	34,815.000	34,618.000	34,078.000
765.000	33,240.000	32,206.000	31,032.000	29,832.000	28,511.000
780.000	26,910.000	25,162.000	23,484.000	21,921.000	20,443.000
795.000	19,039.000	17,702.000	16,443.000	15,296.000	14,289.000
810.000	13,477.000	12,850.000	12,301.000	11,770.000	11,192.000
825.000	10,547.000	9,896.000	9,287.000	8,777.000	8,408.000
840.000	8,191.000	8,118.000	8,113.000	8,112.000	8,038.000
855.000	7,851.000	7,558.000	7,187.000	6,808.000	6,480.000
870.000	6,249.000	6,137.000	6,164.000	6,314.000	6,496.000
885.000	6,651.000	6,716.000	6,653.000	6,464.000	6,158.000
900.000	5,768.000	5,316.000	4,860.000	4,450.000	4,149.000
915.000	4,011.000	4,073.000	4,349.000	4,763.000	5,214.000
930.000	5,601.000	5,851.000	5,932.000	5,846.000	5,623.000
945.000	5,297.000	4,897.000	4,440.000	3,955.000	3,470.000
960.000	3,037.000	2,712.000	2,533.000	2,533.000	2,731.000
975.000	3,126.000	3,655.000	4,223.000	4,741.000	5,125.000
990.000	5,350.000	5,409.000	5,316.000	5,101.000	4,791.000
1,005.000	4,406.000	3,972.000	3,525.000	3,087.000	2,680.000
1,020.000	2,315.000	1,997.000	1,725.000	1,492.000	1,300.000
1,035.000	1,157.000	1,085.000	1,127.000	1,311.000	1,658.000
1,050.000	2,164.000	2,786.000	3,467.000	4,098.000	4,615.000
1,065.000	4,965.000	5,124.000	5,112.000	4,956.000	4,695.000
1,080.000	4,354.000	3,963.000	3,537.000	3,107.000	2,698.000
1,095.000	2,327.000	2,004.000	1,727.000	1,494.000	1,296.000
1,110.000	1,128.000	985.000	861.000	755.000	664.000
1,125.000	586.000	519.000	462.000	413.000	371.000
1,140.000	336.000	319.000	334.000	424.000	623.000
1,155.000	958.000	1,448.000	2,076.000	2,803.000	3,526.000
1,170.000	4,165.000	4,645.000	4,934.000	5,041.000	4,982.000
1,185.000	4,796.000	4,511.000	4,156.000	3,747.000	3,320.000
1,200.000	2,902.000	2,511.000	2,166.000	1,868.000	1,615.000
1,215.000	1,400.000	1,217.000	1,061.000	927.000	813.000
1,230.000	714.000	630.000	557.000	496.000	443.000
1,245.000	397.000	358.000	325.000	297.000	273.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	252.000	234.000	219.000	206.000	195.000
1,275.000	185.000	178.000	195.000	286.000	490.000
1,290.000	833.000	1,329.000	1,987.000	2,757.000	3,527.000
1,305.000	4,207.000	4,718.000	5,023.000	5,130.000	5,067.000
1,320.000	4,875.000	4,587.000	4,223.000	3,800.000	3,357.000
1,335.000	2,921.000	2,521.000	2,170.000	1,869.000	1,616.000
1,350.000	1,402.000	1,218.000	1,062.000	929.000	814.000
1,365.000	715.000	630.000	558.000	496.000	443.000
1,380.000	398.000	359.000	325.000	297.000	273.000
1,395.000	252.000	234.000	219.000	206.000	195.000
1,410.000	185.000	177.000	170.000	164.000	158.000
1,425.000	153.000	148.000	144.000	140.000	137.000
1,440.000	135.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	1.000	3.000
420.000	6.000	10.000	17.000	27.000	42.000
435.000	61.000	94.000	144.000	187.000	214.000
450.000	236.000	257.000	279.000	301.000	324.000
465.000	348.000	373.000	399.000	426.000	453.000
480.000	482.000	511.000	540.000	571.000	602.000
495.000	634.000	667.000	700.000	734.000	769.000
510.000	805.000	841.000	879.000	917.000	956.000
525.000	996.000	1,037.000	1,079.000	1,122.000	1,166.000
540.000	1,211.000	1,258.000	1,308.000	1,362.000	1,420.000
555.000	1,486.000	1,559.000	1,640.000	1,730.000	1,826.000
570.000	1,928.000	2,034.000	2,144.000	2,258.000	2,373.000
585.000	2,490.000	2,608.000	2,727.000	2,848.000	2,970.000
600.000	3,093.000	3,219.000	3,348.000	3,479.000	3,614.000
615.000	3,753.000	3,896.000	4,043.000	4,196.000	4,354.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	4,518.000	4,691.000	4,875.000	5,075.000	5,293.000
645.000	5,535.000	5,804.000	6,102.000	6,427.000	6,779.000
660.000	7,156.000	7,558.000	7,985.000	8,441.000	8,930.000
675.000	9,456.000	10,025.000	10,640.000	11,303.000	12,018.000
690.000	12,787.000	13,618.000	14,521.000	15,508.000	16,601.000
705.000	17,836.000	19,261.000	20,945.000	23,000.000	25,650.000
720.000	29,208.000	32,989.000	36,956.000	42,484.000	49,170.000
735.000	55,939.000	62,609.000	68,691.000	73,515.000	76,580.000
750.000	77,947.000	78,238.000	77,593.000	76,149.000	74,100.000
765.000	71,344.000	67,943.000	64,157.000	60,404.000	56,849.000
780.000	53,558.000	50,567.000	47,856.000	45,404.000	43,179.000
795.000	41,154.000	39,301.000	37,618.000	36,209.000	34,972.000
810.000	33,739.000	32,571.000	31,480.000	30,472.000	29,537.000
825.000	28,520.000	27,300.000	26,003.000	24,740.000	23,546.000
840.000	22,427.000	21,386.000	20,421.000	19,529.000	18,706.000
855.000	17,949.000	17,252.000	16,613.000	16,026.000	15,487.000
870.000	14,993.000	14,541.000	14,126.000	13,743.000	13,388.000
885.000	13,060.000	12,757.000	12,477.000	12,218.000	11,977.000
900.000	11,750.000	11,533.000	11,322.000	11,111.000	10,895.000
915.000	10,670.000	10,431.000	10,180.000	9,918.000	9,649.000
930.000	9,379.000	9,111.000	8,852.000	8,603.000	8,368.000
945.000	8,148.000	7,944.000	7,756.000	7,584.000	7,426.000
960.000	7,280.000	7,147.000	7,023.000	6,907.000	6,800.000
975.000	6,699.000	6,604.000	6,514.000	6,429.000	6,348.000
990.000	6,271.000	6,197.000	6,127.000	6,059.000	5,994.000
1,005.000	5,931.000	5,870.000	5,811.000	5,754.000	5,699.000
1,020.000	5,645.000	5,592.000	5,540.000	5,490.000	5,442.000
1,035.000	5,394.000	5,348.000	5,302.000	5,258.000	5,215.000
1,050.000	5,173.000	5,131.000	5,090.000	5,050.000	5,011.000
1,065.000	4,973.000	4,935.000	4,898.000	4,862.000	4,827.000
1,080.000	4,792.000	4,756.000	4,720.000	4,682.000	4,642.000
1,095.000	4,597.000	4,548.000	4,494.000	4,436.000	4,376.000
1,110.000	4,314.000	4,253.000	4,192.000	4,134.000	4,078.000
1,125.000	4,025.000	3,976.000	3,930.000	3,887.000	3,847.000
1,140.000	3,810.000	3,775.000	3,742.000	3,711.000	3,682.000
1,155.000	3,654.000	3,627.000	3,601.000	3,576.000	3,553.000
1,170.000	3,529.000	3,507.000	3,485.000	3,464.000	3,444.000
1,185.000	3,423.000	3,403.000	3,384.000	3,365.000	3,346.000
1,200.000	3,328.000	3,310.000	3,292.000	3,274.000	3,257.000
1,215.000	3,240.000	3,223.000	3,207.000	3,191.000	3,174.000
1,230.000	3,158.000	3,143.000	3,127.000	3,111.000	3,096.000
1,245.000	3,081.000	3,065.000	3,050.000	3,036.000	3,021.000

Subsection: Time vs. Volume
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,007.000	2,993.000	2,979.000	2,966.000	2,953.000
1,275.000	2,940.000	2,926.000	2,914.000	2,901.000	2,888.000
1,290.000	2,875.000	2,863.000	2,851.000	2,838.000	2,826.000
1,305.000	2,814.000	2,802.000	2,791.000	2,779.000	2,768.000
1,320.000	2,756.000	2,745.000	2,734.000	2,723.000	2,712.000
1,335.000	2,701.000	2,690.000	2,679.000	2,668.000	2,657.000
1,350.000	2,647.000	2,636.000	2,626.000	2,615.000	2,605.000
1,365.000	2,595.000	2,585.000	2,576.000	2,566.000	2,556.000
1,380.000	2,547.000	2,537.000	2,528.000	2,518.000	2,509.000
1,395.000	2,500.000	2,491.000	2,482.000	2,473.000	2,465.000
1,410.000	2,456.000	2,448.000	2,440.000	2,432.000	2,423.000
1,425.000	2,415.000	2,407.000	2,398.000	2,390.000	2,382.000
1,440.000	2,372.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	2.000	5.000	9.000	15.000	25.000
375.000	38.000	58.000	91.000	143.000	190.000
390.000	221.000	248.000	273.000	299.000	327.000
405.000	356.000	386.000	418.000	450.000	484.000
420.000	518.000	553.000	589.000	626.000	664.000
435.000	702.000	741.000	781.000	821.000	862.000
450.000	904.000	947.000	990.000	1,034.000	1,079.000
465.000	1,124.000	1,170.000	1,217.000	1,265.000	1,313.000
480.000	1,363.000	1,413.000	1,464.000	1,516.000	1,569.000
495.000	1,623.000	1,678.000	1,734.000	1,791.000	1,849.000
510.000	1,908.000	1,969.000	2,030.000	2,093.000	2,157.000
525.000	2,223.000	2,289.000	2,358.000	2,427.000	2,499.000
540.000	2,572.000	2,649.000	2,731.000	2,822.000	2,924.000
555.000	3,041.000	3,175.000	3,326.000	3,494.000	3,677.000
570.000	3,871.000	4,074.000	4,282.000	4,494.000	4,708.000
585.000	4,922.000	5,136.000	5,349.000	5,561.000	5,773.000
600.000	5,986.000	6,200.000	6,417.000	6,637.000	6,862.000
615.000	7,091.000	7,326.000	7,567.000	7,815.000	8,071.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	8,336.000	8,616.000	8,914.000	9,241.000	9,602.000
645.000	10,008.000	10,466.000	10,976.000	11,537.000	12,146.000
660.000	12,796.000	13,485.000	14,211.000	14,977.000	15,788.000
675.000	16,647.000	17,561.000	18,533.000	19,568.000	20,669.000
690.000	21,838.000	23,083.000	24,414.000	25,842.000	27,388.000
705.000	29,092.000	30,643.000	32,027.000	33,662.000	35,797.000
720.000	38,870.000	43,270.000	48,931.000	55,723.000	63,664.000
735.000	72,247.000	79,843.000	85,795.000	90,406.000	93,636.000
750.000	95,266.000	95,444.000	94,423.000	92,382.000	89,585.000
765.000	86,182.000	82,490.000	78,606.000	74,827.000	71,038.000
780.000	67,082.000	63,274.000	59,825.000	56,711.000	53,889.000
795.000	51,321.000	48,968.000	46,816.000	44,853.000	43,060.000
810.000	41,418.000	39,914.000	38,524.000	37,271.000	36,185.000
825.000	35,172.000	34,142.000	33,134.000	32,167.000	31,248.000
840.000	30,383.000	29,573.000	28,708.000	27,681.000	26,579.000
855.000	25,517.000	24,525.000	23,612.000	22,773.000	22,004.000
870.000	21,300.000	20,658.000	20,068.000	19,525.000	19,023.000
885.000	18,559.000	18,131.000	17,735.000	17,369.000	17,028.000
900.000	16,707.000	16,400.000	16,099.000	15,797.000	15,484.000
915.000	15,152.000	14,796.000	14,417.000	14,018.000	13,607.000
930.000	13,192.000	12,781.000	12,381.000	11,998.000	11,636.000
945.000	11,299.000	10,987.000	10,701.000	10,439.000	10,202.000
960.000	9,984.000	9,785.000	9,602.000	9,433.000	9,276.000
975.000	9,130.000	8,993.000	8,864.000	8,742.000	8,627.000
990.000	8,518.000	8,414.000	8,315.000	8,220.000	8,129.000
1,005.000	8,041.000	7,957.000	7,875.000	7,797.000	7,720.000
1,020.000	7,646.000	7,574.000	7,504.000	7,435.000	7,369.000
1,035.000	7,304.000	7,241.000	7,179.000	7,118.000	7,059.000
1,050.000	7,002.000	6,945.000	6,890.000	6,836.000	6,783.000
1,065.000	6,730.000	6,679.000	6,629.000	6,580.000	6,531.000
1,080.000	6,483.000	6,434.000	6,384.000	6,329.000	6,269.000
1,095.000	6,202.000	6,126.000	6,041.000	5,948.000	5,851.000
1,110.000	5,750.000	5,650.000	5,551.000	5,455.000	5,364.000
1,125.000	5,278.000	5,198.000	5,125.000	5,057.000	4,996.000
1,140.000	4,939.000	4,886.000	4,837.000	4,791.000	4,748.000
1,155.000	4,708.000	4,669.000	4,633.000	4,598.000	4,565.000
1,170.000	4,533.000	4,502.000	4,472.000	4,443.000	4,415.000
1,185.000	4,388.000	4,361.000	4,335.000	4,310.000	4,285.000
1,200.000	4,261.000	4,237.000	4,213.000	4,190.000	4,167.000
1,215.000	4,145.000	4,123.000	4,101.000	4,080.000	4,058.000
1,230.000	4,038.000	4,017.000	3,997.000	3,977.000	3,957.000
1,245.000	3,937.000	3,918.000	3,898.000	3,879.000	3,861.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,842.000	3,823.000	3,805.000	3,787.000	3,770.000
1,275.000	3,752.000	3,735.000	3,718.000	3,701.000	3,685.000
1,290.000	3,669.000	3,653.000	3,637.000	3,621.000	3,605.000
1,305.000	3,590.000	3,574.000	3,559.000	3,543.000	3,528.000
1,320.000	3,513.000	3,497.000	3,482.000	3,468.000	3,453.000
1,335.000	3,439.000	3,425.000	3,411.000	3,397.000	3,383.000
1,350.000	3,370.000	3,356.000	3,343.000	3,330.000	3,318.000
1,365.000	3,305.000	3,292.000	3,280.000	3,267.000	3,255.000
1,380.000	3,243.000	3,231.000	3,218.000	3,206.000	3,194.000
1,395.000	3,181.000	3,169.000	3,157.000	3,145.000	3,134.000
1,410.000	3,123.000	3,112.000	3,100.000	3,089.000	3,078.000
1,425.000	3,067.000	3,056.000	3,045.000	3,034.000	3,024.000
1,440.000	3,011.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	2.000	5.000	9.000	16.000
300.000	27.000	42.000	63.000	101.000	155.000
315.000	198.000	227.000	252.000	277.000	302.000
330.000	329.000	356.000	385.000	415.000	445.000
345.000	476.000	509.000	541.000	575.000	609.000
360.000	644.000	680.000	717.000	756.000	799.000
375.000	845.000	895.000	950.000	1,008.000	1,071.000
390.000	1,137.000	1,206.000	1,276.000	1,348.000	1,421.000
405.000	1,494.000	1,568.000	1,641.000	1,715.000	1,789.000
420.000	1,862.000	1,936.000	2,010.000	2,085.000	2,160.000
435.000	2,235.000	2,312.000	2,388.000	2,466.000	2,544.000
450.000	2,623.000	2,703.000	2,784.000	2,865.000	2,948.000
465.000	3,032.000	3,116.000	3,202.000	3,290.000	3,378.000
480.000	3,468.000	3,559.000	3,652.000	3,745.000	3,841.000
495.000	3,938.000	4,036.000	4,136.000	4,238.000	4,342.000
510.000	4,447.000	4,554.000	4,663.000	4,775.000	4,888.000
525.000	5,004.000	5,121.000	5,242.000	5,364.000	5,490.000
540.000	5,619.000	5,756.000	5,907.000	6,081.000	6,283.000
555.000	6,526.000	6,815.000	7,151.000	7,532.000	7,949.000
570.000	8,394.000	8,857.000	9,330.000	9,805.000	10,280.000
585.000	10,749.000	11,209.000	11,659.000	12,100.000	12,534.000
600.000	12,961.000	13,385.000	13,808.000	14,233.000	14,660.000
615.000	15,092.000	15,530.000	15,976.000	16,431.000	16,897.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	17,379.000	17,885.000	18,429.000	19,030.000	19,704.000
645.000	20,473.000	21,351.000	22,339.000	23,433.000	24,619.000
660.000	25,883.000	27,213.000	28,601.000	29,877.000	30,894.000
675.000	31,806.000	32,754.000	33,746.000	34,786.000	35,873.000
690.000	37,009.000	38,283.000	39,678.000	41,046.000	42,400.000
705.000	43,823.000	45,379.000	47,167.000	49,353.000	52,265.000
720.000	56,299.000	61,798.000	69,096.000	77,589.000	86,456.000
735.000	95,507.000	104,398.000	112,672.000	118,983.000	122,914.000
750.000	124,637.000	124,381.000	122,542.000	119,370.000	115,254.000
765.000	110,393.000	105,321.000	100,236.000	95,299.000	90,610.000
780.000	86,336.000	82,506.000	79,061.000	75,961.000	73,150.000
795.000	70,236.000	67,182.000	64,350.000	61,746.000	59,347.000
810.000	57,130.000	55,080.000	53,166.000	51,374.000	49,672.000
825.000	48,048.000	46,481.000	44,964.000	43,503.000	42,104.000
840.000	40,775.000	39,523.000	38,352.000	37,292.000	36,381.000
855.000	35,545.000	34,719.000	33,939.000	33,216.000	32,552.000
870.000	31,945.000	31,391.000	30,881.000	30,412.000	29,978.000
885.000	29,577.000	29,197.000	28,753.000	28,230.000	27,708.000
900.000	27,203.000	26,714.000	26,229.000	25,732.000	25,207.000
915.000	24,634.000	24,008.000	23,325.000	22,598.000	21,839.000
930.000	21,068.000	20,300.000	19,553.000	18,838.000	18,164.000
945.000	17,537.000	16,961.000	16,438.000	15,964.000	15,537.000
960.000	15,151.000	14,802.000	14,484.000	14,194.000	13,927.000
975.000	13,681.000	13,453.000	13,241.000	13,042.000	12,856.000
990.000	12,680.000	12,514.000	12,357.000	12,207.000	12,064.000
1,005.000	11,928.000	11,797.000	11,671.000	11,550.000	11,433.000
1,020.000	11,320.000	11,209.000	11,102.000	10,998.000	10,897.000
1,035.000	10,798.000	10,702.000	10,608.000	10,518.000	10,429.000
1,050.000	10,343.000	10,258.000	10,175.000	10,094.000	10,015.000
1,065.000	9,937.000	9,860.000	9,786.000	9,713.000	9,641.000
1,080.000	9,570.000	9,497.000	9,421.000	9,336.000	9,239.000
1,095.000	9,125.000	8,994.000	8,844.000	8,679.000	8,503.000
1,110.000	8,320.000	8,138.000	7,958.000	7,785.000	7,621.000
1,125.000	7,468.000	7,328.000	7,199.000	7,083.000	6,977.000
1,140.000	6,881.000	6,793.000	6,713.000	6,639.000	6,570.000
1,155.000	6,506.000	6,446.000	6,389.000	6,335.000	6,284.000
1,170.000	6,235.000	6,189.000	6,144.000	6,102.000	6,060.000
1,185.000	6,020.000	5,982.000	5,944.000	5,907.000	5,871.000
1,200.000	5,836.000	5,801.000	5,767.000	5,734.000	5,701.000
1,215.000	5,669.000	5,637.000	5,607.000	5,576.000	5,546.000
1,230.000	5,517.000	5,488.000	5,460.000	5,432.000	5,404.000
1,245.000	5,377.000	5,350.000	5,323.000	5,297.000	5,271.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	5,245.000	5,219.000	5,194.000	5,169.000	5,144.000
1,275.000	5,120.000	5,096.000	5,072.000	5,048.000	5,025.000
1,290.000	5,002.000	4,980.000	4,957.000	4,935.000	4,913.000
1,305.000	4,891.000	4,870.000	4,848.000	4,827.000	4,805.000
1,320.000	4,784.000	4,763.000	4,742.000	4,721.000	4,701.000
1,335.000	4,680.000	4,660.000	4,641.000	4,621.000	4,602.000
1,350.000	4,583.000	4,564.000	4,545.000	4,527.000	4,508.000
1,365.000	4,490.000	4,472.000	4,455.000	4,437.000	4,420.000
1,380.000	4,403.000	4,386.000	4,369.000	4,352.000	4,335.000
1,395.000	4,318.000	4,302.000	4,285.000	4,269.000	4,253.000
1,410.000	4,237.000	4,222.000	4,206.000	4,191.000	4,176.000
1,425.000	4,161.000	4,146.000	4,131.000	4,116.000	4,102.000
1,440.000	4,085.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R3
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	0.000	0.000
465.000	0.000	0.000	0.000	0.000	0.000
480.000	0.000	0.000	0.000	0.000	0.000
495.000	0.000	0.000	0.000	0.000	0.000
510.000	0.000	0.000	0.000	1.000	3.000
525.000	6.000	10.000	14.000	19.000	23.000
540.000	26.000	29.000	30.000	31.000	31.000
555.000	31.000	30.000	30.000	33.000	43.000
570.000	66.000	104.000	154.000	208.000	257.000
585.000	296.000	320.000	331.000	328.000	320.000
600.000	315.000	326.000	362.000	422.000	496.000
615.000	567.000	619.000	647.000	650.000	630.000

Subsection: Time vs. Volume
 Label: R3
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	595.000	559.000	537.000	544.000	590.000
645.000	667.000	756.000	833.000	882.000	902.000
660.000	906.000	910.000	937.000	996.000	1,073.000
675.000	1,145.000	1,198.000	1,239.000	1,280.000	1,333.000
690.000	1,402.000	1,481.000	1,567.000	1,658.000	1,767.000
705.000	1,902.000	2,062.000	2,262.000	2,549.000	2,971.000
720.000	3,544.000	4,276.000	5,181.000	6,216.000	7,276.000
735.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
750.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
765.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
780.000	7,866.000	7,866.000	7,118.000	5,606.000	4,734.000
795.000	4,138.000	3,680.000	3,322.000	3,069.000	2,935.000
810.000	2,915.000	2,945.000	2,953.000	2,903.000	2,785.000
825.000	2,602.000	2,394.000	2,219.000	2,129.000	2,168.000
840.000	2,309.000	2,474.000	2,594.000	2,624.000	2,556.000
855.000	2,407.000	2,192.000	1,959.000	1,771.000	1,675.000
870.000	1,698.000	1,847.000	2,072.000	2,290.000	2,440.000
885.000	2,487.000	2,428.000	2,282.000	2,073.000	1,839.000
900.000	1,612.000	1,409.000	1,239.000	1,128.000	1,119.000
915.000	1,235.000	1,475.000	1,789.000	2,082.000	2,282.000
930.000	2,370.000	2,348.000	2,225.000	2,032.000	1,809.000
945.000	1,588.000	1,388.000	1,212.000	1,056.000	919.000
960.000	809.000	747.000	768.000	900.000	1,165.000
975.000	1,521.000	1,878.000	2,159.000	2,312.000	2,333.000
990.000	2,248.000	2,077.000	1,861.000	1,642.000	1,436.000
1,005.000	1,252.000	1,091.000	951.000	830.000	725.000
1,020.000	635.000	557.000	491.000	433.000	384.000
1,035.000	350.000	355.000	433.000	627.000	952.000
1,050.000	1,359.000	1,766.000	2,084.000	2,261.000	2,304.000
1,065.000	2,234.000	2,074.000	1,865.000	1,648.000	1,443.000
1,080.000	1,260.000	1,099.000	959.000	837.000	732.000
1,095.000	641.000	563.000	495.000	437.000	387.000
1,110.000	344.000	307.000	274.000	246.000	221.000
1,125.000	200.000	181.000	163.000	147.000	133.000
1,140.000	121.000	110.000	115.000	168.000	304.000
1,155.000	569.000	961.000	1,409.000	1,826.000	2,128.000
1,170.000	2,282.000	2,301.000	2,208.000	2,033.000	1,820.000
1,185.000	1,605.000	1,405.000	1,226.000	1,070.000	933.000
1,200.000	815.000	712.000	624.000	548.000	483.000
1,215.000	427.000	378.000	336.000	300.000	268.000
1,230.000	241.000	217.000	196.000	177.000	160.000
1,245.000	144.000	130.000	118.000	108.000	99.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	91.000	84.000	77.000	72.000	67.000
1,275.000	62.000	58.000	69.000	128.000	272.000
1,290.000	546.000	949.000	1,407.000	1,833.000	2,141.000
1,305.000	2,297.000	2,318.000	2,224.000	2,048.000	1,834.000
1,320.000	1,616.000	1,414.000	1,234.000	1,077.000	939.000
1,335.000	820.000	717.000	628.000	552.000	486.000
1,350.000	429.000	380.000	338.000	301.000	269.000
1,365.000	242.000	218.000	197.000	178.000	161.000
1,380.000	145.000	131.000	119.000	108.000	99.000
1,395.000	91.000	84.000	77.000	72.000	67.000
1,410.000	62.000	59.000	55.000	52.000	49.000
1,425.000	46.000	44.000	41.000	39.000	37.000
1,440.000	35.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	1.000	2.000
420.000	3.000	6.000	10.000	16.000	24.000
435.000	33.000	45.000	58.000	72.000	88.000
450.000	105.000	123.000	142.000	161.000	180.000
465.000	199.000	219.000	238.000	257.000	276.000
480.000	295.000	314.000	332.000	351.000	369.000
495.000	387.000	405.000	423.000	441.000	459.000
510.000	477.000	495.000	513.000	531.000	549.000
525.000	568.000	586.000	605.000	624.000	643.000
540.000	662.000	682.000	703.000	727.000	753.000
555.000	783.000	818.000	856.000	897.000	939.000
570.000	982.000	1,024.000	1,066.000	1,108.000	1,148.000
585.000	1,188.000	1,228.000	1,267.000	1,307.000	1,346.000
600.000	1,386.000	1,427.000	1,468.000	1,510.000	1,552.000
615.000	1,596.000	1,641.000	1,687.000	1,734.000	1,783.000

Subsection: Time vs. Volume
 Label: R3
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	1,834.000	1,888.000	1,945.000	2,009.000	2,083.000
645.000	2,166.000	2,260.000	2,362.000	2,472.000	2,585.000
660.000	2,701.000	2,819.000	2,941.000	3,070.000	3,208.000
675.000	3,357.000	3,520.000	3,696.000	3,881.000	4,076.000
690.000	4,282.000	4,500.000	4,735.000	4,990.000	5,279.000
705.000	5,613.000	6,004.000	6,467.000	7,052.000	7,849.000
720.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
735.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
750.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
765.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
780.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
795.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
810.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
825.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
840.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
855.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
870.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
885.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
900.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
915.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
930.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
945.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
960.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
975.000	7,375.000	4,897.000	3,661.000	2,992.000	2,609.000
990.000	2,381.000	2,240.000	2,150.000	2,091.000	2,049.000
1,005.000	2,019.000	1,995.000	1,975.000	1,958.000	1,943.000
1,020.000	1,930.000	1,917.000	1,905.000	1,893.000	1,881.000
1,035.000	1,870.000	1,859.000	1,848.000	1,837.000	1,826.000
1,050.000	1,816.000	1,806.000	1,795.000	1,785.000	1,776.000
1,065.000	1,766.000	1,757.000	1,748.000	1,739.000	1,730.000
1,080.000	1,721.000	1,711.000	1,701.000	1,690.000	1,677.000
1,095.000	1,661.000	1,643.000	1,624.000	1,603.000	1,582.000
1,110.000	1,563.000	1,545.000	1,528.000	1,513.000	1,499.000
1,125.000	1,487.000	1,475.000	1,465.000	1,455.000	1,447.000
1,140.000	1,439.000	1,431.000	1,424.000	1,417.000	1,410.000
1,155.000	1,404.000	1,397.000	1,391.000	1,385.000	1,379.000
1,170.000	1,373.000	1,367.000	1,362.000	1,356.000	1,350.000
1,185.000	1,345.000	1,339.000	1,334.000	1,328.000	1,323.000
1,200.000	1,318.000	1,313.000	1,308.000	1,303.000	1,298.000
1,215.000	1,294.000	1,289.000	1,284.000	1,280.000	1,275.000
1,230.000	1,270.000	1,266.000	1,261.000	1,257.000	1,252.000
1,245.000	1,248.000	1,243.000	1,239.000	1,234.000	1,230.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	1,226.000	1,222.000	1,218.000	1,214.000	1,210.000
1,275.000	1,206.000	1,202.000	1,198.000	1,194.000	1,190.000
1,290.000	1,186.000	1,183.000	1,179.000	1,175.000	1,172.000
1,305.000	1,168.000	1,165.000	1,162.000	1,158.000	1,155.000
1,320.000	1,151.000	1,148.000	1,144.000	1,141.000	1,137.000
1,335.000	1,133.000	1,130.000	1,127.000	1,123.000	1,120.000
1,350.000	1,117.000	1,113.000	1,110.000	1,107.000	1,104.000
1,365.000	1,101.000	1,098.000	1,095.000	1,092.000	1,089.000
1,380.000	1,086.000	1,083.000	1,080.000	1,077.000	1,074.000
1,395.000	1,071.000	1,068.000	1,065.000	1,062.000	1,060.000
1,410.000	1,057.000	1,055.000	1,052.000	1,050.000	1,047.000
1,425.000	1,044.000	1,042.000	1,039.000	1,036.000	1,033.000
1,440.000	1,030.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R3
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	1.000
360.000	1.000	3.000	5.000	9.000	15.000
375.000	22.000	32.000	45.000	59.000	76.000
390.000	94.000	115.000	136.000	158.000	181.000
405.000	204.000	227.000	250.000	273.000	296.000
420.000	318.000	340.000	362.000	384.000	405.000
435.000	426.000	447.000	467.000	487.000	508.000
450.000	528.000	547.000	567.000	587.000	607.000
465.000	627.000	647.000	667.000	687.000	707.000
480.000	727.000	747.000	768.000	788.000	809.000
495.000	830.000	851.000	872.000	894.000	915.000
510.000	937.000	959.000	982.000	1,005.000	1,028.000
525.000	1,051.000	1,075.000	1,099.000	1,123.000	1,148.000
540.000	1,173.000	1,199.000	1,228.000	1,262.000	1,302.000
555.000	1,351.000	1,408.000	1,472.000	1,539.000	1,609.000
570.000	1,679.000	1,747.000	1,812.000	1,874.000	1,935.000
585.000	1,993.000	2,050.000	2,106.000	2,162.000	2,217.000
600.000	2,272.000	2,328.000	2,385.000	2,443.000	2,502.000
615.000	2,562.000	2,623.000	2,687.000	2,752.000	2,818.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	2,888.000	2,962.000	3,043.000	3,136.000	3,246.000
645.000	3,374.000	3,520.000	3,679.000	3,848.000	4,021.000
660.000	4,196.000	4,370.000	4,546.000	4,728.000	4,919.000
675.000	5,124.000	5,344.000	5,577.000	5,822.000	6,077.000
690.000	6,344.000	6,625.000	6,920.000	7,235.000	7,578.000
705.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
720.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
735.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
750.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
765.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
780.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
795.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
810.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
825.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
840.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
855.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
870.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
885.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
900.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
915.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
930.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
945.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
960.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
975.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
990.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,005.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,020.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,035.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,050.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,065.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,080.000	7,866.000	7,866.000	7,866.000	7,866.000	6,670.000
1,095.000	4,534.000	3,437.000	2,826.000	2,464.000	2,239.000
1,110.000	2,093.000	1,996.000	1,928.000	1,880.000	1,844.000
1,125.000	1,816.000	1,794.000	1,776.000	1,761.000	1,748.000
1,140.000	1,736.000	1,725.000	1,715.000	1,705.000	1,696.000
1,155.000	1,688.000	1,680.000	1,672.000	1,665.000	1,657.000
1,170.000	1,650.000	1,643.000	1,636.000	1,629.000	1,623.000
1,185.000	1,616.000	1,610.000	1,603.000	1,597.000	1,591.000
1,200.000	1,584.000	1,578.000	1,572.000	1,565.000	1,559.000
1,215.000	1,553.000	1,547.000	1,542.000	1,536.000	1,530.000
1,230.000	1,525.000	1,520.000	1,514.000	1,508.000	1,503.000
1,245.000	1,498.000	1,492.000	1,487.000	1,481.000	1,476.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	1,471.000	1,466.000	1,461.000	1,456.000	1,451.000
1,275.000	1,446.000	1,441.000	1,436.000	1,432.000	1,427.000
1,290.000	1,422.000	1,418.000	1,414.000	1,409.000	1,405.000
1,305.000	1,400.000	1,396.000	1,392.000	1,388.000	1,383.000
1,320.000	1,379.000	1,375.000	1,370.000	1,366.000	1,361.000
1,335.000	1,357.000	1,353.000	1,349.000	1,345.000	1,341.000
1,350.000	1,337.000	1,333.000	1,329.000	1,326.000	1,322.000
1,365.000	1,319.000	1,315.000	1,311.000	1,307.000	1,304.000
1,380.000	1,300.000	1,296.000	1,293.000	1,289.000	1,286.000
1,395.000	1,282.000	1,279.000	1,275.000	1,272.000	1,268.000
1,410.000	1,265.000	1,262.000	1,259.000	1,255.000	1,252.000
1,425.000	1,248.000	1,245.000	1,241.000	1,238.000	1,235.000
1,440.000	1,231.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	1.000	1.000	3.000	6.000	10.000
300.000	17.000	25.000	36.000	48.000	63.000
315.000	80.000	99.000	118.000	139.000	160.000
330.000	182.000	204.000	226.000	248.000	270.000
345.000	291.000	312.000	333.000	353.000	373.000
360.000	393.000	413.000	432.000	453.000	476.000
375.000	500.000	527.000	555.000	586.000	617.000
390.000	648.000	680.000	711.000	741.000	771.000
405.000	800.000	828.000	856.000	883.000	910.000
420.000	937.000	963.000	989.000	1,015.000	1,041.000
435.000	1,067.000	1,093.000	1,119.000	1,145.000	1,171.000
450.000	1,197.000	1,223.000	1,249.000	1,276.000	1,302.000
465.000	1,329.000	1,356.000	1,383.000	1,410.000	1,438.000
480.000	1,466.000	1,494.000	1,523.000	1,551.000	1,580.000
495.000	1,609.000	1,639.000	1,669.000	1,699.000	1,729.000
510.000	1,760.000	1,792.000	1,823.000	1,856.000	1,888.000
525.000	1,922.000	1,955.000	1,989.000	2,024.000	2,060.000
540.000	2,096.000	2,134.000	2,180.000	2,238.000	2,312.000
555.000	2,407.000	2,523.000	2,652.000	2,789.000	2,928.000
570.000	3,062.000	3,188.000	3,306.000	3,415.000	3,517.000
585.000	3,613.000	3,705.000	3,795.000	3,882.000	3,969.000
600.000	4,054.000	4,140.000	4,225.000	4,311.000	4,398.000
615.000	4,488.000	4,580.000	4,676.000	4,774.000	4,875.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
630.000	4,980.000	5,092.000	5,218.000	5,366.000	5,548.000
645.000	5,766.000	6,016.000	6,289.000	6,575.000	6,863.000
660.000	7,147.000	7,424.000	7,696.000	7,866.000	7,866.000
675.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
690.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
705.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
720.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
735.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
750.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
765.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
780.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
795.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
810.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
825.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
840.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
855.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
870.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
885.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
900.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
915.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
930.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
945.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
960.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
975.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
990.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,005.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,020.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,035.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,050.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,065.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,080.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,095.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,110.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,125.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,140.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,155.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,170.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,185.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,200.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,215.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,230.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,245.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000

Subsection: Time vs. Volume
Label: R3
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,275.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,290.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,305.000	7,866.000	7,866.000	7,866.000	7,866.000	7,866.000
1,320.000	7,866.000	7,866.000	7,866.000	7,866.000	7,656.000
1,335.000	4,887.000	3,514.000	2,770.000	2,342.000	2,087.000
1,350.000	1,930.000	1,832.000	1,769.000	1,728.000	1,700.000
1,365.000	1,680.000	1,666.000	1,656.000	1,648.000	1,641.000
1,380.000	1,635.000	1,629.000	1,624.000	1,619.000	1,614.000
1,395.000	1,610.000	1,605.000	1,600.000	1,596.000	1,592.000
1,410.000	1,588.000	1,583.000	1,579.000	1,575.000	1,571.000
1,425.000	1,567.000	1,562.000	1,558.000	1,554.000	1,550.000
1,440.000	1,545.000	(N/A)	(N/A)	(N/A)	(N/A)

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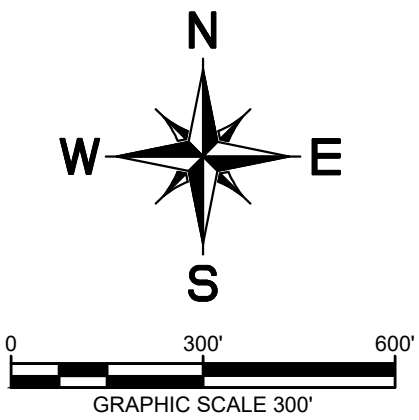
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APPENDIX C

PRELIMINARY PROPOSED DRAINAGE AREA MAP



PROPOSED DRAINAGE PEAK FLOW SUMMARY AT POA D				
AREA	2-YR Q	10-YR Q	25-YR Q	100-YR Q
POA D	303.17 CFS	933.56 CFS	1,256.61 CFS	1,807.35 CFS

**GATEWAY TRACT
PROPOSED DRAINAGE AREA MAP**
A SUBDIVISION OF 38.10 ACRES IN THE
CITY OF TAYLOR,
WILLIAMSON COUNTY, TEXAS
SUBMITTAL DATE: 9/13/2024



TEXAS REGISTERED ENGINEERING FIRM F-928				
Scale	Drawn by	Checked by	Date	KHA Project No.
1" = 300'	RRJ	JAP	SEPTEMBER 2024	069405306
				Sheet No.
				OF 3

APPENDIX D

PRELIMINARY PROPOSED DRAINAGE CALCULATIONS

Project Summary	
Title	Taylor Gateway
Engineer	John Pickens, P.E.
Company	Kimley-Horn & Associates, Inc.
Date	9/13/2024

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	Runoff CN-Area, 25 years (EX 25)	82
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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)
PROP AREA C	EX 2	2	587,792.000	756.000	79.66
PROP AREA C	EX 10	10	1,460,300.000	756.000	214.84
PROP AREA C	EX 25	25	2,005,601.000	756.000	278.46
PROP AREA C	EX 100	100	3,044,204.000	756.000	384.90
PROP AREA B	EX 2	2	621,937.000	753.000	93.10
PROP AREA B	EX 10	10	1,700,726.000	750.000	280.93
PROP AREA B	EX 25	25	2,392,974.000	750.000	373.90
PROP AREA B	EX 100	100	3,726,557.000	750.000	530.04
PROP AREA A1	EX 2	2	800,993.000	753.000	114.31
PROP AREA A1	EX 10	10	2,181,713.000	753.000	344.29
PROP AREA A1	EX 25	25	3,067,288.000	753.000	458.07
PROP AREA A1	EX 100	100	4,772,632.000	753.000	649.43
PROP AREA A2	EX 2	2	164,921.000	741.000	29.78
PROP AREA A2	EX 10	10	452,004.000	741.000	94.12
PROP AREA A2	EX 25	25	635,864.000	741.000	124.62
PROP AREA A2	EX 100	100	989,991.000	741.000	175.46
EX AREA E	EX 2	2	246,762.000	729.000	56.20
EX AREA E	EX 10	10	525,339.000	729.000	140.51
EX AREA E	EX 25	25	692,893.000	729.000	173.70
EX AREA E	EX 100	100	1,007,954.000	729.000	229.03
EX AREA D	EX 2	2	186,197.000	741.000	33.87
EX AREA D	EX 10	10	510,356.000	741.000	107.11
EX AREA D	EX 25	25	717,945.000	741.000	141.73
EX AREA D	EX 100	100	1,117,770.000	738.000	199.85

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)
POA D	EX 2	2	2,581,475.000	756.000	303.17
POA D	EX 10	10	6,766,863.000	753.000	933.56
POA D	EX 25	25	9,437,650.000	753.000	1,256.61
POA D	EX 100	100	14,568,172.000	753.000	1,807.35
POA A	EX 2	2	965,915.000	753.000	139.60
POA A	EX 10	10	2,633,718.000	750.000	418.91
POA A	EX 25	25	3,703,152.000	750.000	557.35
POA A	EX 100	100	5,762,623.000	747.000	791.53
POA B	EX 2	2	1,587,735.000	753.000	230.50
POA B	EX 10	10	4,332,663.000	750.000	697.60
POA B	EX 25	25	6,093,870.000	750.000	929.40
POA B	EX 100	100	9,486,127.000	750.000	1,320.74

Pond Summary

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ft ³)	Time to Peak (min)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft ³)
POND A (IN)	EX 2	2	587,792.000	756.000	79.66	(N/A)	(N/A)
POND A (OUT)	EX 2	2	567,291.000	792.000	45.09	584.09	182,308.000
POND A (IN)	EX 10	10	1,460,300.000	756.000	214.84	(N/A)	(N/A)
POND A (OUT)	EX 10	10	1,419,893.000	780.000	147.07	586.61	401,569.000
POND A (IN)	EX 25	25	2,005,601.000	756.000	278.46	(N/A)	(N/A)
POND A (OUT)	EX 25	25	1,958,912.000	777.000	201.98	587.70	496,187.000
POND A (IN)	EX 100	100	3,044,204.000	756.000	384.90	(N/A)	(N/A)
POND A (OUT)	EX 100	100	2,987,867.000	774.000	297.87	589.38	643,031.000
POND B (IN)	EX 2	2	432,959.000	732.000	83.62	(N/A)	(N/A)
POND B (OUT)	EX 2	2	426,584.000	753.000	46.56	582.64	122,092.000
POND B (IN)	EX 10	10	1,035,695.000	732.000	228.57	(N/A)	(N/A)
POND B (OUT)	EX 10	10	1,016,696.000	750.000	142.44	585.58	257,679.000
POND B (IN)	EX 25	25	1,410,838.000	732.000	292.01	(N/A)	(N/A)
POND B (OUT)	EX 25	25	1,387,900.000	747.000	192.78	586.83	315,277.000
POND B (IN)	EX 100	100	2,125,724.000	732.000	398.18	(N/A)	(N/A)
POND B (OUT)	EX 100	100	2,098,291.000	747.000	279.57	588.75	403,973.000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time-Depth Curve: 10 Yr Storm

Label	10 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	10 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0008	0.0017	0.0025	0.0034
5.000	0.0042	0.0051	0.0059	0.0068	0.0076
10.000	0.0085	0.0093	0.0102	0.0110	0.0119
15.000	0.0127	0.0136	0.0145	0.0153	0.0162
20.000	0.0170	0.0179	0.0188	0.0196	0.0205
25.000	0.0214	0.0222	0.0231	0.0240	0.0248
30.000	0.0257	0.0266	0.0275	0.0283	0.0292
35.000	0.0301	0.0310	0.0318	0.0327	0.0336
40.000	0.0345	0.0354	0.0362	0.0371	0.0380
45.000	0.0389	0.0398	0.0407	0.0416	0.0425
50.000	0.0434	0.0443	0.0452	0.0461	0.0470
55.000	0.0479	0.0488	0.0497	0.0506	0.0515
60.000	0.0524	0.0533	0.0542	0.0551	0.0560
65.000	0.0569	0.0578	0.0587	0.0596	0.0605
70.000	0.0615	0.0624	0.0633	0.0642	0.0651
75.000	0.0661	0.0670	0.0679	0.0688	0.0697
80.000	0.0707	0.0716	0.0725	0.0735	0.0744
85.000	0.0753	0.0763	0.0772	0.0781	0.0791
90.000	0.0800	0.0810	0.0819	0.0828	0.0838
95.000	0.0847	0.0857	0.0866	0.0876	0.0885
100.000	0.0895	0.0904	0.0914	0.0923	0.0933
105.000	0.0942	0.0952	0.0962	0.0971	0.0981
110.000	0.0990	0.1000	0.1010	0.1019	0.1029
115.000	0.1039	0.1049	0.1058	0.1068	0.1078
120.000	0.1088	0.1097	0.1107	0.1117	0.1127
125.000	0.1137	0.1146	0.1156	0.1166	0.1176
130.000	0.1186	0.1196	0.1206	0.1216	0.1226
135.000	0.1236	0.1246	0.1256	0.1266	0.1276
140.000	0.1286	0.1296	0.1306	0.1316	0.1326
145.000	0.1336	0.1346	0.1356	0.1367	0.1377
150.000	0.1387	0.1397	0.1407	0.1418	0.1428
155.000	0.1438	0.1448	0.1459	0.1469	0.1479
160.000	0.1490	0.1500	0.1510	0.1521	0.1531
165.000	0.1542	0.1552	0.1562	0.1573	0.1583
170.000	0.1594	0.1604	0.1615	0.1625	0.1636

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.1647	0.1657	0.1668	0.1678	0.1689
180.000	0.1700	0.1710	0.1721	0.1732	0.1742
185.000	0.1753	0.1764	0.1775	0.1785	0.1796
190.000	0.1807	0.1818	0.1829	0.1840	0.1850
195.000	0.1861	0.1872	0.1883	0.1894	0.1905
200.000	0.1916	0.1927	0.1938	0.1949	0.1960
205.000	0.1971	0.1982	0.1993	0.2005	0.2016
210.000	0.2027	0.2038	0.2049	0.2060	0.2072
215.000	0.2083	0.2094	0.2105	0.2117	0.2128
220.000	0.2139	0.2151	0.2162	0.2174	0.2185
225.000	0.2196	0.2208	0.2219	0.2231	0.2242
230.000	0.2254	0.2265	0.2277	0.2289	0.2300
235.000	0.2312	0.2323	0.2335	0.2347	0.2358
240.000	0.2370	0.2382	0.2394	0.2405	0.2417
245.000	0.2429	0.2441	0.2453	0.2465	0.2477
250.000	0.2488	0.2500	0.2512	0.2524	0.2536
255.000	0.2548	0.2560	0.2573	0.2585	0.2597
260.000	0.2609	0.2621	0.2633	0.2645	0.2658
265.000	0.2670	0.2682	0.2694	0.2707	0.2719
270.000	0.2731	0.2744	0.2756	0.2769	0.2781
275.000	0.2794	0.2806	0.2819	0.2831	0.2844
280.000	0.2856	0.2869	0.2881	0.2894	0.2907
285.000	0.2920	0.2932	0.2945	0.2958	0.2971
290.000	0.2983	0.2996	0.3009	0.3022	0.3035
295.000	0.3048	0.3061	0.3074	0.3087	0.3100
300.000	0.3113	0.3126	0.3139	0.3152	0.3165
305.000	0.3179	0.3192	0.3205	0.3218	0.3232
310.000	0.3245	0.3258	0.3272	0.3285	0.3299
315.000	0.3312	0.3326	0.3339	0.3353	0.3366
320.000	0.3380	0.3393	0.3407	0.3421	0.3435
325.000	0.3448	0.3462	0.3476	0.3490	0.3504
330.000	0.3517	0.3531	0.3545	0.3559	0.3573
335.000	0.3587	0.3601	0.3615	0.3630	0.3644
340.000	0.3658	0.3672	0.3686	0.3701	0.3715
345.000	0.3729	0.3744	0.3758	0.3772	0.3787
350.000	0.3801	0.3816	0.3831	0.3845	0.3860
355.000	0.3874	0.3889	0.3904	0.3919	0.3933
360.000	0.3948	0.3965	0.3982	0.3999	0.4016
365.000	0.4033	0.4050	0.4067	0.4084	0.4101
370.000	0.4118	0.4135	0.4152	0.4169	0.4187
375.000	0.4204	0.4221	0.4239	0.4256	0.4274
380.000	0.4291	0.4309	0.4326	0.4344	0.4362
385.000	0.4379	0.4397	0.4415	0.4433	0.4451

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.4469	0.4487	0.4505	0.4523	0.4541
395.000	0.4559	0.4577	0.4595	0.4614	0.4632
400.000	0.4650	0.4669	0.4687	0.4706	0.4724
405.000	0.4743	0.4762	0.4780	0.4799	0.4818
410.000	0.4837	0.4856	0.4875	0.4894	0.4913
415.000	0.4932	0.4951	0.4970	0.4989	0.5009
420.000	0.5028	0.5047	0.5067	0.5086	0.5106
425.000	0.5125	0.5145	0.5165	0.5185	0.5204
430.000	0.5224	0.5244	0.5264	0.5284	0.5304
435.000	0.5324	0.5345	0.5365	0.5385	0.5406
440.000	0.5426	0.5446	0.5467	0.5488	0.5508
445.000	0.5529	0.5550	0.5571	0.5591	0.5612
450.000	0.5633	0.5655	0.5676	0.5697	0.5718
455.000	0.5740	0.5761	0.5782	0.5804	0.5826
460.000	0.5847	0.5869	0.5891	0.5913	0.5935
465.000	0.5957	0.5979	0.6001	0.6023	0.6045
470.000	0.6068	0.6090	0.6113	0.6135	0.6158
475.000	0.6180	0.6203	0.6226	0.6249	0.6272
480.000	0.6295	0.6318	0.6342	0.6365	0.6388
485.000	0.6412	0.6435	0.6459	0.6483	0.6506
490.000	0.6530	0.6554	0.6578	0.6602	0.6627
495.000	0.6651	0.6675	0.6700	0.6724	0.6749
500.000	0.6774	0.6799	0.6823	0.6848	0.6874
505.000	0.6899	0.6924	0.6949	0.6975	0.7001
510.000	0.7026	0.7052	0.7078	0.7104	0.7130
515.000	0.7156	0.7182	0.7209	0.7235	0.7262
520.000	0.7289	0.7315	0.7342	0.7369	0.7396
525.000	0.7424	0.7451	0.7478	0.7506	0.7534
530.000	0.7561	0.7589	0.7617	0.7645	0.7674
535.000	0.7702	0.7730	0.7759	0.7788	0.7817
540.000	0.7846	0.7883	0.7920	0.7958	0.7996
545.000	0.8034	0.8072	0.8110	0.8149	0.8187
550.000	0.8226	0.8265	0.8304	0.8343	0.8383
555.000	0.8422	0.8462	0.8502	0.8542	0.8582
560.000	0.8623	0.8664	0.8705	0.8746	0.8787
565.000	0.8828	0.8870	0.8912	0.8954	0.8996
570.000	0.9039	0.9082	0.9125	0.9168	0.9211
575.000	0.9254	0.9298	0.9342	0.9386	0.9431
580.000	0.9476	0.9520	0.9566	0.9611	0.9657
585.000	0.9702	0.9749	0.9795	0.9842	0.9888
590.000	0.9936	0.9983	1.0031	1.0079	1.0127
595.000	1.0176	1.0224	1.0273	1.0323	1.0373
600.000	1.0423	1.0473	1.0523	1.0574	1.0626

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	1.0677	1.0729	1.0781	1.0834	1.0887
610.000	1.0940	1.0994	1.1048	1.1102	1.1157
615.000	1.1212	1.1268	1.1324	1.1380	1.1437
620.000	1.1494	1.1551	1.1609	1.1668	1.1727
625.000	1.1786	1.1846	1.1906	1.1967	1.2028
630.000	1.2089	1.2164	1.2239	1.2315	1.2391
635.000	1.2468	1.2546	1.2624	1.2703	1.2783
640.000	1.2863	1.2944	1.3025	1.3108	1.3191
645.000	1.3274	1.3359	1.3444	1.3530	1.3617
650.000	1.3705	1.3794	1.3883	1.3974	1.4065
655.000	1.4157	1.4250	1.4345	1.4440	1.4536
660.000	1.4634	1.4744	1.4855	1.4968	1.5081
665.000	1.5197	1.5313	1.5431	1.5550	1.5671
670.000	1.5794	1.5918	1.6043	1.6170	1.6299
675.000	1.6430	1.6563	1.6698	1.6834	1.6973
680.000	1.7114	1.7257	1.7403	1.7551	1.7702
685.000	1.7855	1.8012	1.8171	1.8333	1.8499
690.000	1.8668	1.8856	1.9049	1.9245	1.9446
695.000	1.9652	1.9863	2.0079	2.0301	2.0529
700.000	2.0764	2.1006	2.1256	2.1514	2.1782
705.000	2.2061	2.2438	2.2828	2.3234	2.3656
710.000	2.4098	2.4561	2.5050	2.5569	2.6193
715.000	2.6862	2.7718	2.8643	2.9666	3.1226
720.000	3.2786	3.4346	3.5906	3.7466	3.8435
725.000	3.9323	4.0018	4.0664	4.1269	4.1772
730.000	4.2248	4.2700	4.3131	4.3545	4.3943
735.000	4.4327	4.4610	4.4883	4.5146	4.5400
740.000	4.5646	4.5885	4.6116	4.6341	4.6560
745.000	4.6774	4.6982	4.7185	4.7384	4.7578
750.000	4.7768	4.7939	4.8107	4.8271	4.8431
755.000	4.8589	4.8744	4.8896	4.9045	4.9192
760.000	4.9337	4.9479	4.9619	4.9756	4.9892
765.000	5.0026	5.0158	5.0288	5.0416	5.0542
770.000	5.0667	5.0790	5.0912	5.1032	5.1150
775.000	5.1268	5.1383	5.1498	5.1611	5.1723
780.000	5.1834	5.1932	5.2029	5.2124	5.2219
785.000	5.2313	5.2406	5.2497	5.2588	5.2678
790.000	5.2767	5.2856	5.2943	5.3029	5.3115
795.000	5.3200	5.3284	5.3368	5.3450	5.3532
800.000	5.3613	5.3694	5.3774	5.3853	5.3931
805.000	5.4009	5.4087	5.4163	5.4239	5.4315
810.000	5.4390	5.4452	5.4513	5.4574	5.4635
815.000	5.4695	5.4754	5.4813	5.4872	5.4930

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	5.4988	5.5045	5.5102	5.5159	5.5215
825.000	5.5271	5.5326	5.5381	5.5435	5.5490
830.000	5.5543	5.5597	5.5650	5.5703	5.5755
835.000	5.5807	5.5859	5.5910	5.5962	5.6012
840.000	5.6063	5.6113	5.6163	5.6213	5.6262
845.000	5.6311	5.6360	5.6408	5.6456	5.6504
850.000	5.6551	5.6599	5.6646	5.6693	5.6739
855.000	5.6785	5.6831	5.6877	5.6923	5.6968
860.000	5.7013	5.7058	5.7102	5.7147	5.7191
865.000	5.7235	5.7278	5.7322	5.7365	5.7408
870.000	5.7451	5.7493	5.7536	5.7578	5.7620
875.000	5.7662	5.7703	5.7745	5.7786	5.7827
880.000	5.7868	5.7908	5.7949	5.7989	5.8029
885.000	5.8069	5.8109	5.8148	5.8187	5.8227
890.000	5.8266	5.8304	5.8343	5.8382	5.8420
895.000	5.8458	5.8496	5.8534	5.8572	5.8609
900.000	5.8647	5.8676	5.8705	5.8733	5.8762
905.000	5.8791	5.8819	5.8847	5.8875	5.8904
910.000	5.8931	5.8959	5.8987	5.9015	5.9042
915.000	5.9069	5.9097	5.9124	5.9151	5.9178
920.000	5.9205	5.9231	5.9258	5.9284	5.9311
925.000	5.9337	5.9363	5.9389	5.9415	5.9441
930.000	5.9467	5.9493	5.9518	5.9544	5.9569
935.000	5.9595	5.9620	5.9645	5.9670	5.9695
940.000	5.9720	5.9745	5.9769	5.9794	5.9818
945.000	5.9843	5.9867	5.9891	5.9916	5.9940
950.000	5.9964	5.9988	6.0011	6.0035	6.0059
955.000	6.0083	6.0106	6.0129	6.0153	6.0176
960.000	6.0199	6.0222	6.0245	6.0268	6.0291
965.000	6.0314	6.0337	6.0360	6.0382	6.0405
970.000	6.0427	6.0450	6.0472	6.0494	6.0516
975.000	6.0538	6.0560	6.0582	6.0604	6.0626
980.000	6.0648	6.0670	6.0691	6.0713	6.0734
985.000	6.0756	6.0777	6.0798	6.0820	6.0841
990.000	6.0862	6.0883	6.0904	6.0925	6.0946
995.000	6.0967	6.0987	6.1008	6.1029	6.1049
1,000.000	6.1070	6.1090	6.1111	6.1131	6.1151
1,005.000	6.1171	6.1192	6.1212	6.1232	6.1252
1,010.000	6.1272	6.1292	6.1312	6.1331	6.1351
1,015.000	6.1371	6.1390	6.1410	6.1429	6.1449
1,020.000	6.1468	6.1488	6.1507	6.1526	6.1546
1,025.000	6.1565	6.1584	6.1603	6.1622	6.1641
1,030.000	6.1660	6.1679	6.1698	6.1716	6.1735

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	6.1754	6.1772	6.1791	6.1809	6.1828
1,040.000	6.1846	6.1865	6.1883	6.1902	6.1920
1,045.000	6.1938	6.1956	6.1974	6.1992	6.2010
1,050.000	6.2028	6.2046	6.2064	6.2082	6.2100
1,055.000	6.2118	6.2136	6.2153	6.2171	6.2189
1,060.000	6.2206	6.2224	6.2241	6.2259	6.2276
1,065.000	6.2293	6.2311	6.2328	6.2345	6.2363
1,070.000	6.2380	6.2397	6.2414	6.2431	6.2448
1,075.000	6.2465	6.2482	6.2499	6.2516	6.2533
1,080.000	6.2549	6.2564	6.2579	6.2594	6.2608
1,085.000	6.2623	6.2638	6.2652	6.2667	6.2682
1,090.000	6.2696	6.2711	6.2725	6.2740	6.2754
1,095.000	6.2768	6.2783	6.2797	6.2811	6.2826
1,100.000	6.2840	6.2854	6.2868	6.2882	6.2896
1,105.000	6.2911	6.2925	6.2939	6.2953	6.2967
1,110.000	6.2980	6.2994	6.3008	6.3022	6.3036
1,115.000	6.3050	6.3063	6.3077	6.3091	6.3105
1,120.000	6.3118	6.3132	6.3145	6.3159	6.3172
1,125.000	6.3186	6.3199	6.3213	6.3226	6.3240
1,130.000	6.3253	6.3266	6.3280	6.3293	6.3306
1,135.000	6.3320	6.3333	6.3346	6.3359	6.3372
1,140.000	6.3385	6.3398	6.3412	6.3425	6.3438
1,145.000	6.3451	6.3464	6.3476	6.3489	6.3502
1,150.000	6.3515	6.3528	6.3541	6.3554	6.3566
1,155.000	6.3579	6.3592	6.3604	6.3617	6.3630
1,160.000	6.3642	6.3655	6.3668	6.3680	6.3693
1,165.000	6.3705	6.3718	6.3730	6.3743	6.3755
1,170.000	6.3767	6.3780	6.3792	6.3804	6.3817
1,175.000	6.3829	6.3841	6.3853	6.3866	6.3878
1,180.000	6.3890	6.3902	6.3914	6.3926	6.3938
1,185.000	6.3951	6.3963	6.3975	6.3987	6.3999
1,190.000	6.4011	6.4022	6.4034	6.4046	6.4058
1,195.000	6.4070	6.4082	6.4094	6.4105	6.4117
1,200.000	6.4129	6.4141	6.4152	6.4164	6.4176
1,205.000	6.4188	6.4199	6.4211	6.4222	6.4234
1,210.000	6.4246	6.4257	6.4269	6.4280	6.4292
1,215.000	6.4303	6.4314	6.4326	6.4337	6.4349
1,220.000	6.4360	6.4371	6.4383	6.4394	6.4405
1,225.000	6.4417	6.4428	6.4439	6.4450	6.4461
1,230.000	6.4473	6.4484	6.4495	6.4506	6.4517
1,235.000	6.4528	6.4539	6.4550	6.4561	6.4572
1,240.000	6.4583	6.4594	6.4605	6.4616	6.4627
1,245.000	6.4638	6.4649	6.4660	6.4671	6.4682

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	6.4693	6.4703	6.4714	6.4725	6.4736
1,255.000	6.4747	6.4757	6.4768	6.4779	6.4789
1,260.000	6.4800	6.4811	6.4821	6.4832	6.4843
1,265.000	6.4853	6.4864	6.4874	6.4885	6.4895
1,270.000	6.4906	6.4916	6.4927	6.4937	6.4948
1,275.000	6.4958	6.4969	6.4979	6.4990	6.5000
1,280.000	6.5010	6.5021	6.5031	6.5041	6.5052
1,285.000	6.5062	6.5072	6.5082	6.5093	6.5103
1,290.000	6.5113	6.5123	6.5133	6.5144	6.5154
1,295.000	6.5164	6.5174	6.5184	6.5194	6.5204
1,300.000	6.5214	6.5225	6.5235	6.5245	6.5255
1,305.000	6.5265	6.5275	6.5285	6.5295	6.5304
1,310.000	6.5314	6.5324	6.5334	6.5344	6.5354
1,315.000	6.5364	6.5374	6.5383	6.5393	6.5403
1,320.000	6.5413	6.5423	6.5432	6.5442	6.5452
1,325.000	6.5462	6.5471	6.5481	6.5491	6.5500
1,330.000	6.5510	6.5520	6.5529	6.5539	6.5549
1,335.000	6.5558	6.5568	6.5577	6.5587	6.5596
1,340.000	6.5606	6.5615	6.5625	6.5634	6.5644
1,345.000	6.5653	6.5663	6.5672	6.5682	6.5691
1,350.000	6.5700	6.5710	6.5719	6.5729	6.5738
1,355.000	6.5747	6.5757	6.5766	6.5775	6.5785
1,360.000	6.5794	6.5803	6.5813	6.5822	6.5831
1,365.000	6.5840	6.5849	6.5859	6.5868	6.5877
1,370.000	6.5886	6.5895	6.5904	6.5914	6.5923
1,375.000	6.5932	6.5941	6.5950	6.5959	6.5968
1,380.000	6.5977	6.5986	6.5995	6.6004	6.6013
1,385.000	6.6022	6.6031	6.6040	6.6049	6.6058
1,390.000	6.6067	6.6076	6.6085	6.6094	6.6103
1,395.000	6.6112	6.6121	6.6129	6.6138	6.6147
1,400.000	6.6156	6.6165	6.6174	6.6182	6.6191
1,405.000	6.6200	6.6209	6.6218	6.6226	6.6235
1,410.000	6.6244	6.6253	6.6261	6.6270	6.6279
1,415.000	6.6287	6.6296	6.6305	6.6313	6.6322
1,420.000	6.6331	6.6339	6.6348	6.6356	6.6365
1,425.000	6.6374	6.6382	6.6391	6.6399	6.6408
1,430.000	6.6416	6.6425	6.6433	6.6442	6.6450
1,435.000	6.6459	6.6467	6.6476	6.6484	6.6493
1,440.000	6.6501	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time-Depth Curve: 100 Yr Storm	
Label	100 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	100 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0014	0.0028	0.0042	0.0057
5.000	0.0071	0.0085	0.0099	0.0113	0.0128
10.000	0.0142	0.0156	0.0170	0.0185	0.0199
15.000	0.0213	0.0228	0.0242	0.0257	0.0271
20.000	0.0285	0.0300	0.0314	0.0329	0.0343
25.000	0.0358	0.0372	0.0387	0.0401	0.0416
30.000	0.0431	0.0445	0.0460	0.0475	0.0489
35.000	0.0504	0.0519	0.0533	0.0548	0.0563
40.000	0.0578	0.0592	0.0607	0.0622	0.0637
45.000	0.0652	0.0667	0.0682	0.0697	0.0711
50.000	0.0726	0.0741	0.0756	0.0771	0.0786
55.000	0.0802	0.0817	0.0832	0.0847	0.0862
60.000	0.0877	0.0892	0.0908	0.0923	0.0938
65.000	0.0953	0.0969	0.0984	0.0999	0.1014
70.000	0.1030	0.1045	0.1061	0.1076	0.1091
75.000	0.1107	0.1122	0.1138	0.1153	0.1169
80.000	0.1184	0.1200	0.1215	0.1231	0.1247
85.000	0.1262	0.1278	0.1294	0.1309	0.1325
90.000	0.1341	0.1357	0.1372	0.1388	0.1404
95.000	0.1420	0.1436	0.1452	0.1468	0.1484
100.000	0.1500	0.1516	0.1532	0.1548	0.1564
105.000	0.1580	0.1596	0.1612	0.1628	0.1644
110.000	0.1660	0.1677	0.1693	0.1709	0.1725
115.000	0.1742	0.1758	0.1774	0.1791	0.1807
120.000	0.1823	0.1840	0.1856	0.1873	0.1889
125.000	0.1906	0.1922	0.1939	0.1956	0.1972
130.000	0.1989	0.2005	0.2022	0.2039	0.2056
135.000	0.2072	0.2089	0.2106	0.2123	0.2140
140.000	0.2157	0.2173	0.2190	0.2207	0.2224
145.000	0.2241	0.2258	0.2275	0.2292	0.2309
150.000	0.2327	0.2344	0.2361	0.2378	0.2395
155.000	0.2412	0.2430	0.2447	0.2464	0.2482
160.000	0.2499	0.2517	0.2534	0.2551	0.2569
165.000	0.2586	0.2604	0.2622	0.2639	0.2657
170.000	0.2674	0.2692	0.2710	0.2727	0.2745

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.2763	0.2781	0.2799	0.2816	0.2834
180.000	0.2852	0.2870	0.2888	0.2906	0.2924
185.000	0.2942	0.2960	0.2978	0.2996	0.3015
190.000	0.3033	0.3051	0.3069	0.3088	0.3106
195.000	0.3124	0.3143	0.3161	0.3179	0.3198
200.000	0.3216	0.3235	0.3253	0.3272	0.3291
205.000	0.3309	0.3328	0.3347	0.3365	0.3384
210.000	0.3403	0.3422	0.3440	0.3459	0.3478
215.000	0.3497	0.3516	0.3535	0.3554	0.3573
220.000	0.3592	0.3611	0.3631	0.3650	0.3669
225.000	0.3688	0.3708	0.3727	0.3746	0.3766
230.000	0.3785	0.3805	0.3824	0.3844	0.3863
235.000	0.3883	0.3902	0.3922	0.3942	0.3962
240.000	0.3981	0.4001	0.4021	0.4041	0.4061
245.000	0.4081	0.4101	0.4121	0.4141	0.4161
250.000	0.4181	0.4201	0.4221	0.4241	0.4262
255.000	0.4282	0.4302	0.4323	0.4343	0.4363
260.000	0.4384	0.4404	0.4425	0.4445	0.4466
265.000	0.4487	0.4507	0.4528	0.4549	0.4570
270.000	0.4591	0.4611	0.4632	0.4653	0.4674
275.000	0.4695	0.4716	0.4738	0.4759	0.4780
280.000	0.4801	0.4822	0.4844	0.4865	0.4887
285.000	0.4908	0.4929	0.4951	0.4973	0.4994
290.000	0.5016	0.5038	0.5059	0.5081	0.5103
295.000	0.5125	0.5147	0.5169	0.5191	0.5213
300.000	0.5235	0.5257	0.5279	0.5301	0.5323
305.000	0.5346	0.5368	0.5390	0.5413	0.5435
310.000	0.5458	0.5480	0.5503	0.5526	0.5548
315.000	0.5571	0.5594	0.5617	0.5640	0.5663
320.000	0.5686	0.5709	0.5732	0.5755	0.5778
325.000	0.5801	0.5825	0.5848	0.5871	0.5895
330.000	0.5918	0.5942	0.5965	0.5989	0.6013
335.000	0.6036	0.6060	0.6084	0.6108	0.6132
340.000	0.6156	0.6180	0.6204	0.6228	0.6252
345.000	0.6277	0.6301	0.6325	0.6350	0.6374
350.000	0.6399	0.6423	0.6448	0.6473	0.6497
355.000	0.6522	0.6547	0.6572	0.6597	0.6622
360.000	0.6647	0.6679	0.6711	0.6744	0.6776
365.000	0.6808	0.6841	0.6873	0.6906	0.6939
370.000	0.6971	0.7004	0.7037	0.7070	0.7103
375.000	0.7136	0.7170	0.7203	0.7236	0.7270
380.000	0.7303	0.7337	0.7370	0.7404	0.7438
385.000	0.7472	0.7506	0.7540	0.7574	0.7608

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.7643	0.7677	0.7712	0.7746	0.7781
395.000	0.7816	0.7850	0.7885	0.7920	0.7955
400.000	0.7991	0.8026	0.8061	0.8097	0.8132
405.000	0.8168	0.8203	0.8239	0.8275	0.8311
410.000	0.8347	0.8383	0.8419	0.8456	0.8492
415.000	0.8529	0.8565	0.8602	0.8639	0.8676
420.000	0.8713	0.8750	0.8787	0.8824	0.8862
425.000	0.8899	0.8937	0.8974	0.9012	0.9050
430.000	0.9088	0.9126	0.9164	0.9203	0.9241
435.000	0.9280	0.9318	0.9357	0.9396	0.9435
440.000	0.9474	0.9513	0.9552	0.9592	0.9631
445.000	0.9671	0.9710	0.9750	0.9790	0.9830
450.000	0.9870	0.9911	0.9951	0.9992	1.0032
455.000	1.0073	1.0114	1.0155	1.0196	1.0237
460.000	1.0279	1.0320	1.0362	1.0404	1.0446
465.000	1.0488	1.0530	1.0572	1.0615	1.0657
470.000	1.0700	1.0743	1.0786	1.0829	1.0872
475.000	1.0915	1.0959	1.1003	1.1046	1.1090
480.000	1.1134	1.1179	1.1223	1.1267	1.1312
485.000	1.1357	1.1402	1.1447	1.1492	1.1538
490.000	1.1583	1.1629	1.1675	1.1721	1.1767
495.000	1.1814	1.1860	1.1907	1.1954	1.2001
500.000	1.2048	1.2095	1.2143	1.2191	1.2238
505.000	1.2287	1.2335	1.2383	1.2432	1.2481
510.000	1.2530	1.2579	1.2628	1.2677	1.2727
515.000	1.2777	1.2827	1.2877	1.2928	1.2979
520.000	1.3030	1.3081	1.3132	1.3183	1.3235
525.000	1.3287	1.3339	1.3391	1.3444	1.3497
530.000	1.3550	1.3603	1.3656	1.3710	1.3764
535.000	1.3818	1.3872	1.3927	1.3981	1.4036
540.000	1.4092	1.4174	1.4257	1.4340	1.4424
545.000	1.4508	1.4592	1.4677	1.4762	1.4847
550.000	1.4932	1.5018	1.5105	1.5191	1.5278
555.000	1.5365	1.5453	1.5541	1.5630	1.5718
560.000	1.5808	1.5897	1.5987	1.6078	1.6168
565.000	1.6259	1.6351	1.6443	1.6535	1.6628
570.000	1.6721	1.6815	1.6909	1.7003	1.7098
575.000	1.7194	1.7289	1.7386	1.7482	1.7580
580.000	1.7677	1.7776	1.7874	1.7973	1.8073
585.000	1.8173	1.8274	1.8375	1.8476	1.8579
590.000	1.8681	1.8785	1.8888	1.8993	1.9098
595.000	1.9203	1.9309	1.9416	1.9523	1.9631
600.000	1.9740	1.9849	1.9958	2.0000	2.0180

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	2.0292	2.0404	2.0517	2.0631	2.0745
610.000	2.0860	2.0976	2.1092	2.1210	2.1328
615.000	2.1446	2.1566	2.1686	2.1807	2.1929
620.000	2.2052	2.2176	2.2300	2.2426	2.2552
625.000	2.2679	2.2807	2.2936	2.3066	2.3197
630.000	2.3329	2.3502	2.3676	2.3851	2.4027
635.000	2.4205	2.4383	2.4563	2.4745	2.4927
640.000	2.5111	2.5296	2.5483	2.5671	2.5861
645.000	2.6052	2.6244	2.6438	2.6634	2.6831
650.000	2.7030	2.7230	2.7433	2.7637	2.7842
655.000	2.8050	2.8259	2.8471	2.8684	2.8900
660.000	2.9117	2.9355	2.9596	2.9838	3.0083
665.000	3.0331	3.0580	3.0833	3.1088	3.1346
670.000	3.1607	3.1870	3.2137	3.2406	3.2679
675.000	3.2955	3.3235	3.3518	3.3804	3.4095
680.000	3.4389	3.4688	3.4991	3.5298	3.5610
685.000	3.5926	3.6248	3.6575	3.6907	3.7245
690.000	3.7589	3.7897	3.8212	3.8534	3.8862
695.000	3.9199	3.9544	3.9897	4.0260	4.0633
700.000	4.1017	4.1412	4.1820	4.2242	4.2680
705.000	4.3133	4.3752	4.4392	4.5056	4.5746
710.000	4.6468	4.7224	4.8021	4.8867	4.9854
715.000	5.0913	5.2264	5.3727	5.5350	5.7870
720.000	6.0390	6.2910	6.5430	6.7950	6.9485
725.000	7.0888	7.1990	7.3010	7.3967	7.4787
730.000	7.5563	7.6302	7.7007	7.7684	7.8336
735.000	7.8964	7.9427	7.9873	8.0302	8.0717
740.000	8.1119	8.1508	8.1886	8.2254	8.2612
745.000	8.2961	8.3302	8.3635	8.3960	8.4278
750.000	8.4589	8.4936	8.5278	8.5613	8.5942
755.000	8.6267	8.6586	8.6900	8.7209	8.7514
760.000	8.7815	8.8111	8.8403	8.8692	8.8977
765.000	8.9258	8.9536	8.9810	9.0082	9.0350
770.000	9.0615	9.0877	9.1136	9.1393	9.1646
775.000	9.1898	9.2146	9.2392	9.2636	9.2878
780.000	9.3117	9.3336	9.3552	9.3766	9.3979
785.000	9.4189	9.4398	9.4604	9.4809	9.5012
790.000	9.5214	9.5414	9.5612	9.5808	9.6003
795.000	9.6196	9.6388	9.6578	9.6767	9.6954
800.000	9.7140	9.7325	9.7508	9.7690	9.7871
805.000	9.8050	9.8228	9.8405	9.8581	9.8755
810.000	9.8929	9.9061	9.9193	9.9323	9.9452
815.000	9.9581	9.9709	9.9835	9.9961	10.0086

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	10.0210	10.0333	10.0456	10.0577	10.0698
825.000	10.0818	10.0937	10.1055	10.1173	10.1290
830.000	10.1406	10.1522	10.1636	10.1750	10.1864
835.000	10.1976	10.2088	10.2200	10.2310	10.2420
840.000	10.2530	10.2638	10.2747	10.2854	10.2961
845.000	10.3068	10.3173	10.3279	10.3383	10.3487
850.000	10.3591	10.3694	10.3796	10.3898	10.4000
855.000	10.4101	10.4201	10.4301	10.4400	10.4499
860.000	10.4598	10.4696	10.4793	10.4890	10.4986
865.000	10.5083	10.5178	10.5273	10.5368	10.5462
870.000	10.5556	10.5650	10.5743	10.5835	10.5927
875.000	10.6019	10.6110	10.6201	10.6292	10.6382
880.000	10.6472	10.6561	10.6650	10.6739	10.6827
885.000	10.6915	10.7002	10.7090	10.7176	10.7263
890.000	10.7349	10.7435	10.7520	10.7605	10.7690
895.000	10.7774	10.7859	10.7942	10.8026	10.8109
900.000	10.8192	10.8247	10.8302	10.8357	10.8412
905.000	10.8466	10.8520	10.8574	10.8628	10.8681
910.000	10.8735	10.8788	10.8841	10.8893	10.8946
915.000	10.8998	10.9050	10.9102	10.9153	10.9205
920.000	10.9256	10.9307	10.9358	10.9408	10.9459
925.000	10.9509	10.9559	10.9609	10.9658	10.9708
930.000	10.9757	10.9806	10.9855	10.9903	10.9952
935.000	11.0000	11.0048	11.0096	11.0144	11.0192
940.000	11.0239	11.0287	11.0334	11.0381	11.0427
945.000	11.0474	11.0520	11.0567	11.0613	11.0659
950.000	11.0705	11.0750	11.0796	11.0841	11.0886
955.000	11.0931	11.0976	11.1021	11.1066	11.1110
960.000	11.1154	11.1198	11.1242	11.1286	11.1330
965.000	11.1374	11.1417	11.1460	11.1504	11.1547
970.000	11.1589	11.1632	11.1675	11.1717	11.1760
975.000	11.1802	11.1844	11.1886	11.1928	11.1970
980.000	11.2011	11.2053	11.2094	11.2135	11.2176
985.000	11.2217	11.2258	11.2299	11.2339	11.2380
990.000	11.2420	11.2460	11.2501	11.2541	11.2580
995.000	11.2620	11.2660	11.2700	11.2739	11.2778
1,000.000	11.2817	11.2857	11.2896	11.2934	11.2973
1,005.000	11.3012	11.3051	11.3089	11.3127	11.3166
1,010.000	11.3204	11.3242	11.3280	11.3317	11.3355
1,015.000	11.3393	11.3430	11.3468	11.3505	11.3542
1,020.000	11.3579	11.3616	11.3653	11.3690	11.3727
1,025.000	11.3763	11.3800	11.3837	11.3873	11.3909
1,030.000	11.3945	11.3982	11.4018	11.4053	11.4089

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	11.4125	11.4161	11.4196	11.4232	11.4267
1,040.000	11.4302	11.4338	11.4373	11.4408	11.4443
1,045.000	11.4477	11.4512	11.4547	11.4582	11.4616
1,050.000	11.4650	11.4685	11.4719	11.4753	11.4787
1,055.000	11.4821	11.4855	11.4889	11.4923	11.4957
1,060.000	11.4990	11.5024	11.5058	11.5091	11.5124
1,065.000	11.5158	11.5191	11.5224	11.5257	11.5290
1,070.000	11.5323	11.5356	11.5388	11.5427	11.5454
1,075.000	11.5486	11.5518	11.5551	11.5583	11.5615
1,080.000	11.5648	11.5673	11.5698	11.5723	11.5748
1,085.000	11.5773	11.5797	11.5822	11.5847	11.5872
1,090.000	11.5896	11.5921	11.5945	11.5970	11.5994
1,095.000	11.6019	11.6043	11.6067	11.6091	11.6115
1,100.000	11.6139	11.6163	11.6187	11.6211	11.6235
1,105.000	11.6259	11.6283	11.6307	11.6330	11.6354
1,110.000	11.6377	11.6401	11.6424	11.6448	11.6471
1,115.000	11.6495	11.6518	11.6541	11.6564	11.6587
1,120.000	11.6610	11.6633	11.6656	11.6679	11.6702
1,125.000	11.6725	11.6748	11.6771	11.6793	11.6816
1,130.000	11.6838	11.6861	11.6883	11.6906	11.6928
1,135.000	11.6951	11.6973	11.6995	11.7018	11.7040
1,140.000	11.7062	11.7084	11.7106	11.7128	11.7150
1,145.000	11.7172	11.7194	11.7216	11.7237	11.7259
1,150.000	11.7281	11.7303	11.7324	11.7346	11.7367
1,155.000	11.7389	11.7410	11.7432	11.7453	11.7474
1,160.000	11.7496	11.7517	11.7538	11.7559	11.7581
1,165.000	11.7602	11.7623	11.7644	11.7665	11.7686
1,170.000	11.7707	11.7727	11.7748	11.7769	11.7790
1,175.000	11.7811	11.7831	11.7852	11.7872	11.7893
1,180.000	11.7914	11.7934	11.7955	11.7975	11.7995
1,185.000	11.8016	11.8036	11.8056	11.8076	11.8097
1,190.000	11.8117	11.8137	11.8157	11.8177	11.8197
1,195.000	11.8217	11.8237	11.8257	11.8277	11.8297
1,200.000	11.8316	11.8336	11.8356	11.8376	11.8395
1,205.000	11.8415	11.8435	11.8454	11.8474	11.8493
1,210.000	11.8513	11.8532	11.8551	11.8571	11.8590
1,215.000	11.8609	11.8629	11.8648	11.8667	11.8686
1,220.000	11.8705	11.8725	11.8744	11.8763	11.8782
1,225.000	11.8801	11.8820	11.8839	11.8857	11.8876
1,230.000	11.8895	11.8914	11.8933	11.8951	11.8970
1,235.000	11.8989	11.9007	11.9026	11.9045	11.9063
1,240.000	11.9082	11.9100	11.9119	11.9137	11.9156
1,245.000	11.9174	11.9192	11.9211	11.9229	11.9247

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	11.9265	11.9284	11.9302	11.9320	11.9338
1,255.000	11.9356	11.9374	11.9392	11.9410	11.9428
1,260.000	11.9446	11.9464	11.9482	11.9500	11.9518
1,265.000	11.9536	11.9553	11.9571	11.9589	11.9607
1,270.000	11.9624	11.9642	11.9660	11.9677	11.9695
1,275.000	11.9712	11.9730	11.9747	11.9765	11.9782
1,280.000	11.9800	11.9817	11.9834	11.9852	11.9869
1,285.000	11.9886	11.9904	11.9921	11.9938	11.9955
1,290.000	11.9972	11.9989	12.0007	12.0024	12.0041
1,295.000	12.0058	12.0075	12.0092	12.0109	12.0126
1,300.000	12.0143	12.0159	12.0176	12.0193	12.0210
1,305.000	12.0227	12.0244	12.0260	12.0277	12.0294
1,310.000	12.0310	12.0327	12.0344	12.0360	12.0377
1,315.000	12.0393	12.0410	12.0426	12.0443	12.0459
1,320.000	12.0476	12.0492	12.0509	12.0525	12.0541
1,325.000	12.0558	12.0574	12.0590	12.0606	12.0623
1,330.000	12.0639	12.0655	12.0671	12.0687	12.0703
1,335.000	12.0720	12.0736	12.0752	12.0768	12.0784
1,340.000	12.0800	12.0816	12.0832	12.0848	12.0864
1,345.000	12.0879	12.0895	12.0911	12.0927	12.0943
1,350.000	12.0959	12.0974	12.0990	12.1006	12.1021
1,355.000	12.1037	12.1053	12.1068	12.1084	12.1100
1,360.000	12.1115	12.1131	12.1146	12.1162	12.1177
1,365.000	12.1193	12.1208	12.1224	12.1239	12.1255
1,370.000	12.1270	12.1285	12.1301	12.1316	12.1331
1,375.000	12.1347	12.1362	12.1377	12.1392	12.1407
1,380.000	12.1423	12.1438	12.1453	12.1468	12.1483
1,385.000	12.1498	12.1513	12.1528	12.1543	12.1558
1,390.000	12.1573	12.1588	12.1603	12.1618	12.1633
1,395.000	12.1648	12.1663	12.1678	12.1693	12.1707
1,400.000	12.1722	12.1737	12.1752	12.1767	12.1781
1,405.000	12.1796	12.1811	12.1825	12.1840	12.1855
1,410.000	12.1869	12.1884	12.1899	12.1913	12.1928
1,415.000	12.1942	12.1957	12.1971	12.1986	12.2000
1,420.000	12.2015	12.2029	12.2044	12.2058	12.2072
1,425.000	12.2087	12.2101	12.2115	12.2130	12.2144
1,430.000	12.2158	12.2173	12.2187	12.2201	12.2215
1,435.000	12.2230	12.2244	12.2258	12.2272	12.2286
1,440.000	12.2300	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time-Depth Curve: 2 Yr Storm

Label	2 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	2 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0000	0.0000	0.0000	0.0000
5.000	0.0000	0.0000	0.0000	0.0000	0.0000
10.000	0.0000	0.0000	0.0000	0.0000	0.0000
15.000	0.0000	0.0000	0.0000	0.0000	0.0000
20.000	0.0000	0.0000	0.0000	0.0000	0.0000
25.000	0.0000	0.0000	0.0000	0.0000	0.0000
30.000	0.0000	0.0000	0.0000	0.0000	0.0000
35.000	0.0000	0.0000	0.0000	0.0000	0.0000
40.000	0.0000	0.0000	0.0000	0.0000	0.0000
45.000	0.0000	0.0000	0.0000	0.0000	0.0000
50.000	0.0000	0.0000	0.0000	0.0000	0.0000
55.000	0.0000	0.0000	0.0000	0.0000	0.0000
60.000	0.0000	0.0000	0.0000	0.0000	0.0000
65.000	0.0000	0.0000	0.0000	0.0000	0.0000
70.000	0.0000	0.0000	0.0000	0.0000	0.0000
75.000	0.0000	0.0000	0.0000	0.0000	0.0000
80.000	0.0000	0.0000	0.0000	0.0000	0.0000
85.000	0.0000	0.0200	0.0400	0.0600	0.0800
90.000	0.1000	0.1000	0.1000	0.1000	0.1000
95.000	0.1000	0.1000	0.1000	0.1000	0.1000
100.000	0.1000	0.1000	0.1000	0.1000	0.1000
105.000	0.1000	0.1000	0.1000	0.1000	0.1000
110.000	0.1000	0.1000	0.1000	0.1000	0.1000
115.000	0.1000	0.1000	0.1000	0.1000	0.1000
120.000	0.1000	0.1000	0.1000	0.1000	0.1000
125.000	0.1000	0.1000	0.1000	0.1000	0.1000
130.000	0.1000	0.1000	0.1000	0.1000	0.1000
135.000	0.1000	0.1000	0.1000	0.1000	0.1000
140.000	0.1000	0.1000	0.1000	0.1000	0.1000
145.000	0.1000	0.1000	0.1000	0.1000	0.1000
150.000	0.1000	0.1000	0.1000	0.1000	0.1000
155.000	0.1000	0.1000	0.1000	0.1000	0.1000
160.000	0.1000	0.1000	0.1000	0.1000	0.1000
165.000	0.1000	0.1000	0.1000	0.1000	0.1000
170.000	0.1000	0.1000	0.1000	0.1000	0.1000

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.1000	0.1000	0.1000	0.1000	0.1000
180.000	0.1000	0.1000	0.1000	0.1000	0.1000
185.000	0.1000	0.1000	0.1000	0.1000	0.1000
190.000	0.1000	0.1000	0.1000	0.1000	0.1000
195.000	0.1000	0.1000	0.1000	0.1000	0.1000
200.000	0.1000	0.1000	0.1000	0.1000	0.1000
205.000	0.1000	0.1000	0.1000	0.1000	0.1000
210.000	0.1000	0.1000	0.1000	0.1000	0.1000
215.000	0.1000	0.1000	0.1000	0.1000	0.1000
220.000	0.1000	0.1000	0.1000	0.1000	0.1000
225.000	0.1000	0.1000	0.1000	0.1000	0.1000
230.000	0.1000	0.1000	0.1000	0.1000	0.1000
235.000	0.1000	0.1000	0.1000	0.1000	0.1000
240.000	0.1000	0.1200	0.1400	0.1600	0.1800
245.000	0.2000	0.2000	0.2000	0.2000	0.2000
250.000	0.2000	0.2000	0.2000	0.2000	0.2000
255.000	0.2000	0.2000	0.2000	0.2000	0.2000
260.000	0.2000	0.2000	0.2000	0.2000	0.2000
265.000	0.2000	0.2000	0.2000	0.2000	0.2000
270.000	0.2000	0.2000	0.2000	0.2000	0.2000
275.000	0.2000	0.2000	0.2000	0.2000	0.2000
280.000	0.2000	0.2000	0.2000	0.2000	0.2000
285.000	0.2000	0.2000	0.2000	0.2000	0.2000
290.000	0.2000	0.2000	0.2000	0.2000	0.2000
295.000	0.2000	0.2000	0.2000	0.2000	0.2000
300.000	0.2000	0.2000	0.2000	0.2000	0.2000
305.000	0.2000	0.2000	0.2000	0.2000	0.2000
310.000	0.2000	0.2000	0.2000	0.2000	0.2000
315.000	0.2000	0.2000	0.2000	0.2000	0.2000
320.000	0.2000	0.2000	0.2000	0.2000	0.2000
325.000	0.2000	0.2000	0.2000	0.2000	0.2000
330.000	0.2000	0.2000	0.2000	0.2000	0.2000
335.000	0.2000	0.2000	0.2000	0.2000	0.2000
340.000	0.2000	0.2000	0.2000	0.2000	0.2000
345.000	0.2000	0.2000	0.2000	0.2000	0.2000
350.000	0.2000	0.2000	0.2000	0.2000	0.2000
355.000	0.2000	0.2000	0.2000	0.2000	0.2000
360.000	0.2000	0.2200	0.2400	0.2600	0.2800
365.000	0.3000	0.3000	0.3000	0.3000	0.3000
370.000	0.3000	0.3000	0.3000	0.3000	0.3000
375.000	0.3000	0.3000	0.3000	0.3000	0.3000
380.000	0.3000	0.3000	0.3000	0.3000	0.3000
385.000	0.3000	0.3000	0.3000	0.3000	0.3000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.3000	0.3000	0.3000	0.3000	0.3000
395.000	0.3000	0.3000	0.3000	0.3000	0.3000
400.000	0.3000	0.3000	0.3000	0.3000	0.3000
405.000	0.3000	0.3000	0.3000	0.3000	0.3000
410.000	0.3000	0.3000	0.3000	0.3000	0.3000
415.000	0.3000	0.3000	0.3000	0.3000	0.3000
420.000	0.3000	0.3000	0.3000	0.3000	0.3000
425.000	0.3000	0.3000	0.3000	0.3000	0.3000
430.000	0.3000	0.3000	0.3000	0.3000	0.3000
435.000	0.3000	0.3000	0.3000	0.3000	0.3000
440.000	0.3000	0.3000	0.3000	0.3000	0.3000
445.000	0.3000	0.3000	0.3000	0.3000	0.3000
450.000	0.3000	0.3200	0.3400	0.3600	0.3800
455.000	0.4000	0.4000	0.4000	0.4000	0.4000
460.000	0.4000	0.4000	0.4000	0.4000	0.4000
465.000	0.4000	0.4000	0.4000	0.4000	0.4000
470.000	0.4000	0.4000	0.4000	0.4000	0.4000
475.000	0.4000	0.4000	0.4000	0.4000	0.4000
480.000	0.4000	0.4000	0.4000	0.4000	0.4000
485.000	0.4000	0.4000	0.4000	0.4000	0.4000
490.000	0.4000	0.4000	0.4000	0.4000	0.4000
495.000	0.4000	0.4000	0.4000	0.4000	0.4000
500.000	0.4000	0.4000	0.4000	0.4000	0.4000
505.000	0.4000	0.4000	0.4000	0.4000	0.4000
510.000	0.4000	0.4200	0.4400	0.4600	0.4800
515.000	0.5000	0.5000	0.5000	0.5000	0.5000
520.000	0.5000	0.5000	0.5000	0.5000	0.5000
525.000	0.5000	0.5000	0.5000	0.5000	0.5000
530.000	0.5000	0.5000	0.5000	0.5000	0.5000
535.000	0.5000	0.5000	0.5000	0.5000	0.5000
540.000	0.5000	0.5000	0.5000	0.5000	0.5000
545.000	0.5000	0.5000	0.5000	0.5000	0.5000
550.000	0.5000	0.5000	0.5000	0.5000	0.5000
555.000	0.5000	0.5000	0.5000	0.5000	0.5000
560.000	0.5000	0.5200	0.5400	0.5600	0.5800
565.000	0.6000	0.6000	0.6000	0.6000	0.6000
570.000	0.6000	0.6000	0.6000	0.6000	0.6000
575.000	0.6000	0.6000	0.6000	0.6000	0.6000
580.000	0.6000	0.6000	0.6000	0.6000	0.6000
585.000	0.6000	0.6000	0.6000	0.6000	0.6000
590.000	0.6000	0.6000	0.6000	0.6000	0.6000
595.000	0.6000	0.6200	0.6400	0.6600	0.6800
600.000	0.7000	0.7000	0.7000	0.7000	0.7000

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	0.7000	0.7000	0.7000	0.7000	0.7000
610.000	0.7000	0.7000	0.7000	0.7000	0.7000
615.000	0.7000	0.7000	0.7000	0.7000	0.7000
620.000	0.7000	0.7000	0.7000	0.7000	0.7000
625.000	0.7000	0.7000	0.7000	0.7000	0.7000
630.000	0.7000	0.7200	0.7400	0.7600	0.7800
635.000	0.8000	0.8000	0.8000	0.8000	0.8000
640.000	0.8000	0.8000	0.8000	0.8000	0.8000
645.000	0.8000	0.8000	0.8000	0.8000	0.8000
650.000	0.8000	0.8000	0.8000	0.8000	0.8000
655.000	0.8000	0.8200	0.8400	0.8600	0.8800
660.000	0.9000	0.9000	0.9000	0.9000	0.9000
665.000	0.9000	0.9000	0.9000	0.9000	0.9000
670.000	0.9000	0.9000	0.9000	0.9000	0.9000
675.000	0.9000	0.9200	0.9400	0.9600	0.9800
680.000	1.0000	1.0000	1.0000	1.0000	1.0000
685.000	1.0000	1.0000	1.0000	1.0000	1.0000
690.000	1.0000	1.0200	1.0400	1.0600	1.0800
695.000	1.1000	1.1000	1.1000	1.1000	1.1000
700.000	1.1000	1.1200	1.1400	1.1600	1.1800
705.000	1.2000	1.2200	1.2400	1.2600	1.2800
710.000	1.3000	1.3400	1.3800	1.4200	1.4600
715.000	1.5000	1.5400	1.5800	1.6200	1.6600
720.000	1.7000	1.7600	1.8200	1.8800	1.9400
725.000	2.0000	2.0200	2.0400	2.0600	2.0800
730.000	2.1000	2.1200	2.1400	2.1600	2.1800
735.000	2.2000	2.2200	2.2400	2.2600	2.2800
740.000	2.3000	2.3200	2.3400	2.3600	2.3800
745.000	2.4000	2.4000	2.4000	2.4000	2.4000
750.000	2.4000	2.4000	2.4000	2.4000	2.4000
755.000	2.4000	2.4200	2.4400	2.4600	2.4800
760.000	2.5000	2.5000	2.5000	2.5000	2.5000
765.000	2.5000	2.5000	2.5000	2.5000	2.5000
770.000	2.5000	2.5200	2.5400	2.5600	2.5800
775.000	2.6000	2.6000	2.6000	2.6000	2.6000
780.000	2.6000	2.6000	2.6000	2.6000	2.6000
785.000	2.6000	2.6000	2.6000	2.6000	2.6000
790.000	2.6000	2.6000	2.6000	2.6000	2.6000
795.000	2.6000	2.6200	2.6400	2.6600	2.6800
800.000	2.7000	2.7000	2.7000	2.7000	2.7000
805.000	2.7000	2.7000	2.7000	2.7000	2.7000
810.000	2.7000	2.7000	2.7000	2.7000	2.7000
815.000	2.7000	2.7000	2.7000	2.7000	2.7000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	2.7000	2.7000	2.7000	2.7000	2.7000
825.000	2.7000	2.7200	2.7400	2.7600	2.7800
830.000	2.8000	2.8000	2.8000	2.8000	2.8000
835.000	2.8000	2.8000	2.8000	2.8000	2.8000
840.000	2.8000	2.8000	2.8000	2.8000	2.8000
845.000	2.8000	2.8000	2.8000	2.8000	2.8000
850.000	2.8000	2.8000	2.8000	2.8000	2.8000
855.000	2.8000	2.8000	2.8000	2.8000	2.8000
860.000	2.8000	2.8200	2.8400	2.8600	2.8800
865.000	2.9000	2.9000	2.9000	2.9000	2.9000
870.000	2.9000	2.9000	2.9000	2.9000	2.9000
875.000	2.9000	2.9000	2.9000	2.9000	2.9000
880.000	2.9000	2.9000	2.9000	2.9000	2.9000
885.000	2.9000	2.9000	2.9000	2.9000	2.9000
890.000	2.9000	2.9000	2.9000	2.9000	2.9000
895.000	2.9000	2.9000	2.9000	2.9000	2.9000
900.000	2.9000	2.9000	2.9000	2.9000	2.9000
905.000	2.9000	2.9200	2.9400	2.9600	2.9800
910.000	3.0000	3.0000	3.0000	3.0000	3.0000
915.000	3.0000	3.0000	3.0000	3.0000	3.0000
920.000	3.0000	3.0000	3.0000	3.0000	3.0000
925.000	3.0000	3.0000	3.0000	3.0000	3.0000
930.000	3.0000	3.0000	3.0000	3.0000	3.0000
935.000	3.0000	3.0000	3.0000	3.0000	3.0000
940.000	3.0000	3.0000	3.0000	3.0000	3.0000
945.000	3.0000	3.0000	3.0000	3.0000	3.0000
950.000	3.0000	3.0000	3.0000	3.0000	3.0000
955.000	3.0000	3.0000	3.0000	3.0000	3.0000
960.000	3.0000	3.0200	3.0400	3.0600	3.0800
965.000	3.1000	3.1000	3.1000	3.1000	3.1000
970.000	3.1000	3.1000	3.1000	3.1000	3.1000
975.000	3.1000	3.1000	3.1000	3.1000	3.1000
980.000	3.1000	3.1000	3.1000	3.1000	3.1000
985.000	3.1000	3.1000	3.1000	3.1000	3.1000
990.000	3.1000	3.1000	3.1000	3.1000	3.1000
995.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,000.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,005.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,010.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,015.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,020.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,025.000	3.1000	3.1000	3.1000	3.1000	3.1000
1,030.000	3.1000	3.1000	3.1000	3.1000	3.1000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	3.1000	3.1200	3.1400	3.1600	3.1800
1,040.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,045.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,050.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,055.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,060.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,065.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,070.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,075.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,080.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,085.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,090.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,095.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,100.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,105.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,110.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,115.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,120.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,125.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,130.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,135.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,140.000	3.2000	3.2000	3.2000	3.2000	3.2000
1,145.000	3.2000	3.2200	3.2400	3.2600	3.2800
1,150.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,155.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,160.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,165.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,170.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,175.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,180.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,185.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,190.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,195.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,200.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,205.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,210.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,215.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,220.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,225.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,230.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,235.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,240.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,245.000	3.3000	3.3000	3.3000	3.3000	3.3000

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,255.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,260.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,265.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,270.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,275.000	3.3000	3.3000	3.3000	3.3000	3.3000
1,280.000	3.3000	3.3200	3.3400	3.3600	3.3800
1,285.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,290.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,295.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,300.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,305.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,310.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,315.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,320.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,325.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,330.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,335.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,340.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,345.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,350.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,355.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,360.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,365.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,370.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,375.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,380.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,385.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,390.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,395.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,400.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,405.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,410.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,415.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,420.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,425.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,430.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,435.000	3.4000	3.4000	3.4000	3.4000	3.4000
1,440.000	3.4000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time-Depth Curve: 25 Yr Storm

Label	25 Yr Storm
Start Time	0.000 min
Increment	1.000 min
End Time	1,440.000 min
Return Event	25 years

CUMULATIVE RAINFALL (in)

Output Time Increment = 1.000 min

Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0000	0.0011	0.0021	0.0032	0.0042
5.000	0.0053	0.0064	0.0074	0.0085	0.0095
10.000	0.0106	0.0117	0.0127	0.0138	0.0149
15.000	0.0160	0.0170	0.0181	0.0192	0.0203
20.000	0.0213	0.0224	0.0235	0.0246	0.0257
25.000	0.0267	0.0278	0.0289	0.0300	0.0311
30.000	0.0322	0.0333	0.0344	0.0355	0.0366
35.000	0.0377	0.0388	0.0399	0.0410	0.0421
40.000	0.0432	0.0443	0.0454	0.0465	0.0476
45.000	0.0487	0.0498	0.0509	0.0520	0.0531
50.000	0.0543	0.0554	0.0565	0.0576	0.0587
55.000	0.0599	0.0610	0.0621	0.0633	0.0644
60.000	0.0655	0.0666	0.0678	0.0689	0.0701
65.000	0.0712	0.0723	0.0735	0.0746	0.0758
70.000	0.0769	0.0781	0.0792	0.0804	0.0815
75.000	0.0827	0.0838	0.0850	0.0861	0.0873
80.000	0.0884	0.0896	0.0908	0.0919	0.0931
85.000	0.0943	0.0954	0.0966	0.0978	0.0989
90.000	0.1001	0.1013	0.1025	0.1037	0.1048
95.000	0.1060	0.1072	0.1084	0.1096	0.1108
100.000	0.1120	0.1132	0.1143	0.1155	0.1167
105.000	0.1179	0.1191	0.1203	0.1215	0.1228
110.000	0.1240	0.1252	0.1264	0.1276	0.1288
115.000	0.1300	0.1312	0.1325	0.1337	0.1349
120.000	0.1361	0.1373	0.1386	0.1398	0.1410
125.000	0.1423	0.1435	0.1447	0.1460	0.1472
130.000	0.1484	0.1497	0.1509	0.1522	0.1534
135.000	0.1547	0.1559	0.1572	0.1584	0.1597
140.000	0.1609	0.1622	0.1635	0.1647	0.1660
145.000	0.1673	0.1685	0.1698	0.1711	0.1723
150.000	0.1736	0.1749	0.1762	0.1775	0.1787
155.000	0.1800	0.1813	0.1826	0.1839	0.1852
160.000	0.1865	0.1878	0.1891	0.1904	0.1917
165.000	0.1930	0.1943	0.1956	0.1969	0.1982
170.000	0.1995	0.2008	0.2022	0.2035	0.2048

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
175.000	0.2061	0.2075	0.2088	0.2101	0.2114
180.000	0.2128	0.2141	0.2155	0.2168	0.2181
185.000	0.2195	0.2208	0.2222	0.2235	0.2249
190.000	0.2262	0.2276	0.2289	0.2303	0.2317
195.000	0.2330	0.2344	0.2358	0.2371	0.2385
200.000	0.2399	0.2413	0.2427	0.2440	0.2454
205.000	0.2468	0.2482	0.2496	0.2510	0.2524
210.000	0.2538	0.2552	0.2566	0.2580	0.2594
215.000	0.2608	0.2622	0.2636	0.2650	0.2665
220.000	0.2679	0.2693	0.2707	0.2722	0.2736
225.000	0.2750	0.2765	0.2779	0.2793	0.2808
230.000	0.2822	0.2837	0.2851	0.2866	0.2880
235.000	0.2895	0.2909	0.2924	0.2939	0.2953
240.000	0.2968	0.2983	0.2997	0.3012	0.3027
245.000	0.3042	0.3057	0.3071	0.3086	0.3101
250.000	0.3116	0.3131	0.3146	0.3161	0.3176
255.000	0.3191	0.3207	0.3222	0.3237	0.3252
260.000	0.3267	0.3282	0.3298	0.3313	0.3328
265.000	0.3344	0.3359	0.3375	0.3390	0.3405
270.000	0.3421	0.3436	0.3452	0.3468	0.3483
275.000	0.3499	0.3515	0.3530	0.3546	0.3562
280.000	0.3578	0.3593	0.3609	0.3625	0.3641
285.000	0.3657	0.3673	0.3689	0.3705	0.3721
290.000	0.3737	0.3753	0.3769	0.3785	0.3802
295.000	0.3818	0.3834	0.3850	0.3867	0.3883
300.000	0.3900	0.3916	0.3932	0.3949	0.3965
305.000	0.3982	0.3999	0.4015	0.4032	0.4049
310.000	0.4065	0.4082	0.4099	0.4116	0.4133
315.000	0.4150	0.4166	0.4183	0.4200	0.4217
320.000	0.4235	0.4252	0.4269	0.4286	0.4303
325.000	0.4320	0.4338	0.4355	0.4372	0.4390
330.000	0.4407	0.4425	0.4442	0.4460	0.4477
335.000	0.4495	0.4513	0.4530	0.4548	0.4566
340.000	0.4584	0.4601	0.4619	0.4637	0.4655
345.000	0.4673	0.4691	0.4709	0.4727	0.4746
350.000	0.4764	0.4782	0.4800	0.4819	0.4837
355.000	0.4855	0.4874	0.4892	0.4911	0.4929
360.000	0.4948	0.4970	0.4992	0.5015	0.5037
365.000	0.5059	0.5082	0.5104	0.5127	0.5149
370.000	0.5172	0.5195	0.5217	0.5240	0.5263
375.000	0.5286	0.5309	0.5332	0.5355	0.5378
380.000	0.5401	0.5424	0.5448	0.5471	0.5494
385.000	0.5518	0.5541	0.5565	0.5588	0.5612

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
390.000	0.5636	0.5660	0.5683	0.5707	0.5731
395.000	0.5755	0.5779	0.5803	0.5828	0.5852
400.000	0.5876	0.5900	0.5925	0.5949	0.5974
405.000	0.5998	0.6023	0.6048	0.6073	0.6097
410.000	0.6122	0.6147	0.6172	0.6197	0.6223
415.000	0.6248	0.6273	0.6299	0.6324	0.6349
420.000	0.6375	0.6401	0.6426	0.6452	0.6478
425.000	0.6504	0.6530	0.6556	0.6582	0.6608
430.000	0.6634	0.6661	0.6687	0.6714	0.6740
435.000	0.6767	0.6793	0.6820	0.6847	0.6874
440.000	0.6901	0.6928	0.6955	0.6982	0.7010
445.000	0.7037	0.7065	0.7092	0.7120	0.7147
450.000	0.7175	0.7203	0.7231	0.7259	0.7287
455.000	0.7315	0.7344	0.7372	0.7400	0.7429
460.000	0.7457	0.7486	0.7515	0.7544	0.7573
465.000	0.7602	0.7631	0.7660	0.7690	0.7719
470.000	0.7748	0.7778	0.7808	0.7838	0.7867
475.000	0.7897	0.7928	0.7958	0.7988	0.8018
480.000	0.8049	0.8080	0.8110	0.8141	0.8172
485.000	0.8203	0.8234	0.8265	0.8297	0.8328
490.000	0.8359	0.8391	0.8423	0.8455	0.8487
495.000	0.8519	0.8551	0.8583	0.8616	0.8648
500.000	0.8681	0.8713	0.8746	0.8779	0.8812
505.000	0.8846	0.8879	0.8913	0.8946	0.8980
510.000	0.9014	0.9048	0.9082	0.9116	0.9150
515.000	0.9185	0.9220	0.9254	0.9289	0.9324
520.000	0.9359	0.9395	0.9430	0.9466	0.9502
525.000	0.9537	0.9573	0.9610	0.9646	0.9682
530.000	0.9719	0.9756	0.9793	0.9830	0.9867
535.000	0.9904	0.9942	0.9980	1.0018	1.0056
540.000	1.0094	1.0146	1.0198	1.0250	1.0302
545.000	1.0354	1.0407	1.0460	1.0513	1.0567
550.000	1.0620	1.0674	1.0728	1.0783	1.0837
555.000	1.0892	1.0947	1.1002	1.1058	1.1113
560.000	1.1169	1.1226	1.1282	1.1339	1.1396
565.000	1.1453	1.1511	1.1569	1.1627	1.1685
570.000	1.1744	1.1803	1.1862	1.1921	1.1981
575.000	1.2041	1.2102	1.2162	1.2223	1.2285
580.000	1.2346	1.2408	1.2470	1.2533	1.2596
585.000	1.2659	1.2722	1.2786	1.2851	1.2915
590.000	1.2980	1.3045	1.3111	1.3177	1.3243
595.000	1.3310	1.3377	1.3445	1.3513	1.3581
600.000	1.3650	1.3719	1.3789	1.3859	1.3929

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
605.000	1.4000	1.4071	1.4143	1.4215	1.4288
610.000	1.4361	1.4435	1.4509	1.4584	1.4659
615.000	1.4734	1.4810	1.4887	1.4964	1.5042
620.000	1.5120	1.5199	1.5278	1.5358	1.5439
625.000	1.5520	1.5602	1.5684	1.5768	1.5851
630.000	1.5936	1.6042	1.6149	1.6256	1.6365
635.000	1.6474	1.6584	1.6695	1.6807	1.6919
640.000	1.7033	1.7147	1.7263	1.7379	1.7497
645.000	1.7615	1.7734	1.7855	1.7976	1.8099
650.000	1.8223	1.8347	1.8473	1.8601	1.8729
655.000	1.8859	1.8990	1.9122	1.9256	1.9391
660.000	1.9527	1.9679	1.9831	1.9986	2.0142
665.000	2.0299	2.0459	2.0620	2.0783	2.0948
670.000	2.1115	2.1284	2.1456	2.1629	2.1805
675.000	2.1983	2.2163	2.2346	2.2532	2.2720
680.000	2.2911	2.3105	2.3302	2.3503	2.3706
685.000	2.3913	2.4124	2.4339	2.4557	2.4780
690.000	2.5007	2.5241	2.5479	2.5723	2.5972
695.000	2.6227	2.6488	2.6756	2.7031	2.7314
700.000	2.7605	2.7905	2.8214	2.8534	2.8866
705.000	2.9210	2.9678	3.0163	3.0666	3.1189
710.000	3.1736	3.2309	3.2914	3.3556	3.4314
715.000	3.5127	3.6166	3.7290	3.8535	4.0455
720.000	4.2375	4.4295	4.6215	4.8135	4.9313
725.000	5.0391	5.1237	5.2021	5.2756	5.3379
730.000	5.3967	5.4527	5.5062	5.5574	5.6068
735.000	5.6544	5.6895	5.7233	5.7559	5.7873
740.000	5.8178	5.8473	5.8760	5.9039	5.9310
745.000	5.9574	5.9832	6.0084	6.0331	6.0572
750.000	6.0808	6.1037	6.1262	6.1483	6.1699
755.000	6.1912	6.2121	6.2326	6.2528	6.2727
760.000	6.2922	6.3115	6.3305	6.3491	6.3676
765.000	6.3857	6.4037	6.4214	6.4388	6.4560
770.000	6.4731	6.4899	6.5065	6.5229	6.5391
775.000	6.5551	6.5710	6.5867	6.6022	6.6176
780.000	6.6327	6.6465	6.6601	6.6735	6.6868
785.000	6.7000	6.7130	6.7259	6.7387	6.7514
790.000	6.7639	6.7763	6.7886	6.8009	6.8130
795.000	6.8249	6.8368	6.8486	6.8603	6.8719
800.000	6.8834	6.8948	6.9061	6.9173	6.9285
805.000	6.9395	6.9505	6.9614	6.9722	6.9829
810.000	6.9935	7.0020	7.0104	7.0188	7.0270
815.000	7.0352	7.0434	7.0515	7.0595	7.0675

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
820.000	7.0754	7.0832	7.0910	7.0988	7.1065
825.000	7.1141	7.1217	7.1293	7.1367	7.1442
830.000	7.1516	7.1589	7.1662	7.1734	7.1806
835.000	7.1878	7.1949	7.2020	7.2090	7.2160
840.000	7.2229	7.2298	7.2367	7.2435	7.2502
845.000	7.2570	7.2637	7.2703	7.2770	7.2835
850.000	7.2901	7.2966	7.3031	7.3095	7.3159
855.000	7.3223	7.3286	7.3349	7.3412	7.3474
860.000	7.3537	7.3598	7.3660	7.3721	7.3782
865.000	7.3842	7.3902	7.3962	7.4022	7.4081
870.000	7.4140	7.4199	7.4258	7.4316	7.4374
875.000	7.4431	7.4489	7.4546	7.4603	7.4659
880.000	7.4716	7.4772	7.4828	7.4883	7.4939
885.000	7.4994	7.5049	7.5103	7.5158	7.5212
890.000	7.5266	7.5320	7.5373	7.5427	7.5480
895.000	7.5533	7.5585	7.5638	7.5690	7.5742
900.000	7.5794	7.5832	7.5870	7.5908	7.5946
905.000	7.5983	7.6021	7.6058	7.6095	7.6132
910.000	7.6169	7.6206	7.6243	7.6279	7.6315
915.000	7.6351	7.6387	7.6423	7.6459	7.6494
920.000	7.6530	7.6565	7.6600	7.6635	7.6670
925.000	7.6705	7.6739	7.6774	7.6808	7.6842
930.000	7.6876	7.6910	7.6944	7.6977	7.7011
935.000	7.7044	7.7078	7.7111	7.7144	7.7177
940.000	7.7210	7.7242	7.7275	7.7307	7.7340
945.000	7.7372	7.7404	7.7436	7.7468	7.7500
950.000	7.7531	7.7563	7.7595	7.7626	7.7657
955.000	7.7688	7.7719	7.7750	7.7781	7.7812
960.000	7.7842	7.7873	7.7903	7.7933	7.7964
965.000	7.7994	7.8024	7.8054	7.8083	7.8113
970.000	7.8143	7.8172	7.8202	7.8231	7.8261
975.000	7.8290	7.8319	7.8348	7.8377	7.8406
980.000	7.8434	7.8463	7.8492	7.8520	7.8549
985.000	7.8577	7.8605	7.8633	7.8661	7.8689
990.000	7.8717	7.8745	7.8773	7.8800	7.8828
995.000	7.8855	7.8883	7.8910	7.8937	7.8965
1,000.000	7.8992	7.9019	7.9046	7.9073	7.9099
1,005.000	7.9126	7.9153	7.9179	7.9206	7.9232
1,010.000	7.9259	7.9285	7.9311	7.9337	7.9363
1,015.000	7.9389	7.9415	7.9441	7.9467	7.9493
1,020.000	7.9518	7.9544	7.9570	7.9595	7.9620
1,025.000	7.9646	7.9671	7.9696	7.9721	7.9746
1,030.000	7.9771	7.9796	7.9821	7.9846	7.9871

Subsection: Time-Depth Curve
 Label: COA - Type III 24hr
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,035.000	7.9896	7.9920	7.9945	7.9969	7.9994
1,040.000	8.0018	8.0042	8.0067	8.0091	8.0115
1,045.000	8.0139	8.0163	8.0187	8.0211	8.0235
1,050.000	8.0259	8.0282	8.0306	8.0330	8.0353
1,055.000	8.0377	8.0400	8.0424	8.0447	8.0470
1,060.000	8.0494	8.0517	8.0540	8.0563	8.0586
1,065.000	8.0609	8.0632	8.0655	8.0678	8.0700
1,070.000	8.0723	8.0746	8.0768	8.0791	8.0813
1,075.000	8.0836	8.0858	8.0881	8.0903	8.0925
1,080.000	8.0948	8.0966	8.0985	8.1003	8.1022
1,085.000	8.1040	8.1059	8.1077	8.1095	8.1114
1,090.000	8.1132	8.1150	8.1168	8.1187	8.1205
1,095.000	8.1223	8.1241	8.1259	8.1277	8.1295
1,100.000	8.1312	8.1330	8.1348	8.1366	8.1383
1,105.000	8.1401	8.1419	8.1436	8.1454	8.1471
1,110.000	8.1489	8.1506	8.1524	8.1541	8.1558
1,115.000	8.1576	8.1593	8.1610	8.1627	8.1645
1,120.000	8.1662	8.1679	8.1696	8.1713	8.1730
1,125.000	8.1747	8.1764	8.1781	8.1797	8.1814
1,130.000	8.1831	8.1848	8.1864	8.1881	8.1898
1,135.000	8.1914	8.1931	8.1947	8.1964	8.1980
1,140.000	8.1997	8.2013	8.2030	8.2046	8.2062
1,145.000	8.2079	8.2095	8.2111	8.2127	8.2144
1,150.000	8.2160	8.2176	8.2192	8.2208	8.2224
1,155.000	8.2240	8.2256	8.2272	8.2288	8.2303
1,160.000	8.2319	8.2335	8.2351	8.2367	8.2382
1,165.000	8.2398	8.2414	8.2429	8.2445	8.2460
1,170.000	8.2476	8.2491	8.2507	8.2522	8.2538
1,175.000	8.2553	8.2569	8.2584	8.2599	8.2615
1,180.000	8.2630	8.2645	8.2660	8.2675	8.2691
1,185.000	8.2706	8.2721	8.2736	8.2751	8.2766
1,190.000	8.2781	8.2796	8.2811	8.2826	8.2841
1,195.000	8.2856	8.2870	8.2885	8.2900	8.2915
1,200.000	8.2929	8.2944	8.2959	8.2974	8.2988
1,205.000	8.3003	8.3017	8.3032	8.3046	8.3061
1,210.000	8.3075	8.3090	8.3104	8.3119	8.3133
1,215.000	8.3148	8.3162	8.3176	8.3190	8.3205
1,220.000	8.3219	8.3233	8.3247	8.3262	8.3276
1,225.000	8.3290	8.3304	8.3318	8.3332	8.3346
1,230.000	8.3360	8.3374	8.3388	8.3402	8.3416
1,235.000	8.3430	8.3444	8.3458	8.3471	8.3485
1,240.000	8.3499	8.3513	8.3527	8.3540	8.3554
1,245.000	8.3568	8.3581	8.3595	8.3609	8.3622

Subsection: Time-Depth Curve
Label: COA - Type III 24hr
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

CUMULATIVE RAINFALL (in)
Output Time Increment = 1.000 min
Time on left represents time for first value in each row.

Time (min)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
1,250.000	8.3636	8.3649	8.3663	8.3676	8.3690
1,255.000	8.3703	8.3717	8.3730	8.3744	8.3757
1,260.000	8.3770	8.3784	8.3797	8.3810	8.3824
1,265.000	8.3837	8.3850	8.3863	8.3877	8.3890
1,270.000	8.3903	8.3916	8.3929	8.3942	8.3955
1,275.000	8.3968	8.3982	8.3995	8.4008	8.4021
1,280.000	8.4034	8.4047	8.4059	8.4072	8.4085
1,285.000	8.4098	8.4111	8.4124	8.4137	8.4149
1,290.000	8.4162	8.4175	8.4188	8.4201	8.4213
1,295.000	8.4226	8.4239	8.4251	8.4264	8.4277
1,300.000	8.4289	8.4302	8.4314	8.4327	8.4339
1,305.000	8.4352	8.4364	8.4377	8.4389	8.4402
1,310.000	8.4414	8.4427	8.4439	8.4451	8.4464
1,315.000	8.4476	8.4488	8.4501	8.4513	8.4525
1,320.000	8.4538	8.4550	8.4562	8.4574	8.4586
1,325.000	8.4599	8.4611	8.4623	8.4635	8.4647
1,330.000	8.4659	8.4671	8.4683	8.4695	8.4707
1,335.000	8.4719	8.4731	8.4743	8.4755	8.4767
1,340.000	8.4779	8.4791	8.4803	8.4815	8.4827
1,345.000	8.4839	8.4851	8.4862	8.4874	8.4886
1,350.000	8.4898	8.4910	8.4921	8.4933	8.4945
1,355.000	8.4956	8.4968	8.4980	8.4991	8.5003
1,360.000	8.5015	8.5026	8.5038	8.5049	8.5061
1,365.000	8.5073	8.5084	8.5096	8.5107	8.5119
1,370.000	8.5130	8.5142	8.5153	8.5164	8.5176
1,375.000	8.5187	8.5199	8.5210	8.5221	8.5233
1,380.000	8.5244	8.5255	8.5267	8.5278	8.5289
1,385.000	8.5300	8.5312	8.5323	8.5334	8.5345
1,390.000	8.5356	8.5368	8.5379	8.5390	8.5401
1,395.000	8.5412	8.5423	8.5434	8.5446	8.5457
1,400.000	8.5468	8.5479	8.5490	8.5501	8.5512
1,405.000	8.5523	8.5534	8.5545	8.5556	8.5567
1,410.000	8.5577	8.5588	8.5599	8.5610	8.5621
1,415.000	8.5632	8.5643	8.5654	8.5664	8.5675
1,420.000	8.5686	8.5697	8.5708	8.5718	8.5729
1,425.000	8.5740	8.5750	8.5761	8.5772	8.5783
1,430.000	8.5793	8.5804	8.5815	8.5825	8.5836
1,435.000	8.5846	8.5857	8.5868	8.5878	8.5889
1,440.000	8.5899	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.010 ft/ft
2 Year 24 Hour Depth	2.4000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.929 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,415.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	13.936 min
Segment #3: TR-55 Channel Flow	
Flow Area	7.0 ft ²
Hydraulic Length	370.00 ft
Manning's n	0.013
Slope	0.005 ft/ft
Wetted Perimeter	9.40 ft
Average Velocity	6.66 ft/s
Segment Time of Concentration	0.926 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	29.791 min

Subsection: Time of Concentration Calculations
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	49.459 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow

Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min

Segment #2: TR-55 Shallow Concentrated Flow

Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min

Segment #3: TR-55 Channel Flow

Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min

Time of Concentration (Composite)

Time of Concentration (Composite)	49.459 min
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Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	49.459 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	2,550.00 ft
Is Paved?	False
Slope	0.007 ft/ft
Average Velocity	1.35 ft/s
Segment Time of Concentration	31.484 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	650.00 ft
Manning's n	0.030
Slope	0.005 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	3.51 ft/s
Segment Time of Concentration	3.085 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	49.459 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
Average Velocity	1.69 ft/s
Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	940.00 ft
Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
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Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
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Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	980.00 ft
Is Paved?	False
Slope	0.011 ft/ft
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Segment Time of Concentration	9.652 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
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Manning's n	0.030
Slope	0.003 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	2.72 ft/s
Segment Time of Concentration	5.759 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	30.302 min

Subsection: Time of Concentration Calculations
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,490.00 ft
Is Paved?	False
Slope	0.006 ft/ft
Average Velocity	1.25 ft/s
Segment Time of Concentration	19.870 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,675.00 ft
Manning's n	0.030
Slope	0.007 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.16 ft/s
Segment Time of Concentration	10.729 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	45.490 min

Subsection: Time of Concentration Calculations
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,455.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	8.684 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	54.444 min

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
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Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
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Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	8.684 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	54.444 min

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
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Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
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Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	8.684 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	54.444 min

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	100.00 ft
Manning's n	0.150
Slope	0.005 ft/ft
2 Year 24 Hour Depth	4.2000 in
Average Velocity	0.11 ft/s
Segment Time of Concentration	14.891 min
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	1,890.00 ft
Is Paved?	False
Slope	0.004 ft/ft
Average Velocity	1.02 ft/s
Segment Time of Concentration	30.869 min
Segment #3: TR-55 Channel Flow	
Flow Area	50.0 ft ²
Hydraulic Length	2,455.00 ft
Manning's n	0.030
Slope	0.009 ft/ft
Wetted Perimeter	50.00 ft
Average Velocity	4.71 ft/s
Segment Time of Concentration	8.684 min
Time of Concentration (Composite)	
Time of Concentration (Composite)	54.444 min

Subsection: Time of Concentration Calculations
Label: PROP AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

==== SCS Channel Flow

Tc = $R = Qa / Wp$
 $V = (1.49 * (R^{2/3}) * (Sf^{-0.5})) / n$

Where: $(Lf / V) / 3600$
R= Hydraulic radius
Aq= Flow area, square feet
Wp= Wetted perimeter, feet
V= Velocity, ft/sec
Sf= Slope, ft/ft
n= Manning's n
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc = Unpaved surface:
 $V = 16.1345 * (Sf^{0.5})$

Paved Surface:
 $V = 20.3282 * (Sf^{0.5})$

Where: $(Lf / V) / 3600$
V= Velocity, ft/sec
Sf= Slope, ft/ft
Tc= Time of concentration, hours
Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc = $(0.007 * ((n * Lf)^{0.8})) / ((P^{0.5}) * (Sf^{0.4}))$

Where: Tc= Time of concentration, hours
n= Manning's n
Lf= Flow length, feet
P= 2yr, 24hr Rain depth, inches
Sf= Slope, %

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	23.930	80.0	0.0	94.600
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	23.930	(N/A)	(N/A)	94.600

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	23.930	80.0	0.0	94.600
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	23.930	(N/A)	(N/A)	94.600

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	23.930	80.0	0.0	94.600
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	23.930	(N/A)	(N/A)	94.600

Subsection: Runoff CN-Area
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	23.930	80.0	0.0	94.600
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	23.930	(N/A)	(N/A)	94.600

Subsection: Runoff CN-Area
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	27.910	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	27.910	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	27.910	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	27.910	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	27.910	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	27.910	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	27.910	0.1	0.0	81.009
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	27.910	(N/A)	(N/A)	81.009

Subsection: Runoff CN-Area
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	105.250	0.1	0.0	81.017
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	105.250	(N/A)	(N/A)	81.017

Subsection: Runoff CN-Area
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	80.560	30.0	0.0	86.100
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	80.560	(N/A)	(N/A)	86.100

Subsection: Runoff CN-Area
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	80.560	30.0	0.0	86.100
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	80.560	(N/A)	(N/A)	86.100

Subsection: Runoff CN-Area
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	80.560	30.0	0.0	86.100
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	80.560	(N/A)	(N/A)	86.100

Subsection: Runoff CN-Area
Label: PROP AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Runoff Curve Number Data

Soil/Surface Description	CN	Area (acres)	C (%)	UC (%)	Adjusted CN
Row crops - Cont & terraced(C&T), good - Soil D	81.000	80.560	30.0	0.0	86.100
COMPOSITE AREA & WEIGHTED CN --->	(N/A)	80.560	(N/A)	(N/A)	86.100

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method (Computational Notes)

Definition of Terms

At	Total area (acres): $A_t = A_i + A_p$
Ai	Impervious area (acres)
Ap	Pervious area (acres)
CNi	Runoff curve number for impervious area
CNp	Runoff curve number for pervious area
fLoss	f loss constant infiltration (depth/time)
gKs	Saturated Hydraulic Conductivity (depth/time)
Md	Volumetric Moisture Deficit
Psi	Capillary Suction (length)
hK	Horton Infiltration Decay Rate (time^{-1})
fo	Initial Infiltration Rate (depth/time)
fc	Ultimate(capacity)Infiltration Rate (depth/time)
Ia	Initial Abstraction (length)
dt	Computational increment (duration of unit excess rainfall) Default dt is smallest value of $0.1333T_c$, r_{tm} , and t_h (Smallest dt is then adjusted to match up with T_p)
UDdt	User specified override computational main time increment (only used if UDdt is $\Rightarrow .1333T_c$)
D(t)	Point on distribution curve (fraction of P) for time step t
K	$2 / (1 + (T_r/T_p))$: default $K = 0.75$: (for $T_r/T_p = 1.67$)
Ks	Hydrograph shape factor = Unit Conversions * K : $= ((1\text{hr}/3600\text{sec}) * (1\text{ft}/12\text{in}) * ((5280\text{ft})^2/\text{sq.mi})) * K$ Default $K_s = 645.333 * 0.75 = 484$
Lag	Lag time from center of excess runoff (dt) to T_p : $\text{Lag} = 0.6T_c$
P	Total precipitation depth, inches
Pa(t)	Accumulated rainfall at time step t
Pi(t)	Incremental rainfall at time step t
qp	Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi. $= (K_s * A * Q) / T_p$ (where $Q = 1\text{in. runoff}$, $A = \text{sq.mi.}$)
Qu(t)	Unit hydrograph ordinate (cfs) at time step t
Q(t)	Final hydrograph ordinate (cfs) at time step t
Rai(t)	Accumulated runoff (inches) at time step t for impervious area
Rap(t)	Accumulated runoff (inches) at time step t for pervious area
Rii(t)	Incremental runoff (inches) at time step t for impervious area
Rip(t)	Incremental runoff (inches) at time step t for pervious area
R(t)	Incremental weighted total runoff (inches)
Rtm	Time increment for rainfall table
Si	S for impervious area: $S_i = (1000/CN_i) - 10$
Sp	S for pervious area: $S_p = (1000/CN_p) - 10$
t	Time step (row) number
Tc	Time of concentration
Tb	Time (hrs) of entire unit hydrograph: $T_b = T_p + T_r$
Tp	Time (hrs) to peak of a unit hydrograph: $T_p = (dt/2) + \text{Lag}$
Tr	Time (hrs) of receding limb of unit hydrograph: $T_r = \text{ratio of } T_p$

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method

Computational Notes

Precipitation

Column (1)	Time for time step t
Column (2)	$D(t)$ = Point on distribution curve for time step t
Column (3)	$P_i(t) = P_a(t) - P_a(t-1)$: Col.(4) - Preceding Col.(4)
Column (4)	$P_a(t) = D(t) \times P$: Col.(2) x P

Pervious Area Runoff (using SCS Runoff CN Method)

Column (5)	$Rap(t)$ = Accumulated pervious runoff for time step t If $(P_a(t) \leq 0.2Sp)$ then use: $Rap(t) = 0.0$ If $(P_a(t) > 0.2Sp)$ then use: $Rap(t) = (Col.(4) - 0.2Sp) \times 2 / (Col.(4) + 0.8Sp)$
Column (6)	$Rip(t)$ = Incremental pervious runoff for time step t $Rip(t) = Rap(t) - Rap(t-1)$ $Rip(t) = Col.(5)$ for current row - $Col.(5)$ for preceding row.

Impervious Area Runoff

Column (7 & 8)...	Did not specify to use impervious areas.
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Incremental Weighted Runoff

Column (9)	$R(t) = (A_p/A_t) \times Rip(t) + (A_i/A_t) \times Rii(t)$ $R(t) = (A_p/A_t) \times Col.(6) + (A_i/A_t) \times Col.(8)$
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SCS Unit Hydrograph Method

Column (10)	$Q(t)$ is computed with the SCS unit hydrograph method using $R(t)$ and $Qu(t)$.
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Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	742.793 min
Flow (Peak, Computed)	33.95 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	33.87 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	186,207.393 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	186,197.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
513.000	0.00	0.00	0.01	0.01	0.02
528.000	0.03	0.03	0.03	0.03	0.02
543.000	0.02	0.01	0.01	0.01	0.01
558.000	0.01	0.01	0.04	0.10	0.19
573.000	0.30	0.40	0.45	0.45	0.43
588.000	0.38	0.31	0.25	0.23	0.28
603.000	0.41	0.62	0.82	0.98	1.01
618.000	0.97	0.88	0.75	0.59	0.48
633.000	0.46	0.55	0.76	1.06	1.32
648.000	1.49	1.50	1.41	1.33	1.29
663.000	1.34	1.60	1.87	2.08	2.13
678.000	2.15	2.20	2.33	2.55	2.79
693.000	3.02	3.25	3.51	3.95	4.42
708.000	5.01	5.82	7.34	9.40	12.07
723.000	15.42	19.66	24.03	28.11	31.55
738.000	33.30	33.87	33.57	32.55	30.73
753.000	28.37	25.71	22.92	20.27	18.10
768.000	16.23	14.60	13.26	12.31	11.71
783.000	11.35	10.76	9.81	8.72	7.59
798.000	6.64	6.01	5.86	6.14	6.64
813.000	6.74	6.51	5.98	5.23	4.38
828.000	3.81	3.65	4.03	4.94	5.66
843.000	6.06	5.99	5.48	4.79	3.96
858.000	3.04	2.51	2.44	2.80	3.58
873.000	4.64	5.39	5.75	5.66	5.16
888.000	4.44	3.62	2.77	2.14	1.67
903.000	1.33	1.15	1.37	2.01	3.00
918.000	4.20	5.15	5.50	5.45	5.06
933.000	4.40	3.54	2.75	2.10	1.63
948.000	1.30	1.02	0.78	0.61	0.57
963.000	0.82	1.40	2.37	3.68	4.75
978.000	5.42	5.54	5.17	4.57	3.80
993.000	2.92	2.26	1.75	1.36	1.07
1,008.000	0.83	0.65	0.51	0.39	0.31

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,023.000	0.24	0.19	0.15	0.12	0.20
1,038.000	0.51	1.14	2.22	3.52	4.65
1,053.000	5.40	5.48	5.16	4.59	3.83
1,068.000	2.97	2.30	1.77	1.38	1.09
1,083.000	0.85	0.67	0.52	0.40	0.32
1,098.000	0.25	0.19	0.15	0.12	0.09
1,113.000	0.07	0.06	0.05	0.04	0.03
1,128.000	0.02	0.01	0.00	0.00	0.00
1,143.000	0.00	0.17	0.58	1.30	2.56
1,158.000	3.85	4.91	5.52	5.47	5.09
1,173.000	4.46	3.63	2.82	2.18	1.69
1,188.000	1.32	1.04	0.81	0.63	0.49
1,203.000	0.38	0.30	0.24	0.18	0.14
1,218.000	0.11	0.09	0.07	0.05	0.04
1,233.000	0.03	0.03	0.02	0.01	0.00
1,248.000	0.00	0.00	0.00	0.00	0.00
1,263.000	0.00	0.00	0.00	0.00	0.00
1,278.000	0.00	0.17	0.58	1.31	2.58
1,293.000	3.88	4.96	5.58	5.52	5.14
1,308.000	4.50	3.67	2.85	2.20	1.70
1,323.000	1.33	1.05	0.82	0.64	0.50
1,338.000	0.39	0.31	0.24	0.19	0.15
1,353.000	0.11	0.09	0.07	0.06	0.04
1,368.000	0.03	0.03	0.02	0.01	0.00
1,383.000	0.00	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	107.95 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	107.11 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	512,494.718 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	510,356.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.00	0.01	0.01
423.000	0.02	0.03	0.04	0.05	0.07
438.000	0.08	0.10	0.11	0.13	0.15
453.000	0.17	0.19	0.21	0.23	0.25
468.000	0.27	0.29	0.31	0.33	0.36
483.000	0.38	0.40	0.43	0.45	0.48
498.000	0.50	0.53	0.55	0.58	0.61
513.000	0.64	0.67	0.70	0.73	0.76
528.000	0.79	0.83	0.86	0.89	0.93
543.000	0.97	1.02	1.08	1.14	1.23
558.000	1.31	1.41	1.50	1.59	1.68
573.000	1.76	1.85	1.93	2.02	2.10
588.000	2.19	2.28	2.37	2.46	2.55
603.000	2.65	2.75	2.86	2.97	3.08
618.000	3.20	3.32	3.45	3.58	3.72
633.000	3.88	4.06	4.27	4.53	4.81
648.000	5.12	5.45	5.79	6.14	6.49
663.000	6.85	7.25	7.69	8.18	8.72
678.000	9.32	9.95	10.62	11.32	12.09
693.000	12.93	13.86	14.91	16.20	17.73
708.000	19.58	21.83	25.11	29.85	36.65
723.000	46.11	59.68	74.72	89.13	101.07
738.000	106.47	107.11	103.87	97.32	88.00
753.000	78.74	70.08	62.31	55.71	49.99
768.000	45.00	40.69	37.09	34.03	31.38
783.000	29.08	27.12	25.39	23.80	22.34
798.000	21.04	19.88	18.83	17.88	17.04
813.000	16.28	15.56	14.88	14.21	13.56
828.000	12.93	12.34	11.79	11.31	10.89
843.000	10.52	10.19	9.90	9.64	9.40
858.000	9.18	8.98	8.79	8.61	8.45
873.000	8.29	8.15	8.01	7.88	7.75
888.000	7.63	7.51	7.40	7.29	7.18
903.000	7.07	6.93	6.77	6.57	6.34

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	6.11	5.87	5.66	5.46	5.29
933.000	5.14	5.02	4.92	4.82	4.74
948.000	4.66	4.59	4.53	4.46	4.41
963.000	4.35	4.30	4.25	4.20	4.16
978.000	4.12	4.07	4.03	3.99	3.96
993.000	3.92	3.88	3.84	3.81	3.77
1,008.000	3.74	3.70	3.67	3.64	3.61
1,023.000	3.58	3.55	3.52	3.49	3.47
1,038.000	3.44	3.41	3.38	3.36	3.33
1,053.000	3.30	3.28	3.26	3.23	3.21
1,068.000	3.19	3.16	3.14	3.12	3.10
1,083.000	3.07	3.04	3.00	2.96	2.90
1,098.000	2.85	2.79	2.74	2.69	2.65
1,113.000	2.61	2.58	2.55	2.53	2.50
1,128.000	2.48	2.46	2.44	2.43	2.41
1,143.000	2.40	2.38	2.37	2.35	2.34
1,158.000	2.32	2.31	2.29	2.28	2.27
1,173.000	2.26	2.24	2.23	2.22	2.20
1,188.000	2.19	2.18	2.17	2.16	2.15
1,203.000	2.13	2.12	2.11	2.10	2.09
1,218.000	2.08	2.07	2.06	2.05	2.04
1,233.000	2.03	2.02	2.01	2.00	1.99
1,248.000	1.98	1.97	1.96	1.95	1.94
1,263.000	1.93	1.93	1.92	1.91	1.90
1,278.000	1.89	1.88	1.87	1.86	1.86
1,293.000	1.85	1.84	1.83	1.82	1.82
1,308.000	1.81	1.80	1.80	1.79	1.78
1,323.000	1.77	1.76	1.76	1.75	1.74
1,338.000	1.73	1.73	1.72	1.71	1.71
1,353.000	1.70	1.69	1.69	1.68	1.67
1,368.000	1.67	1.66	1.65	1.65	1.64
1,383.000	1.64	1.63	1.62	1.62	1.61
1,398.000	1.60	1.60	1.59	1.59	1.58
1,413.000	1.58	1.57	1.57	1.56	1.55
1,428.000	1.55	1.54	1.54	1.53	1.52

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	143.23 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	141.73 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	720,692.010 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	717,945.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
351.000	0.00	0.00	0.00	0.01	0.01
366.000	0.02	0.03	0.04	0.05	0.07
381.000	0.09	0.11	0.13	0.15	0.17
396.000	0.20	0.22	0.25	0.27	0.30
411.000	0.32	0.35	0.38	0.41	0.43
426.000	0.46	0.49	0.52	0.55	0.58
441.000	0.61	0.64	0.67	0.70	0.73
456.000	0.77	0.80	0.84	0.87	0.91
471.000	0.94	0.98	1.01	1.05	1.09
486.000	1.13	1.17	1.21	1.25	1.29
501.000	1.34	1.38	1.42	1.47	1.52
516.000	1.56	1.61	1.66	1.71	1.77
531.000	1.82	1.87	1.93	1.99	2.06
546.000	2.14	2.25	2.38	2.55	2.72
561.000	2.90	3.08	3.26	3.42	3.58
576.000	3.73	3.88	4.03	4.17	4.32
591.000	4.46	4.61	4.77	4.92	5.08
606.000	5.24	5.41	5.59	5.77	5.96
621.000	6.15	6.35	6.57	6.79	7.05
636.000	7.35	7.71	8.16	8.67	9.22
651.000	9.81	10.40	11.00	11.59	12.19
666.000	12.83	13.52	14.27	15.09	15.99
681.000	16.93	17.92	18.96	20.07	21.26
696.000	22.55	23.95	25.58	27.49	29.80
711.000	32.64	36.88	43.04	51.93	64.30
726.000	81.94	101.38	119.87	134.98	141.46
741.000	141.73	137.01	128.02	115.52	103.23
756.000	91.82	81.70	73.22	65.97	59.71
771.000	54.35	49.90	46.11	42.82	39.94
786.000	37.46	35.26	33.24	31.39	29.72
801.000	28.23	26.87	25.64	24.53	23.52
816.000	22.54	21.59	20.62	19.65	18.71
831.000	17.81	16.98	16.25	15.61	15.05
846.000	14.56	14.14	13.76	13.41	13.08

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
861.000	12.80	12.53	12.28	12.04	11.82
876.000	11.61	11.42	11.23	11.05	10.88
891.000	10.72	10.56	10.41	10.26	10.09
906.000	9.89	9.64	9.33	8.98	8.60
921.000	8.24	7.90	7.59	7.32	7.10
936.000	6.92	6.76	6.61	6.49	6.38
951.000	6.28	6.18	6.09	6.01	5.94
966.000	5.87	5.80	5.73	5.67	5.61
981.000	5.55	5.49	5.44	5.39	5.33
996.000	5.28	5.24	5.19	5.14	5.10
1,011.000	5.05	5.01	4.96	4.92	4.88
1,026.000	4.84	4.80	4.76	4.72	4.68
1,041.000	4.65	4.61	4.58	4.54	4.51
1,056.000	4.47	4.44	4.41	4.37	4.34
1,071.000	4.31	4.28	4.25	4.22	4.18
1,086.000	4.13	4.08	4.00	3.91	3.81
1,101.000	3.72	3.63	3.55	3.48	3.42
1,116.000	3.37	3.33	3.29	3.26	3.22
1,131.000	3.20	3.17	3.14	3.12	3.10
1,146.000	3.08	3.06	3.04	3.02	3.00
1,161.000	2.98	2.96	2.95	2.93	2.91
1,176.000	2.90	2.88	2.86	2.85	2.83
1,191.000	2.82	2.80	2.79	2.77	2.76
1,206.000	2.74	2.73	2.71	2.70	2.68
1,221.000	2.67	2.66	2.65	2.63	2.62
1,236.000	2.60	2.59	2.58	2.56	2.55
1,251.000	2.54	2.53	2.52	2.50	2.49
1,266.000	2.48	2.47	2.46	2.45	2.43
1,281.000	2.42	2.41	2.40	2.39	2.38
1,296.000	2.37	2.36	2.35	2.34	2.33
1,311.000	2.32	2.31	2.30	2.29	2.28
1,326.000	2.27	2.26	2.25	2.24	2.23
1,341.000	2.22	2.21	2.20	2.20	2.19
1,356.000	2.18	2.17	2.16	2.15	2.15
1,371.000	2.14	2.13	2.12	2.11	2.10
1,386.000	2.10	2.09	2.08	2.07	2.06
1,401.000	2.06	2.05	2.04	2.03	2.03
1,416.000	2.02	2.01	2.00	2.00	1.99
1,431.000	1.98	1.97	1.97	1.95	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres
Computational Time Increment	3.972 min
Time to Peak (Computed)	738.820 min
Flow (Peak, Computed)	202.08 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	738.000 min
Flow (Peak Interpolated Output)	199.85 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	31.510 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	1,121,535.165 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,117,770.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	29.791 min
Computational Time Increment	3.972 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	71.90 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	19.861 min
Unit receding limb, Tr	79.443 min
Total unit time, Tb	99.304 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	29.791 min
Area (User Defined)	31.510 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.01	0.01	0.02
297.000	0.03	0.04	0.06	0.08	0.09
312.000	0.11	0.13	0.15	0.18	0.20
327.000	0.22	0.24	0.27	0.29	0.31
342.000	0.34	0.36	0.39	0.41	0.44
357.000	0.47	0.49	0.52	0.55	0.59
372.000	0.64	0.69	0.74	0.80	0.86
387.000	0.92	0.97	1.03	1.08	1.13
402.000	1.18	1.23	1.29	1.34	1.39
417.000	1.44	1.49	1.55	1.60	1.65
432.000	1.71	1.76	1.82	1.87	1.93
447.000	1.99	2.04	2.10	2.16	2.22
462.000	2.29	2.35	2.41	2.48	2.54
477.000	2.61	2.68	2.74	2.81	2.89
492.000	2.96	3.03	3.11	3.18	3.26
507.000	3.34	3.42	3.50	3.59	3.67
522.000	3.76	3.85	3.94	4.03	4.13
537.000	4.23	4.33	4.46	4.64	4.87
552.000	5.19	5.58	6.01	6.44	6.88
567.000	7.28	7.65	8.00	8.32	8.62
582.000	8.91	9.20	9.48	9.76	10.04
597.000	10.33	10.62	10.90	11.20	11.50
612.000	11.81	12.14	12.48	12.84	13.20
627.000	13.58	13.99	14.45	15.00	15.71
642.000	16.61	17.63	18.74	19.91	21.08
657.000	22.22	23.34	24.44	25.57	26.77
672.000	28.06	29.46	30.95	32.50	34.12
687.000	35.80	37.55	39.33	41.14	42.97
702.000	44.86	47.01	49.65	53.02	58.41
717.000	66.58	78.63	95.56	119.77	146.35
732.000	171.45	191.70	199.85	199.44	192.14
747.000	179.02	161.20	143.89	128.00	114.11
762.000	102.83	93.44	85.49	78.82	73.35
777.000	68.70	64.60	60.96	57.76	54.86

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA D
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
792.000	52.16	49.65	47.35	45.28	43.37
807.000	41.62	40.04	38.56	37.09	35.60
822.000	34.05	32.42	30.82	29.28	27.85
837.000	26.60	25.51	24.56	23.74	23.04
852.000	22.40	21.83	21.30	20.84	20.41
867.000	20.00	19.63	19.28	18.95	18.64
882.000	18.34	18.06	17.79	17.53	17.28
897.000	17.05	16.80	16.52	16.18	15.72
912.000	15.13	14.46	13.74	13.02	12.37
927.000	11.78	11.27	10.86	10.52	10.23
942.000	9.98	9.76	9.57	9.40	9.24
957.000	9.10	8.98	8.86	8.74	8.64
972.000	8.54	8.44	8.35	8.26	8.17
987.000	8.09	8.01	7.93	7.86	7.79
1,002.000	7.72	7.65	7.58	7.51	7.44
1,017.000	7.38	7.31	7.25	7.19	7.13
1,032.000	7.07	7.01	6.96	6.90	6.85
1,047.000	6.80	6.75	6.69	6.64	6.59
1,062.000	6.54	6.50	6.45	6.40	6.36
1,077.000	6.32	6.27	6.22	6.14	6.03
1,092.000	5.89	5.71	5.53	5.35	5.18
1,107.000	5.04	4.91	4.81	4.72	4.65
1,122.000	4.59	4.53	4.48	4.44	4.40
1,137.000	4.36	4.32	4.29	4.26	4.23
1,152.000	4.20	4.17	4.15	4.12	4.09
1,167.000	4.07	4.05	4.02	4.00	3.98
1,182.000	3.95	3.93	3.91	3.89	3.86
1,197.000	3.84	3.82	3.80	3.78	3.76
1,212.000	3.74	3.72	3.70	3.68	3.66
1,227.000	3.64	3.62	3.61	3.59	3.57
1,242.000	3.55	3.53	3.52	3.50	3.48
1,257.000	3.47	3.45	3.43	3.42	3.40
1,272.000	3.38	3.37	3.35	3.34	3.32
1,287.000	3.31	3.29	3.28	3.26	3.25
1,302.000	3.23	3.22	3.20	3.19	3.17
1,317.000	3.16	3.15	3.13	3.12	3.11
1,332.000	3.09	3.08	3.07	3.06	3.04
1,347.000	3.03	3.02	3.01	2.99	2.98
1,362.000	2.97	2.96	2.94	2.93	2.92
1,377.000	2.91	2.90	2.89	2.88	2.86
1,392.000	2.85	2.84	2.83	2.82	2.81
1,407.000	2.80	2.79	2.78	2.77	2.76
1,422.000	2.75	2.74	2.73	2.72	2.71

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,437.000	2.70	2.68	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres
Computational Time Increment	2.000 min
Time to Peak (Computed)	730.000 min
Flow (Peak, Computed)	57.03 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	729.000 min
Flow (Peak Interpolated Output)	56.20 ft ³ /s
Drainage Area	
SCS CN (Composite)	95.000
Area (User Defined)	23.930 acres
Maximum Retention (Pervious)	0.5263 in
Maximum Retention (Pervious, 20 percent)	0.1053 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.8409 in
Runoff Volume (Pervious)	246,778.755 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	246,762.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	15.000 min
Computational Time Increment	2.000 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	108.45 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	10.000 min
Unit receding limb, Tr	40.000 min
Total unit time, Tb	50.000 min

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
240.000	0.00	0.09	0.45	1.11	1.51
255.000	1.36	0.96	0.58	0.35	0.22
270.000	0.13	0.08	0.05	0.03	0.02
285.000	0.01	0.01	0.00	0.00	0.00
300.000	0.00	0.00	0.00	0.00	0.00
315.000	0.00	0.00	0.00	0.00	0.00
330.000	0.00	0.00	0.00	0.00	0.00
345.000	0.00	0.00	0.00	0.00	0.00
360.000	0.00	0.38	1.57	3.23	3.97
375.000	3.39	2.29	1.40	0.84	0.53
390.000	0.32	0.20	0.12	0.07	0.05
405.000	0.03	0.02	0.00	0.00	0.00
420.000	0.00	0.00	0.00	0.00	0.00
435.000	0.00	0.00	0.00	0.00	0.00
450.000	0.00	0.56	2.28	4.57	5.54
465.000	4.68	3.15	1.91	1.16	0.72
480.000	0.44	0.27	0.16	0.10	0.06
495.000	0.04	0.02	0.01	0.00	0.00
510.000	0.00	0.68	2.75	5.47	6.59
525.000	5.55	3.72	2.27	1.37	0.85
540.000	0.51	0.32	0.19	0.12	0.07
555.000	0.05	0.03	0.16	1.23	4.20
570.000	6.90	7.06	5.55	3.53	2.10
585.000	1.31	0.79	0.49	0.30	0.68
600.000	2.52	5.85	7.92	7.13	5.07
615.000	3.08	1.85	1.16	0.70	0.43
630.000	0.26	1.04	3.61	6.98	8.33
645.000	6.98	4.65	2.83	1.72	1.61
660.000	3.29	6.72	8.86	7.91	5.62
675.000	3.51	2.99	5.28	8.49	9.20
690.000	7.66	6.01	6.83	9.43	10.57
705.000	11.14	13.19	16.96	21.96	28.75
720.000	36.16	43.58	51.19	56.20	55.50
735.000	49.16	41.72	36.30	32.96	30.41

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
750.000	26.71	20.64	14.34	10.96	11.31
765.000	12.68	11.68	8.78	6.98	9.02
780.000	11.45	10.87	8.30	5.24	3.11
795.000	2.05	2.25	5.31	9.24	10.19
810.000	8.54	5.63	3.32	2.07	1.24
825.000	0.87	1.54	4.88	9.00	10.06
840.000	8.47	5.58	3.27	2.04	1.24
855.000	0.77	0.46	0.50	1.91	5.96
870.000	9.63	9.77	7.64	4.84	2.88
885.000	1.80	1.09	0.67	0.41	0.25
900.000	0.15	0.10	0.27	2.13	6.08
915.000	9.50	9.83	7.47	4.62	2.83
930.000	1.73	1.07	0.64	0.40	0.24
945.000	0.15	0.09	0.06	0.03	0.00
960.000	0.00	1.08	4.33	8.49	10.16
975.000	8.52	5.68	3.46	2.10	1.30
990.000	0.79	0.49	0.30	0.18	0.11
1,005.000	0.07	0.04	0.01	0.00	0.00
1,020.000	0.00	0.00	0.00	0.00	0.00
1,035.000	0.11	1.08	4.62	8.88	10.01
1,050.000	8.45	5.58	3.29	2.06	1.24
1,065.000	0.77	0.47	0.29	0.18	0.11
1,080.000	0.07	0.04	0.01	0.00	0.00
1,095.000	0.00	0.00	0.00	0.00	0.00
1,110.000	0.00	0.00	0.00	0.00	0.00
1,125.000	0.00	0.00	0.00	0.00	0.00
1,140.000	0.00	0.00	0.21	2.12	6.10
1,155.000	9.54	9.87	7.50	4.64	2.84
1,170.000	1.73	1.07	0.65	0.40	0.24
1,185.000	0.15	0.09	0.06	0.03	0.00
1,200.000	0.00	0.00	0.00	0.00	0.00
1,215.000	0.00	0.00	0.00	0.00	0.00
1,230.000	0.00	0.00	0.00	0.00	0.00
1,245.000	0.00	0.00	0.00	0.00	0.00
1,260.000	0.00	0.00	0.00	0.00	0.00
1,275.000	0.00	0.00	0.21	1.75	5.88
1,290.000	9.62	9.80	7.68	4.87	2.90
1,305.000	1.81	1.10	0.68	0.41	0.26
1,320.000	0.16	0.10	0.06	0.03	0.00
1,335.000	0.00	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres
Computational Time Increment	2.000 min
Time to Peak (Computed)	730.000 min
Flow (Peak, Computed)	143.19 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	729.000 min
Flow (Peak Interpolated Output)	140.51 ft ³ /s
Drainage Area	
SCS CN (Composite)	95.000
Area (User Defined)	23.930 acres
Maximum Retention (Pervious)	0.5263 in
Maximum Retention (Pervious, 20 percent)	0.1053 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.0577 in
Runoff Volume (Pervious)	526,207.169 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	525,339.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	15.000 min
Computational Time Increment	2.000 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	108.45 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	10.000 min
Unit receding limb, Tr	40.000 min
Total unit time, Tb	50.000 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
120.000	0.00	0.00	0.01	0.02	0.03
135.000	0.04	0.05	0.07	0.08	0.10
150.000	0.11	0.13	0.14	0.16	0.17
165.000	0.19	0.20	0.22	0.23	0.25
180.000	0.26	0.28	0.29	0.30	0.32
195.000	0.33	0.35	0.36	0.38	0.39
210.000	0.41	0.42	0.44	0.45	0.47
225.000	0.48	0.50	0.51	0.53	0.54
240.000	0.55	0.57	0.58	0.60	0.61
255.000	0.63	0.64	0.66	0.67	0.69
270.000	0.70	0.72	0.73	0.75	0.76
285.000	0.78	0.79	0.81	0.82	0.84
300.000	0.85	0.87	0.88	0.90	0.92
315.000	0.93	0.95	0.97	0.98	1.00
330.000	1.01	1.03	1.04	1.06	1.08
345.000	1.10	1.11	1.13	1.15	1.17
360.000	1.18	1.21	1.25	1.30	1.36
375.000	1.40	1.44	1.47	1.50	1.52
390.000	1.55	1.57	1.60	1.62	1.64
405.000	1.67	1.69	1.72	1.74	1.77
420.000	1.79	1.82	1.84	1.87	1.90
435.000	1.92	1.95	1.98	2.00	2.03
450.000	2.06	2.09	2.12	2.15	2.18
465.000	2.21	2.24	2.28	2.31	2.34
480.000	2.37	2.40	2.44	2.47	2.51
495.000	2.55	2.58	2.62	2.66	2.70
510.000	2.74	2.78	2.82	2.87	2.91
525.000	2.96	3.00	3.05	3.10	3.15
540.000	3.20	3.28	3.46	3.72	3.99
555.000	4.21	4.38	4.51	4.62	4.73
570.000	4.82	4.92	5.01	5.10	5.20
585.000	5.30	5.40	5.51	5.61	5.72
600.000	5.83	5.95	6.07	6.20	6.34
615.000	6.48	6.62	6.78	6.94	7.10

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
630.000	7.28	7.52	7.91	8.47	9.04
645.000	9.55	9.98	10.36	10.72	11.09
660.000	11.46	11.91	12.51	13.29	14.11
675.000	14.90	15.65	16.39	17.17	18.01
690.000	18.93	20.04	21.47	23.30	25.35
705.000	27.69	30.68	35.51	42.17	51.86
720.000	66.85	92.70	123.09	140.51	139.43
735.000	121.90	100.89	82.55	67.15	55.39
750.000	46.32	39.94	34.89	30.94	27.72
765.000	25.24	23.23	21.69	20.44	19.42
780.000	18.53	17.70	16.82	15.86	14.95
795.000	14.20	13.58	13.08	12.63	12.25
810.000	11.90	11.51	11.00	10.35	9.72
825.000	9.21	8.82	8.52	8.27	8.05
840.000	7.87	7.70	7.54	7.39	7.25
855.000	7.11	6.98	6.86	6.75	6.64
870.000	6.53	6.43	6.33	6.23	6.14
885.000	6.05	5.97	5.88	5.80	5.72
900.000	5.65	5.53	5.32	4.99	4.68
915.000	4.43	4.26	4.14	4.05	3.97
930.000	3.90	3.85	3.79	3.75	3.71
945.000	3.66	3.62	3.58	3.55	3.51
960.000	3.47	3.43	3.40	3.36	3.33
975.000	3.30	3.27	3.23	3.20	3.18
990.000	3.15	3.12	3.09	3.06	3.03
1,005.000	3.01	2.98	2.96	2.93	2.91
1,020.000	2.88	2.85	2.83	2.81	2.79
1,035.000	2.77	2.75	2.72	2.70	2.68
1,050.000	2.66	2.64	2.62	2.61	2.59
1,065.000	2.57	2.55	2.53	2.52	2.50
1,080.000	2.48	2.45	2.40	2.32	2.25
1,095.000	2.19	2.15	2.12	2.10	2.08
1,110.000	2.06	2.05	2.03	2.01	2.00
1,125.000	1.99	1.97	1.96	1.95	1.94
1,140.000	1.93	1.92	1.91	1.90	1.89
1,155.000	1.87	1.86	1.85	1.84	1.83
1,170.000	1.83	1.81	1.80	1.79	1.78
1,185.000	1.77	1.76	1.75	1.75	1.73
1,200.000	1.72	1.71	1.71	1.70	1.69
1,215.000	1.69	1.68	1.66	1.65	1.65
1,230.000	1.64	1.63	1.62	1.61	1.60
1,245.000	1.60	1.59	1.59	1.58	1.57
1,260.000	1.57	1.56	1.55	1.54	1.54

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,275.000	1.53	1.52	1.52	1.51	1.50
1,290.000	1.49	1.49	1.48	1.47	1.47
1,305.000	1.46	1.46	1.45	1.45	1.44
1,320.000	1.43	1.42	1.42	1.41	1.41
1,335.000	1.40	1.40	1.39	1.39	1.38
1,350.000	1.37	1.37	1.36	1.36	1.35
1,365.000	1.35	1.34	1.34	1.33	1.33
1,380.000	1.32	1.32	1.31	1.30	1.30
1,395.000	1.30	1.30	1.29	1.29	1.28
1,410.000	1.27	1.27	1.27	1.26	1.26
1,425.000	1.25	1.25	1.24	1.23	1.23
1,440.000	1.23	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres
Computational Time Increment	2.000 min
Time to Peak (Computed)	730.000 min
Flow (Peak, Computed)	176.94 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	729.000 min
Flow (Peak Interpolated Output)	173.70 ft ³ /s
Drainage Area	
SCS CN (Composite)	95.000
Area (User Defined)	23.930 acres
Maximum Retention (Pervious)	0.5263 in
Maximum Retention (Pervious, 20 percent)	0.1053 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	7.9891 in
Runoff Volume (Pervious)	693,977.091 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	692,893.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	15.000 min
Computational Time Increment	2.000 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	108.45 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	10.000 min
Unit receding limb, Tr	40.000 min
Total unit time, Tb	50.000 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
96.000	0.00	0.00	0.01	0.01	0.03
111.000	0.04	0.06	0.08	0.11	0.13
126.000	0.15	0.17	0.19	0.21	0.23
141.000	0.25	0.28	0.30	0.32	0.34
156.000	0.36	0.38	0.40	0.42	0.44
171.000	0.46	0.48	0.50	0.52	0.54
186.000	0.56	0.58	0.60	0.62	0.64
201.000	0.66	0.68	0.70	0.72	0.74
216.000	0.76	0.77	0.79	0.81	0.83
231.000	0.85	0.87	0.89	0.91	0.93
246.000	0.95	0.97	0.98	1.00	1.02
261.000	1.04	1.06	1.08	1.10	1.12
276.000	1.14	1.16	1.18	1.20	1.22
291.000	1.24	1.26	1.28	1.30	1.32
306.000	1.34	1.36	1.38	1.40	1.42
321.000	1.44	1.46	1.48	1.50	1.52
336.000	1.54	1.56	1.59	1.61	1.63
351.000	1.65	1.67	1.69	1.72	1.75
366.000	1.82	1.92	2.01	2.10	2.16
381.000	2.21	2.25	2.29	2.32	2.36
396.000	2.39	2.42	2.46	2.49	2.52
411.000	2.55	2.59	2.62	2.65	2.69
426.000	2.72	2.76	2.79	2.83	2.86
441.000	2.90	2.94	2.98	3.02	3.05
456.000	3.09	3.13	3.17	3.21	3.25
471.000	3.29	3.34	3.38	3.42	3.47
486.000	3.52	3.56	3.61	3.66	3.71
501.000	3.76	3.81	3.86	3.91	3.97
516.000	4.02	4.08	4.14	4.20	4.26
531.000	4.32	4.38	4.45	4.52	4.65
546.000	4.93	5.36	5.80	6.16	6.42
561.000	6.62	6.79	6.94	7.08	7.21
576.000	7.34	7.46	7.59	7.73	7.86
591.000	8.00	8.15	8.30	8.45	8.62

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
606.000	8.79	8.96	9.14	9.33	9.53
621.000	9.73	9.94	10.17	10.40	10.77
636.000	11.37	12.27	13.18	13.97	14.62
651.000	15.17	15.69	16.19	16.70	17.30
666.000	18.08	19.08	20.12	21.12	22.08
681.000	23.04	24.04	25.12	26.31	27.66
696.000	29.19	30.99	33.05	35.59	39.09
711.000	44.99	53.22	65.09	83.35	115.01
726.000	152.40	173.70	172.15	150.43	124.58
741.000	102.10	83.20	68.69	57.48	49.70
756.000	43.76	39.40	35.92	33.21	30.93
771.000	29.12	27.62	26.36	25.25	24.21
786.000	23.11	21.92	20.80	19.86	19.07
801.000	18.42	17.84	17.33	16.87	16.32
816.000	15.55	14.55	13.57	12.78	12.20
831.000	11.76	11.40	11.10	10.83	10.60
846.000	10.38	10.18	9.99	9.81	9.63
861.000	9.47	9.32	9.17	9.02	8.88
876.000	8.75	8.62	8.50	8.38	8.26
891.000	8.15	8.04	7.93	7.83	7.66
906.000	7.33	6.83	6.34	5.95	5.69
921.000	5.51	5.37	5.27	5.17	5.10
936.000	5.03	4.97	4.91	4.85	4.79
951.000	4.74	4.69	4.64	4.59	4.54
966.000	4.50	4.45	4.41	4.37	4.32
981.000	4.28	4.24	4.20	4.16	4.13
996.000	4.09	4.05	4.02	3.98	3.95
1,011.000	3.92	3.88	3.85	3.81	3.78
1,026.000	3.76	3.73	3.70	3.67	3.64
1,041.000	3.61	3.59	3.56	3.53	3.51
1,056.000	3.48	3.45	3.43	3.41	3.38
1,071.000	3.36	3.33	3.31	3.28	3.25
1,086.000	3.17	3.04	2.90	2.80	2.73
1,101.000	2.69	2.65	2.62	2.59	2.57
1,116.000	2.56	2.54	2.52	2.50	2.49
1,131.000	2.47	2.45	2.44	2.43	2.41
1,146.000	2.40	2.38	2.37	2.36	2.35
1,161.000	2.33	2.32	2.30	2.29	2.28
1,176.000	2.26	2.25	2.24	2.23	2.22
1,191.000	2.20	2.19	2.18	2.17	2.15
1,206.000	2.14	2.13	2.12	2.11	2.10
1,221.000	2.09	2.08	2.07	2.06	2.05
1,236.000	2.04	2.03	2.02	2.01	2.00

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,251.000	1.99	1.98	1.97	1.96	1.95
1,266.000	1.94	1.94	1.93	1.92	1.91
1,281.000	1.90	1.89	1.88	1.87	1.87
1,296.000	1.86	1.85	1.84	1.83	1.82
1,311.000	1.82	1.81	1.81	1.80	1.79
1,326.000	1.78	1.77	1.77	1.76	1.75
1,341.000	1.74	1.74	1.73	1.73	1.72
1,356.000	1.71	1.70	1.70	1.69	1.68
1,371.000	1.68	1.67	1.67	1.66	1.65
1,386.000	1.65	1.64	1.63	1.62	1.62
1,401.000	1.62	1.61	1.61	1.60	1.59
1,416.000	1.58	1.58	1.57	1.57	1.56
1,431.000	1.56	1.55	1.55	1.54	(N/A)

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres
Computational Time Increment	2.000 min
Time to Peak (Computed)	730.000 min
Flow (Peak, Computed)	233.20 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	729.000 min
Flow (Peak Interpolated Output)	229.03 ft ³ /s
Drainage Area	
SCS CN (Composite)	95.000
Area (User Defined)	23.930 acres
Maximum Retention (Pervious)	0.5263 in
Maximum Retention (Pervious, 20 percent)	0.1053 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.6203 in
Runoff Volume (Pervious)	1,009,409.305 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,007,954.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	15.000 min
Computational Time Increment	2.000 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	108.45 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	10.000 min
Unit receding limb, Tr	40.000 min
Total unit time, Tb	50.000 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: EX AREA E
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	15.000 min
Area (User Defined)	23.930 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
75.000	0.00	0.01	0.02	0.04	0.06
90.000	0.09	0.13	0.16	0.20	0.23
105.000	0.27	0.30	0.34	0.37	0.40
120.000	0.44	0.47	0.50	0.54	0.57
135.000	0.60	0.63	0.66	0.69	0.72
150.000	0.75	0.78	0.81	0.84	0.87
165.000	0.90	0.93	0.96	0.99	1.02
180.000	1.04	1.07	1.10	1.13	1.15
195.000	1.18	1.21	1.24	1.26	1.29
210.000	1.32	1.34	1.37	1.39	1.42
225.000	1.45	1.48	1.50	1.53	1.55
240.000	1.58	1.60	1.63	1.66	1.68
255.000	1.71	1.73	1.76	1.78	1.81
270.000	1.84	1.86	1.89	1.91	1.94
285.000	1.97	2.00	2.02	2.05	2.07
300.000	2.10	2.13	2.15	2.18	2.21
315.000	2.23	2.26	2.29	2.32	2.35
330.000	2.37	2.40	2.43	2.46	2.49
345.000	2.52	2.54	2.57	2.60	2.63
360.000	2.66	2.72	2.85	3.05	3.25
375.000	3.41	3.53	3.61	3.69	3.74
390.000	3.80	3.85	3.90	3.95	3.99
405.000	4.04	4.09	4.13	4.18	4.23
420.000	4.28	4.33	4.38	4.43	4.48
435.000	4.52	4.58	4.63	4.68	4.74
450.000	4.79	4.84	4.90	4.96	5.02
465.000	5.08	5.14	5.20	5.26	5.32
480.000	5.38	5.44	5.51	5.58	5.64
495.000	5.71	5.79	5.86	5.93	6.01
510.000	6.08	6.16	6.24	6.32	6.41
525.000	6.49	6.58	6.67	6.76	6.86
540.000	6.95	7.19	7.73	8.62	9.52
555.000	10.23	10.74	11.11	11.40	11.65
570.000	11.87	12.08	12.28	12.48	12.68

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
585.000	12.88	13.10	13.31	13.53	13.75
600.000	13.99	14.24	14.49	14.75	15.03
615.000	15.31	15.60	15.91	16.23	16.57
630.000	16.93	17.52	18.59	20.21	21.87
645.000	23.26	24.35	25.25	26.07	26.85
660.000	27.61	28.51	29.65	31.09	32.59
675.000	34.03	35.40	36.76	38.18	39.72
690.000	41.38	43.01	44.35	45.44	46.87
705.000	49.26	53.27	60.77	71.45	86.93
720.000	110.70	152.13	201.19	229.03	226.73
735.000	198.06	164.10	134.64	109.83	90.73
750.000	75.97	65.96	58.91	54.50	51.24
765.000	48.61	46.17	44.07	42.24	40.64
780.000	39.17	37.76	36.24	34.58	33.00
795.000	31.65	30.53	29.58	28.73	27.98
810.000	27.30	26.45	25.16	23.39	21.66
825.000	20.29	19.29	18.57	17.98	17.50
840.000	17.08	16.72	16.39	16.08	15.79
855.000	15.51	15.26	15.01	14.78	14.55
870.000	14.33	14.12	13.92	13.72	13.53
885.000	13.35	13.17	13.00	12.84	12.68
900.000	12.53	12.23	11.60	10.62	9.66
915.000	8.92	8.42	8.09	7.85	7.67
930.000	7.52	7.40	7.29	7.20	7.11
945.000	7.02	6.94	6.87	6.80	6.73
960.000	6.65	6.59	6.52	6.45	6.39
975.000	6.33	6.27	6.21	6.15	6.09
990.000	6.03	5.98	5.93	5.88	5.82
1,005.000	5.77	5.73	5.68	5.63	5.58
1,020.000	5.53	5.48	5.44	5.40	5.36
1,035.000	5.32	5.28	5.24	5.20	5.16
1,050.000	5.12	5.08	5.04	5.00	4.97
1,065.000	4.94	4.90	4.87	4.84	4.80
1,080.000	4.77	4.70	4.55	4.30	4.05
1,095.000	3.86	3.74	3.66	3.59	3.55
1,110.000	3.51	3.48	3.45	3.43	3.40
1,125.000	3.38	3.36	3.34	3.32	3.29
1,140.000	3.28	3.26	3.24	3.22	3.20
1,155.000	3.18	3.16	3.14	3.12	3.11
1,170.000	3.09	3.07	3.05	3.03	3.02
1,185.000	3.00	2.99	2.97	2.95	2.94
1,200.000	2.92	2.91	2.89	2.88	2.86
1,215.000	2.85	2.83	2.81	2.80	2.79

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: EX AREA E
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,230.000	2.77	2.76	2.74	2.73	2.72
1,245.000	2.71	2.69	2.68	2.66	2.65
1,260.000	2.64	2.63	2.62	2.61	2.59
1,275.000	2.58	2.57	2.56	2.55	2.53
1,290.000	2.52	2.51	2.50	2.49	2.48
1,305.000	2.47	2.45	2.44	2.43	2.42
1,320.000	2.41	2.41	2.39	2.38	2.37
1,335.000	2.36	2.35	2.35	2.34	2.33
1,350.000	2.32	2.31	2.30	2.29	2.28
1,365.000	2.27	2.26	2.25	2.25	2.24
1,380.000	2.23	2.22	2.21	2.20	2.19
1,395.000	2.18	2.18	2.17	2.16	2.15
1,410.000	2.14	2.14	2.13	2.12	2.12
1,425.000	2.11	2.10	2.09	2.08	2.07
1,440.000	2.07	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	114.34 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	114.31 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6401 in
Runoff Volume (Pervious)	801,008.304 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	800,993.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
507.000	0.00	0.00	0.00	0.01	0.01
522.000	0.02	0.04	0.05	0.06	0.08
537.000	0.09	0.10	0.10	0.10	0.10
552.000	0.09	0.09	0.08	0.09	0.14
567.000	0.19	0.31	0.43	0.61	0.81
582.000	0.98	1.13	1.24	1.27	1.31
597.000	1.39	1.46	1.62	1.78	2.04
612.000	2.33	2.59	2.81	2.96	2.98
627.000	2.99	2.97	2.94	3.01	3.08
642.000	3.27	3.49	3.76	4.07	4.38
657.000	4.70	5.02	5.34	5.67	6.13
672.000	6.59	7.05	7.51	7.90	8.25
687.000	8.71	9.34	9.99	10.72	11.45
702.000	12.35	13.25	14.89	16.71	19.71
717.000	23.50	28.54	35.46	42.95	52.70
732.000	62.45	73.41	84.38	93.43	102.03
747.000	107.88	111.94	114.31	114.25	113.33
762.000	109.22	105.09	98.78	92.47	86.48
777.000	80.58	75.12	69.95	65.10	60.72
792.000	56.51	52.87	49.24	46.11	42.98
807.000	40.66	38.51	36.70	35.08	33.31
822.000	31.33	29.43	27.82	26.23	25.33
837.000	24.42	24.24	24.19	24.00	23.72
852.000	23.14	22.19	21.24	20.33	19.43
867.000	18.95	18.47	18.49	18.60	18.89
882.000	19.28	19.39	19.17	18.78	17.89
897.000	16.98	15.63	14.28	13.24	12.25
912.000	11.87	11.81	12.37	13.66	14.90
927.000	15.99	17.03	16.98	16.94	16.25
942.000	15.45	14.37	13.15	11.74	10.13
957.000	8.72	7.86	7.07	7.33	7.59
972.000	8.79	10.13	11.62	13.17	14.34
987.000	15.08	15.54	15.27	14.95	14.05
1,002.000	13.14	11.85	10.50	9.16	7.82

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,017.000	6.67	5.71	4.85	4.22	3.62
1,032.000	3.20	2.79	3.08	3.45	4.40
1,047.000	5.61	7.26	9.36	11.30	12.87
1,062.000	14.31	14.62	14.92	14.45	13.89
1,077.000	12.98	11.91	10.64	9.18	7.82
1,092.000	6.70	5.62	4.89	4.17	3.65
1,107.000	3.16	2.74	2.34	2.00	1.73
1,122.000	1.48	1.28	1.08	0.94	0.80
1,137.000	0.70	0.60	0.75	1.01	1.64
1,152.000	2.61	3.86	5.68	7.52	9.58
1,167.000	11.63	12.83	13.96	14.27	14.27
1,182.000	13.88	13.14	12.23	10.99	9.73
1,197.000	8.41	7.10	6.13	5.19	4.49
1,212.000	3.87	3.35	2.91	2.50	2.16
1,227.000	1.84	1.60	1.37	1.19	1.02
1,242.000	0.88	0.76	0.65	0.57	0.49
1,257.000	0.42	0.36	0.31	0.27	0.23
1,272.000	0.20	0.17	0.15	0.51	1.16
1,287.000	2.09	3.49	5.04	7.39	9.74
1,302.000	11.57	13.38	14.16	14.63	14.57
1,317.000	14.09	13.42	12.41	11.32	9.84
1,332.000	8.35	7.19	6.05	5.21	4.47
1,347.000	3.87	3.38	2.92	2.51	2.13
1,362.000	1.86	1.59	1.39	1.19	1.03
1,377.000	0.88	0.76	0.66	0.56	0.49
1,392.000	0.42	0.37	0.31	0.27	0.23
1,407.000	0.20	0.17	0.15	0.13	0.11
1,422.000	0.10	0.09	0.07	0.06	0.05
1,437.000	0.03	0.02	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	346.07 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	344.29 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4990 in
Runoff Volume (Pervious)	2,197,220.614 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,181,713.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
402.000	0.00	0.00	0.00	0.01	0.01
417.000	0.02	0.03	0.05	0.07	0.09
432.000	0.12	0.16	0.20	0.24	0.30
447.000	0.35	0.41	0.47	0.54	0.61
462.000	0.68	0.75	0.83	0.91	0.99
477.000	1.07	1.16	1.25	1.34	1.43
492.000	1.52	1.62	1.72	1.82	1.93
507.000	2.03	2.14	2.25	2.36	2.48
522.000	2.60	2.72	2.84	2.97	3.10
537.000	3.24	3.37	3.52	3.68	3.86
552.000	4.05	4.26	4.50	4.75	5.05
567.000	5.34	5.66	5.98	6.32	6.66
582.000	7.00	7.36	7.71	8.07	8.43
597.000	8.80	9.16	9.55	9.93	10.34
612.000	10.75	11.17	11.61	12.05	12.52
627.000	13.00	13.53	14.05	14.67	15.29
642.000	16.02	16.79	17.67	18.62	19.63
657.000	20.75	21.90	23.19	24.47	25.97
672.000	27.47	29.18	30.96	32.92	35.01
687.000	37.25	39.72	42.29	45.30	48.30
702.000	52.15	56.01	61.24	66.82	75.01
717.000	84.93	98.51	117.57	138.57	167.32
732.000	196.08	229.62	263.17	289.95	315.10
747.000	330.81	340.40	344.29	339.91	333.42
762.000	318.99	304.50	284.72	264.95	246.31
777.000	227.93	211.66	196.71	182.98	170.93
792.000	159.32	149.26	139.23	131.02	122.81
807.000	115.76	108.97	102.90	97.27	92.02
822.000	87.30	82.73	78.66	74.61	71.09
837.000	67.57	64.48	61.47	58.77	56.24
852.000	53.90	51.80	49.79	48.07	46.36
867.000	44.91	43.46	42.18	40.93	39.80
882.000	38.75	37.79	36.96	36.15	35.44
897.000	34.73	34.09	33.44	32.80	32.16

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
912.000	31.48	30.78	30.04	29.26	28.48
927.000	27.68	26.88	26.12	25.36	24.67
942.000	23.99	23.38	22.81	22.28	21.82
957.000	21.37	20.98	20.59	20.25	19.92
972.000	19.62	19.33	19.06	18.80	18.55
987.000	18.32	18.10	17.89	17.68	17.48
1,002.000	17.29	17.11	16.93	16.76	16.59
1,017.000	16.43	16.27	16.12	15.97	15.82
1,032.000	15.68	15.54	15.40	15.27	15.14
1,047.000	15.01	14.89	14.77	14.65	14.53
1,062.000	14.42	14.30	14.19	14.09	13.98
1,077.000	13.88	13.77	13.66	13.55	13.43
1,092.000	13.30	13.16	13.00	12.83	12.66
1,107.000	12.48	12.30	12.12	11.94	11.78
1,122.000	11.62	11.47	11.33	11.20	11.08
1,137.000	10.97	10.86	10.76	10.67	10.58
1,152.000	10.49	10.41	10.33	10.26	10.19
1,167.000	10.12	10.05	9.99	9.93	9.86
1,182.000	9.81	9.75	9.69	9.63	9.58
1,197.000	9.52	9.47	9.41	9.36	9.31
1,212.000	9.26	9.21	9.16	9.11	9.06
1,227.000	9.01	8.96	8.92	8.87	8.82
1,242.000	8.78	8.73	8.69	8.65	8.60
1,257.000	8.56	8.52	8.48	8.44	8.40
1,272.000	8.36	8.32	8.28	8.25	8.21
1,287.000	8.17	8.13	8.10	8.06	8.02
1,302.000	7.99	7.95	7.92	7.88	7.85
1,317.000	7.82	7.78	7.75	7.72	7.68
1,332.000	7.65	7.62	7.58	7.55	7.52
1,347.000	7.49	7.45	7.42	7.39	7.36
1,362.000	7.33	7.30	7.28	7.25	7.22
1,377.000	7.19	7.16	7.13	7.11	7.08
1,392.000	7.05	7.03	7.00	6.97	6.95
1,407.000	6.92	6.90	6.87	6.85	6.82
1,422.000	6.80	6.77	6.75	6.73	6.70
1,437.000	6.68	6.63	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	460.87 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	458.07 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3213 in
Runoff Volume (Pervious)	3,087,200.419 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	3,067,288.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
348.000	0.00	0.00	0.00	0.01	0.02
363.000	0.03	0.04	0.06	0.09	0.12
378.000	0.16	0.21	0.26	0.32	0.38
393.000	0.46	0.53	0.62	0.70	0.79
408.000	0.88	0.98	1.08	1.18	1.29
423.000	1.39	1.50	1.61	1.73	1.84
438.000	1.96	2.08	2.20	2.33	2.45
453.000	2.58	2.71	2.84	2.98	3.11
468.000	3.25	3.40	3.54	3.69	3.83
483.000	3.99	4.14	4.30	4.46	4.62
498.000	4.79	4.95	5.13	5.30	5.48
513.000	5.66	5.85	6.04	6.23	6.43
528.000	6.63	6.84	7.05	7.27	7.49
543.000	7.75	8.01	8.32	8.66	9.03
558.000	9.49	9.96	10.51	11.06	11.67
573.000	12.28	12.91	13.54	14.18	14.82
588.000	15.45	16.08	16.71	17.34	17.97
603.000	18.61	19.26	19.93	20.61	21.30
618.000	22.02	22.75	23.51	24.28	25.13
633.000	25.98	26.99	28.01	29.25	30.54
648.000	32.04	33.67	35.42	37.34	39.31
663.000	41.50	43.68	46.14	48.61	51.35
678.000	54.18	57.24	60.46	63.87	67.56
693.000	71.37	75.68	79.99	85.25	90.52
708.000	97.45	104.80	115.48	128.40	146.05
723.000	170.80	198.05	235.19	272.34	315.32
738.000	358.29	392.14	423.77	443.02	454.23
753.000	458.07	451.23	441.71	422.18	402.56
768.000	376.60	350.65	326.52	302.81	281.96
783.000	262.89	245.40	230.13	215.40	202.63
798.000	189.90	179.43	168.96	159.93	151.21
813.000	143.38	136.07	129.23	123.00	116.93
828.000	111.42	105.93	101.03	96.14	91.78
843.000	87.53	83.69	80.08	76.74	73.73

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
858.000	70.85	68.40	65.96	63.90	61.84
873.000	60.03	58.27	56.69	55.21	53.86
888.000	52.68	51.54	50.54	49.54	48.62
903.000	47.71	46.78	45.86	44.86	43.82
918.000	42.71	41.53	40.33	39.10	37.87
933.000	36.70	35.53	34.47	33.42	32.49
948.000	31.62	30.82	30.12	29.45	28.87
963.000	28.29	27.80	27.31	26.88	26.46
978.000	26.07	25.70	25.35	25.03	24.71
993.000	24.41	24.12	23.85	23.58	23.33
1,008.000	23.08	22.84	22.61	22.39	22.17
1,023.000	21.96	21.75	21.55	21.36	21.17
1,038.000	20.98	20.80	20.62	20.45	20.28
1,053.000	20.12	19.96	19.80	19.64	19.49
1,068.000	19.34	19.19	19.05	18.90	18.76
1,083.000	18.60	18.44	18.27	18.06	17.86
1,098.000	17.60	17.34	17.05	16.76	16.46
1,113.000	16.17	15.89	15.61	15.35	15.12
1,128.000	14.88	14.69	14.50	14.33	14.17
1,143.000	14.03	13.89	13.76	13.64	13.52
1,158.000	13.41	13.30	13.21	13.11	13.02
1,173.000	12.92	12.84	12.75	12.67	12.59
1,188.000	12.52	12.44	12.37	12.29	12.22
1,203.000	12.15	12.08	12.01	11.94	11.88
1,218.000	11.81	11.75	11.68	11.62	11.56
1,233.000	11.50	11.44	11.38	11.32	11.26
1,248.000	11.21	11.15	11.09	11.04	10.98
1,263.000	10.93	10.88	10.82	10.77	10.72
1,278.000	10.67	10.62	10.57	10.52	10.48
1,293.000	10.43	10.38	10.34	10.29	10.25
1,308.000	10.20	10.15	10.11	10.06	10.02
1,323.000	9.97	9.93	9.88	9.84	9.80
1,338.000	9.76	9.72	9.67	9.63	9.59
1,353.000	9.56	9.52	9.48	9.44	9.40
1,368.000	9.37	9.33	9.29	9.26	9.22
1,383.000	9.18	9.15	9.11	9.07	9.04
1,398.000	9.00	8.97	8.94	8.90	8.87
1,413.000	8.84	8.80	8.77	8.73	8.70
1,428.000	8.67	8.64	8.60	8.57	8.51

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres
Computational Time Increment	6.595 min
Time to Peak (Computed)	751.774 min
Flow (Peak, Computed)	653.99 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	649.43 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.170
Area (User Defined)	134.540 acres
Maximum Retention (Pervious)	2.3198 in
Maximum Retention (Pervious, 20 percent)	0.4640 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8283 in
Runoff Volume (Pervious)	4,799,931.587 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	4,772,632.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	49.459 min
Computational Time Increment	6.595 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	184.93 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	32.973 min
Unit receding limb, Tr	131.890 min
Total unit time, Tb	164.863 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	49.459 min
Area (User Defined)	134.540 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
276.000	0.00	0.00	0.00	0.01	0.02
291.000	0.03	0.05	0.07	0.10	0.13
306.000	0.17	0.22	0.27	0.33	0.40
321.000	0.47	0.54	0.62	0.70	0.79
336.000	0.87	0.97	1.06	1.15	1.25
351.000	1.35	1.45	1.55	1.66	1.77
366.000	1.89	2.01	2.15	2.29	2.46
381.000	2.63	2.81	3.01	3.21	3.42
396.000	3.63	3.85	4.06	4.28	4.50
411.000	4.72	4.94	5.16	5.38	5.60
426.000	5.82	6.04	6.27	6.49	6.72
441.000	6.95	7.18	7.42	7.66	7.89
456.000	8.14	8.38	8.63	8.88	9.14
471.000	9.40	9.66	9.92	10.19	10.47
486.000	10.75	11.03	11.32	11.61	11.90
501.000	12.20	12.51	12.82	13.14	13.46
516.000	13.79	14.13	14.47	14.82	15.17
531.000	15.54	15.91	16.30	16.69	17.16
546.000	17.66	18.27	18.97	19.78	20.78
561.000	21.83	23.09	24.35	25.73	27.12
576.000	28.52	29.93	31.33	32.71	34.06
591.000	35.36	36.65	37.91	39.17	40.42
606.000	41.67	42.94	44.22	45.52	46.84
621.000	48.18	49.57	50.98	52.52	54.06
636.000	55.94	57.83	60.17	62.64	65.52
651.000	68.70	72.09	75.82	79.63	83.79
666.000	87.94	92.48	97.03	101.95	106.97
681.000	112.29	117.82	123.56	129.62	135.79
696.000	142.40	149.01	156.36	163.73	172.84
711.000	182.40	196.33	213.19	236.54	269.62
726.000	306.12	356.14	406.18	463.90	521.63
741.000	566.45	608.15	632.67	646.07	649.43
756.000	638.26	623.58	595.71	567.75	532.17
771.000	496.59	464.24	432.62	405.13	380.21

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA A1
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
786.000	357.43	337.66	318.57	301.94	285.36
801.000	271.49	257.61	245.47	233.69	222.97
816.000	212.88	203.32	194.47	185.77	177.61
831.000	169.47	161.98	154.48	147.68	141.02
846.000	134.95	129.22	123.90	119.10	114.52
861.000	110.62	106.74	103.49	100.24	97.40
876.000	94.63	92.15	89.83	87.71	85.85
891.000	84.05	82.46	80.88	79.41	77.95
906.000	76.42	74.88	73.15	71.33	69.35
921.000	67.20	65.01	62.72	60.44	58.25
936.000	56.06	54.09	52.15	50.43	48.83
951.000	47.38	46.12	44.93	43.91	42.91
966.000	42.07	41.23	40.51	39.80	39.16
981.000	38.56	37.99	37.46	36.94	36.47
996.000	36.01	35.59	35.16	34.77	34.39
1,011.000	34.02	33.67	33.32	32.99	32.67
1,026.000	32.36	32.05	31.76	31.46	31.18
1,041.000	30.91	30.64	30.38	30.13	29.88
1,056.000	29.64	29.40	29.17	28.94	28.72
1,071.000	28.50	28.29	28.07	27.85	27.62
1,086.000	27.36	27.08	26.74	26.39	25.94
1,101.000	25.49	24.97	24.44	23.90	23.37
1,116.000	22.86	22.37	21.90	21.48	21.07
1,131.000	20.74	20.41	20.13	19.86	19.63
1,146.000	19.40	19.19	19.00	18.81	18.64
1,161.000	18.48	18.33	18.18	18.04	17.90
1,176.000	17.77	17.65	17.53	17.41	17.30
1,191.000	17.19	17.08	16.97	16.87	16.77
1,206.000	16.66	16.57	16.47	16.38	16.28
1,221.000	16.19	16.10	16.02	15.93	15.84
1,236.000	15.76	15.68	15.60	15.52	15.44
1,251.000	15.36	15.28	15.21	15.13	15.05
1,266.000	14.98	14.91	14.83	14.76	14.69
1,281.000	14.62	14.55	14.48	14.42	14.35
1,296.000	14.28	14.22	14.16	14.09	14.03
1,311.000	13.96	13.90	13.84	13.77	13.71
1,326.000	13.65	13.59	13.53	13.47	13.41
1,341.000	13.35	13.29	13.23	13.18	13.12
1,356.000	13.07	13.01	12.96	12.91	12.85
1,371.000	12.80	12.75	12.70	12.65	12.60
1,386.000	12.55	12.50	12.45	12.40	12.35
1,401.000	12.30	12.25	12.21	12.16	12.12
1,416.000	12.07	12.03	11.98	11.94	11.89

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA A1
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,431.000	11.85	11.81	11.76	11.68	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	743.401 min
Flow (Peak, Computed)	29.83 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	29.78 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	27.910 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	164,933.302 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	164,921.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	62.62 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
513.000	0.00	0.00	0.00	0.01	0.02
528.000	0.02	0.02	0.02	0.02	0.02
543.000	0.02	0.01	0.01	0.01	0.01
558.000	0.01	0.01	0.04	0.09	0.17
573.000	0.27	0.35	0.39	0.40	0.38
588.000	0.33	0.28	0.22	0.20	0.24
603.000	0.35	0.51	0.69	0.81	0.87
618.000	0.86	0.79	0.68	0.56	0.43
633.000	0.42	0.49	0.64	0.90	1.13
648.000	1.28	1.32	1.25	1.19	1.16
663.000	1.20	1.40	1.63	1.80	1.88
678.000	1.89	1.95	2.06	2.25	2.45
693.000	2.65	2.86	3.08	3.48	3.89
708.000	4.41	5.11	6.42	8.18	10.47
723.000	13.40	17.05	20.83	24.47	27.47
738.000	29.08	29.78	29.66	28.80	27.26
753.000	25.29	22.99	20.56	18.31	16.33
768.000	14.67	13.23	12.02	11.09	10.45
783.000	10.12	9.54	8.75	7.79	6.81
798.000	6.05	5.52	5.26	5.63	5.93
813.000	6.05	5.83	5.27	4.59	3.88
828.000	3.24	3.14	3.47	4.12	4.85
843.000	5.17	5.17	4.89	4.35	3.63
858.000	2.92	2.33	2.20	2.52	3.16
873.000	4.05	4.77	5.03	4.97	4.62
888.000	4.02	3.29	2.57	1.95	1.54
903.000	1.22	1.05	1.13	1.64	2.49
918.000	3.50	4.43	4.78	4.80	4.52
933.000	4.02	3.29	2.59	1.98	1.54
948.000	1.23	0.97	0.75	0.59	0.55
963.000	0.74	1.20	2.05	3.12	4.07
978.000	4.73	4.80	4.55	4.09	3.45
993.000	2.71	2.10	1.62	1.26	1.00
1,008.000	0.79	0.61	0.48	0.38	0.30

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,023.000	0.23	0.18	0.14	0.11	0.14
1,038.000	0.37	0.94	1.82	2.91	3.98
1,053.000	4.56	4.76	4.61	4.16	3.49
1,068.000	2.79	2.14	1.66	1.31	1.04
1,083.000	0.81	0.64	0.50	0.39	0.30
1,098.000	0.24	0.19	0.15	0.12	0.09
1,113.000	0.07	0.06	0.04	0.04	0.03
1,128.000	0.02	0.01	0.01	0.00	0.00
1,143.000	0.00	0.16	0.54	1.18	2.20
1,158.000	3.33	4.23	4.78	4.79	4.47
1,173.000	3.95	3.27	2.55	1.98	1.53
1,188.000	1.20	0.95	0.75	0.59	0.46
1,203.000	0.36	0.28	0.22	0.17	0.14
1,218.000	0.11	0.08	0.07	0.05	0.04
1,233.000	0.03	0.03	0.02	0.01	0.00
1,248.000	0.00	0.00	0.00	0.00	0.00
1,263.000	0.00	0.00	0.00	0.00	0.00
1,278.000	0.02	0.11	0.53	1.26	2.25
1,293.000	3.47	4.37	4.80	4.84	4.56
1,308.000	3.97	3.26	2.53	1.93	1.52
1,323.000	1.20	0.95	0.73	0.58	0.46
1,338.000	0.36	0.28	0.22	0.17	0.14
1,353.000	0.11	0.08	0.07	0.05	0.04
1,368.000	0.03	0.03	0.02	0.01	0.00
1,383.000	0.00	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	94.83 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	94.12 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	27.910 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	453,942.481 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	452,004.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	62.62 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
408.000	0.00	0.00	0.00	0.01	0.01
423.000	0.02	0.03	0.03	0.05	0.06
438.000	0.07	0.08	0.10	0.11	0.13
453.000	0.15	0.16	0.18	0.20	0.22
468.000	0.23	0.25	0.27	0.29	0.31
483.000	0.33	0.35	0.37	0.40	0.42
498.000	0.44	0.46	0.49	0.51	0.54
513.000	0.56	0.59	0.62	0.64	0.67
528.000	0.70	0.73	0.76	0.79	0.82
543.000	0.86	0.90	0.95	1.01	1.08
558.000	1.15	1.23	1.32	1.40	1.47
573.000	1.55	1.63	1.70	1.78	1.85
588.000	1.93	2.01	2.09	2.17	2.25
603.000	2.34	2.43	2.52	2.62	2.72
618.000	2.82	2.93	3.04	3.16	3.28
633.000	3.42	3.58	3.76	3.98	4.23
648.000	4.51	4.80	5.10	5.40	5.71
663.000	6.04	6.38	6.77	7.20	7.67
678.000	8.19	8.75	9.33	9.95	10.63
693.000	11.36	12.18	13.10	14.23	15.56
708.000	17.18	19.14	22.00	26.12	31.92
723.000	40.21	51.88	64.75	77.53	87.89
738.000	92.87	94.12	91.95	86.27	78.51
753.000	70.48	62.73	55.98	50.14	44.96
768.000	40.40	36.64	33.42	30.61	28.16
783.000	26.15	24.38	22.78	21.32	20.04
798.000	18.86	17.80	16.83	16.00	15.24
813.000	14.55	13.89	13.28	12.67	12.09
828.000	11.53	11.01	10.53	10.09	9.71
843.000	9.38	9.08	8.82	8.58	8.37
858.000	8.17	7.98	7.81	7.66	7.51
873.000	7.37	7.24	7.11	7.00	6.88
888.000	6.77	6.67	6.57	6.47	6.38
903.000	6.27	6.16	6.02	5.84	5.64

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
918.000	5.44	5.23	5.04	4.86	4.71
933.000	4.57	4.47	4.37	4.28	4.21
948.000	4.14	4.08	4.02	3.96	3.91
963.000	3.86	3.82	3.77	3.73	3.69
978.000	3.65	3.61	3.58	3.54	3.51
993.000	3.47	3.44	3.41	3.38	3.35
1,008.000	3.32	3.29	3.26	3.23	3.20
1,023.000	3.17	3.15	3.12	3.10	3.07
1,038.000	3.05	3.02	3.00	2.98	2.95
1,053.000	2.93	2.91	2.89	2.87	2.85
1,068.000	2.83	2.81	2.79	2.77	2.75
1,083.000	2.72	2.70	2.66	2.62	2.57
1,098.000	2.52	2.48	2.43	2.39	2.35
1,113.000	2.32	2.29	2.27	2.24	2.22
1,128.000	2.20	2.19	2.17	2.15	2.14
1,143.000	2.12	2.11	2.10	2.08	2.07
1,158.000	2.06	2.05	2.03	2.02	2.01
1,173.000	2.00	1.99	1.98	1.97	1.95
1,188.000	1.94	1.93	1.92	1.91	1.90
1,203.000	1.89	1.88	1.87	1.86	1.85
1,218.000	1.84	1.84	1.83	1.82	1.81
1,233.000	1.80	1.79	1.78	1.77	1.76
1,248.000	1.75	1.75	1.74	1.73	1.72
1,263.000	1.71	1.71	1.70	1.69	1.68
1,278.000	1.68	1.67	1.66	1.65	1.65
1,293.000	1.64	1.63	1.63	1.62	1.61
1,308.000	1.60	1.60	1.59	1.58	1.57
1,323.000	1.57	1.56	1.56	1.55	1.54
1,338.000	1.54	1.53	1.52	1.52	1.51
1,353.000	1.51	1.50	1.49	1.49	1.48
1,368.000	1.48	1.47	1.47	1.46	1.45
1,383.000	1.45	1.44	1.44	1.43	1.43
1,398.000	1.42	1.42	1.41	1.41	1.40
1,413.000	1.40	1.39	1.39	1.38	1.38
1,428.000	1.37	1.37	1.36	1.36	1.35

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	125.80 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	124.62 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	27.910 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	638,353.348 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	635,864.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	62.62 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
354.000	0.00	0.00	0.00	0.01	0.02
369.000	0.02	0.03	0.05	0.06	0.08
384.000	0.09	0.11	0.13	0.15	0.17
399.000	0.19	0.22	0.24	0.26	0.28
414.000	0.31	0.33	0.36	0.38	0.41
429.000	0.43	0.46	0.48	0.51	0.54
444.000	0.56	0.59	0.62	0.65	0.68
459.000	0.71	0.74	0.77	0.80	0.83
474.000	0.86	0.89	0.93	0.96	1.00
489.000	1.03	1.07	1.10	1.14	1.18
504.000	1.22	1.26	1.30	1.34	1.38
519.000	1.42	1.47	1.51	1.56	1.61
534.000	1.65	1.70	1.76	1.82	1.89
549.000	1.98	2.10	2.24	2.39	2.55
564.000	2.71	2.86	3.01	3.15	3.29
579.000	3.42	3.55	3.67	3.80	3.93
594.000	4.07	4.20	4.34	4.48	4.63
609.000	4.78	4.93	5.09	5.26	5.43
624.000	5.61	5.79	5.99	6.21	6.48
639.000	6.79	7.17	7.63	8.12	8.63
654.000	9.15	9.68	10.21	10.74	11.29
669.000	11.90	12.57	13.28	14.07	14.90
684.000	15.77	16.68	17.66	18.71	19.84
699.000	21.06	22.49	24.17	26.19	28.67
714.000	32.36	37.71	45.30	56.13	71.30
729.000	87.93	104.34	117.46	123.46	124.62
744.000	121.33	113.54	103.12	92.44	82.22
759.000	73.41	65.90	59.31	53.58	48.90
774.000	44.91	41.42	38.38	35.86	33.63
789.000	31.60	29.75	28.11	26.61	25.24
804.000	24.00	22.92	21.93	21.00	20.12
819.000	19.25	18.39	17.53	16.68	15.90
834.000	15.17	14.50	13.92	13.42	12.98
849.000	12.59	12.24	11.93	11.64	11.38

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
864.000	11.14	10.91	10.70	10.50	10.32
879.000	10.14	9.98	9.81	9.66	9.52
894.000	9.38	9.24	9.11	8.96	8.78
909.000	8.57	8.29	7.99	7.66	7.33
924.000	7.03	6.76	6.52	6.32	6.16
939.000	6.01	5.88	5.77	5.67	5.57
954.000	5.49	5.41	5.34	5.27	5.21
969.000	5.15	5.09	5.03	4.98	4.93
984.000	4.88	4.83	4.78	4.73	4.69
999.000	4.64	4.60	4.56	4.52	4.48
1,014.000	4.44	4.40	4.36	4.33	4.29
1,029.000	4.26	4.22	4.19	4.15	4.12
1,044.000	4.09	4.06	4.03	3.99	3.96
1,059.000	3.93	3.91	3.88	3.85	3.82
1,074.000	3.79	3.77	3.74	3.71	3.67
1,089.000	3.61	3.55	3.47	3.38	3.30
1,104.000	3.22	3.15	3.09	3.04	2.99
1,119.000	2.95	2.92	2.89	2.86	2.84
1,134.000	2.81	2.79	2.77	2.75	2.73
1,149.000	2.71	2.69	2.68	2.66	2.64
1,164.000	2.63	2.61	2.60	2.58	2.57
1,179.000	2.55	2.54	2.52	2.51	2.50
1,194.000	2.48	2.47	2.46	2.44	2.43
1,209.000	2.41	2.40	2.39	2.38	2.37
1,224.000	2.36	2.34	2.33	2.32	2.31
1,239.000	2.30	2.29	2.28	2.26	2.25
1,254.000	2.24	2.23	2.22	2.21	2.20
1,269.000	2.19	2.18	2.17	2.16	2.15
1,284.000	2.14	2.13	2.12	2.11	2.10
1,299.000	2.09	2.08	2.07	2.06	2.06
1,314.000	2.05	2.04	2.03	2.02	2.01
1,329.000	2.00	2.00	1.99	1.98	1.97
1,344.000	1.96	1.95	1.95	1.94	1.93
1,359.000	1.92	1.92	1.91	1.90	1.89
1,374.000	1.89	1.88	1.87	1.86	1.86
1,389.000	1.85	1.84	1.84	1.83	1.82
1,404.000	1.82	1.81	1.80	1.80	1.79
1,419.000	1.78	1.77	1.77	1.76	1.76
1,434.000	1.75	1.74	1.73	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres
Computational Time Increment	4.040 min
Time to Peak (Computed)	739.361 min
Flow (Peak, Computed)	177.47 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	741.000 min
Flow (Peak Interpolated Output)	175.46 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	27.910 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	993,400.395 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	989,991.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	30.302 min
Computational Time Increment	4.040 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	62.62 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	20.201 min
Unit receding limb, Tr	80.805 min
Total unit time, Tb	101.006 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	30.302 min
Area (User Defined)	27.910 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.01	0.01	0.02
297.000	0.03	0.04	0.05	0.07	0.08
312.000	0.10	0.12	0.13	0.15	0.17
327.000	0.19	0.21	0.23	0.25	0.28
342.000	0.30	0.32	0.34	0.36	0.39
357.000	0.41	0.43	0.46	0.49	0.52
372.000	0.56	0.61	0.65	0.70	0.76
387.000	0.81	0.86	0.90	0.95	1.00
402.000	1.04	1.09	1.13	1.18	1.22
417.000	1.27	1.32	1.36	1.41	1.46
432.000	1.50	1.55	1.60	1.65	1.70
447.000	1.75	1.80	1.86	1.91	1.96
462.000	2.02	2.07	2.13	2.19	2.24
477.000	2.30	2.36	2.42	2.48	2.55
492.000	2.61	2.68	2.74	2.81	2.88
507.000	2.95	3.02	3.09	3.17	3.24
522.000	3.32	3.40	3.48	3.56	3.65
537.000	3.73	3.83	3.95	4.10	4.30
552.000	4.59	4.91	5.28	5.66	6.03
567.000	6.39	6.73	7.04	7.32	7.59
582.000	7.85	8.11	8.36	8.61	8.86
597.000	9.11	9.37	9.63	9.89	10.16
612.000	10.44	10.73	11.02	11.33	11.65
627.000	11.98	12.32	12.73	13.22	13.82
642.000	14.59	15.51	16.49	17.52	18.55
657.000	19.56	20.56	21.53	22.53	23.59
672.000	24.73	25.95	27.25	28.62	30.04
687.000	31.52	33.06	34.64	36.24	37.86
702.000	39.52	41.42	43.73	46.67	51.37
717.000	58.45	68.70	83.53	104.33	127.07
732.000	149.37	166.94	174.55	175.46	170.24
747.000	158.86	143.98	128.90	114.63	102.58
762.000	92.55	83.94	76.63	70.81	65.88
777.000	61.57	57.80	54.60	51.72	49.07

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA A2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
792.000	46.61	44.38	42.33	40.43	38.68
807.000	37.16	35.73	34.38	33.07	31.71
822.000	30.32	28.90	27.48	26.13	24.88
837.000	23.74	22.74	21.91	21.17	20.52
852.000	19.94	19.43	18.96	18.53	18.14
867.000	17.78	17.45	17.13	16.84	16.56
882.000	16.29	16.04	15.80	15.57	15.35
897.000	15.13	14.92	14.67	14.36	13.99
912.000	13.46	12.87	12.24	11.61	11.03
927.000	10.51	10.05	9.67	9.37	9.11
942.000	8.88	8.68	8.51	8.35	8.21
957.000	8.09	7.97	7.86	7.76	7.67
972.000	7.58	7.49	7.41	7.33	7.25
987.000	7.18	7.11	7.04	6.97	6.90
1,002.000	6.84	6.78	6.72	6.66	6.60
1,017.000	6.55	6.49	6.43	6.38	6.32
1,032.000	6.27	6.22	6.17	6.12	6.07
1,047.000	6.03	5.98	5.94	5.89	5.85
1,062.000	5.80	5.76	5.72	5.68	5.64
1,077.000	5.60	5.56	5.51	5.44	5.34
1,092.000	5.22	5.08	4.92	4.76	4.62
1,107.000	4.48	4.37	4.28	4.20	4.13
1,122.000	4.08	4.02	3.98	3.94	3.90
1,137.000	3.87	3.84	3.81	3.78	3.75
1,152.000	3.73	3.70	3.68	3.65	3.63
1,167.000	3.61	3.59	3.56	3.54	3.52
1,182.000	3.50	3.48	3.46	3.44	3.42
1,197.000	3.41	3.39	3.37	3.35	3.33
1,212.000	3.31	3.30	3.28	3.26	3.25
1,227.000	3.23	3.21	3.20	3.18	3.16
1,242.000	3.15	3.13	3.12	3.10	3.09
1,257.000	3.07	3.06	3.04	3.03	3.01
1,272.000	3.00	2.99	2.97	2.96	2.94
1,287.000	2.93	2.92	2.90	2.89	2.88
1,302.000	2.86	2.85	2.84	2.83	2.81
1,317.000	2.80	2.79	2.78	2.76	2.75
1,332.000	2.74	2.73	2.72	2.71	2.70
1,347.000	2.69	2.67	2.66	2.65	2.64
1,362.000	2.63	2.62	2.61	2.60	2.59
1,377.000	2.58	2.57	2.56	2.55	2.54
1,392.000	2.53	2.52	2.51	2.50	2.49
1,407.000	2.48	2.47	2.46	2.46	2.45
1,422.000	2.44	2.43	2.42	2.41	2.40

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA A2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,437.000	2.39	2.37	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	93.54 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	753.000 min
Flow (Peak Interpolated Output)	93.10 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.6280 in
Runoff Volume (Pervious)	621,971.696 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	621,937.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
510.000	0.00	0.00	0.01	0.01	0.02
525.000	0.03	0.04	0.05	0.06	0.06
540.000	0.06	0.06	0.06	0.05	0.05
555.000	0.04	0.04	0.07	0.10	0.18
570.000	0.27	0.43	0.58	0.73	0.87
585.000	0.94	1.01	1.00	0.98	1.01
600.000	1.06	1.17	1.29	1.54	1.82
615.000	2.06	2.29	2.36	2.39	2.34
630.000	2.25	2.23	2.22	2.30	2.41
645.000	2.68	3.02	3.30	3.56	3.78
660.000	3.98	4.19	4.43	4.76	5.17
675.000	5.56	5.94	6.29	6.62	7.07
690.000	7.62	8.20	8.82	9.43	10.04
705.000	10.95	12.20	14.02	16.53	19.90
720.000	24.41	30.08	37.40	45.42	54.53
735.000	63.46	72.09	79.75	85.56	90.14
750.000	92.14	93.10	91.63	89.32	84.88
765.000	80.14	74.52	69.05	64.10	59.28
780.000	54.96	50.83	47.50	44.25	41.31
795.000	38.40	35.65	32.98	30.85	28.83
810.000	27.86	26.88	25.75	24.59	22.99
825.000	21.40	20.20	19.02	18.42	17.82
840.000	17.86	17.90	17.98	18.05	17.48
855.000	16.87	15.94	14.97	14.28	13.61
870.000	13.35	13.14	13.71	14.39	14.88
885.000	15.34	15.13	14.78	13.94	12.97
900.000	11.76	10.47	9.50	8.63	8.52
915.000	8.67	9.67	11.01	12.21	13.36
930.000	13.79	13.87	13.48	12.84	11.92
945.000	10.85	9.57	8.14	6.89	5.78
960.000	5.17	4.95	5.22	5.91	7.06
975.000	8.66	10.17	11.59	12.52	12.89
990.000	12.89	12.43	11.76	10.81	9.70
1,005.000	8.35	7.09	5.97	4.99	4.25

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA B
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,020.000	3.59	3.10	2.64	2.25	1.98
1,035.000	1.97	2.16	2.86	3.76	5.26
1,050.000	6.83	8.65	10.30	11.35	12.20
1,065.000	12.20	12.10	11.42	10.68	9.56
1,080.000	8.42	7.19	6.00	5.11	4.25
1,095.000	3.66	3.09	2.66	2.24	1.92
1,110.000	1.60	1.38	1.16	1.00	0.84
1,125.000	0.72	0.60	0.52	0.44	0.38
1,140.000	0.32	0.42	0.55	1.24	2.00
1,155.000	3.40	4.91	6.83	8.84	10.28
1,170.000	11.61	12.02	12.21	11.81	11.23
1,185.000	10.30	9.25	8.04	6.77	5.73
1,200.000	4.78	4.06	3.45	2.96	2.53
1,215.000	2.16	1.82	1.55	1.32	1.13
1,230.000	0.96	0.81	0.69	0.59	0.50
1,245.000	0.43	0.36	0.31	0.26	0.22
1,260.000	0.19	0.16	0.14	0.12	0.10
1,275.000	0.09	0.08	0.32	0.93	1.80
1,290.000	3.09	4.70	6.87	8.87	10.55
1,305.000	11.83	12.27	12.44	12.01	11.43
1,320.000	10.50	9.46	8.09	6.79	5.74
1,335.000	4.76	4.07	3.43	2.98	2.54
1,350.000	2.17	1.81	1.56	1.32	1.14
1,365.000	0.95	0.82	0.68	0.59	0.50
1,380.000	0.43	0.36	0.31	0.26	0.23
1,395.000	0.19	0.16	0.14	0.12	0.10
1,410.000	0.09	0.08	0.06	0.05	0.04
1,425.000	0.03	0.01	0.00	0.00	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	282.49 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	280.93 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.4806 in
Runoff Volume (Pervious)	1,711,839.703 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,700,726.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
405.000	0.00	0.00	0.00	0.01	0.01
420.000	0.02	0.03	0.04	0.06	0.09
435.000	0.11	0.15	0.18	0.22	0.27
450.000	0.31	0.36	0.42	0.47	0.53
465.000	0.59	0.65	0.71	0.77	0.84
480.000	0.91	0.98	1.05	1.12	1.20
495.000	1.28	1.35	1.43	1.52	1.60
510.000	1.69	1.77	1.86	1.96	2.05
525.000	2.15	2.24	2.34	2.45	2.55
540.000	2.66	2.78	2.91	3.05	3.20
555.000	3.39	3.58	3.81	4.03	4.29
570.000	4.54	4.81	5.08	5.35	5.63
585.000	5.90	6.18	6.46	6.74	7.03
600.000	7.33	7.63	7.93	8.25	8.58
615.000	8.92	9.26	9.62	9.99	10.39
630.000	10.79	11.24	11.70	12.24	12.81
645.000	13.48	14.19	15.00	15.85	16.78
660.000	17.74	18.79	19.88	21.08	22.34
675.000	23.72	25.20	26.81	28.52	30.39
690.000	32.39	34.60	36.99	39.67	42.65
705.000	46.17	50.29	55.44	61.85	70.95
720.000	83.61	99.72	120.80	144.81	173.41
735.000	201.44	228.48	251.62	267.34	278.70
750.000	280.93	279.95	271.51	261.04	245.31
765.000	229.18	211.91	195.19	180.25	165.95
780.000	153.98	142.42	132.59	123.06	115.00
795.000	107.14	100.49	93.99	88.47	83.05
810.000	78.49	73.99	70.17	66.39	63.12
825.000	59.86	56.98	54.11	51.60	49.08
840.000	46.91	44.74	42.89	41.05	39.48
855.000	37.92	36.60	35.29	34.17	33.06
870.000	32.15	31.26	30.51	29.78	29.14
885.000	28.51	27.95	27.41	26.91	26.42
900.000	25.95	25.49	25.02	24.54	24.01

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
915.000	23.46	22.86	22.24	21.61	20.97
930.000	20.35	19.74	19.17	18.63	18.14
945.000	17.68	17.28	16.91	16.57	16.26
960.000	15.98	15.71	15.46	15.23	15.01
975.000	14.80	14.61	14.42	14.25	14.08
990.000	13.91	13.76	13.61	13.46	13.32
1,005.000	13.19	13.06	12.93	12.80	12.68
1,020.000	12.56	12.45	12.33	12.22	12.11
1,035.000	12.01	11.91	11.81	11.71	11.62
1,050.000	11.52	11.43	11.34	11.25	11.16
1,065.000	11.08	10.99	10.91	10.83	10.75
1,080.000	10.67	10.58	10.50	10.40	10.30
1,095.000	10.17	10.05	9.90	9.76	9.61
1,110.000	9.46	9.32	9.18	9.05	8.92
1,125.000	8.82	8.71	8.62	8.54	8.46
1,140.000	8.38	8.31	8.24	8.18	8.12
1,155.000	8.06	8.01	7.95	7.90	7.85
1,170.000	7.80	7.75	7.70	7.65	7.60
1,185.000	7.56	7.51	7.47	7.42	7.38
1,200.000	7.34	7.30	7.26	7.22	7.19
1,215.000	7.15	7.11	7.07	7.04	7.00
1,230.000	6.96	6.93	6.89	6.86	6.82
1,245.000	6.79	6.75	6.72	6.68	6.65
1,260.000	6.62	6.59	6.56	6.53	6.50
1,275.000	6.47	6.44	6.41	6.38	6.35
1,290.000	6.32	6.29	6.26	6.23	6.21
1,305.000	6.18	6.15	6.13	6.10	6.08
1,320.000	6.05	6.02	6.00	5.97	5.95
1,335.000	5.92	5.90	5.87	5.85	5.82
1,350.000	5.80	5.78	5.75	5.73	5.71
1,365.000	5.68	5.66	5.64	5.62	5.59
1,380.000	5.57	5.55	5.53	5.51	5.49
1,395.000	5.46	5.44	5.42	5.41	5.39
1,410.000	5.37	5.35	5.33	5.31	5.29
1,425.000	5.27	5.25	5.23	5.21	5.19
1,440.000	5.16	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	375.16 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	373.90 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.3008 in
Runoff Volume (Pervious)	2,407,262.266 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,392,974.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
351.000	0.00	0.00	0.00	0.01	0.02
366.000	0.03	0.04	0.06	0.08	0.12
381.000	0.15	0.20	0.24	0.30	0.35
396.000	0.42	0.48	0.55	0.62	0.70
411.000	0.77	0.85	0.93	1.02	1.10
426.000	1.19	1.27	1.36	1.46	1.55
441.000	1.64	1.74	1.84	1.94	2.04
456.000	2.14	2.25	2.35	2.46	2.57
471.000	2.68	2.80	2.91	3.03	3.15
486.000	3.27	3.40	3.52	3.65	3.78
501.000	3.91	4.05	4.19	4.33	4.47
516.000	4.62	4.77	4.93	5.08	5.24
531.000	5.41	5.57	5.75	5.92	6.12
546.000	6.33	6.59	6.85	7.20	7.56
561.000	7.99	8.43	8.92	9.40	9.91
576.000	10.41	10.92	11.43	11.92	12.42
591.000	12.90	13.39	13.88	14.38	14.89
606.000	15.40	15.93	16.46	17.01	17.57
621.000	18.16	18.76	19.39	20.04	20.77
636.000	21.53	22.43	23.38	24.53	25.76
651.000	27.15	28.64	30.24	31.90	33.68
666.000	35.52	37.49	39.54	41.75	44.07
681.000	46.56	49.19	52.01	55.00	58.22
696.000	61.66	65.42	69.50	74.22	79.64
711.000	86.36	94.67	106.51	122.98	143.91
726.000	171.27	202.34	239.24	275.25	309.70
741.000	338.98	358.38	372.11	373.90	371.58
756.000	359.66	345.20	324.16	302.74	280.19
771.000	258.44	239.24	220.90	205.67	190.98
786.000	178.54	166.47	156.27	146.32	137.85
801.000	129.55	122.48	115.53	109.66	103.85
816.000	98.86	93.91	89.53	85.16	81.19
831.000	77.24	73.69	70.14	67.03	63.92
846.000	61.25	58.59	56.33	54.08	52.19

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
861.000	50.31	48.71	47.14	45.84	44.58
876.000	43.51	42.47	41.56	40.67	39.88
891.000	39.10	38.40	37.71	37.04	36.39
906.000	35.70	35.01	34.22	33.40	32.49
921.000	31.54	30.56	29.57	28.61	27.67
936.000	26.79	25.95	25.20	24.51	23.90
951.000	23.34	22.84	22.38	21.95	21.56
966.000	21.20	20.86	20.55	20.25	19.97
981.000	19.71	19.45	19.21	18.98	18.77
996.000	18.56	18.35	18.16	17.97	17.79
1,011.000	17.62	17.44	17.28	17.12	16.96
1,026.000	16.81	16.66	16.51	16.37	16.23
1,041.000	16.09	15.96	15.83	15.70	15.58
1,056.000	15.45	15.33	15.21	15.10	14.98
1,071.000	14.87	14.76	14.65	14.54	14.42
1,086.000	14.30	14.15	14.00	13.80	13.60
1,101.000	13.36	13.13	12.88	12.64	12.41
1,116.000	12.17	11.97	11.76	11.60	11.43
1,131.000	11.29	11.16	11.04	10.92	10.82
1,146.000	10.72	10.62	10.53	10.45	10.37
1,161.000	10.30	10.22	10.15	10.08	10.01
1,176.000	9.95	9.89	9.82	9.76	9.70
1,191.000	9.65	9.59	9.54	9.48	9.43
1,206.000	9.38	9.33	9.28	9.23	9.18
1,221.000	9.13	9.08	9.03	8.98	8.94
1,236.000	8.89	8.85	8.80	8.75	8.71
1,251.000	8.67	8.62	8.58	8.54	8.50
1,266.000	8.46	8.42	8.38	8.34	8.30
1,281.000	8.26	8.22	8.18	8.15	8.11
1,296.000	8.07	8.04	8.00	7.96	7.93
1,311.000	7.89	7.86	7.82	7.79	7.75
1,326.000	7.72	7.69	7.65	7.62	7.59
1,341.000	7.56	7.53	7.50	7.47	7.43
1,356.000	7.41	7.38	7.35	7.32	7.29
1,371.000	7.26	7.23	7.21	7.18	7.15
1,386.000	7.12	7.09	7.06	7.03	7.01
1,401.000	6.98	6.95	6.92	6.90	6.87
1,416.000	6.85	6.82	6.80	6.77	6.75
1,431.000	6.72	6.70	6.68	6.63	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres
Computational Time Increment	6.065 min
Time to Peak (Computed)	752.098 min
Flow (Peak, Computed)	530.55 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	750.000 min
Flow (Peak Interpolated Output)	530.04 ft ³ /s
Drainage Area	
SCS CN (Composite)	81.000
Area (User Defined)	105.250 acres
Maximum Retention (Pervious)	2.3457 in
Maximum Retention (Pervious, 20 percent)	0.4691 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.8052 in
Runoff Volume (Pervious)	3,746,162.365 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	3,726,557.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	45.490 min
Computational Time Increment	6.065 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	157.29 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	30.327 min
Unit receding limb, Tr	121.306 min
Total unit time, Tb	151.633 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	45.490 min
Area (User Defined)	105.250 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
279.000	0.00	0.00	0.00	0.01	0.02
294.000	0.03	0.04	0.07	0.09	0.13
309.000	0.16	0.21	0.25	0.31	0.36
324.000	0.42	0.48	0.55	0.62	0.69
339.000	0.76	0.83	0.91	0.99	1.06
354.000	1.14	1.22	1.31	1.40	1.49
369.000	1.59	1.70	1.82	1.95	2.10
384.000	2.25	2.41	2.57	2.74	2.91
399.000	3.08	3.25	3.42	3.60	3.77
414.000	3.94	4.11	4.28	4.45	4.62
429.000	4.80	4.97	5.15	5.33	5.51
444.000	5.69	5.87	6.06	6.25	6.44
459.000	6.63	6.83	7.02	7.22	7.43
474.000	7.64	7.85	8.06	8.28	8.49
489.000	8.72	8.94	9.18	9.41	9.65
504.000	9.89	10.14	10.39	10.65	10.91
519.000	11.17	11.44	11.72	12.00	12.29
534.000	12.58	12.89	13.19	13.57	13.96
549.000	14.49	15.03	15.82	16.60	17.61
564.000	18.63	19.74	20.86	22.01	23.15
579.000	24.27	25.39	26.43	27.47	28.47
594.000	29.46	30.43	31.40	32.38	33.36
609.000	34.35	35.35	36.38	37.42	38.49
624.000	39.57	40.72	41.88	43.21	44.61
639.000	46.29	48.10	50.31	52.70	55.44
654.000	58.35	61.47	64.69	68.08	71.56
669.000	75.19	78.94	82.87	86.94	91.24
684.000	95.71	100.42	105.32	110.42	115.72
699.000	121.23	126.96	133.28	140.26	148.88
714.000	159.47	174.92	196.77	224.77	261.64
729.000	303.56	353.47	402.00	448.16	487.05
744.000	512.11	529.32	530.04	525.19	507.39
759.000	486.28	456.64	426.73	396.02	366.55
774.000	341.11	316.89	297.13	278.08	262.05

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
789.000	246.50	233.28	220.36	209.17	198.20
804.000	188.68	179.32	171.27	163.31	156.31
819.000	149.35	142.96	136.59	130.58	124.58
834.000	119.03	113.48	108.52	103.56	99.26
849.000	94.97	91.32	87.71	84.68	81.68
864.000	79.15	76.66	74.59	72.59	70.88
879.000	69.23	67.78	66.37	65.10	63.87
894.000	62.74	61.64	60.58	59.51	58.37
909.000	57.20	55.81	54.36	52.69	50.94
924.000	49.12	47.26	45.46	43.68	42.04
939.000	40.47	39.08	37.79	36.68	35.67
954.000	34.78	33.97	33.24	32.57	31.96
969.000	31.39	30.87	30.38	29.92	29.49
984.000	29.09	28.71	28.35	28.01	27.68
999.000	27.37	27.07	26.78	26.50	26.23
1,014.000	25.97	25.72	25.47	25.23	24.99
1,029.000	24.77	24.54	24.33	24.12	23.92
1,044.000	23.72	23.52	23.33	23.15	22.96
1,059.000	22.78	22.60	22.43	22.26	22.09
1,074.000	21.93	21.77	21.61	21.43	21.24
1,089.000	21.00	20.74	20.39	20.03	19.60
1,104.000	19.17	18.72	18.27	17.85	17.43
1,119.000	17.06	16.70	16.41	16.12	15.88
1,134.000	15.65	15.46	15.26	15.10	14.93
1,149.000	14.79	14.65	14.52	14.40	14.28
1,164.000	14.17	14.06	13.96	13.86	13.77
1,179.000	13.67	13.58	13.50	13.41	13.33
1,194.000	13.25	13.17	13.09	13.02	12.94
1,209.000	12.87	12.79	12.72	12.65	12.58
1,224.000	12.51	12.45	12.38	12.32	12.25
1,239.000	12.19	12.13	12.07	12.00	11.94
1,254.000	11.88	11.82	11.76	11.70	11.65
1,269.000	11.59	11.53	11.48	11.43	11.37
1,284.000	11.32	11.27	11.22	11.17	11.11
1,299.000	11.06	11.01	10.96	10.91	10.86
1,314.000	10.81	10.76	10.71	10.66	10.62
1,329.000	10.57	10.52	10.48	10.43	10.39
1,344.000	10.34	10.30	10.26	10.21	10.17
1,359.000	10.13	10.09	10.05	10.01	9.97
1,374.000	9.93	9.89	9.85	9.81	9.77
1,389.000	9.73	9.69	9.65	9.62	9.58
1,404.000	9.54	9.50	9.47	9.43	9.40
1,419.000	9.36	9.33	9.29	9.26	9.23

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,434.000	9.19	9.16	9.10	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres
Computational Time Increment	7.259 min
Time to Peak (Computed)	754.951 min
Flow (Peak, Computed)	79.89 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	79.66 ft ³ /s
Drainage Area	
SCS CN (Composite)	86.000
Area (User Defined)	80.560 acres
Maximum Retention (Pervious)	1.6279 in
Maximum Retention (Pervious, 20 percent)	0.3256 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.0101 in
Runoff Volume (Pervious)	587,813.270 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	587,792.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	54.444 min
Computational Time Increment	7.259 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	100.59 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	36.296 min
Unit receding limb, Tr	145.183 min
Total unit time, Tb	181.479 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Storm Event	2 Yr Storm
Return Event	2 years
Duration	1,440.000 min
Depth	3.4000 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
450.000	0.00	0.01	0.03	0.05	0.08
465.000	0.11	0.16	0.20	0.24	0.28
480.000	0.31	0.32	0.33	0.32	0.31
495.000	0.30	0.27	0.25	0.22	0.19
510.000	0.20	0.24	0.29	0.39	0.49
525.000	0.66	0.85	1.03	1.17	1.32
540.000	1.36	1.39	1.38	1.34	1.29
555.000	1.20	1.12	1.07	1.03	1.04
570.000	1.14	1.24	1.48	1.75	1.99
585.000	2.20	2.42	2.45	2.49	2.53
600.000	2.57	2.63	2.74	2.86	3.05
615.000	3.27	3.47	3.63	3.79	3.82
630.000	3.83	3.86	3.90	3.94	4.07
645.000	4.21	4.37	4.56	4.77	5.03
660.000	5.29	5.55	5.81	6.12	6.49
675.000	6.87	7.20	7.54	7.78	7.95
690.000	8.17	8.67	9.18	9.84	10.56
705.000	11.43	12.63	13.84	16.09	18.51
720.000	21.83	26.25	30.72	36.88	43.05
735.000	49.68	56.60	63.20	68.36	73.52
750.000	76.35	78.49	79.66	79.00	78.34
765.000	75.38	72.25	68.69	64.72	60.78
780.000	57.08	53.38	50.06	46.93	43.91
795.000	41.24	38.56	36.23	33.97	31.92
810.000	30.18	28.45	27.05	25.65	24.22
825.000	22.76	21.35	20.29	19.22	18.54
840.000	18.01	17.54	17.19	16.83	16.25
855.000	15.63	15.03	14.46	13.90	13.58
870.000	13.26	13.17	13.20	13.24	13.25
885.000	13.27	13.02	12.71	12.28	11.66
900.000	11.03	10.39	9.76	9.23	8.80
915.000	8.44	8.61	8.78	9.21	9.75
930.000	10.21	10.45	10.69	10.49	10.21
945.000	9.80	9.21	8.62	7.82	7.03

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA C
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
960.000	6.44	5.99	5.65	5.83	6.00
975.000	6.70	7.57	8.38	9.07	9.77
990.000	9.86	9.90	9.72	9.33	8.91
1,005.000	8.26	7.62	6.83	5.97	5.16
1,020.000	4.49	3.83	3.36	2.92	2.63
1,035.000	2.56	2.49	2.85	3.21	3.89
1,050.000	4.80	5.71	6.68	7.65	8.24
1,065.000	8.69	8.97	8.85	8.74	8.30
1,080.000	7.83	7.24	6.51	5.78	5.08
1,095.000	4.37	3.80	3.31	2.84	2.52
1,110.000	2.19	1.93	1.69	1.47	1.29
1,125.000	1.11	0.98	0.85	0.74	0.64
1,140.000	0.56	0.64	0.72	1.06	1.54
1,155.000	2.13	3.06	3.98	5.08	6.22
1,170.000	7.16	7.84	8.51	8.56	8.60
1,185.000	8.40	8.03	7.60	6.95	6.30
1,200.000	5.59	4.86	4.19	3.65	3.11
1,215.000	2.74	2.39	2.09	1.84	1.60
1,230.000	1.41	1.22	1.07	0.94	0.81
1,245.000	0.72	0.62	0.54	0.47	0.41
1,260.000	0.36	0.32	0.28	0.24	0.21
1,275.000	0.19	0.21	0.54	0.88	1.51
1,290.000	2.25	3.14	4.40	5.65	6.67
1,305.000	7.66	8.32	8.59	8.84	8.59
1,320.000	8.34	7.91	7.37	6.77	5.98
1,335.000	5.18	4.52	3.89	3.34	2.93
1,350.000	2.53	2.25	1.98	1.73	1.51
1,365.000	1.30	1.15	1.00	0.88	0.77
1,380.000	0.67	0.59	0.50	0.44	0.39
1,395.000	0.34	0.30	0.26	0.23	0.20
1,410.000	0.17	0.15	0.13	0.12	0.10
1,425.000	0.09	0.08	0.07	0.06	0.05
1,440.000	0.05	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres
Computational Time Increment	7.259 min
Time to Peak (Computed)	754.951 min
Flow (Peak, Computed)	215.92 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	214.84 ft ³ /s
Drainage Area	
SCS CN (Composite)	86.000
Area (User Defined)	80.560 acres
Maximum Retention (Pervious)	1.6279 in
Maximum Retention (Pervious, 20 percent)	0.3256 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.0299 in
Runoff Volume (Pervious)	1,470,894.140 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	1,460,300.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	54.444 min
Computational Time Increment	7.259 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	100.59 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	36.296 min
Unit receding limb, Tr	145.183 min
Total unit time, Tb	181.479 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Storm Event	10 Yr Storm
Return Event	10 years
Duration	1,440.000 min
Depth	6.6501 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
318.000	0.00	0.00	0.00	0.01	0.01
333.000	0.01	0.02	0.03	0.04	0.05
348.000	0.07	0.08	0.10	0.12	0.15
363.000	0.17	0.20	0.22	0.26	0.29
378.000	0.32	0.36	0.40	0.44	0.48
393.000	0.52	0.57	0.61	0.66	0.71
408.000	0.75	0.80	0.85	0.91	0.96
423.000	1.01	1.06	1.12	1.17	1.23
438.000	1.28	1.34	1.40	1.46	1.52
453.000	1.58	1.64	1.70	1.77	1.83
468.000	1.89	1.96	2.03	2.10	2.17
483.000	2.24	2.31	2.38	2.46	2.53
498.000	2.61	2.69	2.77	2.85	2.94
513.000	3.02	3.11	3.20	3.28	3.38
528.000	3.47	3.57	3.67	3.76	3.87
543.000	3.99	4.11	4.24	4.38	4.55
558.000	4.73	4.93	5.14	5.37	5.61
573.000	5.86	6.12	6.39	6.66	6.93
588.000	7.20	7.48	7.75	8.03	8.31
603.000	8.58	8.87	9.16	9.45	9.75
618.000	10.06	10.38	10.70	11.05	11.40
633.000	11.78	12.19	12.60	13.11	13.61
648.000	14.20	14.84	15.50	16.27	17.03
663.000	17.90	18.79	19.75	20.80	21.85
678.000	23.09	24.34	25.72	27.20	28.71
693.000	30.49	32.27	34.37	36.61	39.12
708.000	42.33	45.53	50.84	56.47	64.41
723.000	75.22	86.18	102.43	118.68	136.52
738.000	155.39	173.31	187.18	201.04	207.96
753.000	212.78	214.84	211.76	208.67	200.41
768.000	191.76	181.64	170.11	158.67	148.13
783.000	137.59	128.43	119.95	111.88	105.05
798.000	98.22	92.33	86.63	81.38	76.75
813.000	72.13	68.30	64.47	61.00	57.80

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA C
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
828.000	54.68	51.98	49.28	46.89	44.61
843.000	42.45	40.54	38.62	37.00	35.42
858.000	33.97	32.68	31.40	30.35	29.31
873.000	28.37	27.51	26.67	25.93	25.19
888.000	24.52	23.87	23.27	22.72	22.18
903.000	21.71	21.26	20.81	20.37	19.92
918.000	19.44	18.96	18.47	17.97	17.47
933.000	16.98	16.49	16.05	15.60	15.19
948.000	14.80	14.42	14.11	13.80	13.52
963.000	13.26	13.01	12.80	12.58	12.38
978.000	12.19	12.01	11.85	11.68	11.53
993.000	11.38	11.24	11.11	10.98	10.86
1,008.000	10.74	10.62	10.51	10.40	10.30
1,023.000	10.20	10.10	10.00	9.91	9.81
1,038.000	9.72	9.64	9.55	9.47	9.39
1,053.000	9.31	9.23	9.15	9.08	9.01
1,068.000	8.93	8.87	8.80	8.73	8.66
1,083.000	8.59	8.52	8.45	8.37	8.29
1,098.000	8.19	8.10	8.00	7.89	7.78
1,113.000	7.67	7.56	7.46	7.36	7.26
1,128.000	7.17	7.08	7.00	6.93	6.86
1,143.000	6.79	6.73	6.67	6.61	6.56
1,158.000	6.51	6.46	6.41	6.36	6.32
1,173.000	6.27	6.23	6.19	6.15	6.11
1,188.000	6.07	6.04	6.00	5.96	5.93
1,203.000	5.89	5.86	5.83	5.80	5.76
1,218.000	5.73	5.70	5.67	5.64	5.61
1,233.000	5.58	5.55	5.52	5.49	5.46
1,248.000	5.44	5.41	5.38	5.35	5.33
1,263.000	5.30	5.27	5.25	5.22	5.20
1,278.000	5.18	5.15	5.13	5.10	5.08
1,293.000	5.06	5.03	5.01	4.99	4.97
1,308.000	4.94	4.92	4.90	4.88	4.86
1,323.000	4.84	4.82	4.80	4.77	4.75
1,338.000	4.73	4.71	4.69	4.67	4.65
1,353.000	4.63	4.61	4.59	4.57	4.55
1,368.000	4.54	4.52	4.50	4.48	4.46
1,383.000	4.45	4.43	4.41	4.40	4.38
1,398.000	4.36	4.35	4.33	4.32	4.30
1,413.000	4.28	4.27	4.25	4.24	4.22
1,428.000	4.20	4.19	4.17	4.16	4.13

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres
Computational Time Increment	7.259 min
Time to Peak (Computed)	754.951 min
Flow (Peak, Computed)	279.98 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	278.46 ft ³ /s
Drainage Area	
SCS CN (Composite)	86.000
Area (User Defined)	80.560 acres
Maximum Retention (Pervious)	1.6279 in
Maximum Retention (Pervious, 20 percent)	0.3256 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.9043 in
Runoff Volume (Pervious)	2,019,045.818 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	2,005,601.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	54.444 min
Computational Time Increment	7.259 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	100.59 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	36.296 min
Unit receding limb, Tr	145.183 min
Total unit time, Tb	181.479 min

Subsection: Unit Hydrograph (Hydrograph Table)
Label: PROP AREA C
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Storm Event	25 Yr Storm
Return Event	25 years
Duration	1,440.000 min
Depth	8.5899 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
264.000	0.00	0.00	0.00	0.00	0.01
279.000	0.01	0.02	0.03	0.04	0.05
294.000	0.07	0.09	0.11	0.14	0.16
309.000	0.19	0.22	0.26	0.29	0.33
324.000	0.36	0.40	0.44	0.48	0.52
339.000	0.57	0.61	0.65	0.70	0.75
354.000	0.79	0.84	0.89	0.94	0.99
369.000	1.05	1.10	1.16	1.23	1.30
384.000	1.37	1.45	1.53	1.61	1.69
399.000	1.77	1.86	1.95	2.03	2.12
414.000	2.21	2.30	2.38	2.47	2.56
429.000	2.65	2.74	2.83	2.92	3.01
444.000	3.11	3.20	3.29	3.39	3.49
459.000	3.58	3.68	3.78	3.89	3.99
474.000	4.10	4.20	4.31	4.42	4.53
489.000	4.64	4.76	4.88	4.99	5.11
504.000	5.24	5.36	5.49	5.62	5.75
519.000	5.89	6.02	6.16	6.30	6.45
534.000	6.60	6.75	6.91	7.08	7.28
549.000	7.49	7.71	8.00	8.29	8.64
564.000	9.01	9.39	9.82	10.25	10.70
579.000	11.16	11.62	12.08	12.54	12.99
594.000	13.45	13.90	14.34	14.79	15.24
609.000	15.69	16.15	16.62	17.09	17.58
624.000	18.08	18.60	19.14	19.72	20.35
639.000	20.99	21.79	22.59	23.53	24.56
654.000	25.63	26.87	28.10	29.50	30.92
669.000	32.43	34.06	35.69	37.56	39.44
684.000	41.47	43.62	45.81	48.30	50.79
699.000	53.64	56.63	59.94	64.03	68.13
714.000	74.80	81.87	91.86	105.45	119.23
729.000	139.63	160.03	182.37	205.94	228.29
744.000	245.39	262.48	270.74	276.34	278.46
759.000	274.10	269.75	259.03	247.83	234.89

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA C
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
774.000	220.28	205.80	192.59	179.38	167.96
789.000	157.42	147.40	138.93	130.47	123.15
804.000	116.05	109.50	103.70	97.91	93.05
819.000	88.19	83.75	79.62	75.57	72.00
834.000	68.43	65.22	62.14	59.19	56.57
849.000	53.94	51.69	49.49	47.48	45.68
864.000	43.91	42.45	41.00	39.71	38.51
879.000	37.35	36.33	35.30	34.39	33.50
894.000	32.66	31.91	31.16	30.52	29.89
909.000	29.25	28.62	27.98	27.28	26.58
924.000	25.84	25.09	24.34	23.61	22.87
939.000	22.20	21.53	20.90	20.33	19.75
954.000	19.29	18.83	18.42	18.04	17.68
969.000	17.36	17.04	16.76	16.49	16.23
984.000	16.00	15.76	15.56	15.35	15.15
999.000	14.97	14.79	14.62	14.46	14.30
1,014.000	14.15	14.00	13.86	13.72	13.58
1,029.000	13.45	13.32	13.20	13.08	12.96
1,044.000	12.85	12.74	12.63	12.52	12.42
1,059.000	12.31	12.21	12.12	12.02	11.93
1,074.000	11.83	11.74	11.65	11.56	11.46
1,089.000	11.35	11.23	11.11	10.97	10.82
1,104.000	10.66	10.49	10.32	10.15	9.98
1,119.000	9.81	9.65	9.49	9.35	9.21
1,134.000	9.09	8.97	8.86	8.77	8.67
1,149.000	8.59	8.50	8.43	8.35	8.28
1,164.000	8.22	8.15	8.09	8.03	7.97
1,179.000	7.92	7.86	7.81	7.76	7.71
1,194.000	7.66	7.61	7.57	7.52	7.48
1,209.000	7.44	7.39	7.35	7.31	7.27
1,224.000	7.23	7.19	7.15	7.11	7.07
1,239.000	7.04	7.00	6.96	6.93	6.89
1,254.000	6.86	6.82	6.79	6.75	6.72
1,269.000	6.69	6.66	6.62	6.59	6.56
1,284.000	6.53	6.50	6.47	6.44	6.41
1,299.000	6.38	6.35	6.32	6.29	6.26
1,314.000	6.24	6.21	6.18	6.15	6.13
1,329.000	6.10	6.07	6.05	6.02	5.99
1,344.000	5.97	5.94	5.92	5.89	5.87
1,359.000	5.84	5.82	5.80	5.77	5.75
1,374.000	5.73	5.71	5.68	5.66	5.64
1,389.000	5.62	5.59	5.57	5.55	5.53
1,404.000	5.51	5.48	5.46	5.44	5.42

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA C
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,419.000	5.40	5.38	5.36	5.34	5.32
1,434.000	5.30	5.28	5.24	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres
Computational Time Increment	7.259 min
Time to Peak (Computed)	754.951 min
Flow (Peak, Computed)	387.16 ft ³ /s
Output Increment	3.000 min
Time to Flow (Peak Interpolated Output)	756.000 min
Flow (Peak Interpolated Output)	384.90 ft ³ /s
Drainage Area	
SCS CN (Composite)	86.000
Area (User Defined)	80.560 acres
Maximum Retention (Pervious)	1.6279 in
Maximum Retention (Pervious, 20 percent)	0.3256 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	10.4723 in
Runoff Volume (Pervious)	3,062,457.175 ft ³
Hydrograph Volume (Area under Hydrograph curve)	
Volume	3,044,204.000 ft ³
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	54.444 min
Computational Time Increment	7.259 min
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	100.59 ft ³ /s

Subsection: Unit Hydrograph Summary
Label: PROP AREA C
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

SCS Unit Hydrograph Parameters	
Unit peak time, Tp	36.296 min
Unit receding limb, Tr	145.183 min
Total unit time, Tb	181.479 min

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA C
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Storm Event	100 Yr Storm
Return Event	100 years
Duration	1,440.000 min
Depth	12.2300 in
Time of Concentration (Composite)	54.444 min
Area (User Defined)	80.560 acres

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
207.000	0.00	0.00	0.00	0.01	0.01
222.000	0.02	0.03	0.04	0.06	0.08
237.000	0.11	0.13	0.17	0.20	0.24
252.000	0.28	0.33	0.38	0.43	0.48
267.000	0.53	0.59	0.64	0.70	0.76
282.000	0.82	0.88	0.95	1.01	1.07
297.000	1.14	1.21	1.27	1.34	1.41
312.000	1.48	1.55	1.62	1.69	1.76
327.000	1.83	1.90	1.97	2.05	2.12
342.000	2.20	2.27	2.35	2.43	2.50
357.000	2.58	2.66	2.75	2.84	2.93
372.000	3.04	3.16	3.28	3.42	3.56
387.000	3.72	3.89	4.05	4.23	4.40
402.000	4.58	4.75	4.93	5.10	5.27
417.000	5.44	5.60	5.77	5.93	6.09
432.000	6.25	6.41	6.57	6.74	6.90
447.000	7.06	7.23	7.39	7.56	7.73
462.000	7.90	8.07	8.24	8.41	8.59
477.000	8.77	8.95	9.13	9.32	9.51
492.000	9.70	9.89	10.09	10.28	10.49
507.000	10.69	10.90	11.12	11.33	11.55
522.000	11.77	12.00	12.24	12.47	12.71
537.000	12.96	13.24	13.53	13.87	14.27
552.000	14.68	15.25	15.82	16.53	17.30
567.000	18.10	19.00	19.89	20.82	21.77
582.000	22.70	23.63	24.57	25.45	26.34
597.000	27.19	28.01	28.83	29.64	30.44
612.000	31.25	32.06	32.89	33.75	34.61
627.000	35.54	36.48	37.50	38.61	39.74
642.000	41.17	42.59	44.31	46.18	48.13
657.000	50.38	52.63	55.12	57.67	60.33
672.000	63.16	65.99	69.14	72.30	75.64
687.000	79.13	82.65	86.45	90.26	94.34
702.000	98.54	103.05	108.32	113.58	122.03

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA C
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
717.000	130.95	143.75	161.32	179.15	205.83
732.000	232.51	261.73	292.59	321.78	343.86
747.000	365.93	376.16	382.83	384.90	378.44
762.000	371.97	357.38	342.19	324.91	305.64
777.000	286.57	269.49	252.42	237.78	224.34
792.000	211.58	200.82	190.06	180.65	171.52
807.000	163.01	155.39	147.78	141.23	134.69
822.000	128.61	122.87	117.21	112.07	106.93
837.000	102.19	97.60	93.20	89.21	85.22
852.000	81.76	78.38	75.28	72.50	69.74
867.000	67.50	65.26	63.27	61.43	59.65
882.000	58.07	56.50	55.10	53.74	52.46
897.000	51.31	50.17	49.15	48.15	47.12
912.000	46.08	45.02	43.79	42.57	41.25
927.000	39.90	38.55	37.21	35.87	34.64
942.000	33.42	32.28	31.24	30.20	29.38
957.000	28.56	27.84	27.19	26.56	26.02
972.000	25.49	25.02	24.57	24.15	23.77
987.000	23.39	23.05	22.72	22.42	22.13
1,002.000	21.84	21.58	21.32	21.08	20.85
1,017.000	20.62	20.40	20.19	19.99	19.79
1,032.000	19.60	19.41	19.23	19.06	18.88
1,047.000	18.72	18.55	18.39	18.24	18.08
1,062.000	17.93	17.79	17.65	17.51	17.37
1,077.000	17.24	17.10	16.96	16.80	16.65
1,092.000	16.45	16.25	16.01	15.75	15.48
1,107.000	15.18	14.87	14.57	14.26	13.97
1,122.000	13.69	13.41	13.17	12.93	12.71
1,137.000	12.52	12.33	12.18	12.02	11.88
1,152.000	11.75	11.63	11.52	11.40	11.30
1,167.000	11.20	11.11	11.02	10.93	10.85
1,182.000	10.77	10.69	10.62	10.55	10.48
1,197.000	10.41	10.34	10.28	10.21	10.15
1,212.000	10.09	10.03	9.97	9.92	9.86
1,227.000	9.80	9.75	9.69	9.64	9.59
1,242.000	9.54	9.49	9.44	9.39	9.34
1,257.000	9.29	9.25	9.20	9.15	9.11
1,272.000	9.06	9.02	8.98	8.93	8.89
1,287.000	8.85	8.81	8.76	8.72	8.68
1,302.000	8.64	8.60	8.56	8.53	8.49
1,317.000	8.45	8.41	8.37	8.33	8.30
1,332.000	8.26	8.22	8.19	8.15	8.12
1,347.000	8.08	8.05	8.01	7.98	7.95

Subsection: Unit Hydrograph (Hydrograph Table)
 Label: PROP AREA C
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,362.000	7.91	7.88	7.85	7.81	7.78
1,377.000	7.75	7.72	7.69	7.65	7.62
1,392.000	7.59	7.56	7.53	7.50	7.47
1,407.000	7.44	7.42	7.39	7.36	7.33
1,422.000	7.30	7.28	7.25	7.22	7.20
1,437.000	7.17	7.12	(N/A)	(N/A)	(N/A)

Subsection: Channel Equations
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

**Solution to Manning's Open Channel Flow Equation
 (Computed values are based on normal depth.)**

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Channel Equations
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Solution to Manning's Open Channel Flow Equation
(Computed values are based on normal depth.)

$$Q = (k/n) * A * (R^{**2/3}) * (S^{**1/2})$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Additional Output Variables:

$$Vel = Q/A$$

$$Hd = A/TpW$$

$$F = Vel / (g * Hd)^{**1/2}$$

where:	English Units	SI units
Q = Channel Flow	cfs	cms
k = Manning's Constant	1.485919	1.0
n = Manning's n	no units	no units
R = Hydraulic radius, A/WP	feet	meter
A = X-section flow area	ft^2	m^2
WP = Wetted perimeter	feet	meter
S = Slope	ft/ft	m/m

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.497 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	62.21 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.497 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	62.21 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	139.60 ft ³ /s	Time to Peak (In)	753.000 min
Flow (Peak Out)	138.01 ft ³ /s	Time to Peak (Out)	756.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	965,914.645 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	965,797.748 ft ³
Volume (Retained)	111.785 ft ³
Volume (Unrouted)	-5.112 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Peak Discharge	138.01 ft ³ /s
Time to Peak	756.000 min
Hydrograph Volume	965,797.584 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
519.000	0.00	0.00	0.00	0.01	0.01
534.000	0.01	0.01	0.02	0.02	0.05
549.000	0.09	0.10	0.10	0.09	0.09
564.000	0.12	0.19	0.30	0.47	0.69
579.000	0.92	1.15	1.33	1.45	1.52
594.000	1.53	1.55	1.61	1.75	1.98
609.000	2.30	2.68	3.06	3.37	3.57
624.000	3.65	3.63	3.54	3.44	3.43
639.000	3.54	3.79	4.16	4.56	4.96
654.000	5.29	5.58	5.85	6.16	6.55
669.000	7.07	7.68	8.27	8.81	9.30
684.000	9.77	10.29	10.95	11.71	12.56
699.000	13.46	14.50	15.70	17.22	19.24
714.000	22.10	26.21	31.73	39.10	48.36
729.000	59.48	72.01	85.23	98.48	110.47
744.000	120.73	128.85	134.34	137.40	138.01
759.000	136.54	133.02	127.86	121.57	114.32
774.000	106.94	99.78	93.04	86.87	81.12
789.000	75.65	70.36	65.30	60.58	56.28
804.000	52.44	49.31	46.93	44.91	43.05
819.000	41.05	38.76	36.25	33.80	31.63
834.000	30.09	29.23	28.98	29.13	29.21
849.000	29.02	28.44	27.36	25.93	24.40
864.000	23.04	22.14	21.78	21.97	22.57
879.000	23.22	23.74	23.98	23.75	23.07
894.000	21.98	20.60	19.06	17.41	15.89
909.000	14.65	13.92	13.91	14.62	16.04
924.000	17.76	19.25	20.41	20.93	20.74
939.000	20.01	18.88	17.55	16.09	14.55
954.000	12.89	11.20	9.79	8.78	8.41
969.000	8.82	10.00	11.84	13.92	15.87
984.000	17.42	18.39	18.80	18.61	17.95
999.000	16.99	15.81	14.49	13.03	11.52
1,014.000	10.02	8.60	7.35	6.26	5.34
1,029.000	4.58	3.95	3.45	3.30	3.67
1,044.000	4.66	6.29	8.46	10.95	13.39
1,059.000	15.43	16.96	17.76	17.85	17.43
1,074.000	16.61	15.59	14.40	13.07	11.60
1,089.000	10.07	8.64	7.35	6.25	5.34

Subsection: Channel Routing (Hydrograph output)
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,104.000	4.58	3.95	3.41	2.93	2.51
1,119.000	2.15	1.84	1.58	1.35	1.16
1,134.000	0.99	0.84	0.73	0.70	0.85
1,149.000	1.34	2.33	3.90	6.09	8.68
1,164.000	11.32	13.77	15.64	16.83	17.37
1,179.000	17.25	16.70	15.83	14.76	13.51
1,194.000	12.12	10.68	9.23	7.90	6.74
1,209.000	5.74	4.92	4.22	3.64	3.13
1,224.000	2.69	2.31	1.98	1.71	1.47
1,239.000	1.26	1.08	0.93	0.79	0.68
1,254.000	0.59	0.51	0.44	0.38	0.33
1,269.000	0.28	0.24	0.21	0.19	0.31
1,284.000	0.82	1.84	3.47	5.68	8.37
1,299.000	11.26	13.81	15.84	17.17	17.68
1,314.000	17.57	16.96	16.07	14.99	13.75
1,329.000	12.34	10.78	9.28	7.92	6.73
1,344.000	5.74	4.91	4.23	3.65	3.14
1,359.000	2.69	2.31	1.99	1.71	1.47
1,374.000	1.26	1.08	0.93	0.80	0.68
1,389.000	0.59	0.51	0.44	0.38	0.33
1,404.000	0.28	0.24	0.21	0.18	0.16
1,419.000	0.14	0.12	0.10	0.09	0.08
1,434.000	0.06	0.05	0.03	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.198 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	191.54 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	3.198 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	191.54 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	418.91 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	416.81 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	2,633,717.562 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	2,631,937.179 ft ³
Volume (Retained)	771.418 ft ³
Volume (Unrouted)	-1,008.966 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R-1
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Peak Discharge	416.81 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	2,631,937.036 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
414.000	0.00	0.00	0.00	0.00	0.01
429.000	0.01	0.02	0.02	0.13	0.23
444.000	0.31	0.38	0.45	0.53	0.61
459.000	0.69	0.77	0.86	0.95	1.05
474.000	1.15	1.25	1.35	1.46	1.56
489.000	1.67	1.79	1.90	2.02	2.14
504.000	2.27	2.39	2.52	2.66	2.79
519.000	2.93	3.07	3.22	3.37	3.52
534.000	3.67	3.83	4.00	4.17	4.36
549.000	4.56	4.78	5.03	5.31	5.62
564.000	5.96	6.31	6.69	7.08	7.47
579.000	7.88	8.30	8.72	9.14	9.58
594.000	10.01	10.45	10.90	11.35	11.82
609.000	12.30	12.79	13.29	13.81	14.35
624.000	14.91	15.48	16.09	16.72	17.41
639.000	18.16	18.98	19.91	20.93	22.06
654.000	23.27	24.58	25.97	27.45	29.01
669.000	30.69	32.51	34.47	36.60	38.90
684.000	41.38	44.03	46.90	49.98	53.35
699.000	57.03	61.17	65.89	71.39	77.91
714.000	86.11	96.93	111.31	131.12	158.09
729.000	203.44	245.57	288.20	329.62	364.10
744.000	390.93	408.95	416.66	416.81	409.90
759.000	398.06	382.03	362.64	341.02	317.43
774.000	294.60	272.88	252.67	234.48	217.96
789.000	203.10	189.52	177.11	165.67	155.25
804.000	148.42	140.74	133.04	125.67	118.80
819.000	112.42	106.51	101.01	95.87	91.06
834.000	86.57	82.39	78.49	74.88	71.54
849.000	68.47	65.63	63.03	60.63	58.44
864.000	56.44	54.59	52.88	51.30	49.82
879.000	48.44	47.17	45.99	44.91	43.92
894.000	43.01	42.16	41.36	40.59	39.83
909.000	39.07	38.27	37.43	36.53	35.60
924.000	34.64	33.67	32.70	31.76	30.85
939.000	29.99	29.19	28.43	27.73	27.08
954.000	26.49	25.94	25.44	24.98	24.55
969.000	24.14	23.77	23.42	23.08	22.77
984.000	22.47	22.18	21.91	21.65	21.40

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
999.000	21.16	20.93	20.71	20.50	20.29
1,014.000	20.08	19.89	19.69	19.51	19.32
1,029.000	19.15	18.97	18.80	18.64	18.48
1,044.000	18.32	18.17	18.01	17.87	17.72
1,059.000	17.58	17.44	17.30	17.17	17.04
1,074.000	16.91	16.79	16.66	16.53	16.40
1,089.000	16.26	16.11	15.94	15.75	15.55
1,104.000	15.34	15.12	14.90	14.68	14.47
1,119.000	14.27	14.08	13.90	13.73	13.56
1,134.000	13.42	13.28	13.15	13.02	12.91
1,149.000	12.80	12.69	12.59	12.50	12.41
1,164.000	12.32	12.24	12.15	12.08	12.00
1,179.000	11.93	11.85	11.78	11.71	11.64
1,194.000	11.58	11.51	11.44	11.38	11.32
1,209.000	11.25	11.19	11.13	11.07	11.01
1,224.000	10.95	10.89	10.84	10.78	10.72
1,239.000	10.67	10.61	10.56	10.50	10.45
1,254.000	10.40	10.35	10.30	10.25	10.20
1,269.000	10.15	10.11	10.06	10.01	9.97
1,284.000	9.92	9.88	9.83	9.79	9.74
1,299.000	9.70	9.66	9.61	9.57	9.53
1,314.000	9.49	9.45	9.40	9.36	9.32
1,329.000	9.28	9.24	9.20	9.17	9.13
1,344.000	9.09	9.05	9.01	8.97	8.94
1,359.000	8.90	8.86	8.83	8.79	8.76
1,374.000	8.72	8.69	8.66	8.62	8.59
1,389.000	8.56	8.52	8.49	8.46	8.43
1,404.000	8.39	8.36	8.33	8.30	8.27
1,419.000	8.24	8.21	8.19	8.16	8.13
1,434.000	8.10	8.07	8.03	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.921 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	250.31 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R-1
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.921 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	250.31 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	557.35 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	555.50 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	3,703,151.567 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	3,700,896.225 ft ³
Volume (Retained)	959.533 ft ³
Volume (Unrouted)	-1,295.809 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Peak Discharge	555.50 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	3,700,896.103 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
357.000	0.00	0.00	0.00	0.00	0.01
372.000	0.01	0.01	0.02	0.12	0.24
387.000	0.33	0.42	0.51	0.60	0.69
402.000	0.80	0.90	1.01	1.13	1.25
417.000	1.37	1.49	1.62	1.75	1.89
432.000	2.02	2.16	2.30	2.45	2.59
447.000	2.74	2.89	3.05	3.20	3.36
462.000	3.52	3.69	3.86	4.03	4.20
477.000	4.37	4.55	4.74	4.92	5.11
492.000	5.30	5.50	5.69	5.90	6.10
507.000	6.31	6.53	6.75	6.97	7.20
522.000	7.43	7.67	7.91	8.16	8.41
537.000	8.67	8.94	9.22	9.53	9.87
552.000	10.27	10.72	11.24	11.82	12.45
567.000	13.13	13.84	14.57	15.32	16.08
582.000	16.84	17.61	18.37	19.13	19.90
597.000	20.66	21.42	22.19	22.98	23.77
612.000	24.59	25.42	26.27	27.15	28.05
627.000	28.98	29.96	30.98	32.10	33.33
642.000	34.71	36.27	38.02	39.96	42.06
657.000	44.31	46.70	49.22	51.87	54.67
672.000	57.65	60.83	64.22	67.82	71.66
687.000	75.72	80.04	84.63	89.56	94.83
702.000	100.61	107.02	114.35	122.93	133.63
717.000	147.73	171.31	203.57	241.58	288.19
732.000	340.82	395.32	448.15	491.73	525.16
747.000	547.05	555.50	554.16	543.77	527.16
762.000	505.36	479.45	450.95	420.18	390.60
777.000	362.66	336.76	313.55	292.52	273.64
792.000	256.42	240.67	226.14	212.89	200.89
807.000	189.82	179.74	170.38	161.77	154.00
822.000	148.52	142.12	135.57	129.17	123.03
837.000	117.23	111.76	106.67	101.92	97.54
852.000	93.49	89.76	86.33	83.19	80.32
867.000	77.69	75.27	73.02	70.93	68.99
882.000	67.19	65.52	64.00	62.61	61.33
897.000	60.13	59.00	57.91	56.82	55.72
912.000	54.56	53.30	51.96	50.55	49.08
927.000	47.59	46.10	44.66	43.27	41.95

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
942.000	40.70	39.54	38.47	37.49	36.59
957.000	35.77	35.02	34.34	33.70	33.12
972.000	32.57	32.06	31.58	31.13	30.71
987.000	30.31	29.92	29.56	29.21	28.87
1,002.000	28.55	28.24	27.94	27.65	27.37
1,017.000	27.10	26.84	26.58	26.33	26.09
1,032.000	25.85	25.62	25.39	25.17	24.96
1,047.000	24.75	24.54	24.34	24.15	23.95
1,062.000	23.76	23.58	23.40	23.22	23.04
1,077.000	22.87	22.70	22.52	22.33	22.13
1,092.000	21.89	21.64	21.34	21.02	20.68
1,107.000	20.33	19.97	19.61	19.27	18.94
1,122.000	18.62	18.33	18.06	17.80	17.57
1,137.000	17.36	17.16	16.98	16.81	16.65
1,152.000	16.50	16.36	16.22	16.09	15.97
1,167.000	15.85	15.74	15.63	15.53	15.42
1,182.000	15.32	15.23	15.13	15.04	14.95
1,197.000	14.86	14.78	14.69	14.61	14.52
1,212.000	14.44	14.36	14.28	14.20	14.13
1,227.000	14.05	13.98	13.90	13.83	13.76
1,242.000	13.69	13.62	13.55	13.48	13.41
1,257.000	13.35	13.28	13.21	13.15	13.09
1,272.000	13.02	12.96	12.90	12.84	12.78
1,287.000	12.72	12.66	12.61	12.55	12.49
1,302.000	12.44	12.38	12.33	12.27	12.22
1,317.000	12.16	12.11	12.05	12.00	11.95
1,332.000	11.90	11.84	11.79	11.74	11.69
1,347.000	11.64	11.60	11.55	11.50	11.46
1,362.000	11.41	11.37	11.32	11.27	11.23
1,377.000	11.19	11.14	11.10	11.05	11.01
1,392.000	10.97	10.92	10.88	10.84	10.80
1,407.000	10.76	10.72	10.68	10.64	10.60
1,422.000	10.56	10.52	10.48	10.44	10.40
1,437.000	10.36	10.31	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.668 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	347.74 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
595.00	0.00	0.000	0.000	0.00	0.00	0.00
595.01	0.02	114.000	0.264	0.00	0.02	1.29
596.67	154.66	32,285.000	0.626	0.00	154.66	513.38
598.34	640.16	91,065.000	0.990	0.00	640.16	1,652.00
600.01	1,561.45	176,339.000	1.354	0.00	1,561.45	3,520.78
601.68	3,017.76	288,108.000	1.719	0.00	3,017.76	6,218.97
603.35	5,100.40	426,372.000	2.083	0.00	5,100.40	9,837.87
605.02	7,894.60	591,130.000	2.447	0.00	7,894.60	14,462.71
606.69	11,480.80	782,382.000	2.811	0.00	11,480.80	20,173.94
608.36	15,935.59	1,000,130.000	3.175	0.00	15,935.59	27,048.15
610.03	21,332.34	1,244,371.000	3.540	0.00	21,332.34	35,158.69
611.70	27,741.67	1,515,108.000	3.904	0.00	27,741.67	44,576.19
613.37	35,231.80	1,812,338.000	4.268	0.00	35,231.80	55,368.89
615.04	43,868.90	2,136,064.000	4.632	0.00	43,868.90	67,602.94
616.71	53,717.27	2,486,283.000	4.996	0.00	53,717.27	81,342.65
618.38	64,839.56	2,862,998.000	5.361	0.00	64,839.56	96,650.65
620.05	77,296.90	3,266,207.000	5.725	0.00	77,296.90	113,588.08
621.72	91,149.05	3,695,910.000	6.089	0.00	91,149.05	132,214.73

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
623.39	106,454.55	4,152,108.000	6.453	0.00	106,454.55	152,589.09
625.06	123,270.76	4,634,801.000	6.817	0.00	123,270.76	174,768.55
626.73	141,653.96	5,143,988.000	7.182	0.00	141,653.96	198,809.39
628.40	161,659.47	5,679,670.000	7.546	0.00	161,659.47	224,766.91
630.07	183,341.65	6,241,846.000	7.910	0.00	183,341.65	252,695.49
631.74	206,754.01	6,830,517.000	8.274	0.00	206,754.01	282,648.64
633.41	231,949.25	7,445,682.000	8.639	0.00	231,949.25	314,679.06
635.08	258,979.30	8,087,342.000	9.003	0.00	258,979.30	348,838.66
636.75	287,895.35	8,755,497.000	9.367	0.00	287,895.35	385,178.65
638.42	318,747.92	9,450,146.000	9.731	0.00	318,747.92	423,749.54
640.09	351,586.88	10,171,289.00	10.095	0.00	351,586.88	464,601.21
		0				
641.76	386,461.48	10,918,928.00	10.460	0.00	386,461.48	507,782.89
		0				
643.43	423,420.37	11,693,060.00	10.824	0.00	423,420.37	553,343.26
		0				
645.10	462,511.65	12,493,688.00	11.188	0.00	462,511.65	601,330.40
		0				
646.77	503,782.88	13,320,809.00	11.552	0.00	503,782.88	651,791.87
		0				
648.44	547,281.12	14,174,426.00	11.916	0.00	547,281.12	704,774.73
		0				
650.11	593,052.91	15,054,536.00	12.281	0.00	593,052.91	760,325.54
		0				
651.78	641,144.34	15,961,142.00	12.645	0.00	641,144.34	818,490.37
		0				
653.45	691,601.05	16,894,242.00	13.009	0.00	691,601.05	879,314.85
		0				
655.12	744,468.24	17,853,836.00	13.373	0.00	744,468.24	942,844.20
		0				
656.79	799,790.66	18,839,925.00	13.737	0.00	799,790.66	1,009,123.17
		0				
658.46	857,612.70	19,852,509.00	14.102	0.00	857,612.70	1,078,196.14
		0				
660.13	917,978.34	20,891,587.00	14.466	0.00	917,978.34	1,150,107.09
		0				
661.80	980,931.18	21,957,160.00	14.830	0.00	980,931.18	1,224,899.63
		0				
663.47	1,046,514.47	23,049,227.00	15.194	0.00	1,046,514.47	1,302,616.99
		0				
665.14	1,114,771.08	24,167,789.00	15.559	0.00	1,114,771.08	1,383,302.07
		0				
666.81	1,185,743.56	25,312,845.00	15.923	0.00	1,185,743.56	1,466,997.40
		0				
668.48	1,259,474.14	26,484,396.00	16.287	0.00	1,259,474.14	1,553,745.21
		0				

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 950.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
670.15	1,336,004.69	27,682,442.00 0	16.651	0.00	1,336,004.69	1,643,587.38
671.82	1,415,376.82	28,906,982.00 0	17.015	0.00	1,415,376.82	1,736,565.51
673.49	1,497,631.79	30,158,016.00 0	17.380	0.00	1,497,631.79	1,832,720.87
675.16	1,582,810.61	31,435,546.00 0	17.744	0.00	1,582,810.61	1,932,094.45
676.83	1,670,953.96	32,739,569.00 0	18.108	0.00	1,670,953.96	2,034,726.95
678.33	1,752,869.51	33,936,111.00 0	18.436	0.00	1,752,869.51	2,129,937.41

Subsection: Channel Routing Summary
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	950.00 ft
Travel Time (Channel)	2.668 min
Number of Sections	1
Length (Section)	950.00 ft
Flow (Weighted)	347.74 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	678.33 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	595.00 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	791.53 ft ³ /s	Time to Peak (In)	747.000 min
Flow (Peak Out)	790.70 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	5,762,623.103 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	5,759,569.857 ft ³
Volume (Retained)	1,275.557 ft ³
Volume (Unrouted)	-1,777.689 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R-1
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Peak Discharge	790.70 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	5,759,569.740 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
285.000	0.00	0.00	0.00	0.00	0.01
300.000	0.01	0.02	0.02	0.16	0.27
315.000	0.35	0.44	0.52	0.61	0.70
330.000	0.80	0.90	1.01	1.11	1.22
345.000	1.34	1.46	1.57	1.70	1.82
360.000	1.95	2.08	2.21	2.36	2.52
375.000	2.69	2.88	3.09	3.31	3.54
390.000	3.78	4.03	4.28	4.54	4.80
405.000	5.07	5.33	5.60	5.86	6.13
420.000	6.39	6.66	6.92	7.19	7.46
435.000	7.73	8.01	8.28	8.56	8.84
450.000	9.13	9.42	9.71	10.00	10.30
465.000	10.60	10.91	11.22	11.53	11.85
480.000	12.18	12.51	12.84	13.18	13.53
495.000	13.88	14.23	14.59	14.96	15.34
510.000	15.72	16.10	16.50	16.90	17.31
525.000	17.73	18.16	18.59	19.04	19.49
540.000	19.97	20.48	21.06	21.73	22.53
555.000	23.49	24.63	25.93	27.36	28.92
570.000	30.54	32.21	33.89	35.58	37.26
585.000	38.91	40.55	42.15	43.72	45.26
600.000	46.78	48.30	49.81	51.33	52.87
615.000	54.43	56.02	57.63	59.28	60.97
630.000	62.74	64.59	66.63	68.91	71.50
645.000	74.50	77.87	81.64	85.74	90.13
660.000	94.76	99.59	104.60	109.84	115.32
675.000	121.06	127.10	133.42	140.04	146.94
690.000	154.15	164.52	173.06	181.32	189.89
705.000	198.95	209.11	220.81	235.80	256.16
720.000	283.69	322.50	373.07	435.97	507.10
735.000	580.52	653.04	718.12	759.56	784.60
750.000	790.70	784.28	765.87	739.89	707.66
765.000	670.50	632.07	594.57	555.50	518.53
780.000	484.44	454.10	426.71	402.18	379.82
795.000	359.30	340.29	322.82	306.86	292.02
810.000	278.38	265.59	253.70	242.44	231.76
825.000	221.50	211.59	202.00	192.82	184.05
840.000	175.76	168.01	160.77	154.25	149.74
855.000	144.58	139.40	134.52	129.99	125.82

Subsection: Channel Routing (Hydrograph output)
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
870.000	121.99	118.44	115.15	112.09	109.27
885.000	106.65	104.26	102.07	100.04	98.15
900.000	96.35	94.61	92.85	91.02	89.04
915.000	86.84	84.46	81.89	79.19	76.44
930.000	73.67	70.97	68.38	65.92	63.62
945.000	61.47	59.51	57.71	56.10	54.63
960.000	53.31	52.12	51.02	50.02	49.10
975.000	48.25	47.45	46.71	46.02	45.37
990.000	44.75	44.16	43.61	43.08	42.58
1,005.000	42.10	41.63	41.19	40.76	40.34
1,020.000	39.94	39.55	39.17	38.80	38.44
1,035.000	38.08	37.74	37.41	37.09	36.77
1,050.000	36.46	36.16	35.87	35.58	35.30
1,065.000	35.02	34.75	34.48	34.22	33.96
1,080.000	33.71	33.44	33.15	32.82	32.44
1,095.000	32.00	31.50	30.93	30.32	29.67
1,110.000	29.02	28.38	27.76	27.17	26.61
1,125.000	26.08	25.60	25.16	24.76	24.39
1,140.000	24.06	23.76	23.49	23.23	22.99
1,155.000	22.77	22.56	22.36	22.17	21.99
1,170.000	21.82	21.65	21.50	21.34	21.20
1,185.000	21.06	20.92	20.78	20.65	20.52
1,200.000	20.40	20.27	20.15	20.03	19.92
1,215.000	19.80	19.69	19.58	19.47	19.37
1,230.000	19.26	19.16	19.06	18.96	18.86
1,245.000	18.76	18.67	18.57	18.48	18.38
1,260.000	18.29	18.20	18.11	18.02	17.93
1,275.000	17.85	17.76	17.68	17.59	17.51
1,290.000	17.43	17.35	17.27	17.19	17.11
1,305.000	17.03	16.96	16.88	16.80	16.73
1,320.000	16.65	16.58	16.50	16.43	16.35
1,335.000	16.28	16.21	16.14	16.07	16.00
1,350.000	15.93	15.86	15.80	15.73	15.66
1,365.000	15.60	15.54	15.47	15.41	15.35
1,380.000	15.29	15.23	15.17	15.11	15.05
1,395.000	14.99	14.93	14.87	14.81	14.76
1,410.000	14.70	14.64	14.59	14.54	14.48
1,425.000	14.43	14.37	14.32	14.27	14.22
1,440.000	14.15	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.865 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	102.40 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.865 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	102.40 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	230.50 ft ³ /s	Time to Peak (In)	753.000 min
Flow (Peak Out)	230.00 ft ³ /s	Time to Peak (Out)	756.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	1,587,734.916 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	1,587,600.324 ft ³
Volume (Retained)	127.694 ft ³
Volume (Unrouted)	-6.898 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Peak Discharge	230.00 ft ³ /s
Time to Peak	756.000 min
Hydrograph Volume	1,587,600.177 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
519.000	0.00	0.00	0.00	0.00	0.01
534.000	0.01	0.01	0.01	0.02	0.02
549.000	0.02	0.03	0.09	0.12	0.14
564.000	0.17	0.26	0.41	0.64	0.95
579.000	1.30	1.67	2.00	2.25	2.41
594.000	2.48	2.52	2.58	2.73	2.98
609.000	3.37	3.93	4.54	5.12	5.59
624.000	5.87	5.96	5.90	5.78	5.70
639.000	5.73	5.93	6.33	6.94	7.61
654.000	8.26	8.84	9.36	9.87	10.42
669.000	11.10	11.96	12.91	13.86	14.77
684.000	15.62	16.49	17.51	18.70	20.05
699.000	21.49	23.03	24.80	27.03	30.01
714.000	34.11	39.81	47.63	58.01	71.45
729.000	87.94	107.12	128.17	149.89	171.11
744.000	197.26	212.08	222.35	228.27	230.00
759.000	227.83	222.10	213.28	202.45	190.19
774.000	177.68	166.94	157.68	147.44	137.58
789.000	128.38	119.68	111.40	103.51	96.08
804.000	89.31	83.38	78.61	74.94	71.73
819.000	68.62	65.22	61.41	57.55	53.94
834.000	50.93	48.76	47.51	47.11	47.11
849.000	47.12	46.69	45.58	43.83	41.61
864.000	39.36	37.41	36.05	35.41	35.60
879.000	36.53	37.62	38.55	38.93	38.54
894.000	37.40	35.56	33.24	30.55	27.85
909.000	25.43	23.65	22.86	23.26	24.92
924.000	27.40	30.09	32.38	33.85	34.31
939.000	33.78	32.43	30.48	28.10	25.39
954.000	22.52	19.66	17.11	15.20	14.15
969.000	14.17	15.36	17.72	20.88	24.26
984.000	27.33	29.59	30.90	31.22	30.63
999.000	29.33	27.48	25.20	22.63	19.99
1,014.000	17.41	15.01	12.86	10.99	9.39
1,029.000	8.02	6.89	6.05	5.63	5.88
1,044.000	6.98	9.06	12.07	15.78	19.82
1,059.000	23.56	26.60	28.65	29.55	29.44
1,074.000	28.49	26.92	24.89	22.58	20.06
1,089.000	17.52	15.11	12.93	11.03	9.40

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,104.000	8.03	6.87	5.88	5.04	4.31
1,119.000	3.69	3.15	2.69	2.30	1.97
1,134.000	1.68	1.44	1.23	1.13	1.22
1,149.000	1.75	2.93	4.93	7.84	11.58
1,164.000	15.90	20.19	23.96	26.78	28.46
1,179.000	29.05	28.67	27.54	25.84	23.73
1,194.000	21.30	18.78	16.32	14.02	11.98
1,209.000	10.23	8.74	7.48	6.40	5.48
1,224.000	4.70	4.03	3.45	2.95	2.53
1,239.000	2.17	1.85	1.59	1.36	1.16
1,254.000	1.00	0.86	0.73	0.63	0.54
1,269.000	0.47	0.40	0.35	0.30	0.40
1,284.000	0.94	2.15	4.19	7.15	11.06
1,299.000	15.64	20.20	24.21	27.21	28.98
1,314.000	29.58	29.17	28.00	26.28	24.11
1,329.000	21.61	19.00	16.43	14.07	12.00
1,344.000	10.23	8.75	7.49	6.41	5.49
1,359.000	4.71	4.03	3.45	2.96	2.53
1,374.000	2.17	1.86	1.59	1.36	1.17
1,389.000	1.00	0.86	0.74	0.63	0.54
1,404.000	0.47	0.40	0.35	0.30	0.26
1,419.000	0.22	0.19	0.16	0.13	0.10
1,434.000	0.08	0.06	0.05	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.282 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	317.09 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.282 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	317.09 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	697.60 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	697.46 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	4,332,662.739 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	4,330,274.519 ft ³
Volume (Retained)	826.544 ft ³
Volume (Unrouted)	-1,561.676 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Peak Discharge	697.46 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	4,330,274.384 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
417.000	0.00	0.00	0.00	0.00	0.01
432.000	0.01	0.01	0.02	0.11	0.36
447.000	0.52	0.65	0.77	0.90	1.03
462.000	1.17	1.31	1.46	1.61	1.77
477.000	1.93	2.10	2.27	2.45	2.63
492.000	2.81	3.00	3.19	3.39	3.59
507.000	3.80	4.01	4.22	4.44	4.67
522.000	4.90	5.14	5.38	5.63	5.88
537.000	6.14	6.41	6.68	6.98	7.30
552.000	7.64	8.03	8.46	8.94	9.47
567.000	10.04	10.64	11.27	11.92	12.59
582.000	13.27	13.96	14.66	15.36	16.08
597.000	16.80	17.53	18.27	19.03	19.80
612.000	20.60	21.42	22.26	23.13	24.03
627.000	24.97	25.94	26.96	28.05	29.23
642.000	30.52	31.95	33.54	35.30	37.23
657.000	39.31	41.54	43.91	46.43	49.13
672.000	52.02	55.13	58.49	62.12	66.05
687.000	70.27	74.82	79.73	85.07	90.90
702.000	97.37	104.67	113.09	123.05	135.21
717.000	150.89	172.51	211.41	253.29	311.38
732.000	380.99	451.65	521.25	584.53	636.53
747.000	676.20	694.09	697.46	688.34	668.64
762.000	640.95	608.76	573.69	534.26	495.21
777.000	458.23	424.03	392.94	364.78	339.31
792.000	316.21	295.18	275.94	258.36	243.62
807.000	230.82	218.13	206.05	194.74	184.29
822.000	174.58	166.99	160.00	152.41	145.00
837.000	137.97	131.39	125.26	119.57	114.32
852.000	109.48	105.03	100.93	97.17	93.72
867.000	90.55	87.64	84.98	82.54	80.28
882.000	78.20	76.27	74.48	72.83	71.31
897.000	69.89	68.55	67.27	66.03	64.79
912.000	63.52	62.18	60.78	59.29	57.74
927.000	56.15	54.56	52.98	51.45	49.98
942.000	48.59	47.30	46.10	44.99	43.98
957.000	43.05	42.19	41.40	40.67	39.99
972.000	39.36	38.77	38.21	37.68	37.18
987.000	36.70	36.25	35.81	35.40	35.00

Subsection: Channel Routing (Hydrograph output)
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,002.000	34.61	34.24	33.88	33.54	33.20
1,017.000	32.87	32.55	32.24	31.94	31.64
1,032.000	31.36	31.08	30.80	30.54	30.28
1,047.000	30.02	29.77	29.53	29.29	29.05
1,062.000	28.82	28.59	28.37	28.15	27.94
1,077.000	27.73	27.53	27.32	27.10	26.88
1,092.000	26.64	26.38	26.08	25.77	25.42
1,107.000	25.07	24.70	24.34	23.98	23.64
1,122.000	23.31	23.00	22.71	22.44	22.18
1,137.000	21.95	21.73	21.52	21.33	21.15
1,152.000	20.98	20.81	20.65	20.50	20.35
1,167.000	20.21	20.08	19.95	19.82	19.69
1,182.000	19.57	19.45	19.33	19.22	19.11
1,197.000	19.00	18.89	18.78	18.68	18.57
1,212.000	18.47	18.37	18.27	18.18	18.08
1,227.000	17.98	17.89	17.80	17.70	17.61
1,242.000	17.52	17.43	17.34	17.25	17.16
1,257.000	17.08	17.00	16.91	16.83	16.75
1,272.000	16.68	16.60	16.52	16.45	16.37
1,287.000	16.29	16.22	16.15	16.07	16.00
1,302.000	15.93	15.86	15.79	15.72	15.65
1,317.000	15.58	15.52	15.45	15.38	15.32
1,332.000	15.25	15.19	15.12	15.06	15.00
1,347.000	14.93	14.87	14.81	14.75	14.69
1,362.000	14.63	14.57	14.51	14.45	14.39
1,377.000	14.34	14.28	14.22	14.17	14.11
1,392.000	14.06	14.01	13.95	13.90	13.85
1,407.000	13.80	13.75	13.70	13.65	13.60
1,422.000	13.55	13.50	13.46	13.41	13.36
1,437.000	13.31	13.25	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.125 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	414.49 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	2.125 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	414.49 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	929.40 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	927.71 ft ³ /s	Time to Peak (Out)	753.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	6,093,869.809 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	6,090,837.717 ft ³
Volume (Retained)	1,025.846 ft ³
Volume (Unrouted)	-2,006.246 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Peak Discharge	927.71 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	6,090,837.448 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
363.000	0.00	0.00	0.00	0.01	0.01
378.000	0.01	0.02	0.10	0.38	0.56
393.000	0.72	0.87	1.02	1.19	1.36
408.000	1.54	1.72	1.91	2.11	2.32
423.000	2.52	2.74	2.95	3.17	3.40
438.000	3.63	3.87	4.11	4.35	4.60
453.000	4.85	5.10	5.36	5.62	5.89
468.000	6.17	6.44	6.72	7.01	7.30
483.000	7.60	7.90	8.21	8.52	8.84
498.000	9.17	9.50	9.83	10.17	10.52
513.000	10.88	11.24	11.61	11.99	12.38
528.000	12.77	13.18	13.59	14.01	14.44
543.000	14.90	15.38	15.92	16.52	17.21
558.000	18.00	18.90	19.90	20.98	22.13
573.000	23.32	24.55	25.81	27.07	28.33
588.000	29.59	30.85	32.10	33.35	34.61
603.000	35.87	37.15	38.45	39.78	41.13
618.000	42.52	43.94	45.41	46.92	48.49
633.000	50.14	51.90	53.83	55.97	58.37
648.000	61.07	64.09	67.41	71.01	74.86
663.000	78.93	83.22	87.75	92.53	97.61
678.000	103.01	108.76	114.87	121.38	128.29
693.000	135.65	143.51	151.95	161.08	171.40
708.000	187.02	201.11	218.16	240.43	273.04
723.000	319.15	377.95	449.01	532.16	621.94
738.000	722.15	800.70	862.15	904.84	926.01
753.000	927.71	913.42	885.62	847.86	802.12
768.000	752.62	700.63	650.10	605.27	564.53
783.000	524.97	489.14	456.80	427.50	400.84
798.000	376.40	354.05	333.67	315.06	298.02
813.000	282.40	267.97	254.77	243.41	233.05
828.000	222.42	211.98	201.94	192.40	183.42
843.000	175.00	168.13	162.28	155.85	149.62
858.000	143.79	138.42	133.48	128.96	124.82
873.000	121.04	117.57	114.38	111.42	108.69
888.000	106.17	103.84	101.69	99.68	97.79
903.000	95.98	94.21	92.43	90.58	88.62
918.000	86.52	84.28	81.92	79.49	77.04
933.000	74.61	72.25	69.99	67.86	65.87

Subsection: Channel Routing (Hydrograph output)
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
948.000	64.03	62.35	60.81	59.41	58.13
963.000	56.96	55.89	54.89	53.96	53.10
978.000	52.30	51.54	50.82	50.14	49.50
993.000	48.89	48.30	47.74	47.21	46.69
1,008.000	46.19	45.71	45.25	44.80	44.36
1,023.000	43.94	43.52	43.12	42.73	42.34
1,038.000	41.97	41.61	41.25	40.90	40.56
1,053.000	40.23	39.90	39.58	39.27	38.96
1,068.000	38.66	38.36	38.07	37.79	37.50
1,083.000	37.22	36.92	36.60	36.24	35.84
1,098.000	35.39	34.89	34.34	33.77	33.17
1,113.000	32.58	31.99	31.43	30.89	30.38
1,128.000	29.92	29.48	29.09	28.72	28.39
1,143.000	28.08	27.79	27.52	27.27	27.03
1,158.000	26.80	26.59	26.38	26.19	26.00
1,173.000	25.82	25.64	25.47	25.30	25.14
1,188.000	24.98	24.83	24.68	24.54	24.39
1,203.000	24.25	24.11	23.98	23.84	23.71
1,218.000	23.58	23.45	23.32	23.20	23.08
1,233.000	22.96	22.84	22.72	22.60	22.48
1,248.000	22.37	22.26	22.14	22.03	21.92
1,263.000	21.81	21.71	21.60	21.50	21.39
1,278.000	21.29	21.19	21.09	21.00	20.90
1,293.000	20.81	20.71	20.62	20.53	20.43
1,308.000	20.34	20.25	20.16	20.07	19.98
1,323.000	19.89	19.80	19.71	19.63	19.54
1,338.000	19.46	19.38	19.30	19.22	19.14
1,353.000	19.06	18.98	18.90	18.83	18.75
1,368.000	18.68	18.61	18.53	18.46	18.39
1,383.000	18.32	18.24	18.17	18.10	18.03
1,398.000	17.95	17.88	17.81	17.75	17.68
1,413.000	17.61	17.55	17.48	17.42	17.35
1,428.000	17.29	17.22	17.16	17.10	17.02

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	1.982 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	575.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Number of sections = 1 Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
587.50	0.00	0.000	0.000	0.00	0.00	0.00
587.51	0.03	132.000	0.303	0.00	0.03	1.49
589.17	170.56	29,240.000	0.503	0.00	170.56	495.44
590.84	624.00	73,121.000	0.704	0.00	624.00	1,436.46
592.51	1,401.28	131,644.000	0.905	0.00	1,401.28	2,863.99
594.18	2,554.98	204,809.000	1.106	0.00	2,554.98	4,830.63
595.85	4,136.30	292,615.000	1.308	0.00	4,136.30	7,387.58
597.52	6,194.18	395,064.000	1.509	0.00	6,194.18	10,583.78
599.19	8,775.42	512,154.000	1.710	0.00	8,775.42	14,466.02
600.86	11,924.95	643,885.000	1.912	0.00	11,924.95	19,079.23
602.53	15,686.06	790,259.000	2.113	0.00	15,686.06	24,466.71
604.20	20,100.61	951,274.000	2.314	0.00	20,100.61	30,670.32
605.87	25,209.18	1,126,931.000	2.515	0.00	25,209.18	37,730.63
607.54	31,051.22	1,317,229.000	2.717	0.00	31,051.22	45,687.10
609.21	37,665.17	1,522,170.000	2.918	0.00	37,665.17	54,578.17
610.88	45,088.53	1,741,752.000	3.119	0.00	45,088.53	64,441.33
612.55	53,357.97	1,975,975.000	3.320	0.00	53,357.97	75,313.25
614.22	62,509.38	2,224,841.000	3.522	0.00	62,509.38	87,229.83

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
615.89	72,577.95	2,488,348.000	3.723	0.00	72,577.95	100,226.26
617.56	83,598.19	2,766,497.000	3.924	0.00	83,598.19	114,337.05
619.23	95,604.03	3,059,288.000	4.126	0.00	95,604.03	129,596.12
620.90	108,628.80	3,366,720.000	4.327	0.00	108,628.80	146,036.80
622.57	122,705.31	3,688,794.000	4.528	0.00	122,705.31	163,691.92
624.24	137,865.87	4,025,510.000	4.729	0.00	137,865.87	182,593.76
625.91	154,142.30	4,376,868.000	4.931	0.00	154,142.30	202,774.16
627.58	171,566.00	4,742,867.000	5.132	0.00	171,566.00	224,264.52
629.25	190,167.91	5,123,508.000	5.333	0.00	190,167.91	247,095.78
630.92	209,978.62	5,518,791.000	5.534	0.00	209,978.62	271,298.51
632.59	231,028.29	5,928,715.000	5.736	0.00	231,028.29	296,902.90
634.26	253,346.75	6,353,281.000	5.937	0.00	253,346.75	323,938.76
635.93	276,963.47	6,792,489.000	6.138	0.00	276,963.47	352,435.57
637.60	301,907.59	7,246,339.000	6.340	0.00	301,907.59	382,422.47
639.27	328,207.94	7,714,830.000	6.541	0.00	328,207.94	413,928.28
640.94	355,893.05	8,197,963.000	6.742	0.00	355,893.05	446,981.53
642.61	384,991.15	8,695,738.000	6.943	0.00	384,991.15	481,610.46
644.28	415,530.19	9,208,155.000	7.145	0.00	415,530.19	517,843.02
645.95	447,537.87	9,735,213.000	7.346	0.00	447,537.87	555,706.90
647.62	481,041.60	10,276,913.000	7.547	0.00	481,041.60	595,229.52
649.29	516,068.57	10,833,255.000	7.748	0.00	516,068.57	636,438.07
650.96	552,645.73	11,404,238.000	7.950	0.00	552,645.73	679,359.48
652.63	590,799.76	11,989,863.000	8.151	0.00	590,799.76	724,020.47
654.30	630,557.17	12,590,130.000	8.352	0.00	630,557.17	770,447.51
655.97	671,944.22	13,205,039.000	8.554	0.00	671,944.22	818,666.87
657.64	714,986.96	13,834,589.000	8.755	0.00	714,986.96	868,704.62
659.31	759,711.25	14,478,781.000	8.956	0.00	759,711.25	920,586.60
660.98	806,142.75	15,137,615.000	9.157	0.00	806,142.75	974,338.47
662.65	854,306.92	15,811,090.000	9.359	0.00	854,306.92	1,029,985.70
664.32	904,229.04	16,499,208.000	9.560	0.00	904,229.04	1,087,553.57
665.99	955,934.21	17,201,967.000	9.761	0.00	955,934.21	1,147,067.17
667.66	1,009,447.35	17,919,367.000	9.962	0.00	1,009,447.35	1,208,551.43

Subsection: Elevation-Volume-Flow Table (Channel)
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Number of sections = 1
Storage, Area, Infiltration (per 875.00 ft section)

Elevation (ft)	Outflow (ft³/s)	Storage (ft³)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
669.33	1,064,793.23	18,651,410.00 0	10.164	0.00	1,064,793.23	1,272,031.11
670.83	1,116,203.39	19,322,917.00 0	10.345	0.00	1,116,203.39	1,330,902.46

Subsection: Channel Routing Summary
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Modified Puls Results Summary

Length (Channel)	875.00 ft
Travel Time (Channel)	1.982 min
Number of Sections	1
Length (Section)	875.00 ft
Flow (Weighted)	575.70 ft ³ /s
Overflow Channel	No Overflow Data
Elevation (Overflow)	670.83 ft

Infiltration

Infiltration Method (Computed)	No Infiltration
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Initial Conditions

Elevation (Starting Water Surface)	587.50 ft
Volume (Starting, per section)	0.000 ft ³
Flow (Out Starting)	0.00 ft ³ /s
Infiltration (Starting, per section)	0.00 ft ³ /s
Flow (Total Out Starting)	0.00 ft ³ /s
Time Increment	3.000 min

Inflow/Outflow Hydrograph Summary

Flow (Peak In)	1,320.74 ft ³ /s	Time to Peak (In)	750.000 min
Flow (Peak Out)	1,319.14 ft ³ /s	Time to Peak (Out)	750.000 min

Mass Balance (ft³)

Volume (Initial)	0.000 ft ³
Volume (Total Inflow)	9,486,126.817 ft ³
Volume (Total Infiltration)	0.000 ft ³
Volume (Total Outlet Outflow)	9,482,013.727 ft ³
Volume (Retained)	1,360.455 ft ³
Volume (Unrouted)	-2,752.634 ft ³
Error (Mass Balance)	0.0 %

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Peak Discharge	1,319.14 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	9,482,013.632 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
288.000	0.00	0.00	0.00	0.00	0.01
303.000	0.01	0.01	0.02	0.17	0.43
318.000	0.60	0.75	0.89	1.04	1.20
333.000	1.36	1.53	1.70	1.88	2.07
348.000	2.26	2.45	2.65	2.85	3.06
363.000	3.27	3.49	3.72	3.97	4.24
378.000	4.54	4.86	5.21	5.58	5.97
393.000	6.38	6.79	7.22	7.65	8.08
408.000	8.51	8.95	9.38	9.82	10.25
423.000	10.69	11.13	11.57	12.01	12.46
438.000	12.91	13.36	13.82	14.28	14.74
453.000	15.22	15.69	16.17	16.66	17.16
468.000	17.66	18.16	18.68	19.20	19.73
483.000	20.27	20.82	21.37	21.93	22.50
498.000	23.09	23.68	24.28	24.89	25.51
513.000	26.14	26.79	27.45	28.12	28.80
528.000	29.49	30.20	30.93	31.67	32.43
543.000	33.24	34.13	35.16	36.35	37.79
558.000	39.51	41.50	43.75	46.22	48.85
573.000	51.59	54.38	57.19	59.99	62.76
588.000	65.47	68.13	70.73	73.28	75.80
603.000	78.31	80.80	83.31	85.83	88.38
618.000	90.97	93.60	96.28	99.04	101.88
633.000	104.87	108.08	111.63	115.62	120.16
648.000	125.36	131.21	137.69	144.70	152.18
663.000	160.03	168.23	179.23	189.27	198.74
678.000	208.62	218.96	229.80	241.13	252.97
693.000	266.69	281.31	295.14	309.17	323.99
708.000	340.19	358.83	381.62	412.03	454.19
723.000	511.70	588.11	691.26	809.73	931.50
738.000	1,051.68	1,162.21	1,245.20	1,297.03	1,319.14
753.000	1,314.75	1,289.29	1,246.20	1,190.71	1,125.44
768.000	1,057.08	989.04	923.51	860.90	803.84
783.000	752.70	706.72	665.34	627.82	597.07
798.000	565.74	536.35	509.30	484.38	461.36
813.000	440.08	420.21	401.60	383.92	367.04
828.000	350.75	334.97	319.79	305.24	291.42
843.000	278.41	266.26	255.05	245.50	237.00
858.000	228.50	220.44	212.95	206.06	199.77

Subsection: Channel Routing (Hydrograph output)
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
873.000	194.02	188.73	183.86	179.35	175.19
888.000	171.34	168.57	165.66	162.64	159.69
903.000	156.81	153.95	151.03	147.93	144.54
918.000	140.84	136.80	132.50	128.01	123.46
933.000	118.92	114.52	110.30	106.33	102.64
948.000	99.25	96.17	93.38	90.87	88.60
963.000	86.54	84.67	82.96	81.39	79.94
978.000	78.60	77.35	76.18	75.08	74.05
993.000	73.07	72.15	71.26	70.42	69.62
1,008.000	68.85	68.11	67.39	66.70	66.03
1,023.000	65.38	64.75	64.14	63.54	62.96
1,038.000	62.39	61.84	61.30	60.78	60.27
1,053.000	59.77	59.29	58.81	58.34	57.88
1,068.000	57.43	56.99	56.56	56.14	55.72
1,083.000	55.29	54.84	54.33	53.76	53.09
1,098.000	52.31	51.43	50.45	49.41	48.33
1,113.000	47.25	46.19	45.17	44.20	43.30
1,128.000	42.47	41.72	41.03	40.41	39.84
1,143.000	39.33	38.85	38.42	38.01	37.64
1,158.000	37.28	36.95	36.63	36.33	36.04
1,173.000	35.77	35.50	35.25	35.01	34.77
1,188.000	34.55	34.32	34.11	33.89	33.69
1,203.000	33.48	33.28	33.08	32.89	32.70
1,218.000	32.52	32.33	32.16	31.98	31.81
1,233.000	31.64	31.47	31.30	31.14	30.98
1,248.000	30.82	30.66	30.51	30.35	30.20
1,263.000	30.05	29.90	29.75	29.61	29.46
1,278.000	29.32	29.18	29.04	28.90	28.77
1,293.000	28.64	28.50	28.37	28.24	28.12
1,308.000	27.99	27.86	27.73	27.61	27.48
1,323.000	27.36	27.23	27.11	26.99	26.87
1,338.000	26.75	26.64	26.52	26.41	26.29
1,353.000	26.18	26.07	25.96	25.85	25.75
1,368.000	25.64	25.54	25.44	25.33	25.23
1,383.000	25.13	25.03	24.93	24.83	24.73
1,398.000	24.64	24.54	24.44	24.35	24.26
1,413.000	24.16	24.07	23.98	23.89	23.80
1,428.000	23.72	23.63	23.54	23.46	23.36

Subsection: Addition Summary
Label: POA A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA A2
<Catchment to Outflow Node>	PROP AREA A1

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA A2	164,921.477	741.000	29.78
Flow (From)	PROP AREA A1	800,993.168	753.000	114.31
Flow (In)	POA A	965,914.645	753.000	139.60

Subsection: Addition Summary
Label: POA A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA A2
<Catchment to Outflow Node>	PROP AREA A1

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA A2	452,004.108	741.000	94.12
Flow (From)	PROP AREA A1	2,181,713.455	753.000	344.29
Flow (In)	POA A	2,633,717.562	750.000	418.91

Subsection: Addition Summary
Label: POA A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA A2
<Catchment to Outflow Node>	PROP AREA A1

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA A2	635,863.708	741.000	124.62
Flow (From)	PROP AREA A1	3,067,287.859	753.000	458.07
Flow (In)	POA A	3,703,151.567	750.000	557.35

Subsection: Addition Summary
Label: POA A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA A2
<Catchment to Outflow Node>	PROP AREA A1

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA A2	989,990.667	741.000	175.46
Flow (From)	PROP AREA A1	4,772,632.436	753.000	649.43
Flow (In)	POA A	5,762,623.103	747.000	791.53

Subsection: Addition Summary
Label: POA B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	PROP AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	PROP AREA B	621,937.167	753.000	93.10
Flow (From)	R-1	965,797.748	756.000	138.01
Flow (In)	POA B	1,587,734.916	753.000	230.50

Subsection: Addition Summary
Label: POA B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	PROP AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	PROP AREA B	1,700,725.560	750.000	280.93
Flow (From)	R-1	2,631,937.179	753.000	416.81
Flow (In)	POA B	4,332,662.739	750.000	697.60

Subsection: Addition Summary
Label: POA B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	PROP AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA B	2,392,973.584	750.000	373.90
Flow (From)	R-1	3,700,896.225	750.000	555.50
Flow (In)	POA B	6,093,869.809	750.000	929.40

Subsection: Addition Summary
Label: POA B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA B'

Upstream Link	Upstream Node
<Catchment to Outflow Node> R-1	PROP AREA B POA A

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	PROP AREA B	3,726,556.960	750.000	530.04
Flow (From)	R-1	5,759,569.857	750.000	790.70
Flow (In)	POA B	9,486,126.817	750.000	1,320.74

Subsection: Addition Summary
Label: POA D
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
POND B OUTFALL	POND B
POND A STRUCTURE	POND A
R2	POA B

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	POND B OUTFALL	426,584.119	753.000	46.56
Flow (From)	POND A STRUCTURE	567,290.511	792.000	45.09
Flow (From)	R2	1,587,600.324	756.000	230.00
Flow (In)	POA D	2,581,474.953	756.000	303.17

Subsection: Addition Summary
Label: POA D
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
POND B OUTFALL	POND B
POND A STRUCTURE	POND A
R2	POA B

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	POND B OUTFALL	1,016,695.745	750.000	142.44
Flow (From)	POND A STRUCTURE	1,419,893.193	780.000	147.07
Flow (From)	R2	4,330,274.519	753.000	697.46
Flow (In)	POA D	6,766,863.458	753.000	933.56

Subsection: Addition Summary
Label: POA D
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
POND B OUTFALL	POND B
POND A STRUCTURE	POND A
R2	POA B

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	POND B OUTFALL	1,387,900.369	747.000	192.78
Flow (From)	POND A STRUCTURE	1,958,911.779	777.000	201.98
Flow (From)	R2	6,090,837.717	753.000	927.71
Flow (In)	POA D	9,437,649.865	753.000	1,256.61

Subsection: Addition Summary
Label: POA D
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POA D'

Upstream Link	Upstream Node
POND B OUTFALL	POND B
POND A STRUCTURE	POND A
R2	POA B

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	POND B OUTFALL	2,098,290.514	747.000	279.57
Flow (From)	POND A STRUCTURE	2,987,867.397	774.000	297.87
Flow (From)	R2	9,482,013.727	750.000	1,319.14
Flow (In)	POA D	14,568,171.638	753.000	1,807.35

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	582.00	582.00	582.00	582.00	582.00
15.000	582.00	582.00	582.00	582.00	582.00
30.000	582.00	582.00	582.00	582.00	582.00
45.000	582.00	582.00	582.00	582.00	582.00
60.000	582.00	582.00	582.00	582.00	582.00
75.000	582.00	582.00	582.00	582.00	582.00
90.000	582.00	582.00	582.00	582.00	582.00
105.000	582.00	582.00	582.00	582.00	582.00
120.000	582.00	582.00	582.00	582.00	582.00
135.000	582.00	582.00	582.00	582.00	582.00
150.000	582.00	582.00	582.00	582.00	582.00
165.000	582.00	582.00	582.00	582.00	582.00
180.000	582.00	582.00	582.00	582.00	582.00
195.000	582.00	582.00	582.00	582.00	582.00
210.000	582.00	582.00	582.00	582.00	582.00
225.000	582.00	582.00	582.00	582.00	582.00
240.000	582.00	582.00	582.00	582.00	582.00
255.000	582.00	582.00	582.00	582.00	582.00
270.000	582.00	582.00	582.00	582.00	582.00
285.000	582.00	582.00	582.00	582.00	582.00
300.000	582.00	582.00	582.00	582.00	582.00
315.000	582.00	582.00	582.00	582.00	582.00
330.000	582.00	582.00	582.00	582.00	582.00
345.000	582.00	582.00	582.00	582.00	582.00
360.000	582.00	582.00	582.00	582.00	582.00
375.000	582.00	582.00	582.00	582.00	582.00
390.000	582.00	582.00	582.00	582.00	582.00
405.000	582.00	582.00	582.00	582.00	582.00
420.000	582.00	582.00	582.00	582.00	582.00
435.000	582.00	582.00	582.00	582.00	582.00
450.000	582.00	582.00	582.00	582.00	582.00
465.000	582.00	582.00	582.00	582.00	582.00
480.000	582.00	582.00	582.00	582.00	582.00
495.000	582.01	582.01	582.01	582.01	582.01
510.000	582.01	582.01	582.01	582.01	582.01
525.000	582.01	582.01	582.01	582.02	582.02
540.000	582.02	582.02	582.02	582.03	582.03
555.000	582.03	582.03	582.03	582.04	582.04
570.000	582.04	582.04	582.04	582.04	582.05
585.000	582.05	582.05	582.06	582.06	582.07
600.000	582.07	582.07	582.08	582.08	582.09
615.000	582.09	582.09	582.10	582.11	582.11

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	582.12	582.12	582.13	582.13	582.14
645.000	582.14	582.15	582.15	582.16	582.17
660.000	582.17	582.18	582.19	582.20	582.21
675.000	582.22	582.23	582.24	582.25	582.26
690.000	582.27	582.28	582.29	582.30	582.32
705.000	582.33	582.35	582.37	582.39	582.42
720.000	582.45	582.49	582.54	582.60	582.66
735.000	582.74	582.83	582.93	583.03	583.14
750.000	583.26	583.37	583.48	583.59	583.68
765.000	583.77	583.85	583.92	583.97	584.01
780.000	584.05	584.07	584.08	584.09	584.09
795.000	584.09	584.08	584.06	584.05	584.02
810.000	584.00	583.98	583.95	583.92	583.89
825.000	583.86	583.83	583.80	583.76	583.73
840.000	583.70	583.67	583.64	583.61	583.58
855.000	583.55	583.53	583.50	583.47	583.45
870.000	583.42	583.40	583.37	583.35	583.33
885.000	583.31	583.29	583.27	583.26	583.24
900.000	583.22	583.20	583.18	583.16	583.14
915.000	583.12	583.10	583.08	583.07	583.05
930.000	583.04	583.03	583.02	583.01	583.00
945.000	582.99	582.98	582.97	582.96	582.94
960.000	582.93	582.91	582.90	582.89	582.87
975.000	582.86	582.85	582.84	582.83	582.83
990.000	582.83	582.82	582.82	582.82	582.81
1,005.000	582.81	582.80	582.79	582.78	582.77
1,020.000	582.76	582.75	582.74	582.72	582.71
1,035.000	582.70	582.68	582.67	582.66	582.65
1,050.000	582.64	582.64	582.63	582.63	582.63
1,065.000	582.63	582.64	582.64	582.64	582.64
1,080.000	582.64	582.64	582.64	582.63	582.63
1,095.000	582.62	582.62	582.61	582.60	582.59
1,110.000	582.58	582.57	582.56	582.55	582.54
1,125.000	582.53	582.52	582.51	582.50	582.49
1,140.000	582.48	582.48	582.47	582.46	582.45
1,155.000	582.45	582.44	582.44	582.44	582.44
1,170.000	582.44	582.45	582.46	582.47	582.47
1,185.000	582.48	582.49	582.49	582.50	582.50
1,200.000	582.50	582.50	582.50	582.50	582.49
1,215.000	582.49	582.48	582.48	582.47	582.46
1,230.000	582.46	582.45	582.44	582.44	582.43
1,245.000	582.42	582.41	582.40	582.40	582.39

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	582.38	582.37	582.37	582.36	582.35
1,275.000	582.35	582.34	582.33	582.33	582.32
1,290.000	582.32	582.32	582.32	582.32	582.33
1,305.000	582.33	582.34	582.35	582.36	582.37
1,320.000	582.38	582.39	582.40	582.40	582.41
1,335.000	582.41	582.41	582.41	582.41	582.41
1,350.000	582.41	582.40	582.40	582.39	582.39
1,365.000	582.38	582.38	582.37	582.36	582.36
1,380.000	582.35	582.35	582.34	582.33	582.33
1,395.000	582.32	582.31	582.31	582.30	582.30
1,410.000	582.29	582.28	582.28	582.27	582.27
1,425.000	582.26	582.26	582.25	582.25	582.24
1,440.000	582.24	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	582.00	582.00	582.00	582.00	582.00
15.000	582.00	582.00	582.00	582.00	582.00
30.000	582.00	582.00	582.00	582.00	582.00
45.000	582.00	582.00	582.00	582.00	582.00
60.000	582.00	582.00	582.00	582.00	582.00
75.000	582.00	582.00	582.00	582.00	582.00
90.000	582.00	582.00	582.00	582.00	582.00
105.000	582.00	582.00	582.00	582.00	582.00
120.000	582.00	582.00	582.00	582.00	582.00
135.000	582.00	582.00	582.00	582.00	582.00
150.000	582.00	582.00	582.00	582.00	582.00
165.000	582.00	582.00	582.00	582.00	582.00
180.000	582.00	582.00	582.00	582.00	582.00
195.000	582.00	582.00	582.00	582.00	582.00
210.000	582.00	582.00	582.00	582.00	582.00
225.000	582.00	582.00	582.00	582.00	582.00
240.000	582.00	582.00	582.00	582.00	582.00
255.000	582.00	582.00	582.00	582.00	582.00
270.000	582.00	582.00	582.00	582.00	582.00
285.000	582.00	582.00	582.00	582.00	582.00
300.000	582.00	582.00	582.00	582.00	582.00
315.000	582.00	582.00	582.00	582.00	582.00
330.000	582.00	582.00	582.00	582.00	582.00
345.000	582.00	582.00	582.00	582.00	582.00
360.000	582.00	582.00	582.00	582.00	582.00
375.000	582.00	582.00	582.00	582.01	582.01
390.000	582.01	582.01	582.01	582.01	582.01
405.000	582.01	582.01	582.01	582.02	582.02
420.000	582.02	582.02	582.02	582.02	582.03
435.000	582.03	582.03	582.03	582.03	582.04
450.000	582.04	582.04	582.04	582.05	582.05
465.000	582.05	582.05	582.06	582.06	582.06
480.000	582.06	582.07	582.07	582.07	582.08
495.000	582.08	582.08	582.09	582.09	582.10
510.000	582.10	582.10	582.11	582.11	582.12
525.000	582.12	582.12	582.13	582.13	582.14
540.000	582.14	582.15	582.15	582.16	582.16
555.000	582.17	582.18	582.18	582.19	582.19
570.000	582.20	582.21	582.22	582.23	582.23
585.000	582.24	582.25	582.26	582.27	582.28
600.000	582.29	582.30	582.31	582.33	582.34
615.000	582.35	582.36	582.38	582.39	582.40

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	582.42	582.43	582.45	582.46	582.48
645.000	582.50	582.51	582.53	582.55	582.57
660.000	582.59	582.61	582.63	582.66	582.68
675.000	582.71	582.73	582.76	582.79	582.82
690.000	582.86	582.89	582.93	582.97	583.01
705.000	583.06	583.11	583.16	583.22	583.29
720.000	583.37	583.46	583.57	583.70	583.85
735.000	584.03	584.24	584.47	584.71	584.97
750.000	585.22	585.47	585.70	585.91	586.10
765.000	586.26	586.39	586.49	586.56	586.60
780.000	586.61	586.60	586.57	586.53	586.48
795.000	586.41	586.34	586.27	586.18	586.10
810.000	586.01	585.92	585.83	585.74	585.65
825.000	585.57	585.48	585.39	585.31	585.23
840.000	585.15	585.07	584.99	584.92	584.85
855.000	584.78	584.71	584.64	584.58	584.52
870.000	584.46	584.40	584.35	584.29	584.24
885.000	584.19	584.15	584.10	584.06	584.02
900.000	583.98	583.94	583.90	583.86	583.83
915.000	583.79	583.76	583.73	583.70	583.67
930.000	583.64	583.61	583.58	583.56	583.53
945.000	583.51	583.48	583.46	583.43	583.41
960.000	583.39	583.36	583.34	583.32	583.30
975.000	583.28	583.26	583.24	583.22	583.21
990.000	583.19	583.17	583.16	583.14	583.13
1,005.000	583.11	583.10	583.09	583.07	583.06
1,020.000	583.05	583.04	583.03	583.01	583.00
1,035.000	582.99	582.98	582.97	582.96	582.95
1,050.000	582.95	582.94	582.93	582.92	582.91
1,065.000	582.90	582.89	582.89	582.88	582.87
1,080.000	582.86	582.86	582.85	582.84	582.84
1,095.000	582.83	582.82	582.81	582.81	582.80
1,110.000	582.80	582.79	582.78	582.78	582.77
1,125.000	582.76	582.76	582.75	582.74	582.74
1,140.000	582.73	582.73	582.72	582.72	582.71
1,155.000	582.70	582.70	582.69	582.69	582.68
1,170.000	582.68	582.67	582.67	582.66	582.66
1,185.000	582.66	582.65	582.65	582.64	582.64
1,200.000	582.63	582.63	582.63	582.62	582.62
1,215.000	582.62	582.61	582.61	582.61	582.60
1,230.000	582.60	582.60	582.59	582.59	582.59
1,245.000	582.58	582.58	582.58	582.58	582.57

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	582.57	582.57	582.57	582.56	582.56
1,275.000	582.56	582.56	582.55	582.55	582.55
1,290.000	582.55	582.54	582.54	582.54	582.54
1,305.000	582.54	582.53	582.53	582.53	582.53
1,320.000	582.53	582.52	582.52	582.52	582.52
1,335.000	582.52	582.52	582.51	582.51	582.51
1,350.000	582.51	582.51	582.51	582.50	582.50
1,365.000	582.50	582.50	582.50	582.50	582.50
1,380.000	582.49	582.49	582.49	582.49	582.49
1,395.000	582.49	582.48	582.48	582.48	582.48
1,410.000	582.48	582.48	582.48	582.47	582.47
1,425.000	582.47	582.47	582.47	582.47	582.47
1,440.000	582.46	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	582.00	582.00	582.00	582.00	582.00
15.000	582.00	582.00	582.00	582.00	582.00
30.000	582.00	582.00	582.00	582.00	582.00
45.000	582.00	582.00	582.00	582.00	582.00
60.000	582.00	582.00	582.00	582.00	582.00
75.000	582.00	582.00	582.00	582.00	582.00
90.000	582.00	582.00	582.00	582.00	582.00
105.000	582.00	582.00	582.00	582.00	582.00
120.000	582.00	582.00	582.00	582.00	582.00
135.000	582.00	582.00	582.00	582.00	582.00
150.000	582.00	582.00	582.00	582.00	582.00
165.000	582.00	582.00	582.00	582.00	582.00
180.000	582.00	582.00	582.00	582.00	582.00
195.000	582.00	582.00	582.00	582.00	582.00
210.000	582.00	582.00	582.00	582.00	582.00
225.000	582.00	582.00	582.00	582.00	582.00
240.000	582.00	582.00	582.00	582.00	582.00
255.000	582.00	582.00	582.00	582.00	582.00
270.000	582.00	582.00	582.00	582.00	582.00
285.000	582.00	582.00	582.00	582.00	582.00
300.000	582.00	582.00	582.00	582.00	582.00
315.000	582.00	582.00	582.00	582.00	582.00
330.000	582.01	582.01	582.01	582.01	582.01
345.000	582.01	582.01	582.01	582.01	582.02
360.000	582.02	582.02	582.02	582.02	582.02
375.000	582.03	582.03	582.03	582.03	582.03
390.000	582.04	582.04	582.04	582.04	582.05
405.000	582.05	582.05	582.06	582.06	582.06
420.000	582.07	582.07	582.07	582.08	582.08
435.000	582.08	582.09	582.09	582.10	582.10
450.000	582.11	582.11	582.11	582.12	582.12
465.000	582.13	582.13	582.14	582.14	582.15
480.000	582.16	582.16	582.17	582.17	582.18
495.000	582.18	582.19	582.20	582.20	582.21
510.000	582.22	582.22	582.23	582.24	582.24
525.000	582.25	582.26	582.27	582.27	582.28
540.000	582.29	582.30	582.31	582.31	582.32
555.000	582.33	582.34	582.35	582.36	582.37
570.000	582.38	582.40	582.41	582.42	582.44
585.000	582.45	582.47	582.48	582.50	582.52
600.000	582.53	582.55	582.57	582.59	582.61
615.000	582.62	582.64	582.66	582.68	582.70

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	582.72	582.74	582.76	582.78	582.80
645.000	582.83	582.85	582.87	582.90	582.93
660.000	582.96	582.99	583.02	583.05	583.08
675.000	583.12	583.16	583.20	583.24	583.28
690.000	583.33	583.38	583.43	583.48	583.54
705.000	583.60	583.66	583.73	583.81	583.89
720.000	583.99	584.10	584.23	584.39	584.58
735.000	584.80	585.05	585.32	585.61	585.91
750.000	586.21	586.50	586.77	587.00	587.21
765.000	587.38	587.52	587.61	587.67	587.70
780.000	587.69	587.66	587.61	587.54	587.46
795.000	587.37	587.27	587.17	587.06	586.95
810.000	586.84	586.72	586.61	586.50	586.39
825.000	586.28	586.17	586.07	585.97	585.87
840.000	585.77	585.67	585.58	585.49	585.40
855.000	585.32	585.24	585.16	585.08	585.01
870.000	584.94	584.87	584.80	584.74	584.68
885.000	584.62	584.56	584.51	584.46	584.41
900.000	584.36	584.31	584.27	584.23	584.18
915.000	584.14	584.11	584.07	584.03	584.00
930.000	583.96	583.93	583.90	583.86	583.83
945.000	583.80	583.77	583.74	583.71	583.68
960.000	583.65	583.62	583.60	583.57	583.55
975.000	583.52	583.50	583.48	583.46	583.44
990.000	583.42	583.40	583.38	583.36	583.34
1,005.000	583.32	583.31	583.29	583.28	583.26
1,020.000	583.25	583.23	583.22	583.20	583.19
1,035.000	583.18	583.16	583.15	583.14	583.13
1,050.000	583.12	583.11	583.10	583.09	583.08
1,065.000	583.07	583.06	583.05	583.04	583.03
1,080.000	583.03	583.02	583.01	583.00	583.00
1,095.000	582.99	582.98	582.97	582.97	582.96
1,110.000	582.95	582.94	582.94	582.93	582.92
1,125.000	582.91	582.91	582.90	582.89	582.88
1,140.000	582.88	582.87	582.86	582.85	582.85
1,155.000	582.84	582.83	582.83	582.82	582.81
1,170.000	582.81	582.80	582.79	582.79	582.78
1,185.000	582.78	582.77	582.77	582.76	582.76
1,200.000	582.75	582.74	582.74	582.74	582.73
1,215.000	582.73	582.72	582.72	582.71	582.71
1,230.000	582.70	582.70	582.70	582.69	582.69
1,245.000	582.68	582.68	582.68	582.67	582.67

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	582.67	582.66	582.66	582.66	582.65
1,275.000	582.65	582.65	582.64	582.64	582.64
1,290.000	582.63	582.63	582.63	582.63	582.62
1,305.000	582.62	582.62	582.62	582.61	582.61
1,320.000	582.61	582.61	582.60	582.60	582.60
1,335.000	582.60	582.60	582.59	582.59	582.59
1,350.000	582.59	582.58	582.58	582.58	582.58
1,365.000	582.58	582.57	582.57	582.57	582.57
1,380.000	582.57	582.57	582.56	582.56	582.56
1,395.000	582.56	582.56	582.56	582.55	582.55
1,410.000	582.55	582.55	582.55	582.55	582.54
1,425.000	582.54	582.54	582.54	582.54	582.54
1,440.000	582.54	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	582.00	582.00	582.00	582.00	582.00
15.000	582.00	582.00	582.00	582.00	582.00
30.000	582.00	582.00	582.00	582.00	582.00
45.000	582.00	582.00	582.00	582.00	582.00
60.000	582.00	582.00	582.00	582.00	582.00
75.000	582.00	582.00	582.00	582.00	582.00
90.000	582.00	582.00	582.00	582.00	582.00
105.000	582.00	582.00	582.00	582.00	582.00
120.000	582.00	582.00	582.00	582.00	582.00
135.000	582.00	582.00	582.00	582.00	582.00
150.000	582.00	582.00	582.00	582.00	582.00
165.000	582.00	582.00	582.00	582.00	582.00
180.000	582.00	582.00	582.00	582.00	582.00
195.000	582.00	582.00	582.00	582.00	582.00
210.000	582.00	582.00	582.00	582.00	582.00
225.000	582.00	582.00	582.00	582.00	582.00
240.000	582.00	582.00	582.00	582.00	582.00
255.000	582.00	582.00	582.00	582.01	582.01
270.000	582.01	582.01	582.01	582.01	582.01
285.000	582.01	582.02	582.02	582.02	582.02
300.000	582.02	582.02	582.03	582.03	582.03
315.000	582.03	582.04	582.04	582.04	582.04
330.000	582.05	582.05	582.05	582.06	582.06
345.000	582.06	582.07	582.07	582.07	582.08
360.000	582.08	582.08	582.09	582.09	582.10
375.000	582.10	582.10	582.11	582.11	582.12
390.000	582.12	582.13	582.14	582.14	582.15
405.000	582.15	582.16	582.17	582.17	582.18
420.000	582.19	582.20	582.20	582.21	582.22
435.000	582.23	582.24	582.25	582.25	582.26
450.000	582.27	582.28	582.29	582.30	582.31
465.000	582.32	582.33	582.34	582.35	582.36
480.000	582.37	582.38	582.39	582.40	582.41
495.000	582.42	582.43	582.45	582.46	582.47
510.000	582.48	582.49	582.51	582.52	582.53
525.000	582.54	582.56	582.57	582.58	582.59
540.000	582.60	582.62	582.63	582.64	582.66
555.000	582.67	582.68	582.70	582.71	582.73
570.000	582.75	582.77	582.79	582.81	582.83
585.000	582.86	582.88	582.90	582.93	582.96
600.000	582.99	583.01	583.04	583.07	583.10
615.000	583.13	583.16	583.19	583.22	583.25

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	583.28	583.31	583.34	583.37	583.40
645.000	583.44	583.47	583.51	583.55	583.59
660.000	583.63	583.68	583.73	583.78	583.83
675.000	583.89	583.94	584.00	584.07	584.13
690.000	584.20	584.27	584.35	584.43	584.51
705.000	584.59	584.68	584.77	584.87	584.97
720.000	585.09	585.24	585.40	585.60	585.83
735.000	586.10	586.40	586.74	587.09	587.45
750.000	587.81	588.14	588.45	588.72	588.95
765.000	589.13	589.26	589.34	589.38	589.38
780.000	589.34	589.27	589.18	589.08	588.96
795.000	588.83	588.69	588.55	588.41	588.26
810.000	588.12	587.98	587.83	587.70	587.56
825.000	587.42	587.29	587.16	587.03	586.91
840.000	586.79	586.67	586.55	586.44	586.33
855.000	586.22	586.12	586.02	585.93	585.83
870.000	585.74	585.66	585.57	585.50	585.42
885.000	585.35	585.28	585.21	585.15	585.08
900.000	585.02	584.97	584.91	584.86	584.81
915.000	584.76	584.71	584.67	584.62	584.58
930.000	584.53	584.49	584.44	584.40	584.36
945.000	584.32	584.28	584.23	584.19	584.16
960.000	584.12	584.08	584.05	584.01	583.98
975.000	583.95	583.91	583.88	583.85	583.83
990.000	583.80	583.77	583.75	583.72	583.70
1,005.000	583.68	583.65	583.63	583.61	583.59
1,020.000	583.57	583.55	583.54	583.52	583.50
1,035.000	583.49	583.47	583.46	583.44	583.43
1,050.000	583.41	583.40	583.39	583.38	583.36
1,065.000	583.35	583.34	583.33	583.32	583.31
1,080.000	583.29	583.28	583.27	583.26	583.25
1,095.000	583.24	583.24	583.23	583.22	583.21
1,110.000	583.20	583.19	583.18	583.17	583.15
1,125.000	583.14	583.13	583.12	583.11	583.10
1,140.000	583.09	583.08	583.07	583.06	583.05
1,155.000	583.05	583.04	583.03	583.02	583.01
1,170.000	583.00	583.00	582.99	582.98	582.97
1,185.000	582.97	582.96	582.95	582.94	582.94
1,200.000	582.93	582.92	582.92	582.91	582.91
1,215.000	582.90	582.89	582.89	582.88	582.88
1,230.000	582.87	582.87	582.86	582.86	582.85
1,245.000	582.85	582.84	582.84	582.83	582.83

Subsection: Time vs. Elevation
Label: POND A (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	582.82	582.82	582.81	582.81	582.80
1,275.000	582.80	582.80	582.79	582.79	582.78
1,290.000	582.78	582.78	582.77	582.77	582.77
1,305.000	582.76	582.76	582.76	582.75	582.75
1,320.000	582.75	582.74	582.74	582.74	582.73
1,335.000	582.73	582.73	582.72	582.72	582.72
1,350.000	582.72	582.71	582.71	582.71	582.70
1,365.000	582.70	582.70	582.70	582.69	582.69
1,380.000	582.69	582.69	582.68	582.68	582.68
1,395.000	582.68	582.68	582.67	582.67	582.67
1,410.000	582.67	582.66	582.66	582.66	582.66
1,425.000	582.66	582.65	582.65	582.65	582.65
1,440.000	582.65	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	580.00	580.00	580.00	580.00	580.00
15.000	580.00	580.00	580.00	580.00	580.00
30.000	580.00	580.00	580.00	580.00	580.00
45.000	580.00	580.00	580.00	580.00	580.00
60.000	580.00	580.00	580.00	580.00	580.00
75.000	580.00	580.00	580.00	580.00	580.00
90.000	580.00	580.00	580.00	580.00	580.00
105.000	580.00	580.00	580.00	580.00	580.00
120.000	580.00	580.00	580.00	580.00	580.00
135.000	580.00	580.00	580.00	580.00	580.00
150.000	580.00	580.00	580.00	580.00	580.00
165.000	580.00	580.00	580.00	580.00	580.00
180.000	580.00	580.00	580.00	580.00	580.00
195.000	580.00	580.00	580.00	580.00	580.00
210.000	580.00	580.00	580.00	580.00	580.00
225.000	580.00	580.00	580.00	580.00	580.00
240.000	580.00	580.00	580.00	580.00	580.01
255.000	580.01	580.02	580.02	580.02	580.02
270.000	580.02	580.02	580.02	580.02	580.02
285.000	580.02	580.02	580.02	580.02	580.02
300.000	580.02	580.02	580.02	580.02	580.02
315.000	580.02	580.01	580.01	580.01	580.01
330.000	580.01	580.01	580.01	580.01	580.01
345.000	580.01	580.01	580.01	580.01	580.01
360.000	580.01	580.01	580.01	580.02	580.04
375.000	580.05	580.06	580.06	580.07	580.07
390.000	580.07	580.07	580.06	580.06	580.06
405.000	580.06	580.06	580.06	580.05	580.05
420.000	580.05	580.05	580.05	580.05	580.05
435.000	580.04	580.04	580.04	580.04	580.04
450.000	580.04	580.04	580.04	580.05	580.07
465.000	580.09	580.10	580.11	580.11	580.11
480.000	580.11	580.11	580.11	580.10	580.10
495.000	580.10	580.10	580.09	580.09	580.09
510.000	580.08	580.08	580.09	580.10	580.12
525.000	580.14	580.15	580.16	580.16	580.16
540.000	580.16	580.16	580.16	580.15	580.15
555.000	580.14	580.14	580.14	580.13	580.14
570.000	580.16	580.18	580.20	580.22	580.22
585.000	580.22	580.22	580.22	580.22	580.21
600.000	580.21	580.22	580.25	580.27	580.29
615.000	580.30	580.30	580.31	580.30	580.30

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	580.29	580.29	580.29	580.31	580.33
645.000	580.35	580.37	580.38	580.38	580.38
660.000	580.39	580.40	580.42	580.45	580.47
675.000	580.48	580.49	580.50	580.52	580.55
690.000	580.57	580.59	580.61	580.63	580.66
705.000	580.70	580.73	580.79	580.85	580.95
720.000	581.07	581.23	581.42	581.63	581.85
735.000	582.06	582.23	582.37	582.47	582.56
750.000	582.62	582.64	582.64	582.60	582.55
765.000	582.51	582.46	582.40	582.33	582.26
780.000	582.21	582.16	582.11	582.05	581.98
795.000	581.91	581.84	581.77	581.73	581.69
810.000	581.66	581.63	581.58	581.54	581.49
825.000	581.43	581.38	581.34	581.32	581.31
840.000	581.30	581.29	581.27	581.24	581.21
855.000	581.18	581.14	581.10	581.07	581.05
870.000	581.04	581.05	581.06	581.06	581.05
885.000	581.04	581.02	580.99	580.97	580.94
900.000	580.91	580.88	580.85	580.83	580.82
915.000	580.82	580.84	580.86	580.87	580.87
930.000	580.86	580.85	580.84	580.82	580.80
945.000	580.78	580.75	580.73	580.71	580.68
960.000	580.66	580.64	580.64	580.65	580.67
975.000	580.70	580.72	580.73	580.74	580.73
990.000	580.73	580.72	580.70	580.68	580.67
1,005.000	580.65	580.63	580.61	580.59	580.58
1,020.000	580.56	580.54	580.53	580.51	580.50
1,035.000	580.48	580.47	580.47	580.49	580.52
1,050.000	580.56	580.59	580.60	580.61	580.62
1,065.000	580.62	580.61	580.60	580.59	580.58
1,080.000	580.57	580.55	580.54	580.52	580.51
1,095.000	580.50	580.48	580.47	580.46	580.44
1,110.000	580.43	580.42	580.41	580.39	580.38
1,125.000	580.37	580.36	580.35	580.34	580.33
1,140.000	580.32	580.31	580.30	580.30	580.31
1,155.000	580.34	580.38	580.42	580.45	580.47
1,170.000	580.49	580.50	580.50	580.50	580.50
1,185.000	580.49	580.48	580.47	580.46	580.45
1,200.000	580.44	580.43	580.42	580.41	580.40
1,215.000	580.38	580.37	580.36	580.35	580.34
1,230.000	580.33	580.32	580.31	580.30	580.30
1,245.000	580.29	580.28	580.27	580.26	580.25

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	580.25	580.24	580.23	580.23	580.22
1,275.000	580.21	580.21	580.20	580.20	580.21
1,290.000	580.24	580.29	580.33	580.36	580.39
1,305.000	580.41	580.42	580.43	580.43	580.43
1,320.000	580.42	580.42	580.41	580.40	580.39
1,335.000	580.38	580.37	580.36	580.35	580.34
1,350.000	580.33	580.33	580.32	580.31	580.30
1,365.000	580.29	580.28	580.27	580.27	580.26
1,380.000	580.25	580.24	580.24	580.23	580.22
1,395.000	580.22	580.21	580.20	580.20	580.19
1,410.000	580.19	580.18	580.18	580.17	580.17
1,425.000	580.16	580.16	580.15	580.15	580.14
1,440.000	580.14	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	580.00	580.00	580.00	580.00	580.00
15.000	580.00	580.00	580.00	580.00	580.00
30.000	580.00	580.00	580.00	580.00	580.00
45.000	580.00	580.00	580.00	580.00	580.00
60.000	580.00	580.00	580.00	580.00	580.00
75.000	580.00	580.00	580.00	580.00	580.00
90.000	580.00	580.00	580.00	580.00	580.00
105.000	580.00	580.00	580.00	580.00	580.00
120.000	580.00	580.00	580.00	580.00	580.00
135.000	580.00	580.00	580.00	580.00	580.00
150.000	580.00	580.00	580.00	580.00	580.00
165.000	580.00	580.00	580.01	580.01	580.01
180.000	580.01	580.01	580.01	580.01	580.01
195.000	580.01	580.01	580.01	580.01	580.02
210.000	580.02	580.02	580.02	580.02	580.02
225.000	580.02	580.02	580.03	580.03	580.03
240.000	580.03	580.03	580.03	580.03	580.03
255.000	580.04	580.04	580.04	580.04	580.04
270.000	580.04	580.04	580.05	580.05	580.05
285.000	580.05	580.05	580.05	580.05	580.06
300.000	580.06	580.06	580.06	580.06	580.06
315.000	580.07	580.07	580.07	580.07	580.07
330.000	580.07	580.08	580.08	580.08	580.08
345.000	580.08	580.09	580.09	580.09	580.09
360.000	580.09	580.09	580.10	580.10	580.10
375.000	580.10	580.11	580.11	580.11	580.11
390.000	580.12	580.12	580.12	580.12	580.13
405.000	580.13	580.13	580.13	580.14	580.14
420.000	580.14	580.15	580.15	580.15	580.15
435.000	580.16	580.16	580.16	580.17	580.17
450.000	580.17	580.18	580.18	580.18	580.19
465.000	580.19	580.20	580.20	580.20	580.21
480.000	580.21	580.22	580.22	580.23	580.23
495.000	580.23	580.24	580.24	580.25	580.25
510.000	580.26	580.27	580.27	580.28	580.28
525.000	580.29	580.29	580.30	580.31	580.31
540.000	580.32	580.33	580.33	580.34	580.35
555.000	580.36	580.37	580.38	580.39	580.41
570.000	580.42	580.43	580.45	580.46	580.47
585.000	580.49	580.50	580.52	580.53	580.55
600.000	580.56	580.57	580.59	580.60	580.62
615.000	580.63	580.65	580.66	580.68	580.70

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	580.71	580.73	580.75	580.77	580.79
645.000	580.81	580.84	580.86	580.89	580.92
660.000	580.95	580.98	581.01	581.05	581.09
675.000	581.13	581.17	581.21	581.26	581.31
690.000	581.36	581.42	581.48	581.54	581.61
705.000	581.69	581.78	581.89	582.01	582.18
720.000	582.39	582.69	583.11	583.62	584.17
735.000	584.65	585.04	585.32	585.49	585.58
750.000	585.58	585.52	585.42	585.28	585.13
765.000	584.96	584.79	584.61	584.44	584.27
780.000	584.11	583.96	583.81	583.67	583.54
795.000	583.41	583.29	583.18	583.07	582.97
810.000	582.88	582.79	582.70	582.62	582.54
825.000	582.46	582.39	582.32	582.25	582.19
840.000	582.13	582.07	582.02	581.97	581.92
855.000	581.88	581.83	581.79	581.75	581.72
870.000	581.68	581.65	581.62	581.59	581.56
885.000	581.53	581.51	581.48	581.46	581.44
900.000	581.41	581.39	581.37	581.35	581.33
915.000	581.31	581.29	581.26	581.24	581.22
930.000	581.20	581.18	581.16	581.14	581.13
945.000	581.11	581.09	581.08	581.06	581.05
960.000	581.03	581.02	581.01	580.99	580.98
975.000	580.97	580.96	580.95	580.94	580.93
990.000	580.92	580.91	580.90	580.89	580.88
1,005.000	580.87	580.86	580.85	580.85	580.84
1,020.000	580.83	580.82	580.82	580.81	580.80
1,035.000	580.79	580.79	580.78	580.78	580.77
1,050.000	580.76	580.76	580.75	580.75	580.74
1,065.000	580.74	580.73	580.73	580.72	580.72
1,080.000	580.71	580.71	580.70	580.70	580.69
1,095.000	580.69	580.68	580.68	580.67	580.67
1,110.000	580.66	580.66	580.65	580.65	580.64
1,125.000	580.64	580.63	580.63	580.62	580.62
1,140.000	580.61	580.61	580.61	580.60	580.60
1,155.000	580.59	580.59	580.59	580.58	580.58
1,170.000	580.58	580.57	580.57	580.57	580.57
1,185.000	580.56	580.56	580.56	580.55	580.55
1,200.000	580.55	580.55	580.54	580.54	580.54
1,215.000	580.54	580.54	580.53	580.53	580.53
1,230.000	580.53	580.53	580.52	580.52	580.52
1,245.000	580.52	580.52	580.51	580.51	580.51

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	580.51	580.51	580.51	580.50	580.50
1,275.000	580.50	580.50	580.50	580.50	580.49
1,290.000	580.49	580.49	580.49	580.49	580.49
1,305.000	580.48	580.48	580.48	580.48	580.48
1,320.000	580.48	580.47	580.47	580.47	580.47
1,335.000	580.47	580.47	580.46	580.46	580.46
1,350.000	580.46	580.46	580.46	580.45	580.45
1,365.000	580.45	580.45	580.45	580.45	580.44
1,380.000	580.44	580.44	580.44	580.44	580.44
1,395.000	580.43	580.43	580.43	580.43	580.43
1,410.000	580.43	580.43	580.42	580.42	580.42
1,425.000	580.42	580.42	580.42	580.41	580.41
1,440.000	580.41	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	580.00	580.00	580.00	580.00	580.00
15.000	580.00	580.00	580.00	580.00	580.00
30.000	580.00	580.00	580.00	580.00	580.00
45.000	580.00	580.00	580.00	580.00	580.00
60.000	580.00	580.00	580.00	580.00	580.00
75.000	580.00	580.00	580.00	580.00	580.00
90.000	580.00	580.00	580.00	580.00	580.00
105.000	580.00	580.00	580.00	580.00	580.00
120.000	580.00	580.00	580.00	580.00	580.00
135.000	580.00	580.00	580.01	580.01	580.01
150.000	580.01	580.01	580.01	580.01	580.01
165.000	580.01	580.02	580.02	580.02	580.02
180.000	580.02	580.02	580.02	580.02	580.03
195.000	580.03	580.03	580.03	580.03	580.03
210.000	580.04	580.04	580.04	580.04	580.04
225.000	580.05	580.05	580.05	580.05	580.05
240.000	580.05	580.06	580.06	580.06	580.06
255.000	580.06	580.07	580.07	580.07	580.07
270.000	580.07	580.08	580.08	580.08	580.08
285.000	580.09	580.09	580.09	580.09	580.09
300.000	580.10	580.10	580.10	580.10	580.10
315.000	580.11	580.11	580.11	580.11	580.12
330.000	580.12	580.12	580.12	580.13	580.13
345.000	580.13	580.13	580.14	580.14	580.14
360.000	580.14	580.14	580.15	580.15	580.15
375.000	580.16	580.16	580.17	580.17	580.17
390.000	580.18	580.18	580.19	580.19	580.20
405.000	580.20	580.21	580.21	580.22	580.22
420.000	580.23	580.23	580.24	580.24	580.25
435.000	580.25	580.26	580.26	580.27	580.28
450.000	580.28	580.29	580.29	580.30	580.31
465.000	580.31	580.32	580.33	580.33	580.34
480.000	580.35	580.35	580.36	580.37	580.38
495.000	580.38	580.39	580.40	580.41	580.42
510.000	580.42	580.43	580.44	580.45	580.46
525.000	580.47	580.48	580.49	580.50	580.51
540.000	580.52	580.53	580.54	580.55	580.56
555.000	580.57	580.59	580.61	580.62	580.64
570.000	580.66	580.67	580.69	580.71	580.73
585.000	580.75	580.76	580.78	580.80	580.82
600.000	580.84	580.86	580.88	580.90	580.92
615.000	580.94	580.96	580.98	581.00	581.02

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	581.04	581.06	581.09	581.11	581.14
645.000	581.18	581.21	581.25	581.29	581.33
660.000	581.37	581.41	581.46	581.50	581.55
675.000	581.61	581.66	581.72	581.78	581.85
690.000	581.92	581.99	582.06	582.14	582.23
705.000	582.32	582.42	582.54	582.69	582.88
720.000	583.13	583.49	583.99	584.61	585.25
735.000	585.83	586.28	586.58	586.76	586.83
750.000	586.80	586.70	586.54	586.35	586.14
765.000	585.92	585.70	585.49	585.27	585.07
780.000	584.88	584.69	584.52	584.36	584.20
795.000	584.05	583.91	583.78	583.66	583.54
810.000	583.43	583.33	583.23	583.13	583.04
825.000	582.95	582.87	582.78	582.71	582.63
840.000	582.56	582.49	582.43	582.37	582.31
855.000	582.25	582.20	582.16	582.11	582.07
870.000	582.03	581.99	581.95	581.92	581.88
885.000	581.85	581.82	581.79	581.76	581.74
900.000	581.71	581.69	581.66	581.64	581.62
915.000	581.59	581.56	581.54	581.51	581.48
930.000	581.46	581.43	581.41	581.38	581.36
945.000	581.34	581.32	581.30	581.28	581.26
960.000	581.24	581.22	581.21	581.19	581.17
975.000	581.16	581.15	581.13	581.12	581.11
990.000	581.10	581.08	581.07	581.06	581.05
1,005.000	581.04	581.03	581.02	581.02	581.01
1,020.000	581.00	580.99	580.98	580.98	580.97
1,035.000	580.96	580.95	580.95	580.94	580.93
1,050.000	580.93	580.92	580.91	580.91	580.90
1,065.000	580.89	580.89	580.88	580.88	580.87
1,080.000	580.87	580.86	580.85	580.85	580.84
1,095.000	580.84	580.83	580.82	580.81	580.81
1,110.000	580.80	580.79	580.78	580.78	580.77
1,125.000	580.76	580.76	580.75	580.74	580.74
1,140.000	580.73	580.73	580.72	580.71	580.71
1,155.000	580.70	580.70	580.69	580.69	580.69
1,170.000	580.68	580.68	580.67	580.67	580.66
1,185.000	580.66	580.66	580.65	580.65	580.65
1,200.000	580.64	580.64	580.64	580.63	580.63
1,215.000	580.63	580.62	580.62	580.62	580.61
1,230.000	580.61	580.61	580.61	580.60	580.60
1,245.000	580.60	580.60	580.59	580.59	580.59

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	580.59	580.59	580.58	580.58	580.58
1,275.000	580.58	580.57	580.57	580.57	580.57
1,290.000	580.57	580.57	580.56	580.56	580.56
1,305.000	580.56	580.56	580.55	580.55	580.55
1,320.000	580.55	580.55	580.55	580.54	580.54
1,335.000	580.54	580.54	580.54	580.54	580.54
1,350.000	580.53	580.53	580.53	580.53	580.53
1,365.000	580.53	580.53	580.52	580.52	580.52
1,380.000	580.52	580.52	580.52	580.52	580.52
1,395.000	580.51	580.51	580.51	580.51	580.51
1,410.000	580.51	580.51	580.51	580.50	580.50
1,425.000	580.50	580.50	580.50	580.50	580.50
1,440.000	580.50	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	580.00	580.00	580.00	580.00	580.00
15.000	580.00	580.00	580.00	580.00	580.00
30.000	580.00	580.00	580.00	580.00	580.00
45.000	580.00	580.00	580.00	580.00	580.00
60.000	580.00	580.00	580.00	580.00	580.00
75.000	580.00	580.00	580.00	580.00	580.00
90.000	580.00	580.00	580.00	580.00	580.00
105.000	580.00	580.00	580.01	580.01	580.01
120.000	580.01	580.01	580.01	580.01	580.02
135.000	580.02	580.02	580.02	580.02	580.03
150.000	580.03	580.03	580.03	580.03	580.04
165.000	580.04	580.04	580.04	580.05	580.05
180.000	580.05	580.05	580.06	580.06	580.06
195.000	580.06	580.07	580.07	580.07	580.08
210.000	580.08	580.08	580.08	580.09	580.09
225.000	580.09	580.10	580.10	580.10	580.10
240.000	580.11	580.11	580.11	580.12	580.12
255.000	580.12	580.13	580.13	580.13	580.13
270.000	580.14	580.14	580.14	580.15	580.15
285.000	580.15	580.16	580.16	580.16	580.17
300.000	580.17	580.17	580.18	580.18	580.18
315.000	580.19	580.19	580.19	580.20	580.20
330.000	580.21	580.21	580.21	580.22	580.22
345.000	580.23	580.23	580.24	580.24	580.25
360.000	580.25	580.26	580.26	580.27	580.27
375.000	580.28	580.29	580.30	580.30	580.31
390.000	580.32	580.33	580.34	580.35	580.36
405.000	580.37	580.38	580.39	580.40	580.41
420.000	580.42	580.43	580.44	580.45	580.46
435.000	580.47	580.48	580.49	580.50	580.51
450.000	580.52	580.53	580.54	580.55	580.56
465.000	580.57	580.58	580.59	580.60	580.61
480.000	580.62	580.63	580.64	580.65	580.66
495.000	580.67	580.68	580.69	580.70	580.71
510.000	580.72	580.73	580.74	580.75	580.76
525.000	580.77	580.78	580.79	580.81	580.82
540.000	580.83	580.84	580.85	580.87	580.89
555.000	580.91	580.94	580.96	580.99	581.02
570.000	581.05	581.08	581.11	581.14	581.17
585.000	581.20	581.23	581.26	581.29	581.32
600.000	581.35	581.38	581.41	581.44	581.47
615.000	581.50	581.53	581.56	581.59	581.62

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	581.65	581.68	581.71	581.75	581.80
645.000	581.85	581.90	581.96	582.02	582.09
660.000	582.15	582.22	582.28	582.36	582.43
675.000	582.51	582.59	582.67	582.76	582.85
690.000	582.94	583.04	583.14	583.24	583.34
705.000	583.44	583.55	583.69	583.85	584.07
720.000	584.37	584.81	585.43	586.20	586.99
735.000	587.69	588.21	588.55	588.72	588.75
750.000	588.66	588.47	588.23	587.95	587.66
765.000	587.37	587.09	586.81	586.55	586.30
780.000	586.07	585.85	585.65	585.45	585.27
795.000	585.10	584.93	584.78	584.64	584.50
810.000	584.37	584.25	584.14	584.03	583.92
825.000	583.81	583.70	583.60	583.50	583.41
840.000	583.31	583.23	583.15	583.07	583.00
855.000	582.93	582.87	582.81	582.75	582.70
870.000	582.64	582.60	582.55	582.51	582.47
885.000	582.43	582.39	582.36	582.33	582.29
900.000	582.26	582.23	582.20	582.17	582.14
915.000	582.10	582.06	582.03	581.99	581.95
930.000	581.91	581.87	581.83	581.80	581.77
945.000	581.73	581.70	581.67	581.65	581.62
960.000	581.59	581.57	581.55	581.53	581.51
975.000	581.49	581.47	581.45	581.43	581.42
990.000	581.40	581.39	581.37	581.36	581.34
1,005.000	581.33	581.32	581.30	581.29	581.28
1,020.000	581.27	581.26	581.25	581.24	581.23
1,035.000	581.22	581.21	581.20	581.19	581.18
1,050.000	581.17	581.17	581.16	581.15	581.14
1,065.000	581.14	581.13	581.12	581.12	581.11
1,080.000	581.10	581.10	581.09	581.08	581.07
1,095.000	581.07	581.06	581.05	581.04	581.03
1,110.000	581.01	581.00	580.99	580.98	580.98
1,125.000	580.97	580.96	580.95	580.94	580.93
1,140.000	580.92	580.91	580.91	580.90	580.89
1,155.000	580.88	580.88	580.87	580.86	580.86
1,170.000	580.85	580.84	580.84	580.83	580.83
1,185.000	580.82	580.82	580.81	580.81	580.80
1,200.000	580.80	580.79	580.79	580.78	580.78
1,215.000	580.77	580.77	580.77	580.76	580.76
1,230.000	580.75	580.75	580.75	580.74	580.74
1,245.000	580.74	580.73	580.73	580.73	580.72

Subsection: Time vs. Elevation
Label: POND B (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	580.72	580.72	580.71	580.71	580.71
1,275.000	580.70	580.70	580.70	580.70	580.69
1,290.000	580.69	580.69	580.69	580.68	580.68
1,305.000	580.68	580.68	580.67	580.67	580.67
1,320.000	580.67	580.66	580.66	580.66	580.66
1,335.000	580.66	580.65	580.65	580.65	580.65
1,350.000	580.65	580.64	580.64	580.64	580.64
1,365.000	580.64	580.63	580.63	580.63	580.63
1,380.000	580.63	580.62	580.62	580.62	580.62
1,395.000	580.62	580.62	580.61	580.61	580.61
1,410.000	580.61	580.61	580.61	580.60	580.60
1,425.000	580.60	580.60	580.60	580.60	580.60
1,440.000	580.59	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.00	595.00	595.00	595.00	595.00
390.000	595.00	595.00	595.00	595.00	595.00
405.000	595.00	595.00	595.00	595.00	595.00
420.000	595.00	595.00	595.00	595.00	595.00
435.000	595.00	595.00	595.00	595.00	595.00
450.000	595.00	595.00	595.00	595.00	595.00
465.000	595.00	595.00	595.00	595.00	595.00
480.000	595.00	595.00	595.00	595.00	595.00
495.000	595.00	595.00	595.00	595.00	595.00
510.000	595.00	595.00	595.00	595.00	595.00
525.000	595.00	595.00	595.00	595.00	595.01
540.000	595.01	595.01	595.01	595.01	595.01
555.000	595.01	595.01	595.01	595.01	595.01
570.000	595.01	595.01	595.02	595.02	595.02
585.000	595.02	595.03	595.03	595.03	595.03
600.000	595.03	595.03	595.03	595.03	595.04
615.000	595.04	595.05	595.05	595.05	595.05

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.05	595.05	595.05	595.05	595.05
645.000	595.05	595.06	595.06	595.07	595.07
660.000	595.07	595.08	595.08	595.09	595.09
675.000	595.10	595.10	595.11	595.11	595.12
690.000	595.13	595.14	595.14	595.15	595.17
705.000	595.18	595.19	595.22	595.25	595.29
720.000	595.35	595.43	595.53	595.65	595.78
735.000	595.92	596.07	596.20	596.31	596.39
750.000	596.45	596.48	596.49	596.48	596.44
765.000	596.38	596.31	596.24	596.16	596.08
780.000	596.01	595.94	595.88	595.82	595.77
795.000	595.71	595.66	595.61	595.57	595.54
810.000	595.51	595.49	595.47	595.45	595.43
825.000	595.40	595.37	595.35	595.33	595.32
840.000	595.32	595.32	595.32	595.32	595.32
855.000	595.30	595.29	595.27	595.26	595.25
870.000	595.24	595.25	595.25	595.26	595.26
885.000	595.27	595.26	595.26	595.25	595.23
900.000	595.21	595.20	595.18	595.17	595.16
915.000	595.16	595.17	595.18	595.20	595.22
930.000	595.23	595.23	595.23	595.22	595.21
945.000	595.20	595.18	595.17	595.15	595.13
960.000	595.11	595.10	595.10	595.10	595.12
975.000	595.14	595.16	595.18	595.20	595.21
990.000	595.21	595.21	595.20	595.19	595.18
1,005.000	595.17	595.15	595.13	595.12	595.10
1,020.000	595.09	595.08	595.07	595.06	595.05
1,035.000	595.05	595.05	595.05	595.06	595.08
1,050.000	595.10	595.13	595.15	595.18	595.19
1,065.000	595.20	595.20	595.20	595.19	595.18
1,080.000	595.16	595.15	595.13	595.12	595.10
1,095.000	595.09	595.08	595.07	595.06	595.05
1,110.000	595.05	595.04	595.04	595.03	595.03
1,125.000	595.03	595.02	595.02	595.02	595.02
1,140.000	595.02	595.02	595.02	595.02	595.03
1,155.000	595.05	595.08	595.10	595.13	595.16
1,170.000	595.18	595.19	595.20	595.20	595.19
1,185.000	595.18	595.17	595.15	595.14	595.12
1,200.000	595.11	595.09	595.08	595.07	595.06
1,215.000	595.06	595.05	595.04	595.04	595.03
1,230.000	595.03	595.03	595.03	595.02	595.02
1,245.000	595.02	595.02	595.02	595.02	595.02

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.01	595.01	595.01	595.01	595.01
1,275.000	595.01	595.01	595.01	595.02	595.03
1,290.000	595.05	595.07	595.10	595.13	595.16
1,305.000	595.18	595.19	595.20	595.20	595.19
1,320.000	595.18	595.17	595.16	595.14	595.13
1,335.000	595.11	595.09	595.08	595.07	595.06
1,350.000	595.06	595.05	595.04	595.04	595.03
1,365.000	595.03	595.03	595.03	595.02	595.02
1,380.000	595.02	595.02	595.02	595.02	595.02
1,395.000	595.01	595.01	595.01	595.01	595.01
1,410.000	595.01	595.01	595.01	595.01	595.01
1,425.000	595.01	595.01	595.01	595.01	595.01
1,440.000	595.01	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.00	595.00	595.00	595.00	595.00
390.000	595.00	595.00	595.00	595.00	595.00
405.000	595.00	595.00	595.00	595.00	595.00
420.000	595.00	595.00	595.00	595.00	595.01
435.000	595.01	595.01	595.01	595.01	595.01
450.000	595.01	595.02	595.02	595.02	595.02
465.000	595.02	595.02	595.02	595.02	595.02
480.000	595.02	595.03	595.03	595.03	595.03
495.000	595.03	595.03	595.03	595.03	595.04
510.000	595.04	595.04	595.04	595.04	595.04
525.000	595.04	595.05	595.05	595.05	595.05
540.000	595.05	595.05	595.06	595.06	595.06
555.000	595.06	595.07	595.07	595.07	595.08
570.000	595.08	595.09	595.09	595.09	595.10
585.000	595.10	595.11	595.11	595.12	595.12
600.000	595.13	595.13	595.14	595.14	595.15
615.000	595.15	595.16	595.16	595.17	595.18

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.18	595.19	595.20	595.20	595.21
645.000	595.22	595.23	595.25	595.26	595.27
660.000	595.29	595.30	595.32	595.34	595.36
675.000	595.38	595.40	595.43	595.45	595.48
690.000	595.51	595.55	595.58	595.62	595.67
705.000	595.72	595.78	595.85	595.93	596.05
720.000	596.20	596.42	596.68	596.84	596.98
735.000	597.13	597.27	597.39	597.48	597.54
750.000	597.57	597.57	597.55	597.51	597.45
765.000	597.39	597.31	597.23	597.15	597.08
780.000	597.01	596.94	596.89	596.84	596.79
795.000	596.75	596.71	596.67	596.60	596.52
810.000	596.44	596.36	596.29	596.22	596.15
825.000	596.09	596.04	595.99	595.94	595.89
840.000	595.85	595.81	595.78	595.74	595.71
855.000	595.69	595.66	595.64	595.62	595.60
870.000	595.58	595.56	595.54	595.53	595.52
885.000	595.50	595.49	595.48	595.47	595.46
900.000	595.45	595.45	595.44	595.43	595.42
915.000	595.41	595.40	595.39	595.38	595.37
930.000	595.36	595.35	595.34	595.33	595.32
945.000	595.31	595.31	595.30	595.29	595.29
960.000	595.28	595.28	595.27	595.27	595.26
975.000	595.26	595.26	595.25	595.25	595.25
990.000	595.24	595.24	595.24	595.24	595.23
1,005.000	595.23	595.23	595.23	595.23	595.22
1,020.000	595.22	595.22	595.22	595.22	595.21
1,035.000	595.21	595.21	595.21	595.21	595.20
1,050.000	595.20	595.20	595.20	595.20	595.20
1,065.000	595.20	595.19	595.19	595.19	595.19
1,080.000	595.19	595.19	595.19	595.18	595.18
1,095.000	595.18	595.18	595.18	595.17	595.17
1,110.000	595.17	595.17	595.17	595.16	595.16
1,125.000	595.16	595.16	595.16	595.15	595.15
1,140.000	595.15	595.15	595.15	595.15	595.15
1,155.000	595.14	595.14	595.14	595.14	595.14
1,170.000	595.14	595.14	595.14	595.14	595.14
1,185.000	595.14	595.14	595.13	595.13	595.13
1,200.000	595.13	595.13	595.13	595.13	595.13
1,215.000	595.13	595.13	595.13	595.13	595.13
1,230.000	595.13	595.13	595.12	595.12	595.12
1,245.000	595.12	595.12	595.12	595.12	595.12

Subsection: Time vs. Elevation
 Label: R-1
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.12	595.12	595.12	595.12	595.12
1,275.000	595.12	595.12	595.12	595.12	595.12
1,290.000	595.12	595.11	595.11	595.11	595.11
1,305.000	595.11	595.11	595.11	595.11	595.11
1,320.000	595.11	595.11	595.11	595.11	595.11
1,335.000	595.11	595.11	595.11	595.11	595.11
1,350.000	595.11	595.11	595.11	595.11	595.10
1,365.000	595.10	595.10	595.10	595.10	595.10
1,380.000	595.10	595.10	595.10	595.10	595.10
1,395.000	595.10	595.10	595.10	595.10	595.10
1,410.000	595.10	595.10	595.10	595.10	595.10
1,425.000	595.10	595.10	595.10	595.10	595.10
1,440.000	595.10	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.00	595.00	595.00	595.00
315.000	595.00	595.00	595.00	595.00	595.00
330.000	595.00	595.00	595.00	595.00	595.00
345.000	595.00	595.00	595.00	595.00	595.00
360.000	595.00	595.00	595.00	595.00	595.00
375.000	595.01	595.01	595.01	595.01	595.01
390.000	595.01	595.02	595.02	595.02	595.02
405.000	595.02	595.02	595.02	595.02	595.02
420.000	595.03	595.03	595.03	595.03	595.03
435.000	595.03	595.03	595.04	595.04	595.04
450.000	595.04	595.04	595.04	595.05	595.05
465.000	595.05	595.05	595.05	595.05	595.06
480.000	595.06	595.06	595.06	595.06	595.07
495.000	595.07	595.07	595.07	595.08	595.08
510.000	595.08	595.08	595.08	595.09	595.09
525.000	595.09	595.09	595.10	595.10	595.10
540.000	595.11	595.11	595.11	595.12	595.12
555.000	595.12	595.13	595.14	595.14	595.15
570.000	595.16	595.17	595.17	595.18	595.19
585.000	595.20	595.21	595.22	595.22	595.23
600.000	595.24	595.25	595.26	595.26	595.27
615.000	595.28	595.29	595.30	595.31	595.32

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.33	595.34	595.35	595.37	595.38
645.000	595.40	595.42	595.44	595.46	595.49
660.000	595.51	595.54	595.57	595.60	595.63
675.000	595.66	595.70	595.74	595.78	595.82
690.000	595.87	595.92	595.97	596.03	596.09
705.000	596.16	596.24	596.33	596.44	596.60
720.000	596.73	596.84	596.97	597.13	597.31
735.000	597.50	597.68	597.83	597.94	598.02
750.000	598.05	598.04	598.01	597.95	597.88
765.000	597.79	597.69	597.58	597.48	597.39
780.000	597.30	597.22	597.14	597.08	597.02
795.000	596.97	596.92	596.87	596.83	596.79
810.000	596.76	596.72	596.69	596.66	596.60
825.000	596.54	596.47	596.40	596.33	596.27
840.000	596.21	596.15	596.10	596.06	596.01
855.000	595.97	595.94	595.90	595.87	595.84
870.000	595.82	595.79	595.77	595.75	595.73
885.000	595.71	595.70	595.68	595.67	595.66
900.000	595.64	595.63	595.62	595.61	595.60
915.000	595.58	595.57	595.55	595.54	595.52
930.000	595.50	595.49	595.47	595.46	595.45
945.000	595.43	595.42	595.41	595.40	595.39
960.000	595.39	595.38	595.37	595.37	595.36
975.000	595.35	595.35	595.34	595.34	595.34
990.000	595.33	595.33	595.32	595.32	595.32
1,005.000	595.31	595.31	595.31	595.30	595.30
1,020.000	595.30	595.30	595.29	595.29	595.29
1,035.000	595.28	595.28	595.28	595.28	595.28
1,050.000	595.27	595.27	595.27	595.27	595.26
1,065.000	595.26	595.26	595.26	595.26	595.26
1,080.000	595.25	595.25	595.25	595.25	595.24
1,095.000	595.24	595.24	595.24	595.23	595.23
1,110.000	595.22	595.22	595.22	595.21	595.21
1,125.000	595.21	595.20	595.20	595.20	595.20
1,140.000	595.19	595.19	595.19	595.19	595.19
1,155.000	595.19	595.18	595.18	595.18	595.18
1,170.000	595.18	595.18	595.18	595.18	595.17
1,185.000	595.17	595.17	595.17	595.17	595.17
1,200.000	595.17	595.17	595.17	595.17	595.16
1,215.000	595.16	595.16	595.16	595.16	595.16
1,230.000	595.16	595.16	595.16	595.16	595.16
1,245.000	595.16	595.16	595.15	595.15	595.15

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.15	595.15	595.15	595.15	595.15
1,275.000	595.15	595.15	595.15	595.15	595.15
1,290.000	595.15	595.15	595.14	595.14	595.14
1,305.000	595.14	595.14	595.14	595.14	595.14
1,320.000	595.14	595.14	595.14	595.14	595.14
1,335.000	595.14	595.14	595.14	595.14	595.13
1,350.000	595.13	595.13	595.13	595.13	595.13
1,365.000	595.13	595.13	595.13	595.13	595.13
1,380.000	595.13	595.13	595.13	595.13	595.13
1,395.000	595.13	595.13	595.13	595.13	595.13
1,410.000	595.12	595.12	595.12	595.12	595.12
1,425.000	595.12	595.12	595.12	595.12	595.12
1,440.000	595.12	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	595.00	595.00	595.00	595.00	595.00
15.000	595.00	595.00	595.00	595.00	595.00
30.000	595.00	595.00	595.00	595.00	595.00
45.000	595.00	595.00	595.00	595.00	595.00
60.000	595.00	595.00	595.00	595.00	595.00
75.000	595.00	595.00	595.00	595.00	595.00
90.000	595.00	595.00	595.00	595.00	595.00
105.000	595.00	595.00	595.00	595.00	595.00
120.000	595.00	595.00	595.00	595.00	595.00
135.000	595.00	595.00	595.00	595.00	595.00
150.000	595.00	595.00	595.00	595.00	595.00
165.000	595.00	595.00	595.00	595.00	595.00
180.000	595.00	595.00	595.00	595.00	595.00
195.000	595.00	595.00	595.00	595.00	595.00
210.000	595.00	595.00	595.00	595.00	595.00
225.000	595.00	595.00	595.00	595.00	595.00
240.000	595.00	595.00	595.00	595.00	595.00
255.000	595.00	595.00	595.00	595.00	595.00
270.000	595.00	595.00	595.00	595.00	595.00
285.000	595.00	595.00	595.00	595.00	595.00
300.000	595.00	595.01	595.01	595.01	595.01
315.000	595.01	595.01	595.02	595.02	595.02
330.000	595.02	595.02	595.02	595.02	595.02
345.000	595.02	595.03	595.03	595.03	595.03
360.000	595.03	595.03	595.03	595.04	595.04
375.000	595.04	595.04	595.04	595.05	595.05
390.000	595.05	595.05	595.06	595.06	595.06
405.000	595.06	595.07	595.07	595.07	595.08
420.000	595.08	595.08	595.08	595.09	595.09
435.000	595.09	595.10	595.10	595.10	595.10
450.000	595.11	595.11	595.11	595.12	595.12
465.000	595.12	595.13	595.13	595.13	595.14
480.000	595.14	595.14	595.15	595.15	595.15
495.000	595.16	595.16	595.17	595.17	595.17
510.000	595.18	595.18	595.19	595.19	595.20
525.000	595.20	595.20	595.21	595.21	595.22
540.000	595.22	595.23	595.24	595.24	595.25
555.000	595.26	595.27	595.29	595.30	595.32
570.000	595.34	595.36	595.37	595.39	595.41
585.000	595.43	595.45	595.46	595.48	595.50
600.000	595.51	595.53	595.54	595.56	595.58
615.000	595.59	595.61	595.63	595.65	595.66

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	595.68	595.70	595.73	595.75	595.78
645.000	595.81	595.85	595.89	595.93	595.98
660.000	596.03	596.08	596.13	596.19	596.25
675.000	596.31	596.37	596.44	596.51	596.59
690.000	596.66	596.70	596.73	596.76	596.79
705.000	596.82	596.86	596.90	596.95	597.02
720.000	597.11	597.25	597.42	597.64	597.88
735.000	598.13	598.36	598.48	598.56	598.60
750.000	598.61	598.60	598.57	598.52	598.46
765.000	598.40	598.31	598.18	598.05	597.92
780.000	597.80	597.70	597.61	597.52	597.44
795.000	597.37	597.31	597.25	597.19	597.14
810.000	597.10	597.05	597.01	596.97	596.94
825.000	596.90	596.87	596.83	596.80	596.77
840.000	596.74	596.72	596.69	596.67	596.62
855.000	596.56	596.51	596.45	596.41	596.36
870.000	596.32	596.28	596.25	596.21	596.18
885.000	596.15	596.13	596.11	596.08	596.06
900.000	596.04	596.03	596.01	595.99	595.97
915.000	595.94	595.92	595.89	595.86	595.83
930.000	595.80	595.77	595.74	595.72	595.69
945.000	595.67	595.65	595.63	595.61	595.60
960.000	595.58	595.57	595.56	595.55	595.54
975.000	595.53	595.52	595.51	595.50	595.50
990.000	595.49	595.48	595.48	595.47	595.47
1,005.000	595.46	595.46	595.45	595.45	595.44
1,020.000	595.44	595.43	595.43	595.43	595.42
1,035.000	595.42	595.41	595.41	595.41	595.40
1,050.000	595.40	595.40	595.39	595.39	595.39
1,065.000	595.39	595.38	595.38	595.38	595.37
1,080.000	595.37	595.37	595.37	595.36	595.36
1,095.000	595.35	595.35	595.34	595.34	595.33
1,110.000	595.32	595.31	595.31	595.30	595.30
1,125.000	595.29	595.28	595.28	595.28	595.27
1,140.000	595.27	595.26	595.26	595.26	595.26
1,155.000	595.25	595.25	595.25	595.25	595.25
1,170.000	595.24	595.24	595.24	595.24	595.24
1,185.000	595.24	595.23	595.23	595.23	595.23
1,200.000	595.23	595.23	595.23	595.22	595.22
1,215.000	595.22	595.22	595.22	595.22	595.22
1,230.000	595.22	595.22	595.21	595.21	595.21
1,245.000	595.21	595.21	595.21	595.21	595.21

Subsection: Time vs. Elevation
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	595.21	595.21	595.20	595.20	595.20
1,275.000	595.20	595.20	595.20	595.20	595.20
1,290.000	595.20	595.20	595.20	595.19	595.19
1,305.000	595.19	595.19	595.19	595.19	595.19
1,320.000	595.19	595.19	595.19	595.19	595.19
1,335.000	595.18	595.18	595.18	595.18	595.18
1,350.000	595.18	595.18	595.18	595.18	595.18
1,365.000	595.18	595.18	595.18	595.18	595.17
1,380.000	595.17	595.17	595.17	595.17	595.17
1,395.000	595.17	595.17	595.17	595.17	595.17
1,410.000	595.17	595.17	595.17	595.17	595.17
1,425.000	595.16	595.16	595.16	595.16	595.16
1,440.000	595.16	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.50	587.50	587.50
390.000	587.50	587.50	587.50	587.50	587.50
405.000	587.50	587.50	587.50	587.50	587.50
420.000	587.50	587.50	587.50	587.50	587.50
435.000	587.50	587.50	587.50	587.50	587.50
450.000	587.50	587.50	587.50	587.50	587.50
465.000	587.50	587.50	587.50	587.50	587.50
480.000	587.50	587.50	587.50	587.50	587.50
495.000	587.50	587.50	587.50	587.50	587.50
510.000	587.50	587.50	587.50	587.50	587.50
525.000	587.50	587.50	587.50	587.50	587.50
540.000	587.50	587.51	587.51	587.51	587.51
555.000	587.51	587.51	587.51	587.51	587.51
570.000	587.51	587.52	587.52	587.52	587.53
585.000	587.53	587.53	587.53	587.53	587.53
600.000	587.53	587.54	587.54	587.54	587.55
615.000	587.55	587.56	587.56	587.57	587.57

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.57	587.57	587.57	587.57	587.57
645.000	587.57	587.58	587.58	587.59	587.60
660.000	587.60	587.61	587.61	587.62	587.63
675.000	587.64	587.64	587.65	587.66	587.67
690.000	587.68	587.69	587.70	587.72	587.73
705.000	587.75	587.77	587.80	587.84	587.90
720.000	587.97	588.07	588.21	588.37	588.55
735.000	588.76	588.97	589.17	589.27	589.32
750.000	589.36	589.38	589.39	589.38	589.36
765.000	589.33	589.29	589.24	589.20	589.13
780.000	589.04	588.94	588.85	588.76	588.67
795.000	588.59	588.52	588.44	588.38	588.32
810.000	588.27	588.24	588.21	588.18	588.14
825.000	588.11	588.07	588.03	588.01	587.98
840.000	587.97	587.97	587.97	587.97	587.96
855.000	587.95	587.94	587.91	587.89	587.87
870.000	587.86	587.85	587.86	587.87	587.88
885.000	587.89	587.89	587.88	587.87	587.86
900.000	587.83	587.81	587.78	587.76	587.74
915.000	587.73	587.74	587.75	587.78	587.80
930.000	587.82	587.84	587.84	587.84	587.83
945.000	587.81	587.78	587.76	587.73	587.70
960.000	587.68	587.66	587.65	587.65	587.66
975.000	587.68	587.71	587.75	587.78	587.80
990.000	587.81	587.81	587.81	587.80	587.78
1,005.000	587.76	587.73	587.70	587.68	587.66
1,020.000	587.63	587.62	587.60	587.59	587.58
1,035.000	587.57	587.56	587.57	587.58	587.60
1,050.000	587.63	587.66	587.70	587.74	587.77
1,065.000	587.79	587.80	587.80	587.79	587.77
1,080.000	587.75	587.73	587.70	587.68	587.66
1,095.000	587.64	587.62	587.60	587.59	587.58
1,110.000	587.57	587.56	587.55	587.55	587.54
1,125.000	587.54	587.53	587.53	587.53	587.52
1,140.000	587.52	587.52	587.52	587.53	587.54
1,155.000	587.56	587.59	587.62	587.66	587.71
1,170.000	587.74	587.77	587.79	587.79	587.79
1,185.000	587.78	587.76	587.74	587.72	587.69
1,200.000	587.67	587.65	587.63	587.61	587.59
1,215.000	587.58	587.57	587.56	587.56	587.55
1,230.000	587.54	587.54	587.53	587.53	587.53
1,245.000	587.53	587.52	587.52	587.52	587.52

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.52	587.52	587.52	587.51	587.51
1,275.000	587.51	587.51	587.51	587.52	587.53
1,290.000	587.55	587.58	587.62	587.66	587.71
1,305.000	587.75	587.77	587.79	587.80	587.79
1,320.000	587.78	587.77	587.74	587.72	587.69
1,335.000	587.67	587.65	587.63	587.61	587.59
1,350.000	587.58	587.57	587.56	587.56	587.55
1,365.000	587.54	587.54	587.53	587.53	587.53
1,380.000	587.53	587.52	587.52	587.52	587.52
1,395.000	587.52	587.52	587.52	587.51	587.51
1,410.000	587.51	587.51	587.51	587.51	587.51
1,425.000	587.51	587.51	587.51	587.51	587.51
1,440.000	587.51	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.50	587.50	587.50
390.000	587.50	587.50	587.50	587.50	587.50
405.000	587.50	587.50	587.50	587.50	587.50
420.000	587.50	587.50	587.50	587.50	587.50
435.000	587.50	587.51	587.51	587.51	587.51
450.000	587.52	587.52	587.52	587.52	587.52
465.000	587.52	587.52	587.53	587.53	587.53
480.000	587.53	587.53	587.53	587.54	587.54
495.000	587.54	587.54	587.54	587.54	587.55
510.000	587.55	587.55	587.55	587.56	587.56
525.000	587.56	587.56	587.56	587.57	587.57
540.000	587.57	587.57	587.58	587.58	587.58
555.000	587.59	587.59	587.60	587.60	587.61
570.000	587.61	587.62	587.63	587.63	587.64
585.000	587.65	587.65	587.66	587.67	587.67
600.000	587.68	587.69	587.69	587.70	587.71
615.000	587.72	587.73	587.73	587.74	587.75

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.76	587.77	587.78	587.79	587.81
645.000	587.82	587.84	587.85	587.87	587.89
660.000	587.91	587.94	587.96	587.99	588.02
675.000	588.05	588.08	588.11	588.15	588.19
690.000	588.24	588.29	588.34	588.39	588.46
705.000	588.53	588.61	588.71	588.83	588.98
720.000	589.18	589.32	589.47	589.69	589.95
735.000	590.21	590.46	590.69	590.87	590.95
750.000	590.99	591.00	590.98	590.94	590.88
765.000	590.78	590.65	590.51	590.37	590.23
780.000	590.10	589.99	589.89	589.79	589.71
795.000	589.63	589.56	589.49	589.44	589.39
810.000	589.35	589.30	589.26	589.22	589.18
825.000	589.14	589.07	588.99	588.92	588.85
840.000	588.79	588.73	588.67	588.62	588.58
855.000	588.53	588.49	588.46	588.42	588.39
870.000	588.36	588.34	588.31	588.29	588.27
885.000	588.25	588.23	588.22	588.20	588.19
900.000	588.18	588.16	588.15	588.14	588.13
915.000	588.12	588.10	588.09	588.07	588.06
930.000	588.04	588.03	588.01	588.00	587.98
945.000	587.97	587.96	587.95	587.94	587.93
960.000	587.92	587.91	587.91	587.90	587.89
975.000	587.89	587.88	587.88	587.87	587.87
990.000	587.86	587.86	587.85	587.85	587.85
1,005.000	587.84	587.84	587.84	587.83	587.83
1,020.000	587.83	587.82	587.82	587.82	587.81
1,035.000	587.81	587.81	587.81	587.80	587.80
1,050.000	587.80	587.80	587.79	587.79	587.79
1,065.000	587.79	587.79	587.78	587.78	587.78
1,080.000	587.78	587.78	587.77	587.77	587.77
1,095.000	587.77	587.76	587.76	587.76	587.75
1,110.000	587.75	587.75	587.74	587.74	587.74
1,125.000	587.73	587.73	587.73	587.73	587.72
1,140.000	587.72	587.72	587.72	587.72	587.71
1,155.000	587.71	587.71	587.71	587.71	587.71
1,170.000	587.71	587.70	587.70	587.70	587.70
1,185.000	587.70	587.70	587.70	587.70	587.69
1,200.000	587.69	587.69	587.69	587.69	587.69
1,215.000	587.69	587.69	587.69	587.69	587.68
1,230.000	587.68	587.68	587.68	587.68	587.68
1,245.000	587.68	587.68	587.68	587.68	587.68

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.68	587.67	587.67	587.67	587.67
1,275.000	587.67	587.67	587.67	587.67	587.67
1,290.000	587.67	587.67	587.67	587.67	587.66
1,305.000	587.66	587.66	587.66	587.66	587.66
1,320.000	587.66	587.66	587.66	587.66	587.66
1,335.000	587.66	587.66	587.66	587.66	587.66
1,350.000	587.65	587.65	587.65	587.65	587.65
1,365.000	587.65	587.65	587.65	587.65	587.65
1,380.000	587.65	587.65	587.65	587.65	587.65
1,395.000	587.65	587.65	587.65	587.64	587.64
1,410.000	587.64	587.64	587.64	587.64	587.64
1,425.000	587.64	587.64	587.64	587.64	587.64
1,440.000	587.64	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.50	587.50
315.000	587.50	587.50	587.50	587.50	587.50
330.000	587.50	587.50	587.50	587.50	587.50
345.000	587.50	587.50	587.50	587.50	587.50
360.000	587.50	587.50	587.50	587.50	587.50
375.000	587.50	587.50	587.51	587.51	587.51
390.000	587.52	587.52	587.52	587.52	587.52
405.000	587.52	587.52	587.53	587.53	587.53
420.000	587.53	587.53	587.54	587.54	587.54
435.000	587.54	587.55	587.55	587.55	587.55
450.000	587.55	587.56	587.56	587.56	587.56
465.000	587.57	587.57	587.57	587.58	587.58
480.000	587.58	587.58	587.59	587.59	587.59
495.000	587.60	587.60	587.60	587.61	587.61
510.000	587.61	587.62	587.62	587.62	587.63
525.000	587.63	587.63	587.64	587.64	587.65
540.000	587.65	587.65	587.66	587.66	587.67
555.000	587.68	587.68	587.69	587.70	587.71
570.000	587.73	587.74	587.75	587.76	587.77
585.000	587.79	587.80	587.81	587.82	587.83
600.000	587.85	587.86	587.87	587.88	587.90
615.000	587.91	587.92	587.94	587.95	587.97

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	587.98	588.00	588.01	588.03	588.05
645.000	588.08	588.10	588.13	588.17	588.20
660.000	588.24	588.28	588.32	588.36	588.41
675.000	588.46	588.51	588.57	588.63	588.69
690.000	588.76	588.83	588.91	588.99	589.08
705.000	589.17	589.23	589.28	589.35	589.43
720.000	589.55	589.72	589.93	590.20	590.50
735.000	590.83	591.05	591.22	591.35	591.44
750.000	591.49	591.49	591.46	591.40	591.32
765.000	591.22	591.12	591.00	590.90	590.77
780.000	590.62	590.48	590.34	590.22	590.12
795.000	590.02	589.93	589.85	589.77	589.70
810.000	589.64	589.58	589.53	589.48	589.44
825.000	589.40	589.36	589.32	589.29	589.25
840.000	589.22	589.19	589.15	589.09	589.03
855.000	588.97	588.91	588.86	588.81	588.77
870.000	588.72	588.69	588.65	588.62	588.59
885.000	588.57	588.54	588.52	588.50	588.48
900.000	588.46	588.44	588.43	588.41	588.39
915.000	588.37	588.35	588.33	588.31	588.28
930.000	588.26	588.24	588.21	588.19	588.17
945.000	588.15	588.13	588.12	588.10	588.09
960.000	588.08	588.06	588.05	588.04	588.04
975.000	588.03	588.02	588.01	588.00	588.00
990.000	587.99	587.99	587.98	587.97	587.97
1,005.000	587.96	587.96	587.95	587.95	587.95
1,020.000	587.94	587.94	587.93	587.93	587.93
1,035.000	587.92	587.92	587.91	587.91	587.91
1,050.000	587.90	587.90	587.90	587.90	587.89
1,065.000	587.89	587.89	587.88	587.88	587.88
1,080.000	587.87	587.87	587.87	587.87	587.86
1,095.000	587.86	587.85	587.85	587.84	587.84
1,110.000	587.83	587.83	587.82	587.82	587.81
1,125.000	587.81	587.80	587.80	587.79	587.79
1,140.000	587.79	587.78	587.78	587.78	587.78
1,155.000	587.77	587.77	587.77	587.77	587.76
1,170.000	587.76	587.76	587.76	587.76	587.76
1,185.000	587.75	587.75	587.75	587.75	587.75
1,200.000	587.75	587.75	587.74	587.74	587.74
1,215.000	587.74	587.74	587.74	587.74	587.74
1,230.000	587.73	587.73	587.73	587.73	587.73
1,245.000	587.73	587.73	587.73	587.73	587.72

Subsection: Time vs. Elevation
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.72	587.72	587.72	587.72	587.72
1,275.000	587.72	587.72	587.72	587.72	587.71
1,290.000	587.71	587.71	587.71	587.71	587.71
1,305.000	587.71	587.71	587.71	587.71	587.71
1,320.000	587.70	587.70	587.70	587.70	587.70
1,335.000	587.70	587.70	587.70	587.70	587.70
1,350.000	587.70	587.70	587.69	587.69	587.69
1,365.000	587.69	587.69	587.69	587.69	587.69
1,380.000	587.69	587.69	587.69	587.69	587.69
1,395.000	587.69	587.68	587.68	587.68	587.68
1,410.000	587.68	587.68	587.68	587.68	587.68
1,425.000	587.68	587.68	587.68	587.68	587.68
1,440.000	587.68	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
0.000	587.50	587.50	587.50	587.50	587.50
15.000	587.50	587.50	587.50	587.50	587.50
30.000	587.50	587.50	587.50	587.50	587.50
45.000	587.50	587.50	587.50	587.50	587.50
60.000	587.50	587.50	587.50	587.50	587.50
75.000	587.50	587.50	587.50	587.50	587.50
90.000	587.50	587.50	587.50	587.50	587.50
105.000	587.50	587.50	587.50	587.50	587.50
120.000	587.50	587.50	587.50	587.50	587.50
135.000	587.50	587.50	587.50	587.50	587.50
150.000	587.50	587.50	587.50	587.50	587.50
165.000	587.50	587.50	587.50	587.50	587.50
180.000	587.50	587.50	587.50	587.50	587.50
195.000	587.50	587.50	587.50	587.50	587.50
210.000	587.50	587.50	587.50	587.50	587.50
225.000	587.50	587.50	587.50	587.50	587.50
240.000	587.50	587.50	587.50	587.50	587.50
255.000	587.50	587.50	587.50	587.50	587.50
270.000	587.50	587.50	587.50	587.50	587.50
285.000	587.50	587.50	587.50	587.50	587.50
300.000	587.50	587.50	587.50	587.51	587.51
315.000	587.51	587.52	587.52	587.52	587.52
330.000	587.52	587.52	587.52	587.53	587.53
345.000	587.53	587.53	587.53	587.54	587.54
360.000	587.54	587.54	587.54	587.55	587.55
375.000	587.55	587.55	587.56	587.56	587.56
390.000	587.57	587.57	587.58	587.58	587.58
405.000	587.59	587.59	587.60	587.60	587.61
420.000	587.61	587.61	587.62	587.62	587.63
435.000	587.63	587.64	587.64	587.64	587.65
450.000	587.65	587.66	587.66	587.67	587.67
465.000	587.68	587.68	587.69	587.69	587.70
480.000	587.70	587.71	587.71	587.72	587.72
495.000	587.73	587.73	587.74	587.75	587.75
510.000	587.76	587.76	587.77	587.78	587.78
525.000	587.79	587.80	587.80	587.81	587.82
540.000	587.83	587.83	587.84	587.85	587.86
555.000	587.88	587.89	587.91	587.94	587.96
570.000	587.99	588.01	588.04	588.07	588.09
585.000	588.12	588.15	588.17	588.20	588.22
600.000	588.25	588.27	588.30	588.32	588.35
615.000	588.37	588.40	588.42	588.45	588.47

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
630.000	588.50	588.53	588.56	588.60	588.64
645.000	588.68	588.73	588.79	588.85	588.92
660.000	588.99	589.07	589.15	589.20	589.24
675.000	589.27	589.31	589.35	589.39	589.43
690.000	589.47	589.52	589.58	589.63	589.68
705.000	589.74	589.79	589.86	589.95	590.06
720.000	590.21	590.43	590.71	590.98	591.24
735.000	591.50	591.76	592.00	592.17	592.29
750.000	592.33	592.32	592.27	592.18	592.06
765.000	591.92	591.77	591.62	591.48	591.35
780.000	591.23	591.12	591.02	590.93	590.85
795.000	590.74	590.63	590.52	590.42	590.33
810.000	590.24	590.16	590.09	590.02	589.96
825.000	589.89	589.83	589.78	589.72	589.67
840.000	589.62	589.57	589.52	589.48	589.45
855.000	589.41	589.38	589.35	589.33	589.30
870.000	589.28	589.26	589.24	589.22	589.20
885.000	589.19	589.17	589.15	589.12	589.09
900.000	589.06	589.04	589.01	588.98	588.95
915.000	588.92	588.88	588.84	588.80	588.76
930.000	588.71	588.67	588.62	588.58	588.54
945.000	588.51	588.48	588.45	588.42	588.39
960.000	588.37	588.35	588.33	588.32	588.30
975.000	588.29	588.27	588.26	588.25	588.24
990.000	588.23	588.22	588.21	588.20	588.20
1,005.000	588.19	588.18	588.17	588.17	588.16
1,020.000	588.15	588.15	588.14	588.13	588.13
1,035.000	588.12	588.12	588.11	588.11	588.10
1,050.000	588.10	588.09	588.09	588.08	588.08
1,065.000	588.07	588.07	588.06	588.06	588.06
1,080.000	588.05	588.05	588.04	588.04	588.03
1,095.000	588.03	588.02	588.01	588.00	587.99
1,110.000	587.98	587.97	587.96	587.95	587.94
1,125.000	587.93	587.92	587.92	587.91	587.90
1,140.000	587.90	587.89	587.89	587.88	587.88
1,155.000	587.88	587.87	587.87	587.87	587.86
1,170.000	587.86	587.86	587.86	587.85	587.85
1,185.000	587.85	587.85	587.84	587.84	587.84
1,200.000	587.84	587.84	587.83	587.83	587.83
1,215.000	587.83	587.83	587.82	587.82	587.82
1,230.000	587.82	587.82	587.82	587.81	587.81
1,245.000	587.81	587.81	587.81	587.81	587.81

Subsection: Time vs. Elevation
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Elevation (ft)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)	Elevation (ft)
1,260.000	587.80	587.80	587.80	587.80	587.80
1,275.000	587.80	587.80	587.79	587.79	587.79
1,290.000	587.79	587.79	587.79	587.79	587.78
1,305.000	587.78	587.78	587.78	587.78	587.78
1,320.000	587.78	587.78	587.77	587.77	587.77
1,335.000	587.77	587.77	587.77	587.77	587.77
1,350.000	587.77	587.76	587.76	587.76	587.76
1,365.000	587.76	587.76	587.76	587.76	587.76
1,380.000	587.76	587.75	587.75	587.75	587.75
1,395.000	587.75	587.75	587.75	587.75	587.75
1,410.000	587.75	587.74	587.74	587.74	587.74
1,425.000	587.74	587.74	587.74	587.74	587.74
1,440.000	587.74	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	11.000	22.000
465.000	38.000	61.000	92.000	129.000	173.000
480.000	221.000	272.000	324.000	374.000	422.000
495.000	467.000	508.000	544.000	575.000	600.000
510.000	622.000	649.000	681.000	727.000	790.000
525.000	876.000	992.000	1,138.000	1,309.000	1,503.000
540.000	1,708.000	1,916.000	2,122.000	2,319.000	2,504.000
555.000	2,672.000	2,821.000	2,955.000	3,079.000	3,197.000
570.000	3,322.000	3,462.000	3,629.000	3,838.000	4,089.000
585.000	4,374.000	4,691.000	5,024.000	5,356.000	5,688.000
600.000	6,020.000	6,354.000	6,696.000	7,051.000	7,426.000
615.000	7,829.000	8,261.000	8,716.000	9,189.000	9,669.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	10,143.000	10,611.000	11,074.000	11,534.000	12,000.000
645.000	12,479.000	12,975.000	13,492.000	14,033.000	14,604.000
660.000	15,209.000	15,848.000	16,520.000	17,227.000	17,980.000
675.000	18,783.000	19,632.000	20,523.000	21,447.000	22,387.000
690.000	23,342.000	24,340.000	25,407.000	26,555.000	27,802.000
705.000	29,163.000	30,679.000	32,378.000	34,348.000	36,692.000
720.000	39,496.000	42,928.000	47,048.000	51,951.000	57,752.000
735.000	64,456.000	72,096.000	80,631.000	89,856.000	99,562.000
750.000	109,484.000	119,345.000	129,002.000	138,185.000	146,718.000
765.000	154,432.000	161,159.000	166,904.000	171,652.000	175,424.000
780.000	178,291.000	180,322.000	181,608.000	182,247.000	182,308.000
795.000	181,869.000	180,995.000	179,742.000	178,174.000	176,335.000
810.000	174,290.000	172,070.000	169,708.000	167,242.000	164,674.000
825.000	162,005.000	159,243.000	156,428.000	153,593.000	150,773.000
840.000	148,014.000	145,330.000	142,732.000	140,227.000	137,787.000
855.000	135,386.000	133,020.000	130,692.000	128,392.000	126,131.000
870.000	123,927.000	121,797.000	119,768.000	117,847.000	116,026.000
885.000	114,300.000	112,640.000	111,013.000	109,403.000	107,781.000
900.000	106,131.000	104,453.000	102,747.000	101,024.000	99,304.000
915.000	97,601.000	95,966.000	94,443.000	93,048.000	91,809.000
930.000	90,720.000	89,746.000	88,863.000	88,028.000	87,193.000
945.000	86,335.000	85,422.000	84,440.000	83,374.000	82,209.000
960.000	80,967.000	79,682.000	78,378.000	77,110.000	75,922.000
975.000	74,857.000	73,973.000	73,271.000	72,730.000	72,331.000
990.000	72,017.000	71,728.000	71,437.000	71,108.000	70,718.000
1,005.000	70,251.000	69,688.000	69,020.000	68,232.000	67,328.000
1,020.000	66,329.000	65,251.000	64,115.000	62,943.000	61,753.000
1,035.000	60,577.000	59,433.000	58,360.000	57,392.000	56,554.000
1,050.000	55,888.000	55,410.000	55,116.000	55,005.000	55,036.000
1,065.000	55,157.000	55,338.000	55,526.000	55,686.000	55,791.000
1,080.000	55,812.000	55,738.000	55,551.000	55,242.000	54,820.000
1,095.000	54,289.000	53,667.000	52,974.000	52,224.000	51,433.000
1,110.000	50,616.000	49,779.000	48,931.000	48,074.000	47,216.000
1,125.000	46,360.000	45,509.000	44,668.000	43,839.000	43,019.000
1,140.000	42,202.000	41,404.000	40,636.000	39,922.000	39,297.000
1,155.000	38,780.000	38,409.000	38,212.000	38,199.000	38,385.000
1,170.000	38,752.000	39,255.000	39,868.000	40,532.000	41,190.000
1,185.000	41,820.000	42,385.000	42,867.000	43,244.000	43,496.000
1,200.000	43,621.000	43,614.000	43,483.000	43,246.000	42,919.000
1,215.000	42,519.000	42,063.000	41,558.000	41,017.000	40,443.000
1,230.000	39,843.000	39,222.000	38,584.000	37,935.000	37,277.000
1,245.000	36,613.000	35,946.000	35,278.000	34,612.000	33,948.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	33,289.000	32,635.000	31,987.000	31,347.000	30,715.000
1,275.000	30,091.000	29,481.000	28,915.000	28,421.000	28,024.000
1,290.000	27,757.000	27,641.000	27,719.000	28,018.000	28,514.000
1,305.000	29,178.000	29,974.000	30,836.000	31,726.000	32,597.000
1,320.000	33,405.000	34,135.000	34,762.000	35,275.000	35,653.000
1,335.000	35,881.000	35,974.000	35,950.000	35,823.000	35,613.000
1,350.000	35,334.000	35,001.000	34,626.000	34,213.000	33,768.000
1,365.000	33,293.000	32,797.000	32,285.000	31,760.000	31,225.000
1,380.000	30,683.000	30,136.000	29,586.000	29,035.000	28,486.000
1,395.000	27,940.000	27,397.000	26,859.000	26,326.000	25,799.000
1,410.000	25,279.000	24,765.000	24,259.000	23,761.000	23,270.000
1,425.000	22,788.000	22,314.000	21,848.000	21,391.000	20,942.000
1,440.000	20,502.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	12.000	18.000
345.000	26.000	36.000	48.000	64.000	82.000
360.000	105.000	130.000	160.000	194.000	233.000
375.000	276.000	325.000	378.000	437.000	502.000
390.000	573.000	649.000	732.000	821.000	916.000
405.000	1,018.000	1,126.000	1,241.000	1,362.000	1,490.000
420.000	1,624.000	1,764.000	1,911.000	2,064.000	2,223.000
435.000	2,389.000	2,562.000	2,741.000	2,926.000	3,117.000
450.000	3,315.000	3,520.000	3,731.000	3,948.000	4,172.000
465.000	4,403.000	4,640.000	4,883.000	5,134.000	5,391.000
480.000	5,655.000	5,926.000	6,204.000	6,488.000	6,780.000
495.000	7,079.000	7,385.000	7,699.000	8,020.000	8,348.000
510.000	8,684.000	9,028.000	9,380.000	9,740.000	10,108.000
525.000	10,484.000	10,869.000	11,262.000	11,664.000	12,076.000
540.000	12,497.000	12,928.000	13,371.000	13,828.000	14,299.000
555.000	14,787.000	15,296.000	15,827.000	16,384.000	16,968.000
570.000	17,582.000	18,226.000	18,902.000	19,610.000	20,350.000
585.000	21,123.000	21,928.000	22,765.000	23,632.000	24,530.000
600.000	25,458.000	26,416.000	27,403.000	28,420.000	29,466.000
615.000	30,544.000	31,652.000	32,793.000	33,966.000	35,173.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	36,416.000	37,698.000	39,023.000	40,392.000	41,814.000
645.000	43,296.000	44,831.000	46,418.000	48,057.000	49,759.000
660.000	51,530.000	53,376.000	55,305.000	57,323.000	59,439.000
675.000	61,658.000	63,994.000	66,458.000	69,059.000	71,810.000
690.000	74,718.000	77,804.000	81,084.000	84,578.000	88,313.000
705.000	92,292.000	96,574.000	101,204.000	106,349.000	112,195.000
720.000	118,940.000	126,993.000	136,524.000	147,887.000	161,417.000
735.000	177,115.000	195,011.000	214,913.000	236,188.000	258,315.000
750.000	280,556.000	302,049.000	322,374.000	340,886.000	357,262.000
765.000	371,200.000	382,423.000	391,017.000	396,959.000	400,369.000
780.000	401,569.000	400,848.000	398,507.000	394,877.000	390,172.000
795.000	384,620.000	378,397.000	371,645.000	364,509.000	357,077.000
810.000	349,470.000	341,740.000	333,943.000	326,151.000	318,395.000
825.000	310,726.000	303,161.000	295,712.000	288,391.000	281,213.000
840.000	274,203.000	267,368.000	260,722.000	254,253.000	247,952.000
855.000	241,837.000	235,908.000	230,177.000	224,645.000	219,318.000
870.000	214,188.000	209,233.000	204,453.000	199,844.000	195,404.000
885.000	191,131.000	187,020.000	183,070.000	179,274.000	175,630.000
900.000	172,127.000	168,748.000	165,488.000	162,342.000	159,304.000
915.000	156,367.000	153,523.000	150,764.000	148,082.000	145,471.000
930.000	142,927.000	140,447.000	138,028.000	135,671.000	133,375.000
945.000	131,140.000	128,959.000	126,822.000	124,731.000	122,691.000
960.000	120,700.000	118,763.000	116,878.000	115,047.000	113,269.000
975.000	111,543.000	109,871.000	108,249.000	106,678.000	105,157.000
990.000	103,685.000	102,260.000	100,881.000	99,548.000	98,258.000
1,005.000	97,010.000	95,804.000	94,637.000	93,509.000	92,418.000
1,020.000	91,363.000	90,343.000	89,356.000	88,401.000	87,478.000
1,035.000	86,581.000	85,705.000	84,848.000	84,009.000	83,187.000
1,050.000	82,384.000	81,597.000	80,826.000	80,072.000	79,335.000
1,065.000	78,612.000	77,906.000	77,214.000	76,537.000	75,874.000
1,080.000	75,225.000	74,590.000	73,966.000	73,355.000	72,753.000
1,095.000	72,161.000	71,576.000	70,997.000	70,423.000	69,854.000
1,110.000	69,287.000	68,723.000	68,163.000	67,605.000	67,051.000
1,125.000	66,501.000	65,955.000	65,415.000	64,882.000	64,355.000
1,140.000	63,836.000	63,325.000	62,823.000	62,329.000	61,845.000
1,155.000	61,369.000	60,903.000	60,445.000	59,997.000	59,558.000
1,170.000	59,128.000	58,707.000	58,295.000	57,892.000	57,497.000
1,185.000	57,110.000	56,732.000	56,362.000	56,000.000	55,645.000
1,200.000	55,298.000	54,958.000	54,626.000	54,300.000	53,981.000
1,215.000	53,669.000	53,364.000	53,065.000	52,773.000	52,486.000
1,230.000	52,205.000	51,930.000	51,661.000	51,396.000	51,137.000
1,245.000	50,882.000	50,633.000	50,388.000	50,147.000	49,911.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	49,680.000	49,453.000	49,230.000	49,012.000	48,797.000
1,275.000	48,587.000	48,380.000	48,178.000	47,979.000	47,783.000
1,290.000	47,591.000	47,402.000	47,217.000	47,035.000	46,855.000
1,305.000	46,679.000	46,505.000	46,335.000	46,167.000	46,001.000
1,320.000	45,839.000	45,679.000	45,522.000	45,367.000	45,215.000
1,335.000	45,065.000	44,917.000	44,771.000	44,628.000	44,486.000
1,350.000	44,346.000	44,209.000	44,073.000	43,938.000	43,806.000
1,365.000	43,675.000	43,546.000	43,418.000	43,289.000	43,160.000
1,380.000	43,030.000	42,900.000	42,769.000	42,639.000	42,508.000
1,395.000	42,377.000	42,246.000	42,115.000	41,984.000	41,853.000
1,410.000	41,721.000	41,590.000	41,459.000	41,328.000	41,196.000
1,425.000	41,065.000	40,934.000	40,802.000	40,671.000	40,540.000
1,440.000	40,407.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	11.000	17.000	25.000	35.000	49.000
300.000	65.000	86.000	111.000	140.000	174.000
315.000	212.000	256.000	306.000	361.000	421.000
330.000	487.000	559.000	637.000	720.000	809.000
345.000	905.000	1,006.000	1,113.000	1,226.000	1,345.000
360.000	1,470.000	1,601.000	1,739.000	1,883.000	2,034.000
375.000	2,192.000	2,358.000	2,532.000	2,715.000	2,908.000
390.000	3,110.000	3,322.000	3,544.000	3,776.000	4,019.000
405.000	4,271.000	4,534.000	4,806.000	5,089.000	5,380.000
420.000	5,682.000	5,992.000	6,312.000	6,640.000	6,977.000
435.000	7,324.000	7,678.000	8,042.000	8,414.000	8,795.000
450.000	9,184.000	9,582.000	9,989.000	10,404.000	10,828.000
465.000	11,260.000	11,702.000	12,152.000	12,611.000	13,079.000
480.000	13,556.000	14,042.000	14,538.000	15,043.000	15,557.000
495.000	16,081.000	16,614.000	17,158.000	17,711.000	18,275.000
510.000	18,849.000	19,433.000	20,028.000	20,634.000	21,251.000
525.000	21,880.000	22,520.000	23,172.000	23,836.000	24,513.000
540.000	25,203.000	25,908.000	26,631.000	27,374.000	28,140.000
555.000	28,936.000	29,765.000	30,634.000	31,547.000	32,508.000
570.000	33,522.000	34,590.000	35,713.000	36,893.000	38,128.000
585.000	39,420.000	40,765.000	42,164.000	43,612.000	45,097.000
600.000	46,602.000	48,128.000	49,674.000	51,239.000	52,824.000
615.000	54,429.000	56,054.000	57,702.000	59,372.000	61,068.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
630.000	62,791.000	64,545.000	66,338.000	68,174.000	70,065.000
645.000	72,024.000	74,060.000	76,191.000	78,424.000	80,774.000
660.000	83,251.000	85,863.000	88,615.000	91,492.000	94,501.000
675.000	97,645.000	100,939.000	104,397.000	108,024.000	111,837.000
690.000	115,840.000	120,054.000	124,493.000	129,179.000	134,125.000
705.000	139,337.000	144,888.000	150,827.000	157,356.000	164,701.000
720.000	173,101.000	183,030.000	194,682.000	208,531.000	224,980.000
735.000	243,959.000	265,509.000	289,366.000	314,704.000	340,872.000
750.000	366,941.000	391,860.000	415,126.000	435,978.000	454,047.000
765.000	469,018.000	480,612.000	488,958.000	494,081.000	496,187.000
780.000	495,711.000	493,032.000	488,532.000	482,630.000	475,583.000
795.000	467,669.000	459,101.000	450,041.000	440,656.000	431,033.000
810.000	421,283.000	411,470.000	401,682.000	391,996.000	382,413.000
825.000	372,961.000	363,662.000	354,551.000	345,645.000	336,930.000
840.000	328,417.000	320,110.000	312,032.000	304,189.000	296,570.000
855.000	289,173.000	282,002.000	275,073.000	268,387.000	261,954.000
870.000	255,764.000	249,793.000	244,046.000	238,516.000	233,204.000
885.000	228,104.000	223,211.000	218,520.000	214,012.000	209,669.000
900.000	205,485.000	201,460.000	197,592.000	193,871.000	190,288.000
915.000	186,834.000	183,494.000	180,254.000	177,104.000	174,035.000
930.000	171,027.000	168,067.000	165,153.000	162,289.000	159,476.000
945.000	156,717.000	154,015.000	151,372.000	148,795.000	146,290.000
960.000	143,855.000	141,496.000	139,212.000	137,002.000	134,868.000
975.000	132,808.000	130,822.000	128,898.000	127,028.000	125,210.000
990.000	123,445.000	121,731.000	120,068.000	118,456.000	116,892.000
1,005.000	115,375.000	113,906.000	112,481.000	111,101.000	109,764.000
1,020.000	108,468.000	107,212.000	105,995.000	104,816.000	103,674.000
1,035.000	102,566.000	101,493.000	100,452.000	99,443.000	98,465.000
1,050.000	97,516.000	96,596.000	95,703.000	94,837.000	93,997.000
1,065.000	93,181.000	92,390.000	91,621.000	90,875.000	90,149.000
1,080.000	89,445.000	88,759.000	88,090.000	87,437.000	86,795.000
1,095.000	86,158.000	85,523.000	84,887.000	84,249.000	83,606.000
1,110.000	82,958.000	82,305.000	81,647.000	80,985.000	80,320.000
1,125.000	79,652.000	78,984.000	78,318.000	77,654.000	76,995.000
1,140.000	76,341.000	75,695.000	75,057.000	74,429.000	73,809.000
1,155.000	73,200.000	72,601.000	72,013.000	71,436.000	70,869.000
1,170.000	70,313.000	69,769.000	69,235.000	68,712.000	68,199.000
1,185.000	67,698.000	67,206.000	66,725.000	66,254.000	65,793.000
1,200.000	65,341.000	64,899.000	64,466.000	64,043.000	63,628.000
1,215.000	63,222.000	62,824.000	62,434.000	62,053.000	61,679.000
1,230.000	61,313.000	60,954.000	60,603.000	60,258.000	59,921.000
1,245.000	59,590.000	59,266.000	58,948.000	58,636.000	58,330.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	58,030.000	57,736.000	57,447.000	57,163.000	56,885.000
1,275.000	56,612.000	56,343.000	56,080.000	55,821.000	55,567.000
1,290.000	55,318.000	55,073.000	54,832.000	54,595.000	54,362.000
1,305.000	54,134.000	53,909.000	53,688.000	53,470.000	53,256.000
1,320.000	53,046.000	52,838.000	52,634.000	52,434.000	52,236.000
1,335.000	52,041.000	51,849.000	51,660.000	51,474.000	51,291.000
1,350.000	51,110.000	50,932.000	50,756.000	50,583.000	50,413.000
1,365.000	50,245.000	50,079.000	49,916.000	49,755.000	49,597.000
1,380.000	49,441.000	49,286.000	49,134.000	48,984.000	48,835.000
1,395.000	48,689.000	48,544.000	48,401.000	48,260.000	48,120.000
1,410.000	47,983.000	47,846.000	47,712.000	47,579.000	47,448.000
1,425.000	47,318.000	47,190.000	47,063.000	46,937.000	46,813.000
1,440.000	46,689.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: POND A
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	10.000	16.000	25.000	37.000	53.000
240.000	73.000	97.000	128.000	164.000	207.000
255.000	257.000	314.000	379.000	451.000	532.000
270.000	620.000	716.000	821.000	934.000	1,055.000
285.000	1,184.000	1,322.000	1,467.000	1,621.000	1,784.000
300.000	1,954.000	2,133.000	2,320.000	2,515.000	2,717.000
315.000	2,928.000	3,147.000	3,373.000	3,608.000	3,849.000
330.000	4,099.000	4,356.000	4,621.000	4,893.000	5,173.000
345.000	5,460.000	5,754.000	6,056.000	6,364.000	6,681.000
360.000	7,005.000	7,336.000	7,676.000	8,026.000	8,386.000
375.000	8,758.000	9,143.000	9,543.000	9,960.000	10,395.000
390.000	10,849.000	11,323.000	11,817.000	12,332.000	12,867.000
405.000	13,421.000	13,995.000	14,587.000	15,198.000	15,825.000
420.000	16,468.000	17,127.000	17,800.000	18,488.000	19,190.000
435.000	19,906.000	20,635.000	21,378.000	22,133.000	22,901.000
450.000	23,682.000	24,475.000	25,281.000	26,099.000	26,930.000
465.000	27,773.000	28,629.000	29,497.000	30,378.000	31,272.000
480.000	32,178.000	33,098.000	34,030.000	34,976.000	35,935.000
495.000	36,908.000	37,894.000	38,895.000	39,909.000	40,939.000
510.000	41,983.000	43,042.000	44,112.000	45,183.000	46,251.000
525.000	47,318.000	48,384.000	49,451.000	50,517.000	51,586.000
540.000	52,659.000	53,741.000	54,837.000	55,955.000	57,100.000
555.000	58,288.000	59,530.000	60,837.000	62,224.000	63,696.000
570.000	65,260.000	66,921.000	68,679.000	70,534.000	72,482.000
585.000	74,519.000	76,642.000	78,843.000	81,114.000	83,450.000
600.000	85,842.000	88,281.000	90,747.000	93,231.000	95,733.000
615.000	98,251.000	100,787.000	103,344.000	105,924.000	108,532.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	111,173.000	113,853.000	116,586.000	119,379.000	122,257.000
645.000	125,240.000	128,350.000	131,614.000	135,031.000	138,613.000
660.000	142,377.000	146,333.000	150,495.000	154,867.000	159,460.000
675.000	164,277.000	169,331.000	174,636.000	180,172.000	185,933.000
690.000	191,919.000	198,144.000	204,615.000	211,340.000	218,336.000
705.000	225,596.000	233,169.000	241,099.000	249,635.000	259,049.000
720.000	269,628.000	281,996.000	296,438.000	313,550.000	333,826.000
735.000	357,165.000	383,592.000	412,732.000	443,485.000	474,989.000
750.000	506,036.000	535,259.000	562,084.000	585,576.000	605,329.000
765.000	621,042.000	632,406.000	639,692.000	643,031.000	642,730.000
780.000	639,402.000	633,567.000	625,726.000	616,420.000	605,971.000
795.000	594,717.000	582,906.000	570,715.000	558,308.000	545,758.000
810.000	533,201.000	520,709.000	508,339.000	496,147.000	484,153.000
825.000	472,389.000	460,841.000	449,527.000	438,466.000	427,650.000
840.000	417,074.000	406,738.000	396,669.000	386,865.000	377,320.000
855.000	368,053.000	359,067.000	350,389.000	342,005.000	333,911.000
870.000	326,121.000	318,630.000	311,449.000	304,568.000	297,963.000
885.000	291,613.000	285,511.000	279,657.000	274,041.000	268,662.000
900.000	263,514.000	258,580.000	253,841.000	249,274.000	244,863.000
915.000	240,594.000	236,441.000	232,380.000	228,398.000	224,477.000
930.000	220,609.000	216,790.000	213,002.000	209,243.000	205,522.000
945.000	201,843.000	198,220.000	194,658.000	191,171.000	187,774.000
960.000	184,469.000	181,265.000	178,164.000	175,168.000	172,272.000
975.000	169,463.000	166,739.000	164,100.000	161,546.000	159,076.000
990.000	156,691.000	154,388.000	152,165.000	150,022.000	147,954.000
1,005.000	145,961.000	144,041.000	142,190.000	140,407.000	138,689.000
1,020.000	137,034.000	135,439.000	133,902.000	132,420.000	130,992.000
1,035.000	129,611.000	128,267.000	126,960.000	125,687.000	124,449.000
1,050.000	123,244.000	122,070.000	120,928.000	119,815.000	118,732.000
1,065.000	117,677.000	116,650.000	115,649.000	114,675.000	113,725.000
1,080.000	112,800.000	111,896.000	111,012.000	110,145.000	109,290.000
1,095.000	108,443.000	107,600.000	106,755.000	105,906.000	105,049.000
1,110.000	104,182.000	103,305.000	102,419.000	101,524.000	100,623.000
1,125.000	99,719.000	98,813.000	97,911.000	97,015.000	96,127.000
1,140.000	95,251.000	94,388.000	93,542.000	92,712.000	91,900.000
1,155.000	91,106.000	90,331.000	89,575.000	88,838.000	88,120.000
1,170.000	87,422.000	86,740.000	86,070.000	85,412.000	84,765.000
1,185.000	84,130.000	83,505.000	82,893.000	82,291.000	81,701.000
1,200.000	81,122.000	80,554.000	79,997.000	79,450.000	78,914.000
1,215.000	78,388.000	77,872.000	77,366.000	76,870.000	76,383.000
1,230.000	75,905.000	75,436.000	74,976.000	74,524.000	74,081.000
1,245.000	73,646.000	73,220.000	72,801.000	72,390.000	71,986.000

Subsection: Time vs. Volume
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	71,590.000	71,201.000	70,819.000	70,444.000	70,075.000
1,275.000	69,713.000	69,358.000	69,008.000	68,664.000	68,326.000
1,290.000	67,994.000	67,668.000	67,347.000	67,031.000	66,721.000
1,305.000	66,415.000	66,114.000	65,819.000	65,528.000	65,241.000
1,320.000	64,959.000	64,681.000	64,407.000	64,137.000	63,871.000
1,335.000	63,608.000	63,350.000	63,095.000	62,844.000	62,597.000
1,350.000	62,353.000	62,112.000	61,875.000	61,642.000	61,411.000
1,365.000	61,184.000	60,959.000	60,738.000	60,519.000	60,303.000
1,380.000	60,090.000	59,880.000	59,672.000	59,467.000	59,264.000
1,395.000	59,064.000	58,867.000	58,671.000	58,479.000	58,288.000
1,410.000	58,100.000	57,914.000	57,731.000	57,549.000	57,370.000
1,425.000	57,193.000	57,018.000	56,846.000	56,675.000	56,506.000
1,440.000	56,337.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	8.000	55.000	192.000	419.000
255.000	662.000	848.000	960.000	1,015.000	1,036.000
270.000	1,037.000	1,025.000	1,007.000	984.000	960.000
285.000	935.000	909.000	883.000	857.000	832.000
300.000	808.000	784.000	761.000	739.000	717.000
315.000	696.000	676.000	656.000	637.000	618.000
330.000	600.000	582.000	565.000	549.000	532.000
345.000	517.000	502.000	487.000	473.000	459.000
360.000	445.000	466.000	625.000	1,032.000	1,640.000
375.000	2,245.000	2,683.000	2,931.000	3,044.000	3,076.000
390.000	3,060.000	3,016.000	2,956.000	2,886.000	2,812.000
405.000	2,736.000	2,660.000	2,584.000	2,508.000	2,435.000
420.000	2,363.000	2,294.000	2,227.000	2,161.000	2,098.000
435.000	2,036.000	1,977.000	1,919.000	1,862.000	1,808.000
450.000	1,755.000	1,753.000	1,953.000	2,503.000	3,326.000
465.000	4,134.000	4,707.000	5,017.000	5,142.000	5,158.000
480.000	5,109.000	5,022.000	4,913.000	4,793.000	4,667.000
495.000	4,539.000	4,411.000	4,284.000	4,159.000	4,037.000
510.000	3,919.000	3,864.000	4,055.000	4,666.000	5,601.000
525.000	6,516.000	7,152.000	7,478.000	7,586.000	7,565.000
540.000	7,469.000	7,328.000	7,161.000	6,981.000	6,795.000
555.000	6,608.000	6,421.000	6,251.000	6,195.000	6,507.000
570.000	7,326.000	8,393.000	9,328.000	9,935.000	10,222.000
585.000	10,303.000	10,260.000	10,134.000	9,957.000	9,794.000
600.000	9,836.000	10,350.000	11,359.000	12,488.000	13,363.000
615.000	13,870.000	14,076.000	14,094.000	13,990.000	13,799.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	13,551.000	13,352.000	13,462.000	14,123.000	15,228.000
645.000	16,350.000	17,152.000	17,578.000	17,724.000	17,742.000
660.000	17,887.000	18,484.000	19,583.000	20,803.000	21,744.000
675.000	22,290.000	22,591.000	23,048.000	23,984.000	25,245.000
690.000	26,406.000	27,267.000	28,050.000	29,136.000	30,553.000
705.000	32,125.000	33,935.000	36,281.000	39,477.000	43,849.000
720.000	49,621.000	56,829.000	65,514.000	75,414.000	85,648.000
735.000	95,089.000	102,973.000	109,240.000	114,210.000	118,098.000
750.000	120,827.000	122,092.000	121,744.000	120,137.000	117,965.000
765.000	115,745.000	113,422.000	110,674.000	107,516.000	104,468.000
780.000	101,946.000	99,729.000	97,358.000	94,586.000	91,439.000
795.000	88,088.000	84,752.000	81,828.000	79,675.000	78,129.000
810.000	76,713.000	75,068.000	73,092.000	70,901.000	68,591.000
825.000	66,206.000	63,885.000	62,009.000	60,927.000	60,482.000
840.000	60,163.000	59,572.000	58,599.000	57,335.000	55,876.000
855.000	54,272.000	52,558.000	50,812.000	49,258.000	48,309.000
870.000	48,194.000	48,580.000	48,924.000	48,913.000	48,513.000
885.000	47,824.000	46,920.000	45,843.000	44,623.000	43,300.000
900.000	41,930.000	40,547.000	39,203.000	38,110.000	37,661.000
915.000	38,023.000	38,886.000	39,715.000	40,157.000	40,195.000
930.000	39,938.000	39,450.000	38,759.000	37,902.000	36,930.000
945.000	35,888.000	34,818.000	33,743.000	32,674.000	31,621.000
960.000	30,603.000	29,752.000	29,398.000	29,847.000	30,984.000
975.000	32,272.000	33,251.000	33,806.000	33,994.000	33,898.000
990.000	33,573.000	33,049.000	32,375.000	31,608.000	30,786.000
1,005.000	29,938.000	29,082.000	28,230.000	27,391.000	26,573.000
1,020.000	25,781.000	25,017.000	24,284.000	23,581.000	22,907.000
1,035.000	22,272.000	21,788.000	21,800.000	22,655.000	24,162.000
1,050.000	25,770.000	27,069.000	27,919.000	28,394.000	28,588.000
1,065.000	28,541.000	28,287.000	27,870.000	27,345.000	26,751.000
1,080.000	26,120.000	25,470.000	24,812.000	24,156.000	23,511.000
1,095.000	22,880.000	22,259.000	21,645.000	21,041.000	20,448.000
1,110.000	19,867.000	19,299.000	18,744.000	18,203.000	17,677.000
1,125.000	17,164.000	16,664.000	16,178.000	15,705.000	15,244.000
1,140.000	14,797.000	14,363.000	13,976.000	13,839.000	14,329.000
1,155.000	15,638.000	17,469.000	19,273.000	20,708.000	21,738.000
1,170.000	22,442.000	22,878.000	23,077.000	23,065.000	22,889.000
1,185.000	22,595.000	22,221.000	21,791.000	21,323.000	20,829.000
1,200.000	20,318.000	19,800.000	19,280.000	18,762.000	18,249.000
1,215.000	17,743.000	17,245.000	16,758.000	16,280.000	15,813.000
1,230.000	15,358.000	14,915.000	14,482.000	14,061.000	13,651.000
1,245.000	13,252.000	12,863.000	12,486.000	12,120.000	11,764.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	11,419.000	11,084.000	10,759.000	10,443.000	10,137.000
1,275.000	9,840.000	9,551.000	9,305.000	9,272.000	9,845.000
1,290.000	11,276.000	13,242.000	15,188.000	16,791.000	17,973.000
1,305.000	18,810.000	19,371.000	19,686.000	19,783.000	19,710.000
1,320.000	19,514.000	19,234.000	18,894.000	18,514.000	18,104.000
1,335.000	17,674.000	17,234.000	16,790.000	16,346.000	15,904.000
1,350.000	15,467.000	15,037.000	14,614.000	14,199.000	13,794.000
1,365.000	13,398.000	13,012.000	12,635.000	12,269.000	11,911.000
1,380.000	11,563.000	11,224.000	10,894.000	10,575.000	10,265.000
1,395.000	9,963.000	9,671.000	9,388.000	9,112.000	8,845.000
1,410.000	8,585.000	8,334.000	8,089.000	7,852.000	7,621.000
1,425.000	7,398.000	7,181.000	6,970.000	6,766.000	6,567.000
1,440.000	6,375.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	7.000
135.000	12.000	20.000	30.000	42.000	57.000
150.000	73.000	92.000	113.000	136.000	161.000
165.000	188.000	217.000	248.000	280.000	314.000
180.000	350.000	387.000	426.000	466.000	508.000
195.000	551.000	596.000	641.000	688.000	736.000
210.000	786.000	836.000	888.000	941.000	995.000
225.000	1,050.000	1,105.000	1,162.000	1,220.000	1,278.000
240.000	1,338.000	1,398.000	1,459.000	1,521.000	1,584.000
255.000	1,647.000	1,711.000	1,776.000	1,842.000	1,908.000
270.000	1,976.000	2,043.000	2,112.000	2,181.000	2,251.000
285.000	2,321.000	2,392.000	2,464.000	2,536.000	2,609.000
300.000	2,683.000	2,757.000	2,832.000	2,907.000	2,983.000
315.000	3,059.000	3,136.000	3,214.000	3,293.000	3,372.000
330.000	3,451.000	3,531.000	3,611.000	3,692.000	3,773.000
345.000	3,856.000	3,939.000	4,022.000	4,106.000	4,191.000
360.000	4,277.000	4,364.000	4,453.000	4,549.000	4,651.000
375.000	4,759.000	4,871.000	4,986.000	5,102.000	5,220.000
390.000	5,340.000	5,460.000	5,581.000	5,703.000	5,826.000
405.000	5,948.000	6,071.000	6,196.000	6,322.000	6,449.000
420.000	6,577.000	6,707.000	6,839.000	6,974.000	7,112.000
435.000	7,252.000	7,396.000	7,543.000	7,693.000	7,847.000
450.000	8,005.000	8,166.000	8,331.000	8,500.000	8,673.000
465.000	8,850.000	9,031.000	9,216.000	9,406.000	9,599.000
480.000	9,796.000	9,997.000	10,202.000	10,412.000	10,626.000
495.000	10,845.000	11,068.000	11,296.000	11,529.000	11,767.000
510.000	12,010.000	12,259.000	12,512.000	12,771.000	13,035.000
525.000	13,305.000	13,581.000	13,863.000	14,150.000	14,444.000
540.000	14,745.000	15,055.000	15,388.000	15,759.000	16,177.000
555.000	16,640.000	17,139.000	17,666.000	18,215.000	18,784.000
570.000	19,369.000	19,969.000	20,584.000	21,213.000	21,855.000
585.000	22,510.000	23,178.000	23,851.000	24,522.000	25,193.000
600.000	25,863.000	26,536.000	27,211.000	27,891.000	28,577.000
615.000	29,270.000	29,971.000	30,683.000	31,405.000	32,141.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	32,892.000	33,666.000	34,484.000	35,375.000	36,360.000
645.000	37,435.000	38,587.000	39,805.000	41,082.000	42,416.000
660.000	43,804.000	45,253.000	46,780.000	48,403.000	50,135.000
675.000	51,979.000	53,930.000	55,985.000	58,145.000	60,417.000
690.000	62,816.000	65,368.000	68,121.000	71,130.000	74,443.000
705.000	78,113.000	82,241.000	87,068.000	92,976.000	100,473.000
720.000	110,404.000	124,301.000	143,597.000	167,361.000	192,315.000
735.000	214,912.000	232,861.000	245,644.000	253,653.000	257,470.000
750.000	257,679.000	255,012.000	250,253.000	244,005.000	236,793.000
765.000	229,036.000	221,017.000	212,949.000	205,009.000	197,302.000
780.000	189,894.000	182,821.000	176,076.000	169,639.000	163,503.000
795.000	157,659.000	152,101.000	146,835.000	141,868.000	137,189.000
810.000	132,774.000	128,598.000	124,635.000	120,846.000	117,210.000
825.000	113,717.000	110,360.000	107,143.000	104,072.000	101,150.000
840.000	98,380.000	95,762.000	93,291.000	90,958.000	88,743.000
855.000	86,639.000	84,641.000	82,745.000	80,948.000	79,243.000
870.000	77,627.000	76,094.000	74,641.000	73,263.000	71,956.000
885.000	70,715.000	69,538.000	68,414.000	67,334.000	66,295.000
900.000	65,294.000	64,325.000	63,372.000	62,411.000	61,428.000
915.000	60,425.000	59,415.000	58,407.000	57,411.000	56,432.000
930.000	55,474.000	54,544.000	53,644.000	52,778.000	51,945.000
945.000	51,146.000	50,380.000	49,647.000	48,945.000	48,273.000
960.000	47,630.000	47,015.000	46,426.000	45,860.000	45,312.000
975.000	44,779.000	44,261.000	43,757.000	43,267.000	42,791.000
990.000	42,328.000	41,878.000	41,440.000	41,015.000	40,600.000
1,005.000	40,197.000	39,804.000	39,422.000	39,049.000	38,687.000
1,020.000	38,333.000	37,988.000	37,652.000	37,326.000	37,009.000
1,035.000	36,700.000	36,398.000	36,102.000	35,814.000	35,532.000
1,050.000	35,258.000	34,990.000	34,729.000	34,474.000	34,225.000
1,065.000	33,982.000	33,745.000	33,512.000	33,285.000	33,064.000
1,080.000	32,846.000	32,633.000	32,418.000	32,198.000	31,969.000
1,095.000	31,731.000	31,488.000	31,242.000	30,994.000	30,747.000
1,110.000	30,502.000	30,261.000	30,024.000	29,791.000	29,563.000
1,125.000	29,341.000	29,125.000	28,914.000	28,709.000	28,510.000
1,140.000	28,318.000	28,130.000	27,948.000	27,771.000	27,599.000
1,155.000	27,431.000	27,267.000	27,107.000	26,952.000	26,801.000
1,170.000	26,654.000	26,511.000	26,371.000	26,235.000	26,101.000
1,185.000	25,971.000	25,844.000	25,719.000	25,598.000	25,480.000
1,200.000	25,364.000	25,250.000	25,139.000	25,031.000	24,925.000
1,215.000	24,822.000	24,721.000	24,622.000	24,524.000	24,428.000
1,230.000	24,334.000	24,242.000	24,152.000	24,063.000	23,975.000
1,245.000	23,888.000	23,803.000	23,720.000	23,639.000	23,560.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	23,482.000	23,405.000	23,329.000	23,255.000	23,182.000
1,275.000	23,110.000	23,038.000	22,966.000	22,894.000	22,820.000
1,290.000	22,747.000	22,672.000	22,597.000	22,522.000	22,446.000
1,305.000	22,371.000	22,296.000	22,221.000	22,145.000	22,069.000
1,320.000	21,992.000	21,915.000	21,838.000	21,760.000	21,683.000
1,335.000	21,606.000	21,528.000	21,452.000	21,375.000	21,297.000
1,350.000	21,220.000	21,143.000	21,065.000	20,988.000	20,911.000
1,365.000	20,834.000	20,758.000	20,682.000	20,606.000	20,530.000
1,380.000	20,455.000	20,379.000	20,304.000	20,228.000	20,153.000
1,395.000	20,079.000	20,005.000	19,931.000	19,858.000	19,785.000
1,410.000	19,712.000	19,640.000	19,568.000	19,496.000	19,425.000
1,425.000	19,354.000	19,283.000	19,212.000	19,141.000	19,070.000
1,440.000	18,999.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	6.000	12.000	21.000	34.000
120.000	50.000	69.000	91.000	117.000	145.000
135.000	177.000	211.000	248.000	288.000	331.000
150.000	376.000	423.000	473.000	524.000	578.000
165.000	634.000	692.000	752.000	814.000	877.000
180.000	942.000	1,009.000	1,077.000	1,147.000	1,218.000
195.000	1,290.000	1,364.000	1,439.000	1,515.000	1,593.000
210.000	1,672.000	1,752.000	1,833.000	1,915.000	1,998.000
225.000	2,082.000	2,166.000	2,252.000	2,339.000	2,426.000
240.000	2,514.000	2,603.000	2,693.000	2,784.000	2,875.000
255.000	2,967.000	3,060.000	3,153.000	3,248.000	3,342.000
270.000	3,438.000	3,534.000	3,631.000	3,728.000	3,826.000
285.000	3,925.000	4,024.000	4,124.000	4,224.000	4,324.000
300.000	4,426.000	4,527.000	4,630.000	4,733.000	4,836.000
315.000	4,940.000	5,045.000	5,150.000	5,256.000	5,362.000
330.000	5,469.000	5,577.000	5,684.000	5,793.000	5,902.000
345.000	6,013.000	6,123.000	6,234.000	6,346.000	6,458.000
360.000	6,572.000	6,689.000	6,812.000	6,947.000	7,097.000
375.000	7,262.000	7,437.000	7,621.000	7,810.000	8,005.000
390.000	8,204.000	8,407.000	8,615.000	8,826.000	9,042.000
405.000	9,261.000	9,485.000	9,712.000	9,943.000	10,177.000
420.000	10,415.000	10,658.000	10,904.000	11,154.000	11,409.000
435.000	11,667.000	11,929.000	12,195.000	12,466.000	12,741.000
450.000	13,020.000	13,304.000	13,592.000	13,884.000	14,180.000
465.000	14,481.000	14,786.000	15,097.000	15,412.000	15,732.000
480.000	16,057.000	16,387.000	16,723.000	17,064.000	17,410.000
495.000	17,763.000	18,120.000	18,484.000	18,854.000	19,229.000
510.000	19,611.000	19,999.000	20,394.000	20,796.000	21,205.000
525.000	21,622.000	22,046.000	22,477.000	22,918.000	23,363.000
540.000	23,808.000	24,258.000	24,734.000	25,264.000	25,862.000
555.000	26,525.000	27,237.000	27,984.000	28,755.000	29,544.000
570.000	30,346.000	31,158.000	31,976.000	32,799.000	33,628.000
585.000	34,460.000	35,298.000	36,141.000	36,991.000	37,848.000
600.000	38,713.000	39,588.000	40,473.000	41,372.000	42,283.000
615.000	43,210.000	44,154.000	45,117.000	46,099.000	47,097.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	48,107.000	49,141.000	50,237.000	51,447.000	52,803.000
645.000	54,298.000	55,908.000	57,611.000	59,394.000	61,248.000
660.000	63,167.000	65,156.000	67,237.000	69,444.000	71,788.000
675.000	74,256.000	76,844.000	79,549.000	82,374.000	85,328.000
690.000	88,428.000	91,698.000	95,152.000	98,815.000	102,739.000
705.000	107,008.000	111,774.000	117,349.000	124,201.000	132,980.000
720.000	144,723.000	161,286.000	184,409.000	212,870.000	242,566.000
735.000	269,134.000	289,786.000	303,961.000	312,204.000	315,277.000
750.000	313,958.000	309,197.000	302,046.000	293,312.000	283,656.000
765.000	273,575.000	263,384.000	253,312.000	243,527.000	234,139.000
780.000	225,207.000	216,734.000	208,722.000	201,131.000	193,920.000
795.000	187,088.000	180,632.000	174,539.000	168,801.000	163,413.000
810.000	158,352.000	153,577.000	149,035.000	144,667.000	140,436.000
825.000	136,336.000	132,372.000	128,557.000	124,905.000	121,427.000
840.000	118,130.000	115,013.000	112,062.000	109,263.000	106,614.000
855.000	104,110.000	101,744.000	99,510.000	97,404.000	95,417.000
870.000	93,542.000	91,770.000	90,086.000	88,480.000	86,948.000
885.000	85,485.000	84,090.000	82,757.000	81,484.000	80,268.000
900.000	79,106.000	77,985.000	76,879.000	75,752.000	74,580.000
915.000	73,369.000	72,136.000	70,900.000	69,674.000	68,465.000
930.000	67,274.000	66,106.000	64,970.000	63,871.000	62,809.000
945.000	61,785.000	60,801.000	59,856.000	58,949.000	58,079.000
960.000	57,246.000	56,447.000	55,682.000	54,949.000	54,246.000
975.000	53,573.000	52,927.000	52,308.000	51,714.000	51,143.000
990.000	50,596.000	50,070.000	49,564.000	49,077.000	48,609.000
1,005.000	48,159.000	47,725.000	47,308.000	46,905.000	46,516.000
1,020.000	46,140.000	45,773.000	45,414.000	45,062.000	44,716.000
1,035.000	44,376.000	44,043.000	43,717.000	43,396.000	43,082.000
1,050.000	42,773.000	42,470.000	42,172.000	41,879.000	41,592.000
1,065.000	41,311.000	41,034.000	40,763.000	40,496.000	40,233.000
1,080.000	39,974.000	39,718.000	39,458.000	39,185.000	38,890.000
1,095.000	38,576.000	38,247.000	37,909.000	37,565.000	37,218.000
1,110.000	36,872.000	36,529.000	36,192.000	35,861.000	35,537.000
1,125.000	35,222.000	34,915.000	34,617.000	34,326.000	34,043.000
1,140.000	33,769.000	33,503.000	33,244.000	32,993.000	32,749.000
1,155.000	32,513.000	32,284.000	32,061.000	31,845.000	31,634.000
1,170.000	31,429.000	31,229.000	31,036.000	30,847.000	30,664.000
1,185.000	30,485.000	30,311.000	30,141.000	29,976.000	29,814.000
1,200.000	29,657.000	29,502.000	29,351.000	29,204.000	29,059.000
1,215.000	28,918.000	28,781.000	28,647.000	28,516.000	28,387.000
1,230.000	28,262.000	28,139.000	28,018.000	27,900.000	27,783.000
1,245.000	27,669.000	27,557.000	27,447.000	27,339.000	27,233.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	27,129.000	27,027.000	26,926.000	26,827.000	26,729.000
1,275.000	26,633.000	26,539.000	26,446.000	26,356.000	26,266.000
1,290.000	26,178.000	26,091.000	26,006.000	25,922.000	25,839.000
1,305.000	25,758.000	25,677.000	25,597.000	25,518.000	25,441.000
1,320.000	25,365.000	25,290.000	25,216.000	25,143.000	25,070.000
1,335.000	24,998.000	24,927.000	24,856.000	24,787.000	24,719.000
1,350.000	24,652.000	24,587.000	24,522.000	24,458.000	24,394.000
1,365.000	24,331.000	24,268.000	24,206.000	24,145.000	24,084.000
1,380.000	24,025.000	23,966.000	23,907.000	23,849.000	23,791.000
1,395.000	23,734.000	23,677.000	23,621.000	23,567.000	23,513.000
1,410.000	23,459.000	23,406.000	23,352.000	23,299.000	23,246.000
1,425.000	23,194.000	23,143.000	23,092.000	23,041.000	22,990.000
1,440.000	22,938.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	7.000	16.000
90.000	30.000	48.000	73.000	102.000	137.000
105.000	178.000	223.000	273.000	328.000	387.000
120.000	450.000	518.000	589.000	664.000	742.000
135.000	824.000	909.000	998.000	1,089.000	1,183.000
150.000	1,279.000	1,378.000	1,479.000	1,583.000	1,689.000
165.000	1,796.000	1,906.000	2,018.000	2,131.000	2,246.000
180.000	2,363.000	2,482.000	2,601.000	2,722.000	2,844.000
195.000	2,968.000	3,093.000	3,219.000	3,346.000	3,474.000
210.000	3,604.000	3,734.000	3,865.000	3,996.000	4,129.000
225.000	4,262.000	4,396.000	4,531.000	4,667.000	4,803.000
240.000	4,939.000	5,077.000	5,215.000	5,354.000	5,493.000
255.000	5,633.000	5,773.000	5,914.000	6,054.000	6,196.000
270.000	6,337.000	6,479.000	6,622.000	6,765.000	6,908.000
285.000	7,053.000	7,198.000	7,345.000	7,493.000	7,643.000
300.000	7,796.000	7,952.000	8,110.000	8,271.000	8,436.000
315.000	8,604.000	8,775.000	8,950.000	9,129.000	9,312.000
330.000	9,498.000	9,688.000	9,881.000	10,078.000	10,279.000
345.000	10,483.000	10,691.000	10,902.000	11,117.000	11,336.000
360.000	11,558.000	11,786.000	12,029.000	12,301.000	12,607.000
375.000	12,946.000	13,308.000	13,688.000	14,081.000	14,485.000
390.000	14,897.000	15,316.000	15,741.000	16,172.000	16,607.000
405.000	17,047.000	17,491.000	17,940.000	18,393.000	18,850.000
420.000	19,312.000	19,778.000	20,249.000	20,724.000	21,204.000
435.000	21,688.000	22,176.000	22,669.000	23,166.000	23,661.000
450.000	24,150.000	24,632.000	25,108.000	25,580.000	26,048.000
465.000	26,512.000	26,974.000	27,432.000	27,889.000	28,343.000
480.000	28,796.000	29,247.000	29,698.000	30,149.000	30,601.000
495.000	31,053.000	31,507.000	31,963.000	32,421.000	32,882.000
510.000	33,345.000	33,812.000	34,282.000	34,756.000	35,235.000
525.000	35,719.000	36,208.000	36,704.000	37,205.000	37,713.000
540.000	38,228.000	38,766.000	39,370.000	40,105.000	41,005.000
555.000	42,061.000	43,240.000	44,508.000	45,843.000	47,220.000
570.000	48,614.000	50,012.000	51,409.000	52,799.000	54,180.000
585.000	55,554.000	56,920.000	58,278.000	59,631.000	60,980.000
600.000	62,326.000	63,673.000	65,022.000	66,377.000	67,739.000
615.000	69,111.000	70,492.000	71,875.000	73,264.000	74,663.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	76,077.000	77,535.000	79,108.000	80,897.000	82,963.000
645.000	85,293.000	87,836.000	90,544.000	93,380.000	96,305.000
660.000	99,293.000	102,343.000	105,484.000	108,764.000	112,214.000
675.000	115,834.000	119,597.000	123,479.000	127,483.000	131,623.000
690.000	135,919.000	140,363.000	144,898.000	149,462.000	154,066.000
705.000	158,843.000	164,049.000	170,151.000	177,836.000	187,992.000
720.000	201,957.000	222,123.000	250,731.000	286,096.000	322,753.000
735.000	354,977.000	379,201.000	394,832.000	402,724.000	403,973.000
750.000	399,675.000	391,178.000	379,963.000	367,205.000	353,820.000
765.000	340,383.000	327,215.000	314,496.000	302,381.000	290,948.000
780.000	280,209.000	270,135.000	260,671.000	251,756.000	243,323.000
795.000	235,345.000	227,822.000	220,740.000	214,083.000	207,842.000
810.000	201,983.000	196,452.000	191,167.000	186,020.000	180,935.000
825.000	175,908.000	170,986.000	166,218.000	161,636.000	157,247.000
840.000	153,055.000	149,075.000	145,312.000	141,771.000	138,446.000
855.000	135,314.000	132,355.000	129,563.000	126,931.000	124,449.000
870.000	122,110.000	119,905.000	117,826.000	115,865.000	114,007.000
885.000	112,238.000	110,550.000	108,939.000	107,401.000	105,933.000
900.000	104,530.000	103,169.000	101,799.000	100,346.000	98,767.000
915.000	97,076.000	95,312.000	93,512.000	91,705.000	89,905.000
930.000	88,126.000	86,388.000	84,704.000	83,083.000	81,529.000
945.000	80,043.000	78,628.000	77,281.000	76,002.000	74,787.000
960.000	73,634.000	72,540.000	71,501.000	70,515.000	69,580.000
975.000	68,688.000	67,832.000	67,009.000	66,215.000	65,451.000
990.000	64,715.000	64,006.000	63,322.000	62,663.000	62,028.000
1,005.000	61,414.000	60,822.000	60,250.000	59,697.000	59,161.000
1,020.000	58,643.000	58,140.000	57,652.000	57,180.000	56,723.000
1,035.000	56,279.000	55,849.000	55,432.000	55,026.000	54,633.000
1,050.000	54,250.000	53,877.000	53,514.000	53,160.000	52,816.000
1,065.000	52,480.000	52,154.000	51,836.000	51,526.000	51,224.000
1,080.000	50,929.000	50,636.000	50,332.000	49,997.000	49,621.000
1,095.000	49,205.000	48,759.000	48,294.000	47,818.000	47,337.000
1,110.000	46,858.000	46,387.000	45,923.000	45,468.000	45,021.000
1,125.000	44,582.000	44,154.000	43,737.000	43,330.000	42,935.000
1,140.000	42,550.000	42,177.000	41,815.000	41,463.000	41,121.000
1,155.000	40,788.000	40,464.000	40,150.000	39,845.000	39,549.000
1,170.000	39,262.000	38,983.000	38,711.000	38,446.000	38,188.000
1,185.000	37,936.000	37,691.000	37,453.000	37,220.000	36,993.000
1,200.000	36,772.000	36,555.000	36,345.000	36,139.000	35,938.000
1,215.000	35,742.000	35,549.000	35,361.000	35,177.000	34,996.000
1,230.000	34,820.000	34,648.000	34,478.000	34,313.000	34,150.000
1,245.000	33,991.000	33,836.000	33,683.000	33,532.000	33,384.000

Subsection: Time vs. Volume
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	33,239.000	33,096.000	32,956.000	32,818.000	32,683.000
1,275.000	32,550.000	32,420.000	32,292.000	32,166.000	32,041.000
1,290.000	31,919.000	31,798.000	31,679.000	31,562.000	31,447.000
1,305.000	31,334.000	31,221.000	31,111.000	31,001.000	30,893.000
1,320.000	30,787.000	30,682.000	30,579.000	30,477.000	30,376.000
1,335.000	30,276.000	30,178.000	30,081.000	29,986.000	29,892.000
1,350.000	29,800.000	29,708.000	29,617.000	29,526.000	29,437.000
1,365.000	29,348.000	29,261.000	29,174.000	29,090.000	29,006.000
1,380.000	28,924.000	28,842.000	28,761.000	28,680.000	28,600.000
1,395.000	28,521.000	28,443.000	28,366.000	28,290.000	28,215.000
1,410.000	28,141.000	28,067.000	27,994.000	27,923.000	27,852.000
1,425.000	27,781.000	27,711.000	27,641.000	27,571.000	27,502.000
1,440.000	27,433.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	0.000	0.000
465.000	0.000	0.000	0.000	0.000	0.000
480.000	0.000	0.000	0.000	0.000	0.000
495.000	0.000	0.000	0.000	0.000	0.000
510.000	0.000	0.000	2.000	4.000	9.000
525.000	16.000	26.000	39.000	55.000	72.000
540.000	89.000	107.000	121.000	128.000	130.000
555.000	130.000	129.000	129.000	134.000	149.000
570.000	173.000	208.000	253.000	302.000	348.000
585.000	386.000	412.000	426.000	428.000	432.000
600.000	444.000	473.000	521.000	588.000	668.000
615.000	746.000	810.000	853.000	870.000	864.000

Subsection: Time vs. Volume
 Label: R-1
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	845.000	826.000	824.000	846.000	898.000
645.000	975.000	1,059.000	1,141.000	1,210.000	1,270.000
660.000	1,328.000	1,392.000	1,472.000	1,580.000	1,706.000
675.000	1,830.000	1,942.000	2,045.000	2,143.000	2,251.000
690.000	2,387.000	2,547.000	2,722.000	2,910.000	3,126.000
705.000	3,375.000	3,692.000	4,112.000	4,708.000	5,563.000
720.000	6,711.000	8,245.000	10,171.000	12,483.000	15,091.000
735.000	17,842.000	20,598.000	23,093.000	25,226.000	26,918.000
750.000	28,059.000	28,694.000	28,823.000	28,515.000	27,784.000
765.000	26,711.000	25,401.000	23,894.000	22,358.000	20,868.000
780.000	19,466.000	18,182.000	16,987.000	15,848.000	14,747.000
795.000	13,696.000	12,713.000	11,818.000	11,021.000	10,369.000
810.000	9,873.000	9,454.000	9,066.000	8,651.000	8,173.000
825.000	7,652.000	7,141.000	6,691.000	6,371.000	6,191.000
840.000	6,139.000	6,170.000	6,187.000	6,147.000	6,026.000
855.000	5,802.000	5,504.000	5,187.000	4,903.000	4,716.000
870.000	4,642.000	4,680.000	4,805.000	4,941.000	5,049.000
885.000	5,098.000	5,051.000	4,910.000	4,683.000	4,395.000
900.000	4,074.000	3,732.000	3,416.000	3,157.000	3,005.000
915.000	3,004.000	3,151.000	3,447.000	3,804.000	4,115.000
930.000	4,356.000	4,465.000	4,425.000	4,273.000	4,037.000
945.000	3,760.000	3,458.000	3,138.000	2,791.000	2,440.000
960.000	2,146.000	1,936.000	1,859.000	1,944.000	2,190.000
975.000	2,574.000	3,005.000	3,412.000	3,734.000	3,936.000
990.000	4,022.000	3,981.000	3,844.000	3,644.000	3,398.000
1,005.000	3,124.000	2,820.000	2,506.000	2,194.000	1,899.000
1,020.000	1,639.000	1,412.000	1,221.000	1,063.000	932.000
1,035.000	828.000	795.000	873.000	1,078.000	1,419.000
1,050.000	1,870.000	2,387.000	2,895.000	3,320.000	3,639.000
1,065.000	3,805.000	3,824.000	3,735.000	3,565.000	3,353.000
1,080.000	3,105.000	2,829.000	2,522.000	2,204.000	1,907.000
1,095.000	1,639.000	1,410.000	1,221.000	1,062.000	932.000
1,110.000	819.000	720.000	632.000	557.000	493.000
1,125.000	438.000	391.000	350.000	315.000	285.000
1,140.000	261.000	254.000	287.000	389.000	595.000
1,155.000	921.000	1,377.000	1,915.000	2,465.000	2,975.000
1,170.000	3,363.000	3,610.000	3,723.000	3,700.000	3,584.000
1,185.000	3,404.000	3,180.000	2,919.000	2,631.000	2,333.000
1,200.000	2,030.000	1,752.000	1,512.000	1,305.000	1,133.000
1,215.000	988.000	866.000	761.000	670.000	590.000
1,230.000	522.000	465.000	415.000	372.000	335.000
1,245.000	303.000	275.000	252.000	232.000	215.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	201.000	188.000	177.000	168.000	160.000
1,275.000	153.000	149.000	175.000	280.000	494.000
1,290.000	832.000	1,292.000	1,852.000	2,453.000	2,982.000
1,305.000	3,405.000	3,681.000	3,787.000	3,764.000	3,638.000
1,320.000	3,453.000	3,228.000	2,971.000	2,678.000	2,352.000
1,335.000	2,040.000	1,757.000	1,509.000	1,304.000	1,131.000
1,350.000	989.000	869.000	764.000	670.000	590.000
1,365.000	523.000	465.000	416.000	373.000	335.000
1,380.000	303.000	275.000	252.000	232.000	215.000
1,395.000	201.000	188.000	178.000	168.000	160.000
1,410.000	153.000	147.000	142.000	138.000	134.000
1,425.000	131.000	128.000	125.000	123.000	120.000
1,440.000	117.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	1.000	3.000	7.000
420.000	12.000	21.000	34.000	52.000	76.000
435.000	107.000	137.000	158.000	174.000	189.000
450.000	204.000	220.000	236.000	253.000	271.000
465.000	289.000	308.000	328.000	348.000	369.000
480.000	391.000	413.000	435.000	458.000	482.000
495.000	506.000	530.000	556.000	582.000	608.000
510.000	635.000	663.000	691.000	720.000	749.000
525.000	779.000	810.000	842.000	874.000	908.000
540.000	942.000	978.000	1,016.000	1,058.000	1,105.000
555.000	1,157.000	1,215.000	1,279.000	1,349.000	1,423.000
570.000	1,501.000	1,582.000	1,665.000	1,750.000	1,836.000
585.000	1,923.000	2,012.000	2,102.000	2,192.000	2,284.000
600.000	2,377.000	2,472.000	2,569.000	2,668.000	2,770.000
615.000	2,875.000	2,984.000	3,095.000	3,211.000	3,331.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	3,457.000	3,589.000	3,732.000	3,887.000	4,059.000
645.000	4,252.000	4,464.000	4,698.000	4,952.000	5,224.000
660.000	5,513.000	5,820.000	6,146.000	6,496.000	6,874.000
675.000	7,282.000	7,725.000	8,203.000	8,718.000	9,271.000
690.000	9,866.000	10,507.000	11,210.000	11,974.000	12,837.000
705.000	13,817.000	14,962.000	16,319.000	18,025.000	20,276.000
720.000	23,268.000	27,388.000	32,701.000	38,191.000	43,292.000
735.000	48,453.000	53,468.000	57,643.000	60,891.000	63,073.000
750.000	64,006.000	64,024.000	63,187.000	61,754.000	59,813.000
765.000	57,465.000	54,848.000	51,992.000	49,228.000	46,599.000
780.000	44,151.000	41,949.000	39,949.000	38,150.000	36,507.000
795.000	35,004.000	33,619.000	32,358.000	30,988.000	29,390.000
810.000	27,788.000	26,254.000	24,827.000	23,498.000	22,269.000
825.000	21,124.000	20,056.000	19,055.000	18,121.000	17,250.000
840.000	16,438.000	15,688.000	14,993.000	14,354.000	13,763.000
855.000	13,222.000	12,724.000	12,269.000	11,851.000	11,466.000
870.000	11,112.000	10,782.000	10,475.000	10,188.000	9,923.000
885.000	9,677.000	9,453.000	9,248.000	9,059.000	8,882.000
900.000	8,715.000	8,555.000	8,397.000	8,238.000	8,072.000
915.000	7,896.000	7,711.000	7,516.000	7,316.000	7,114.000
930.000	6,912.000	6,717.000	6,529.000	6,350.000	6,182.000
945.000	6,024.000	5,878.000	5,744.000	5,620.000	5,507.000
960.000	5,402.000	5,306.000	5,216.000	5,133.000	5,055.000
975.000	4,981.000	4,912.000	4,846.000	4,784.000	4,725.000
990.000	4,668.000	4,614.000	4,562.000	4,512.000	4,465.000
1,005.000	4,418.000	4,374.000	4,330.000	4,288.000	4,247.000
1,020.000	4,207.000	4,168.000	4,130.000	4,093.000	4,057.000
1,035.000	4,022.000	3,987.000	3,954.000	3,921.000	3,889.000
1,050.000	3,858.000	3,827.000	3,797.000	3,767.000	3,738.000
1,065.000	3,710.000	3,682.000	3,655.000	3,629.000	3,602.000
1,080.000	3,576.000	3,550.000	3,522.000	3,493.000	3,461.000
1,095.000	3,425.000	3,386.000	3,344.000	3,300.000	3,255.000
1,110.000	3,209.000	3,164.000	3,121.000	3,079.000	3,039.000
1,125.000	3,001.000	2,965.000	2,932.000	2,901.000	2,872.000
1,140.000	2,845.000	2,819.000	2,795.000	2,772.000	2,751.000
1,155.000	2,730.000	2,710.000	2,691.000	2,673.000	2,655.000
1,170.000	2,638.000	2,622.000	2,606.000	2,591.000	2,576.000
1,185.000	2,561.000	2,547.000	2,532.000	2,518.000	2,504.000
1,200.000	2,491.000	2,477.000	2,464.000	2,451.000	2,438.000
1,215.000	2,425.000	2,413.000	2,400.000	2,388.000	2,376.000
1,230.000	2,364.000	2,352.000	2,341.000	2,329.000	2,318.000
1,245.000	2,306.000	2,295.000	2,284.000	2,273.000	2,263.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	2,252.000	2,242.000	2,232.000	2,222.000	2,212.000
1,275.000	2,202.000	2,193.000	2,183.000	2,174.000	2,164.000
1,290.000	2,155.000	2,146.000	2,137.000	2,128.000	2,119.000
1,305.000	2,110.000	2,101.000	2,092.000	2,083.000	2,075.000
1,320.000	2,066.000	2,058.000	2,049.000	2,041.000	2,033.000
1,335.000	2,025.000	2,016.000	2,008.000	2,000.000	1,992.000
1,350.000	1,984.000	1,976.000	1,969.000	1,961.000	1,954.000
1,365.000	1,946.000	1,939.000	1,932.000	1,925.000	1,918.000
1,380.000	1,911.000	1,904.000	1,897.000	1,890.000	1,883.000
1,395.000	1,876.000	1,869.000	1,863.000	1,856.000	1,850.000
1,410.000	1,843.000	1,837.000	1,831.000	1,825.000	1,819.000
1,425.000	1,813.000	1,807.000	1,800.000	1,794.000	1,788.000
1,440.000	1,780.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	1.000	3.000
360.000	5.000	10.000	18.000	30.000	47.000
375.000	71.000	103.000	135.000	160.000	179.000
390.000	197.000	215.000	234.000	254.000	275.000
405.000	297.000	321.000	345.000	369.000	395.000
420.000	421.000	447.000	474.000	502.000	531.000
435.000	559.000	589.000	619.000	649.000	680.000
450.000	712.000	744.000	776.000	809.000	843.000
465.000	877.000	912.000	947.000	983.000	1,020.000
480.000	1,057.000	1,095.000	1,133.000	1,173.000	1,213.000
495.000	1,253.000	1,295.000	1,337.000	1,380.000	1,423.000
510.000	1,468.000	1,513.000	1,560.000	1,607.000	1,655.000
525.000	1,705.000	1,755.000	1,806.000	1,859.000	1,913.000
540.000	1,969.000	2,028.000	2,092.000	2,164.000	2,246.000
555.000	2,340.000	2,448.000	2,568.000	2,700.000	2,841.000
570.000	2,989.000	3,142.000	3,297.000	3,455.000	3,613.000
585.000	3,773.000	3,932.000	4,091.000	4,249.000	4,407.000
600.000	4,567.000	4,727.000	4,890.000	5,056.000	5,225.000
615.000	5,399.000	5,576.000	5,758.000	5,946.000	6,140.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	6,342.000	6,555.000	6,788.000	7,044.000	7,330.000
645.000	7,656.000	8,019.000	8,423.000	8,860.000	9,329.000
660.000	9,826.000	10,350.000	10,901.000	11,484.000	12,104.000
675.000	12,764.000	13,470.000	14,220.000	15,019.000	15,863.000
690.000	16,763.000	17,718.000	18,743.000	19,839.000	21,041.000
705.000	22,374.000	23,900.000	25,684.000	27,912.000	30,845.000
720.000	34,302.000	38,208.000	42,809.000	48,452.000	54,823.000
735.000	61,422.000	67,818.000	73,095.000	77,142.000	79,792.000
750.000	80,815.000	80,653.000	79,395.000	77,383.000	74,745.000
765.000	71,607.000	68,157.000	64,432.000	60,850.000	57,468.000
780.000	54,332.000	51,523.000	48,976.000	46,691.000	44,605.000
795.000	42,699.000	40,939.000	39,335.000	37,883.000	36,542.000
810.000	35,322.000	34,189.000	33,147.000	32,148.000	31,009.000
825.000	29,677.000	28,315.000	26,982.000	25,707.000	24,499.000
840.000	23,361.000	22,301.000	21,314.000	20,403.000	19,559.000
855.000	18,784.000	18,070.000	17,418.000	16,821.000	16,272.000
870.000	15,769.000	15,300.000	14,866.000	14,462.000	14,088.000
885.000	13,741.000	13,425.000	13,136.000	12,869.000	12,620.000
900.000	12,384.000	12,157.000	11,932.000	11,702.000	11,460.000
915.000	11,199.000	10,921.000	10,626.000	10,320.000	10,011.000
930.000	9,702.000	9,400.000	9,111.000	8,837.000	8,578.000
945.000	8,336.000	8,114.000	7,909.000	7,722.000	7,552.000
960.000	7,396.000	7,253.000	7,121.000	7,000.000	6,886.000
975.000	6,780.000	6,681.000	6,587.000	6,498.000	6,415.000
990.000	6,335.000	6,259.000	6,186.000	6,116.000	6,049.000
1,005.000	5,985.000	5,923.000	5,862.000	5,804.000	5,748.000
1,020.000	5,693.000	5,640.000	5,588.000	5,537.000	5,487.000
1,035.000	5,439.000	5,393.000	5,347.000	5,302.000	5,259.000
1,050.000	5,216.000	5,174.000	5,133.000	5,093.000	5,054.000
1,065.000	5,015.000	4,977.000	4,940.000	4,903.000	4,867.000
1,080.000	4,831.000	4,795.000	4,756.000	4,713.000	4,665.000
1,095.000	4,611.000	4,550.000	4,484.000	4,413.000	4,338.000
1,110.000	4,264.000	4,190.000	4,118.000	4,050.000	3,984.000
1,125.000	3,923.000	3,866.000	3,813.000	3,765.000	3,721.000
1,140.000	3,680.000	3,642.000	3,607.000	3,573.000	3,542.000
1,155.000	3,512.000	3,484.000	3,458.000	3,432.000	3,408.000
1,170.000	3,384.000	3,362.000	3,340.000	3,318.000	3,298.000
1,185.000	3,278.000	3,258.000	3,239.000	3,221.000	3,202.000
1,200.000	3,184.000	3,166.000	3,148.000	3,131.000	3,114.000
1,215.000	3,097.000	3,081.000	3,064.000	3,049.000	3,033.000
1,230.000	3,018.000	3,003.000	2,988.000	2,973.000	2,958.000
1,245.000	2,943.000	2,929.000	2,915.000	2,901.000	2,887.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	2,873.000	2,859.000	2,845.000	2,832.000	2,819.000
1,275.000	2,806.000	2,793.000	2,781.000	2,769.000	2,756.000
1,290.000	2,744.000	2,733.000	2,721.000	2,709.000	2,697.000
1,305.000	2,686.000	2,674.000	2,663.000	2,652.000	2,640.000
1,320.000	2,629.000	2,617.000	2,606.000	2,595.000	2,584.000
1,335.000	2,574.000	2,563.000	2,553.000	2,542.000	2,532.000
1,350.000	2,522.000	2,512.000	2,503.000	2,493.000	2,484.000
1,365.000	2,474.000	2,465.000	2,455.000	2,446.000	2,437.000
1,380.000	2,428.000	2,419.000	2,410.000	2,400.000	2,391.000
1,395.000	2,382.000	2,373.000	2,364.000	2,356.000	2,348.000
1,410.000	2,339.000	2,331.000	2,323.000	2,314.000	2,306.000
1,425.000	2,298.000	2,289.000	2,281.000	2,273.000	2,265.000
1,440.000	2,255.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	3.000	6.000	11.000	20.000	33.000
300.000	52.000	78.000	112.000	143.000	165.000
315.000	183.000	200.000	218.000	236.000	256.000
330.000	276.000	297.000	319.000	341.000	365.000
345.000	388.000	412.000	437.000	462.000	488.000
360.000	514.000	542.000	570.000	601.000	634.000
375.000	670.000	709.000	752.000	798.000	846.000
390.000	896.000	948.000	1,001.000	1,055.000	1,109.000
405.000	1,164.000	1,219.000	1,274.000	1,329.000	1,384.000
420.000	1,439.000	1,494.000	1,550.000	1,606.000	1,662.000
435.000	1,718.000	1,775.000	1,833.000	1,891.000	1,950.000
450.000	2,009.000	2,069.000	2,129.000	2,191.000	2,253.000
465.000	2,316.000	2,379.000	2,444.000	2,510.000	2,576.000
480.000	2,644.000	2,712.000	2,782.000	2,852.000	2,924.000
495.000	2,997.000	3,071.000	3,146.000	3,222.000	3,300.000
510.000	3,379.000	3,460.000	3,542.000	3,626.000	3,712.000
525.000	3,799.000	3,887.000	3,978.000	4,070.000	4,165.000
540.000	4,264.000	4,370.000	4,490.000	4,630.000	4,798.000
555.000	4,998.000	5,234.000	5,504.000	5,803.000	6,126.000
570.000	6,463.000	6,811.000	7,161.000	7,512.000	7,861.000
585.000	8,205.000	8,545.000	8,878.000	9,205.000	9,526.000
600.000	9,842.000	10,158.000	10,473.000	10,790.000	11,110.000
615.000	11,434.000	11,764.000	12,100.000	12,443.000	12,795.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	13,162.000	13,548.000	13,973.000	14,446.000	14,986.000
645.000	15,608.000	16,310.000	17,095.000	17,948.000	18,862.000
660.000	19,823.000	20,828.000	21,872.000	22,961.000	24,102.000
675.000	25,296.000	26,552.000	27,867.000	29,245.000	30,680.000
690.000	32,180.000	33,480.000	34,514.000	35,514.000	36,552.000
705.000	37,648.000	38,878.000	40,294.000	42,109.000	44,574.000
720.000	47,907.000	52,606.000	58,729.000	66,344.000	74,956.000
735.000	83,845.000	92,256.000	98,281.000	102,116.000	104,434.000
750.000	104,999.000	104,404.000	102,701.000	100,296.000	97,313.000
765.000	93,873.000	90,086.000	85,545.000	80,815.000	76,339.000
780.000	72,212.000	68,539.000	65,223.000	62,252.000	59,545.000
795.000	57,061.000	54,760.000	52,645.000	50,713.000	48,916.000
810.000	47,264.000	45,716.000	44,277.000	42,913.000	41,620.000
825.000	40,378.000	39,178.000	38,017.000	36,905.000	35,844.000
840.000	34,840.000	33,902.000	33,026.000	32,201.000	31,264.000
855.000	30,188.000	29,111.000	28,095.000	27,153.000	26,286.000
870.000	25,490.000	24,750.000	24,067.000	23,430.000	22,842.000
885.000	22,298.000	21,801.000	21,345.000	20,923.000	20,530.000
900.000	20,155.000	19,793.000	19,427.000	19,046.000	18,634.000
915.000	18,177.000	17,681.000	17,146.000	16,585.000	16,013.000
930.000	15,437.000	14,876.000	14,336.000	13,825.000	13,346.000
945.000	12,899.000	12,490.000	12,117.000	11,780.000	11,476.000
960.000	11,201.000	10,952.000	10,725.000	10,517.000	10,324.000
975.000	10,147.000	9,982.000	9,828.000	9,684.000	9,548.000
990.000	9,419.000	9,298.000	9,182.000	9,073.000	8,968.000
1,005.000	8,868.000	8,772.000	8,679.000	8,590.000	8,503.000
1,020.000	8,419.000	8,338.000	8,258.000	8,181.000	8,106.000
1,035.000	8,033.000	7,962.000	7,893.000	7,826.000	7,760.000
1,050.000	7,696.000	7,633.000	7,572.000	7,512.000	7,453.000
1,065.000	7,395.000	7,339.000	7,284.000	7,230.000	7,176.000
1,080.000	7,122.000	7,067.000	7,007.000	6,939.000	6,859.000
1,095.000	6,768.000	6,663.000	6,544.000	6,417.000	6,283.000
1,110.000	6,148.000	6,014.000	5,885.000	5,762.000	5,645.000
1,125.000	5,536.000	5,435.000	5,343.000	5,260.000	5,184.000
1,140.000	5,116.000	5,054.000	4,996.000	4,943.000	4,893.000
1,155.000	4,846.000	4,802.000	4,761.000	4,722.000	4,684.000
1,170.000	4,648.000	4,614.000	4,582.000	4,550.000	4,520.000
1,185.000	4,491.000	4,462.000	4,434.000	4,406.000	4,380.000
1,200.000	4,353.000	4,328.000	4,303.000	4,278.000	4,254.000
1,215.000	4,230.000	4,207.000	4,184.000	4,161.000	4,139.000
1,230.000	4,117.000	4,096.000	4,075.000	4,054.000	4,033.000
1,245.000	4,013.000	3,993.000	3,973.000	3,954.000	3,935.000

Subsection: Time vs. Volume
Label: R-1
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,915.000	3,897.000	3,878.000	3,859.000	3,841.000
1,275.000	3,823.000	3,805.000	3,787.000	3,770.000	3,752.000
1,290.000	3,735.000	3,718.000	3,702.000	3,685.000	3,669.000
1,305.000	3,653.000	3,637.000	3,621.000	3,606.000	3,590.000
1,320.000	3,574.000	3,558.000	3,543.000	3,527.000	3,512.000
1,335.000	3,497.000	3,482.000	3,467.000	3,452.000	3,438.000
1,350.000	3,424.000	3,410.000	3,396.000	3,382.000	3,369.000
1,365.000	3,355.000	3,342.000	3,329.000	3,316.000	3,303.000
1,380.000	3,291.000	3,278.000	3,265.000	3,253.000	3,240.000
1,395.000	3,228.000	3,215.000	3,203.000	3,191.000	3,180.000
1,410.000	3,168.000	3,157.000	3,145.000	3,134.000	3,122.000
1,425.000	3,111.000	3,100.000	3,089.000	3,078.000	3,067.000
1,440.000	3,053.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R2
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	0.000	0.000
420.000	0.000	0.000	0.000	0.000	0.000
435.000	0.000	0.000	0.000	0.000	0.000
450.000	0.000	0.000	0.000	0.000	0.000
465.000	0.000	0.000	0.000	0.000	0.000
480.000	0.000	0.000	0.000	0.000	0.000
495.000	0.000	0.000	0.000	0.000	0.000
510.000	0.000	0.000	1.000	3.000	6.000
525.000	11.000	18.000	27.000	37.000	48.000
540.000	60.000	72.000	86.000	105.000	126.000
555.000	141.000	147.000	150.000	156.000	170.000
570.000	196.000	235.000	288.000	348.000	412.000
585.000	467.000	511.000	539.000	550.000	557.000
600.000	568.000	592.000	634.000	702.000	797.000
615.000	901.000	1,001.000	1,080.000	1,128.000	1,144.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	1,134.000	1,113.000	1,099.000	1,105.000	1,139.000
645.000	1,208.000	1,310.000	1,426.000	1,536.000	1,635.000
660.000	1,724.000	1,810.000	1,905.000	2,021.000	2,167.000
675.000	2,330.000	2,493.000	2,647.000	2,792.000	2,941.000
690.000	3,115.000	3,319.000	3,548.000	3,794.000	4,057.000
705.000	4,359.000	4,741.000	5,248.000	5,948.000	6,922.000
720.000	8,256.000	10,028.000	12,322.000	15,137.000	18,411.000
735.000	22,005.000	25,713.000	29,293.000	31,824.000	33,258.000
750.000	34,252.000	34,824.000	34,992.000	34,782.000	34,227.000
765.000	33,374.000	32,326.000	31,139.000	29,929.000	28,622.000
780.000	27,042.000	25,293.000	23,611.000	22,041.000	20,555.000
795.000	19,142.000	17,794.000	16,526.000	15,371.000	14,359.000
810.000	13,544.000	12,918.000	12,371.000	11,839.000	11,259.000
825.000	10,608.000	9,950.000	9,334.000	8,820.000	8,450.000
840.000	8,236.000	8,168.000	8,168.000	8,170.000	8,096.000
855.000	7,906.000	7,608.000	7,230.000	6,845.000	6,512.000
870.000	6,280.000	6,171.000	6,204.000	6,361.000	6,549.000
885.000	6,707.000	6,771.000	6,705.000	6,510.000	6,197.000
900.000	5,799.000	5,342.000	4,881.000	4,467.000	4,164.000
915.000	4,029.000	4,096.000	4,381.000	4,803.000	5,262.000
930.000	5,653.000	5,904.000	5,983.000	5,892.000	5,662.000
945.000	5,329.000	4,922.000	4,460.000	3,971.000	3,483.000
960.000	3,047.000	2,721.000	2,542.000	2,545.000	2,748.000
975.000	3,152.000	3,691.000	4,268.000	4,791.000	5,178.000
990.000	5,401.000	5,456.000	5,356.000	5,134.000	4,818.000
1,005.000	4,427.000	3,989.000	3,538.000	3,098.000	2,688.000
1,020.000	2,321.000	2,002.000	1,729.000	1,495.000	1,302.000
1,035.000	1,159.000	1,087.000	1,130.000	1,318.000	1,672.000
1,050.000	2,187.000	2,820.000	3,510.000	4,147.000	4,667.000
1,065.000	5,016.000	5,171.000	5,152.000	4,990.000	4,722.000
1,080.000	4,375.000	3,980.000	3,550.000	3,118.000	2,706.000
1,095.000	2,333.000	2,009.000	1,731.000	1,497.000	1,299.000
1,110.000	1,130.000	986.000	863.000	756.000	665.000
1,125.000	586.000	519.000	462.000	413.000	371.000
1,140.000	336.000	319.000	334.000	425.000	627.000
1,155.000	967.000	1,465.000	2,103.000	2,841.000	3,572.000
1,170.000	4,216.000	4,698.000	4,984.000	5,086.000	5,020.000
1,185.000	4,827.000	4,536.000	4,176.000	3,763.000	3,333.000
1,200.000	2,912.000	2,519.000	2,172.000	1,873.000	1,618.000
1,215.000	1,403.000	1,219.000	1,062.000	929.000	814.000
1,230.000	715.000	630.000	558.000	496.000	443.000
1,245.000	398.000	359.000	325.000	297.000	273.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	252.000	234.000	219.000	206.000	195.000
1,275.000	185.000	178.000	195.000	288.000	494.000
1,290.000	842.000	1,347.000	2,014.000	2,796.000	3,574.000
1,305.000	4,259.000	4,771.000	5,073.000	5,175.000	5,105.000
1,320.000	4,906.000	4,612.000	4,243.000	3,815.000	3,369.000
1,335.000	2,931.000	2,528.000	2,176.000	1,873.000	1,620.000
1,350.000	1,405.000	1,221.000	1,064.000	931.000	815.000
1,365.000	716.000	631.000	559.000	497.000	444.000
1,380.000	398.000	359.000	325.000	297.000	273.000
1,395.000	252.000	234.000	219.000	206.000	195.000
1,410.000	185.000	177.000	170.000	164.000	158.000
1,425.000	153.000	148.000	144.000	140.000	137.000
1,440.000	135.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	0.000	0.000	0.000	0.000	0.000
375.000	0.000	0.000	0.000	0.000	0.000
390.000	0.000	0.000	0.000	0.000	0.000
405.000	0.000	0.000	0.000	1.000	3.000
420.000	6.000	10.000	17.000	27.000	42.000
435.000	61.000	95.000	146.000	188.000	215.000
450.000	237.000	259.000	280.000	303.000	326.000
465.000	350.000	376.000	402.000	429.000	456.000
480.000	485.000	514.000	544.000	575.000	606.000
495.000	638.000	671.000	705.000	739.000	775.000
510.000	811.000	847.000	885.000	924.000	963.000
525.000	1,003.000	1,045.000	1,087.000	1,130.000	1,174.000
540.000	1,220.000	1,267.000	1,318.000	1,372.000	1,431.000
555.000	1,497.000	1,571.000	1,653.000	1,743.000	1,840.000
570.000	1,943.000	2,051.000	2,162.000	2,276.000	2,392.000
585.000	2,510.000	2,629.000	2,749.000	2,871.000	2,993.000
600.000	3,118.000	3,245.000	3,374.000	3,507.000	3,643.000
615.000	3,783.000	3,927.000	4,075.000	4,229.000	4,388.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	4,554.000	4,729.000	4,914.000	5,115.000	5,336.000
645.000	5,580.000	5,852.000	6,152.000	6,481.000	6,836.000
660.000	7,216.000	7,622.000	8,053.000	8,512.000	9,005.000
675.000	9,536.000	10,110.000	10,731.000	11,400.000	12,121.000
690.000	12,898.000	13,736.000	14,647.000	15,643.000	16,746.000
705.000	17,993.000	19,431.000	21,131.000	23,206.000	25,883.000
720.000	29,429.000	33,193.000	37,246.000	42,868.000	49,604.000
735.000	56,442.000	63,177.000	69,301.000	74,064.000	77,051.000
750.000	78,398.000	78,652.000	77,965.000	76,482.000	74,397.000
765.000	71,646.000	68,253.000	64,436.000	60,657.000	57,079.000
780.000	53,769.000	50,761.000	48,036.000	45,571.000	43,335.000
795.000	41,300.000	39,437.000	37,737.000	36,310.000	35,071.000
810.000	33,844.000	32,674.000	31,580.000	30,568.000	29,629.000
825.000	28,631.000	27,437.000	26,143.000	24,876.000	23,677.000
840.000	22,553.000	21,507.000	20,537.000	19,641.000	18,814.000
855.000	18,054.000	17,355.000	16,713.000	16,123.000	15,582.000
870.000	15,086.000	14,633.000	14,215.000	13,831.000	13,475.000
885.000	13,145.000	12,840.000	12,559.000	12,299.000	12,056.000
900.000	11,828.000	11,610.000	11,397.000	11,185.000	10,968.000
915.000	10,741.000	10,500.000	10,247.000	9,982.000	9,711.000
930.000	9,439.000	9,170.000	8,908.000	8,658.000	8,421.000
945.000	8,200.000	7,995.000	7,806.000	7,633.000	7,474.000
960.000	7,328.000	7,194.000	7,069.000	6,953.000	6,845.000
975.000	6,744.000	6,648.000	6,558.000	6,472.000	6,391.000
990.000	6,313.000	6,239.000	6,168.000	6,100.000	6,035.000
1,005.000	5,972.000	5,910.000	5,851.000	5,794.000	5,738.000
1,020.000	5,683.000	5,630.000	5,578.000	5,528.000	5,479.000
1,035.000	5,431.000	5,384.000	5,339.000	5,295.000	5,251.000
1,050.000	5,208.000	5,166.000	5,125.000	5,085.000	5,046.000
1,065.000	5,007.000	4,969.000	4,932.000	4,896.000	4,860.000
1,080.000	4,825.000	4,789.000	4,753.000	4,715.000	4,674.000
1,095.000	4,629.000	4,579.000	4,525.000	4,466.000	4,405.000
1,110.000	4,343.000	4,281.000	4,220.000	4,161.000	4,105.000
1,125.000	4,052.000	4,002.000	3,956.000	3,913.000	3,873.000
1,140.000	3,836.000	3,801.000	3,768.000	3,736.000	3,707.000
1,155.000	3,679.000	3,652.000	3,626.000	3,601.000	3,577.000
1,170.000	3,554.000	3,531.000	3,509.000	3,488.000	3,467.000
1,185.000	3,447.000	3,427.000	3,407.000	3,388.000	3,369.000
1,200.000	3,350.000	3,332.000	3,314.000	3,297.000	3,280.000
1,215.000	3,262.000	3,246.000	3,229.000	3,212.000	3,196.000
1,230.000	3,180.000	3,164.000	3,148.000	3,133.000	3,117.000
1,245.000	3,102.000	3,086.000	3,071.000	3,056.000	3,042.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,028.000	3,014.000	3,000.000	2,986.000	2,973.000
1,275.000	2,960.000	2,947.000	2,934.000	2,921.000	2,908.000
1,290.000	2,895.000	2,883.000	2,870.000	2,858.000	2,845.000
1,305.000	2,833.000	2,822.000	2,810.000	2,798.000	2,787.000
1,320.000	2,775.000	2,764.000	2,753.000	2,741.000	2,730.000
1,335.000	2,719.000	2,708.000	2,697.000	2,686.000	2,676.000
1,350.000	2,665.000	2,654.000	2,644.000	2,633.000	2,623.000
1,365.000	2,613.000	2,603.000	2,593.000	2,583.000	2,574.000
1,380.000	2,564.000	2,555.000	2,545.000	2,536.000	2,526.000
1,395.000	2,517.000	2,508.000	2,499.000	2,490.000	2,482.000
1,410.000	2,473.000	2,465.000	2,457.000	2,448.000	2,440.000
1,425.000	2,432.000	2,423.000	2,415.000	2,406.000	2,398.000
1,440.000	2,388.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)	Volume (ft ³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	0.000	0.000	0.000	0.000
300.000	0.000	0.000	0.000	0.000	0.000
315.000	0.000	0.000	0.000	0.000	0.000
330.000	0.000	0.000	0.000	0.000	0.000
345.000	0.000	0.000	0.000	0.000	0.000
360.000	2.000	5.000	9.000	15.000	25.000
375.000	38.000	58.000	92.000	144.000	192.000
390.000	223.000	249.000	275.000	301.000	329.000
405.000	358.000	389.000	420.000	453.000	487.000
420.000	522.000	557.000	593.000	631.000	668.000
435.000	707.000	746.000	786.000	827.000	869.000
450.000	911.000	954.000	997.000	1,042.000	1,087.000
465.000	1,132.000	1,179.000	1,226.000	1,274.000	1,323.000
480.000	1,373.000	1,424.000	1,475.000	1,528.000	1,581.000
495.000	1,636.000	1,691.000	1,747.000	1,805.000	1,863.000
510.000	1,923.000	1,984.000	2,046.000	2,109.000	2,173.000
525.000	2,239.000	2,307.000	2,375.000	2,446.000	2,518.000
540.000	2,591.000	2,669.000	2,752.000	2,844.000	2,947.000
555.000	3,065.000	3,200.000	3,353.000	3,523.000	3,707.000
570.000	3,903.000	4,108.000	4,318.000	4,531.000	4,747.000
585.000	4,962.000	5,178.000	5,392.000	5,606.000	5,820.000
600.000	6,034.000	6,250.000	6,468.000	6,690.000	6,917.000
615.000	7,148.000	7,384.000	7,627.000	7,877.000	8,135.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	8,403.000	8,684.000	8,986.000	9,315.000	9,679.000
645.000	10,089.000	10,551.000	11,066.000	11,633.000	12,248.000
660.000	12,904.000	13,599.000	14,332.000	15,104.000	15,921.000
675.000	16,788.000	17,710.000	18,691.000	19,735.000	20,845.000
690.000	22,025.000	23,281.000	24,623.000	26,064.000	27,623.000
705.000	29,321.000	30,833.000	32,196.000	33,846.000	36,002.000
720.000	39,157.000	43,620.000	49,310.000	56,187.000	64,233.000
735.000	72,921.000	80,511.000	86,425.000	91,052.000	94,266.000
750.000	95,860.000	95,988.000	94,912.000	92,819.000	89,976.000
765.000	86,532.000	82,805.000	78,890.000	75,086.000	71,308.000
780.000	67,365.000	63,537.000	60,070.000	56,941.000	54,105.000
795.000	51,524.000	49,160.000	46,997.000	45,025.000	43,224.000
810.000	41,574.000	40,063.000	38,667.000	37,389.000	36,289.000
825.000	35,288.000	34,259.000	33,248.000	32,277.000	31,354.000
840.000	30,484.000	29,670.000	28,826.000	27,826.000	26,730.000
855.000	25,666.000	24,671.000	23,754.000	22,912.000	22,139.000
870.000	21,433.000	20,788.000	20,195.000	19,650.000	19,146.000
885.000	18,680.000	18,249.000	17,852.000	17,484.000	17,141.000
900.000	16,818.000	16,510.000	16,207.000	15,903.000	15,588.000
915.000	15,253.000	14,895.000	14,512.000	14,110.000	13,695.000
930.000	13,276.000	12,862.000	12,459.000	12,074.000	11,710.000
945.000	11,370.000	11,057.000	10,769.000	10,507.000	10,268.000
960.000	10,050.000	9,850.000	9,666.000	9,496.000	9,338.000
975.000	9,191.000	9,053.000	8,923.000	8,801.000	8,686.000
990.000	8,576.000	8,472.000	8,372.000	8,276.000	8,184.000
1,005.000	8,096.000	8,011.000	7,929.000	7,850.000	7,773.000
1,020.000	7,699.000	7,626.000	7,556.000	7,487.000	7,420.000
1,035.000	7,354.000	7,291.000	7,228.000	7,168.000	7,108.000
1,050.000	7,050.000	6,993.000	6,938.000	6,883.000	6,830.000
1,065.000	6,777.000	6,726.000	6,675.000	6,625.000	6,576.000
1,080.000	6,528.000	6,479.000	6,428.000	6,373.000	6,313.000
1,095.000	6,245.000	6,168.000	6,082.000	5,989.000	5,890.000
1,110.000	5,789.000	5,687.000	5,587.000	5,491.000	5,399.000
1,125.000	5,313.000	5,233.000	5,159.000	5,091.000	5,029.000
1,140.000	4,972.000	4,919.000	4,870.000	4,824.000	4,781.000
1,155.000	4,740.000	4,701.000	4,665.000	4,630.000	4,596.000
1,170.000	4,564.000	4,533.000	4,503.000	4,474.000	4,445.000
1,185.000	4,418.000	4,391.000	4,365.000	4,340.000	4,315.000
1,200.000	4,290.000	4,266.000	4,243.000	4,219.000	4,196.000
1,215.000	4,174.000	4,151.000	4,129.000	4,108.000	4,087.000
1,230.000	4,066.000	4,045.000	4,025.000	4,004.000	3,984.000
1,245.000	3,964.000	3,945.000	3,925.000	3,906.000	3,887.000

Subsection: Time vs. Volume
 Label: R2
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	3,868.000	3,850.000	3,832.000	3,814.000	3,796.000
1,275.000	3,778.000	3,761.000	3,744.000	3,727.000	3,710.000
1,290.000	3,694.000	3,678.000	3,662.000	3,646.000	3,630.000
1,305.000	3,614.000	3,599.000	3,583.000	3,568.000	3,552.000
1,320.000	3,537.000	3,522.000	3,507.000	3,492.000	3,477.000
1,335.000	3,463.000	3,448.000	3,434.000	3,420.000	3,406.000
1,350.000	3,393.000	3,379.000	3,366.000	3,353.000	3,340.000
1,365.000	3,328.000	3,315.000	3,302.000	3,290.000	3,277.000
1,380.000	3,265.000	3,253.000	3,241.000	3,228.000	3,216.000
1,395.000	3,203.000	3,191.000	3,179.000	3,167.000	3,156.000
1,410.000	3,144.000	3,133.000	3,122.000	3,111.000	3,100.000
1,425.000	3,088.000	3,077.000	3,066.000	3,055.000	3,045.000
1,440.000	3,032.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time vs. Volume
 Label: R2
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
0.000	0.000	0.000	0.000	0.000	0.000
15.000	0.000	0.000	0.000	0.000	0.000
30.000	0.000	0.000	0.000	0.000	0.000
45.000	0.000	0.000	0.000	0.000	0.000
60.000	0.000	0.000	0.000	0.000	0.000
75.000	0.000	0.000	0.000	0.000	0.000
90.000	0.000	0.000	0.000	0.000	0.000
105.000	0.000	0.000	0.000	0.000	0.000
120.000	0.000	0.000	0.000	0.000	0.000
135.000	0.000	0.000	0.000	0.000	0.000
150.000	0.000	0.000	0.000	0.000	0.000
165.000	0.000	0.000	0.000	0.000	0.000
180.000	0.000	0.000	0.000	0.000	0.000
195.000	0.000	0.000	0.000	0.000	0.000
210.000	0.000	0.000	0.000	0.000	0.000
225.000	0.000	0.000	0.000	0.000	0.000
240.000	0.000	0.000	0.000	0.000	0.000
255.000	0.000	0.000	0.000	0.000	0.000
270.000	0.000	0.000	0.000	0.000	0.000
285.000	0.000	2.000	5.000	9.000	16.000
300.000	27.000	42.000	63.000	102.000	156.000
315.000	199.000	228.000	254.000	278.000	304.000
330.000	331.000	359.000	388.000	417.000	448.000
345.000	480.000	512.000	545.000	579.000	613.000
360.000	648.000	684.000	722.000	762.000	805.000
375.000	851.000	902.000	957.000	1,016.000	1,080.000
390.000	1,146.000	1,215.000	1,286.000	1,358.000	1,432.000
405.000	1,506.000	1,580.000	1,654.000	1,728.000	1,803.000
420.000	1,877.000	1,951.000	2,026.000	2,101.000	2,177.000
435.000	2,253.000	2,329.000	2,407.000	2,485.000	2,563.000
450.000	2,643.000	2,724.000	2,805.000	2,887.000	2,971.000
465.000	3,055.000	3,140.000	3,227.000	3,315.000	3,404.000
480.000	3,494.000	3,586.000	3,680.000	3,774.000	3,870.000
495.000	3,968.000	4,067.000	4,168.000	4,270.000	4,375.000
510.000	4,481.000	4,589.000	4,699.000	4,811.000	4,926.000
525.000	5,042.000	5,161.000	5,282.000	5,406.000	5,532.000
540.000	5,662.000	5,801.000	5,953.000	6,128.000	6,332.000
555.000	6,577.000	6,870.000	7,210.000	7,595.000	8,016.000
570.000	8,465.000	8,932.000	9,409.000	9,888.000	10,366.000
585.000	10,838.000	11,301.000	11,755.000	12,199.000	12,635.000
600.000	13,066.000	13,493.000	13,919.000	14,347.000	14,777.000
615.000	15,212.000	15,654.000	16,103.000	16,561.000	17,032.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
630.000	17,517.000	18,027.000	18,576.000	19,181.000	19,861.000
645.000	20,638.000	21,525.000	22,523.000	23,629.000	24,826.000
660.000	26,102.000	27,443.000	28,843.000	30,079.000	31,050.000
675.000	31,967.000	32,923.000	33,924.000	34,972.000	36,069.000
690.000	37,215.000	38,543.000	39,958.000	41,296.000	42,654.000
705.000	44,087.000	45,656.000	47,459.000	49,665.000	52,608.000
720.000	56,687.000	62,253.000	69,648.000	78,185.000	87,105.000
735.000	96,273.000	105,322.000	113,644.000	119,892.000	123,795.000
750.000	125,460.000	125,129.000	123,212.000	119,968.000	115,790.000
765.000	110,875.000	105,728.000	100,606.000	95,672.000	90,958.000
780.000	86,661.000	82,811.000	79,349.000	76,233.000	73,408.000
795.000	70,515.000	67,482.000	64,638.000	62,021.000	59,610.000
810.000	57,382.000	55,322.000	53,399.000	51,599.000	49,887.000
825.000	48,254.000	46,677.000	45,151.000	43,681.000	42,273.000
840.000	40,936.000	39,677.000	38,501.000	37,416.000	36,492.000
855.000	35,669.000	34,847.000	34,067.000	33,342.000	32,676.000
870.000	32,066.000	31,510.000	30,998.000	30,527.000	30,091.000
885.000	29,688.000	29,315.000	28,900.000	28,404.000	27,889.000
900.000	27,384.000	26,894.000	26,406.000	25,906.000	25,377.000
915.000	24,799.000	24,167.000	23,478.000	22,743.000	21,978.000
930.000	21,200.000	20,426.000	19,674.000	18,954.000	18,276.000
945.000	17,646.000	17,067.000	16,541.000	16,066.000	15,637.000
960.000	15,249.000	14,899.000	14,579.000	14,288.000	14,020.000
975.000	13,772.000	13,543.000	13,330.000	13,130.000	12,943.000
990.000	12,767.000	12,600.000	12,442.000	12,291.000	12,147.000
1,005.000	12,010.000	11,878.000	11,752.000	11,630.000	11,512.000
1,020.000	11,398.000	11,287.000	11,179.000	11,074.000	10,972.000
1,035.000	10,873.000	10,776.000	10,682.000	10,591.000	10,502.000
1,050.000	10,415.000	10,330.000	10,246.000	10,165.000	10,085.000
1,065.000	10,006.000	9,929.000	9,854.000	9,781.000	9,708.000
1,080.000	9,637.000	9,564.000	9,487.000	9,401.000	9,303.000
1,095.000	9,189.000	9,056.000	8,905.000	8,737.000	8,560.000
1,110.000	8,376.000	8,191.000	8,010.000	7,836.000	7,671.000
1,125.000	7,518.000	7,376.000	7,247.000	7,130.000	7,024.000
1,140.000	6,927.000	6,839.000	6,759.000	6,684.000	6,615.000
1,155.000	6,551.000	6,490.000	6,433.000	6,379.000	6,327.000
1,170.000	6,278.000	6,232.000	6,187.000	6,144.000	6,102.000
1,185.000	6,062.000	6,023.000	5,985.000	5,948.000	5,912.000
1,200.000	5,876.000	5,842.000	5,807.000	5,774.000	5,741.000
1,215.000	5,709.000	5,677.000	5,646.000	5,615.000	5,585.000
1,230.000	5,556.000	5,527.000	5,498.000	5,470.000	5,442.000
1,245.000	5,414.000	5,387.000	5,360.000	5,334.000	5,307.000

Subsection: Time vs. Volume
Label: R2
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Time vs. Volume (ft³)

Output Time increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)	Volume (ft³)
1,260.000	5,281.000	5,256.000	5,230.000	5,205.000	5,180.000
1,275.000	5,155.000	5,131.000	5,107.000	5,084.000	5,060.000
1,290.000	5,037.000	5,015.000	4,992.000	4,970.000	4,948.000
1,305.000	4,926.000	4,904.000	4,882.000	4,860.000	4,839.000
1,320.000	4,817.000	4,796.000	4,775.000	4,754.000	4,733.000
1,335.000	4,713.000	4,693.000	4,673.000	4,653.000	4,634.000
1,350.000	4,615.000	4,596.000	4,577.000	4,558.000	4,540.000
1,365.000	4,522.000	4,504.000	4,486.000	4,468.000	4,451.000
1,380.000	4,434.000	4,416.000	4,399.000	4,382.000	4,365.000
1,395.000	4,348.000	4,332.000	4,315.000	4,299.000	4,283.000
1,410.000	4,267.000	4,251.000	4,236.000	4,220.000	4,205.000
1,425.000	4,190.000	4,175.000	4,160.000	4,145.000	4,130.000
1,440.000	4,113.000	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
 Label: POND A
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
582.00	0.0	2.000	0.000	0.000	0.000
583.00	0.0	2.000	6.000	87,120.000	87,120.000
584.00	0.0	2.000	6.000	87,120.000	174,240.000
585.00	0.0	2.000	6.000	87,120.000	261,360.000
586.00	0.0	2.000	6.000	87,120.000	348,480.000
587.00	0.0	2.000	6.000	87,120.000	435,600.000
588.00	0.0	2.000	6.000	87,120.000	522,720.000
589.00	0.0	2.000	6.000	87,120.000	609,840.000
590.00	0.0	2.000	6.000	87,120.000	696,960.000

Subsection: Volume Equations
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
582.00	0.0	2.000	0.000	0.000	0.000
583.00	0.0	2.000	6.000	87,120.000	87,120.000
584.00	0.0	2.000	6.000	87,120.000	174,240.000
585.00	0.0	2.000	6.000	87,120.000	261,360.000
586.00	0.0	2.000	6.000	87,120.000	348,480.000
587.00	0.0	2.000	6.000	87,120.000	435,600.000
588.00	0.0	2.000	6.000	87,120.000	522,720.000
589.00	0.0	2.000	6.000	87,120.000	609,840.000
590.00	0.0	2.000	6.000	87,120.000	696,960.000

Subsection: Volume Equations
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
582.00	0.0	2.000	0.000	0.000	0.000
583.00	0.0	2.000	6.000	87,120.000	87,120.000
584.00	0.0	2.000	6.000	87,120.000	174,240.000
585.00	0.0	2.000	6.000	87,120.000	261,360.000
586.00	0.0	2.000	6.000	87,120.000	348,480.000
587.00	0.0	2.000	6.000	87,120.000	435,600.000
588.00	0.0	2.000	6.000	87,120.000	522,720.000
589.00	0.0	2.000	6.000	87,120.000	609,840.000
590.00	0.0	2.000	6.000	87,120.000	696,960.000

Subsection: Volume Equations
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
 Label: POND A
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
582.00	0.0	2.000	0.000	0.000	0.000
583.00	0.0	2.000	6.000	87,120.000	87,120.000
584.00	0.0	2.000	6.000	87,120.000	174,240.000
585.00	0.0	2.000	6.000	87,120.000	261,360.000
586.00	0.0	2.000	6.000	87,120.000	348,480.000
587.00	0.0	2.000	6.000	87,120.000	435,600.000
588.00	0.0	2.000	6.000	87,120.000	522,720.000
589.00	0.0	2.000	6.000	87,120.000	609,840.000
590.00	0.0	2.000	6.000	87,120.000	696,960.000

Subsection: Volume Equations
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve

Label: POND B

Scenario: EX 2

Return Event: 2 years

Storm Event: 2 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
580.00	0.0	1.060	0.000	0.000	0.000
581.00	0.0	1.060	3.180	46,174.000	46,174.000
582.00	0.0	1.060	3.180	46,174.000	92,347.000
583.00	0.0	1.060	3.180	46,174.000	138,521.000
584.00	0.0	1.060	3.180	46,174.000	184,694.000
585.00	0.0	1.060	3.180	46,174.000	230,868.000
586.00	0.0	1.060	3.180	46,174.000	277,042.000
587.00	0.0	1.060	3.180	46,174.000	323,215.000
588.00	0.0	1.060	3.180	46,174.000	369,389.000
589.00	0.0	1.060	3.180	46,174.000	415,562.000

Subsection: Volume Equations
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
580.00	0.0	1.060	0.000	0.000	0.000
581.00	0.0	1.060	3.180	46,174.000	46,174.000
582.00	0.0	1.060	3.180	46,174.000	92,347.000
583.00	0.0	1.060	3.180	46,174.000	138,521.000
584.00	0.0	1.060	3.180	46,174.000	184,694.000
585.00	0.0	1.060	3.180	46,174.000	230,868.000
586.00	0.0	1.060	3.180	46,174.000	277,042.000
587.00	0.0	1.060	3.180	46,174.000	323,215.000
588.00	0.0	1.060	3.180	46,174.000	369,389.000
589.00	0.0	1.060	3.180	46,174.000	415,562.000

Subsection: Volume Equations
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
580.00	0.0	1.060	0.000	0.000	0.000
581.00	0.0	1.060	3.180	46,174.000	46,174.000
582.00	0.0	1.060	3.180	46,174.000	92,347.000
583.00	0.0	1.060	3.180	46,174.000	138,521.000
584.00	0.0	1.060	3.180	46,174.000	184,694.000
585.00	0.0	1.060	3.180	46,174.000	230,868.000
586.00	0.0	1.060	3.180	46,174.000	277,042.000
587.00	0.0	1.060	3.180	46,174.000	323,215.000
588.00	0.0	1.060	3.180	46,174.000	369,389.000
589.00	0.0	1.060	3.180	46,174.000	415,562.000

Subsection: Volume Equations
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Elevation-Area Volume Curve
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ft ³)	Volume (Total) (ft ³)
580.00	0.0	1.060	0.000	0.000	0.000
581.00	0.0	1.060	3.180	46,174.000	46,174.000
582.00	0.0	1.060	3.180	46,174.000	92,347.000
583.00	0.0	1.060	3.180	46,174.000	138,521.000
584.00	0.0	1.060	3.180	46,174.000	184,694.000
585.00	0.0	1.060	3.180	46,174.000	230,868.000
586.00	0.0	1.060	3.180	46,174.000	277,042.000
587.00	0.0	1.060	3.180	46,174.000	323,215.000
588.00	0.0	1.060	3.180	46,174.000	369,389.000
589.00	0.0	1.060	3.180	46,174.000	415,562.000

Subsection: Volume Equations
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Pond Volume Equations

*** Incremental volume computed by the Conic Method for Reservoir Volumes.**

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sqr}(\text{Area1} * \text{Area2}))$$

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	582.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	590.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	582.00 (N/A)	590.00 (N/A)

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	582.00 ft
Weir Length	5.50 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
 Label: POND A
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
 Structure ID = Weir - 1 (Rectangular Weir)

Upstream ID = (Pond Water Surface)
 Downstream ID = Tailwater (Pond Outfall)

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
582.00	0.00	(N/A)	0.00
582.50	5.25	(N/A)	0.00
583.00	14.85	(N/A)	0.00
583.50	27.28	(N/A)	0.00
584.00	42.00	(N/A)	0.00
584.50	58.70	(N/A)	0.00
585.00	77.16	(N/A)	0.00
585.50	97.24	(N/A)	0.00
586.00	118.80	(N/A)	0.00
586.50	141.76	(N/A)	0.00
587.00	166.03	(N/A)	0.00
587.50	191.54	(N/A)	0.00
588.00	218.25	(N/A)	0.00
588.50	246.09	(N/A)	0.00
589.00	275.03	(N/A)	0.00
589.50	305.01	(N/A)	0.00
590.00	336.02	(N/A)	0.00

Computation Messages

H=.00; Htw=.00;
 Qfree=.00;
 H=.50; Htw=.00;
 Qfree=5.25;
 H=1.00; Htw=.00;
 Qfree=14.85;
 H=1.50; Htw=.00;
 Qfree=27.28;
 H=2.00; Htw=.00;
 Qfree=42.00;
 H=2.50; Htw=.00;
 Qfree=58.70;
 H=3.00; Htw=.00;
 Qfree=77.16;
 H=3.50; Htw=.00;
 Qfree=97.24;
 H=4.00; Htw=.00;
 Qfree=118.80;
 H=4.50; Htw=.00;
 Qfree=141.76;
 H=5.00; Htw=.00;
 Qfree=166.03;

Subsection: Individual Outlet Curves
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = Weir - 1 (Rectangular Weir)

Upstream ID = (Pond Water Surface)
Downstream ID = Tailwater (Pond Outfall)

Computation Messages

H=5.50; Htw=.00; Qfree=191.54; H=6.00; Htw=.00; Qfree=218.25; H=6.50; Htw=.00; Qfree=246.09; H=7.00; Htw=.00; Qfree=275.03; H=7.50; Htw=.00; Qfree=305.01; H=8.00; Htw=.00; Qfree=336.02;
--

Subsection: Composite Rating Curve
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
582.00	0.00	(N/A)	0.00
582.50	5.25	(N/A)	0.00
583.00	14.85	(N/A)	0.00
583.50	27.28	(N/A)	0.00
584.00	42.00	(N/A)	0.00
584.50	58.70	(N/A)	0.00
585.00	77.16	(N/A)	0.00
585.50	97.24	(N/A)	0.00
586.00	118.80	(N/A)	0.00
586.50	141.76	(N/A)	0.00
587.00	166.03	(N/A)	0.00
587.50	191.54	(N/A)	0.00
588.00	218.25	(N/A)	0.00
588.50	246.09	(N/A)	0.00
589.00	275.03	(N/A)	0.00
589.50	305.01	(N/A)	0.00
590.00	336.02	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	582.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	590.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	582.00 (N/A)	590.00 (N/A)

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	582.00 ft
Weir Length	5.50 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
582.00	0.00	(N/A)	0.00
582.50	5.25	(N/A)	0.00
583.00	14.85	(N/A)	0.00
583.50	27.28	(N/A)	0.00
584.00	42.00	(N/A)	0.00
584.50	58.70	(N/A)	0.00
585.00	77.16	(N/A)	0.00
585.50	97.24	(N/A)	0.00
586.00	118.80	(N/A)	0.00
586.50	141.76	(N/A)	0.00
587.00	166.03	(N/A)	0.00
587.50	191.54	(N/A)	0.00
588.00	218.25	(N/A)	0.00
588.50	246.09	(N/A)	0.00
589.00	275.03	(N/A)	0.00
589.50	305.01	(N/A)	0.00
590.00	336.02	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	582.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	590.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	TW	582.00	590.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	582.00 ft
Weir Length	5.50 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
582.00	0.00	(N/A)	0.00
582.50	5.25	(N/A)	0.00
583.00	14.85	(N/A)	0.00
583.50	27.28	(N/A)	0.00
584.00	42.00	(N/A)	0.00
584.50	58.70	(N/A)	0.00
585.00	77.16	(N/A)	0.00
585.50	97.24	(N/A)	0.00
586.00	118.80	(N/A)	0.00
586.50	141.76	(N/A)	0.00
587.00	166.03	(N/A)	0.00
587.50	191.54	(N/A)	0.00
588.00	218.25	(N/A)	0.00
588.50	246.09	(N/A)	0.00
589.00	275.03	(N/A)	0.00
589.50	305.01	(N/A)	0.00
590.00	336.02	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	582.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	590.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	582.00 (N/A)	590.00 (N/A)

Subsection: Outlet Input Data
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	582.00 ft
Weir Length	5.50 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
582.00	0.00	(N/A)	0.00
582.50	5.25	(N/A)	0.00
583.00	14.85	(N/A)	0.00
583.50	27.28	(N/A)	0.00
584.00	42.00	(N/A)	0.00
584.50	58.70	(N/A)	0.00
585.00	77.16	(N/A)	0.00
585.50	97.24	(N/A)	0.00
586.00	118.80	(N/A)	0.00
586.50	141.76	(N/A)	0.00
587.00	166.03	(N/A)	0.00
587.50	191.54	(N/A)	0.00
588.00	218.25	(N/A)	0.00
588.50	246.09	(N/A)	0.00
589.00	275.03	(N/A)	0.00
589.50	305.01	(N/A)	0.00
590.00	336.02	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	580.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	589.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	580.00 (N/A)	589.00 (N/A)

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	580.00 ft
Weir Length	4.00 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = Weir - 1 (Rectangular Weir)

Upstream ID = (Pond Water Surface)
Downstream ID = Tailwater (Pond Outfall)

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
580.00	0.00	(N/A)	0.00
580.50	3.82	(N/A)	0.00
581.00	10.80	(N/A)	0.00
581.50	19.84	(N/A)	0.00
582.00	30.55	(N/A)	0.00
582.50	42.69	(N/A)	0.00
583.00	56.12	(N/A)	0.00
583.50	70.72	(N/A)	0.00
584.00	86.40	(N/A)	0.00
584.50	103.10	(N/A)	0.00
585.00	120.75	(N/A)	0.00
585.50	139.31	(N/A)	0.00
586.00	158.73	(N/A)	0.00
586.50	178.98	(N/A)	0.00
587.00	200.02	(N/A)	0.00
587.50	221.83	(N/A)	0.00
588.00	244.38	(N/A)	0.00
588.50	267.64	(N/A)	0.00
589.00	291.60	(N/A)	0.00

Computation Messages

H=.00; Htw=.00;
Qfree=.00;
H=.50; Htw=.00;
Qfree=3.82;
H=1.00; Htw=.00;
Qfree=10.80;
H=1.50; Htw=.00;
Qfree=19.84;
H=2.00; Htw=.00;
Qfree=30.55;
H=2.50; Htw=.00;
Qfree=42.69;
H=3.00; Htw=.00;
Qfree=56.12;
H=3.50; Htw=.00;
Qfree=70.72;
H=4.00; Htw=.00;
Qfree=86.40;
H=4.50; Htw=.00;
Qfree=103.10;

Subsection: Individual Outlet Curves
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = Weir - 1 (Rectangular Weir)

Upstream ID = (Pond Water Surface)
Downstream ID = Tailwater (Pond Outfall)

Computation Messages

H=5.00; Htw=.00; Qfree=120.75; H=5.50; Htw=.00; Qfree=139.31; H=6.00; Htw=.00; Qfree=158.73; H=6.50; Htw=.00; Qfree=178.98; H=7.00; Htw=.00; Qfree=200.02; H=7.50; Htw=.00; Qfree=221.83; H=8.00; Htw=.00; Qfree=244.38; H=8.50; Htw=.00; Qfree=267.64; H=9.00; Htw=.00; Qfree=291.60;

Subsection: Composite Rating Curve
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
580.00	0.00	(N/A)	0.00
580.50	3.82	(N/A)	0.00
581.00	10.80	(N/A)	0.00
581.50	19.84	(N/A)	0.00
582.00	30.55	(N/A)	0.00
582.50	42.69	(N/A)	0.00
583.00	56.12	(N/A)	0.00
583.50	70.72	(N/A)	0.00
584.00	86.40	(N/A)	0.00
584.50	103.10	(N/A)	0.00
585.00	120.75	(N/A)	0.00
585.50	139.31	(N/A)	0.00
586.00	158.73	(N/A)	0.00
586.50	178.98	(N/A)	0.00
587.00	200.02	(N/A)	0.00
587.50	221.83	(N/A)	0.00
588.00	244.38	(N/A)	0.00
588.50	267.64	(N/A)	0.00
589.00	291.60	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	580.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	589.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	580.00 (N/A)	589.00 (N/A)

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	580.00 ft
Weir Length	4.00 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
580.00	0.00	(N/A)	0.00
580.50	3.82	(N/A)	0.00
581.00	10.80	(N/A)	0.00
581.50	19.84	(N/A)	0.00
582.00	30.55	(N/A)	0.00
582.50	42.69	(N/A)	0.00
583.00	56.12	(N/A)	0.00
583.50	70.72	(N/A)	0.00
584.00	86.40	(N/A)	0.00
584.50	103.10	(N/A)	0.00
585.00	120.75	(N/A)	0.00
585.50	139.31	(N/A)	0.00
586.00	158.73	(N/A)	0.00
586.50	178.98	(N/A)	0.00
587.00	200.02	(N/A)	0.00
587.50	221.83	(N/A)	0.00
588.00	244.38	(N/A)	0.00
588.50	267.64	(N/A)	0.00
589.00	291.60	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	580.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	589.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	580.00 (N/A)	589.00 (N/A)

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	580.00 ft
Weir Length	4.00 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
580.00	0.00	(N/A)	0.00
580.50	3.82	(N/A)	0.00
581.00	10.80	(N/A)	0.00
581.50	19.84	(N/A)	0.00
582.00	30.55	(N/A)	0.00
582.50	42.69	(N/A)	0.00
583.00	56.12	(N/A)	0.00
583.50	70.72	(N/A)	0.00
584.00	86.40	(N/A)	0.00
584.50	103.10	(N/A)	0.00
585.00	120.75	(N/A)	0.00
585.50	139.31	(N/A)	0.00
586.00	158.73	(N/A)	0.00
586.50	178.98	(N/A)	0.00
587.00	200.02	(N/A)	0.00
587.50	221.83	(N/A)	0.00
588.00	244.38	(N/A)	0.00
588.50	267.64	(N/A)	0.00
589.00	291.60	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
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Weir - 1
Weir - 1
Weir - 1

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Requested Pond Water Surface Elevations	
Minimum (Headwater)	580.00 ft
Increment (Headwater)	0.50 ft
Maximum (Headwater)	589.00 ft

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir Tailwater Settings	Weir - 1 Tailwater	Forward	TW	580.00 (N/A)	589.00 (N/A)

Subsection: Outlet Input Data
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Structure ID: Weir - 1	
Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	580.00 ft
Weir Length	4.00 ft
Weir Coefficient	2.70 (ft ^{0.5})/s
Structure ID: TW	
Structure Type: TW Setup, DS Channel	
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft ³ /s
Flow Tolerance (Maximum)	10.000 ft ³ /s

Subsection: Individual Outlet Curves
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

RATING TABLE FOR ONE OUTLET TYPE
Structure ID = ()

Upstream ID =
Downstream ID =

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
Contributing Structures			

Subsection: Composite Rating Curve
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft ³ /s)	Tailwater Elevation (ft)	Convergence Error (ft)
580.00	0.00	(N/A)	0.00
580.50	3.82	(N/A)	0.00
581.00	10.80	(N/A)	0.00
581.50	19.84	(N/A)	0.00
582.00	30.55	(N/A)	0.00
582.50	42.69	(N/A)	0.00
583.00	56.12	(N/A)	0.00
583.50	70.72	(N/A)	0.00
584.00	86.40	(N/A)	0.00
584.50	103.10	(N/A)	0.00
585.00	120.75	(N/A)	0.00
585.50	139.31	(N/A)	0.00
586.00	158.73	(N/A)	0.00
586.50	178.98	(N/A)	0.00
587.00	200.02	(N/A)	0.00
587.50	221.83	(N/A)	0.00
588.00	244.38	(N/A)	0.00
588.50	267.64	(N/A)	0.00
589.00	291.60	(N/A)	0.00

Contributing Structures

Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
Weir - 1
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Weir - 1
Weir - 1
Weir - 1

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND A
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	582.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
582.00	0.00	0.000	2.000	0.00	0.00	0.00
582.50	5.25	43,560.000	2.000	0.00	5.25	489.25
583.00	14.85	87,120.000	2.000	0.00	14.85	982.85
583.50	27.28	130,680.000	2.000	0.00	27.28	1,479.28
584.00	42.00	174,240.000	2.000	0.00	42.00	1,978.00
584.50	58.70	217,800.000	2.000	0.00	58.70	2,478.70
585.00	77.16	261,360.000	2.000	0.00	77.16	2,981.16
585.50	97.24	304,920.000	2.000	0.00	97.24	3,485.24
586.00	118.80	348,480.000	2.000	0.00	118.80	3,990.80
586.50	141.76	392,040.000	2.000	0.00	141.76	4,497.76
587.00	166.03	435,600.000	2.000	0.00	166.03	5,006.03
587.50	191.54	479,160.000	2.000	0.00	191.54	5,515.54
588.00	218.25	522,720.000	2.000	0.00	218.25	6,026.25
588.50	246.09	566,280.000	2.000	0.00	246.09	6,538.09
589.00	275.03	609,840.000	2.000	0.00	275.03	7,051.03
589.50	305.01	653,400.000	2.000	0.00	305.01	7,565.01
590.00	336.02	696,960.000	2.000	0.00	336.02	8,080.02

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND A
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	582.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
582.00	0.00	0.000	2.000	0.00	0.00	0.00
582.50	5.25	43,560.000	2.000	0.00	5.25	489.25
583.00	14.85	87,120.000	2.000	0.00	14.85	982.85
583.50	27.28	130,680.000	2.000	0.00	27.28	1,479.28
584.00	42.00	174,240.000	2.000	0.00	42.00	1,978.00
584.50	58.70	217,800.000	2.000	0.00	58.70	2,478.70
585.00	77.16	261,360.000	2.000	0.00	77.16	2,981.16
585.50	97.24	304,920.000	2.000	0.00	97.24	3,485.24
586.00	118.80	348,480.000	2.000	0.00	118.80	3,990.80
586.50	141.76	392,040.000	2.000	0.00	141.76	4,497.76
587.00	166.03	435,600.000	2.000	0.00	166.03	5,006.03
587.50	191.54	479,160.000	2.000	0.00	191.54	5,515.54
588.00	218.25	522,720.000	2.000	0.00	218.25	6,026.25
588.50	246.09	566,280.000	2.000	0.00	246.09	6,538.09
589.00	275.03	609,840.000	2.000	0.00	275.03	7,051.03
589.50	305.01	653,400.000	2.000	0.00	305.01	7,565.01
590.00	336.02	696,960.000	2.000	0.00	336.02	8,080.02

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND A
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	582.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
582.00	0.00	0.000	2.000	0.00	0.00	0.00
582.50	5.25	43,560.000	2.000	0.00	5.25	489.25
583.00	14.85	87,120.000	2.000	0.00	14.85	982.85
583.50	27.28	130,680.000	2.000	0.00	27.28	1,479.28
584.00	42.00	174,240.000	2.000	0.00	42.00	1,978.00
584.50	58.70	217,800.000	2.000	0.00	58.70	2,478.70
585.00	77.16	261,360.000	2.000	0.00	77.16	2,981.16
585.50	97.24	304,920.000	2.000	0.00	97.24	3,485.24
586.00	118.80	348,480.000	2.000	0.00	118.80	3,990.80
586.50	141.76	392,040.000	2.000	0.00	141.76	4,497.76
587.00	166.03	435,600.000	2.000	0.00	166.03	5,006.03
587.50	191.54	479,160.000	2.000	0.00	191.54	5,515.54
588.00	218.25	522,720.000	2.000	0.00	218.25	6,026.25
588.50	246.09	566,280.000	2.000	0.00	246.09	6,538.09
589.00	275.03	609,840.000	2.000	0.00	275.03	7,051.03
589.50	305.01	653,400.000	2.000	0.00	305.01	7,565.01
590.00	336.02	696,960.000	2.000	0.00	336.02	8,080.02

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND A
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	582.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
582.00	0.00	0.000	2.000	0.00	0.00	0.00
582.50	5.25	43,560.000	2.000	0.00	5.25	489.25
583.00	14.85	87,120.000	2.000	0.00	14.85	982.85
583.50	27.28	130,680.000	2.000	0.00	27.28	1,479.28
584.00	42.00	174,240.000	2.000	0.00	42.00	1,978.00
584.50	58.70	217,800.000	2.000	0.00	58.70	2,478.70
585.00	77.16	261,360.000	2.000	0.00	77.16	2,981.16
585.50	97.24	304,920.000	2.000	0.00	97.24	3,485.24
586.00	118.80	348,480.000	2.000	0.00	118.80	3,990.80
586.50	141.76	392,040.000	2.000	0.00	141.76	4,497.76
587.00	166.03	435,600.000	2.000	0.00	166.03	5,006.03
587.50	191.54	479,160.000	2.000	0.00	191.54	5,515.54
588.00	218.25	522,720.000	2.000	0.00	218.25	6,026.25
588.50	246.09	566,280.000	2.000	0.00	246.09	6,538.09
589.00	275.03	609,840.000	2.000	0.00	275.03	7,051.03
589.50	305.01	653,400.000	2.000	0.00	305.01	7,565.01
590.00	336.02	696,960.000	2.000	0.00	336.02	8,080.02

Subsection: Level Pool Pond Routing Summary
Label: POND A (IN)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		582.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		79.66 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		45.09 ft³/s	Time to Peak (Flow, Outlet)
			756.000 min
			792.000 min
Peak Conditions			
Elevation (Water Surface, Peak)		584.09 ft	
Volume (Peak)		182,307.730 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		587,792.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		567,291.000 ft³	
Volume (Retained)		20,062.000 ft³	
Volume (Unrouted)		-440.000 ft³	
Error (Mass Balance)		0.1 %	

Subsection: Level Pool Pond Routing Summary
Label: POND A (IN)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		582.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		214.84 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		147.07 ft³/s	Time to Peak (Flow, Outlet)
			756.000 min
			780.000 min
Elevation (Water Surface, Peak)		586.61 ft	
Volume (Peak)		401,569.453 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		1,460,300.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		1,419,893.000 ft³	
Volume (Retained)		39,540.000 ft³	
Volume (Unrouted)		-867.000 ft³	
Error (Mass Balance)		0.1 %	

Subsection: Level Pool Pond Routing Summary
Label: POND A (IN)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		582.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		278.46 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		201.98 ft³/s	Time to Peak (Flow, Outlet)
			756.000 min
			777.000 min
Elevation (Water Surface, Peak)		587.70 ft	
Volume (Peak)		496,187.067 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		2,005,601.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		1,958,912.000 ft³	
Volume (Retained)		45,641.000 ft³	
Volume (Unrouted)		-1,048.000 ft³	
Error (Mass Balance)		0.1 %	

Subsection: Level Pool Pond Routing Summary
Label: POND A (IN)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		582.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		384.90 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		297.87 ft³/s	Time to Peak (Flow, Outlet)
			756.000 min
			774.000 min
Elevation (Water Surface, Peak)		589.38 ft	
Volume (Peak)		643,030.979 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		3,044,204.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		2,987,867.000 ft³	
Volume (Retained)		54,913.000 ft³	
Volume (Unrouted)		-1,424.000 ft³	
Error (Mass Balance)		0.0 %	

Subsection: Pond Routed Hydrograph (total out)
 Label: POND A (OUT)
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Peak Discharge	45.09 ft ³ /s
Time to Peak	792.000 min
Hydrograph Volume	567,290.432 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
456.000	0.00	0.00	0.00	0.00	0.01
471.000	0.01	0.02	0.02	0.03	0.03
486.000	0.04	0.05	0.05	0.06	0.06
501.000	0.07	0.07	0.07	0.08	0.08
516.000	0.08	0.09	0.10	0.11	0.12
531.000	0.14	0.16	0.18	0.21	0.23
546.000	0.26	0.28	0.30	0.32	0.34
561.000	0.36	0.37	0.39	0.40	0.42
576.000	0.44	0.46	0.49	0.53	0.57
591.000	0.61	0.65	0.69	0.73	0.77
606.000	0.81	0.85	0.90	0.94	1.00
621.000	1.05	1.11	1.17	1.22	1.28
636.000	1.33	1.39	1.45	1.50	1.56
651.000	1.63	1.69	1.76	1.83	1.91
666.000	1.99	2.08	2.17	2.26	2.37
681.000	2.47	2.58	2.70	2.81	2.93
696.000	3.06	3.20	3.35	3.52	3.70
711.000	3.90	4.14	4.42	4.76	5.17
726.000	6.02	7.10	8.38	9.86	11.54
741.000	13.42	15.63	18.40	21.23	24.05
756.000	26.80	29.82	32.70	35.31	37.58
771.000	39.52	41.13	42.46	43.55	44.33
786.000	44.83	45.07	45.09	44.93	44.59
801.000	44.11	43.51	42.81	42.02	41.27
816.000	40.47	39.64	38.77	37.87	36.93
831.000	35.98	35.02	34.07	33.14	32.23
846.000	31.35	30.51	29.68	28.87	28.07
861.000	27.29	26.63	25.98	25.35	24.75
876.000	24.17	23.62	23.10	22.61	22.13
891.000	21.67	21.21	20.75	20.28	19.80
906.000	19.31	18.82	18.33	17.84	17.37
921.000	16.94	16.54	16.19	15.88	15.60
936.000	15.35	15.11	14.87	14.68	14.48
951.000	14.26	14.02	13.77	13.49	13.21
966.000	12.92	12.64	12.38	12.15	11.95
981.000	11.80	11.68	11.59	11.52	11.46
996.000	11.39	11.32	11.24	11.13	11.01
1,011.000	10.86	10.69	10.49	10.27	10.03
1,026.000	9.78	9.52	9.26	9.00	8.75

Subsection: Pond Routed Hydrograph (total out)
Label: POND A (OUT)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,041.000	8.51	8.30	8.11	7.97	7.86
1,056.000	7.80	7.77	7.78	7.81	7.85
1,071.000	7.89	7.92	7.95	7.95	7.93
1,086.000	7.89	7.82	7.73	7.61	7.48
1,101.000	7.32	7.16	6.99	6.81	6.62
1,116.000	6.43	6.25	6.06	5.87	5.68
1,131.000	5.49	5.31	5.19	5.09	4.99
1,146.000	4.90	4.81	4.74	4.67	4.63
1,161.000	4.61	4.60	4.63	4.67	4.73
1,176.000	4.81	4.89	4.96	5.04	5.11
1,191.000	5.17	5.21	5.24	5.26	5.26
1,206.000	5.24	5.21	5.17	5.12	5.07
1,221.000	5.01	4.94	4.87	4.80	4.73
1,236.000	4.65	4.57	4.49	4.41	4.33
1,251.000	4.25	4.17	4.09	4.01	3.93
1,266.000	3.86	3.78	3.70	3.63	3.55
1,281.000	3.49	3.43	3.38	3.35	3.33
1,296.000	3.34	3.38	3.44	3.52	3.61
1,311.000	3.72	3.82	3.93	4.03	4.11
1,326.000	4.19	4.25	4.30	4.32	4.34
1,341.000	4.33	4.32	4.29	4.26	4.22
1,356.000	4.17	4.12	4.07	4.01	3.95
1,371.000	3.89	3.83	3.76	3.70	3.63
1,386.000	3.57	3.50	3.43	3.37	3.30
1,401.000	3.24	3.17	3.11	3.05	2.98
1,416.000	2.92	2.86	2.80	2.75	2.69
1,431.000	2.63	2.58	2.52	2.47	(N/A)

Subsection: Pond Routed Hydrograph (total out)
Label: POND A (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Peak Discharge	147.07 ft ³ /s
Time to Peak	780.000 min
Hydrograph Volume	1,419,892.893 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
336.000	0.00	0.00	0.00	0.00	0.00
351.000	0.01	0.01	0.01	0.01	0.02
366.000	0.02	0.02	0.03	0.03	0.04
381.000	0.05	0.05	0.06	0.07	0.08
396.000	0.09	0.10	0.11	0.12	0.14
411.000	0.15	0.16	0.18	0.20	0.21
426.000	0.23	0.25	0.27	0.29	0.31
441.000	0.33	0.35	0.38	0.40	0.42
456.000	0.45	0.48	0.50	0.53	0.56
471.000	0.59	0.62	0.65	0.68	0.71
486.000	0.75	0.78	0.82	0.85	0.89
501.000	0.93	0.97	1.01	1.05	1.09
516.000	1.13	1.17	1.22	1.26	1.31
531.000	1.36	1.41	1.46	1.51	1.56
546.000	1.61	1.67	1.72	1.78	1.84
561.000	1.91	1.97	2.05	2.12	2.20
576.000	2.28	2.36	2.45	2.55	2.64
591.000	2.74	2.85	2.96	3.07	3.18
606.000	3.30	3.43	3.55	3.68	3.82
621.000	3.95	4.09	4.24	4.39	4.54
636.000	4.70	4.87	5.04	5.22	5.53
651.000	5.88	6.24	6.62	7.01	7.41
666.000	7.84	8.28	8.75	9.24	9.75
681.000	10.30	10.87	11.48	12.12	12.80
696.000	13.52	14.29	15.19	16.33	17.55
711.000	18.87	20.34	22.01	23.93	26.23
726.000	29.26	33.10	37.67	43.10	49.96
741.000	57.59	66.49	75.87	86.01	95.91
756.000	105.88	115.04	123.43	130.77	136.69
771.000	141.22	144.50	146.40	147.07	146.66
786.000	145.36	143.34	140.77	137.85	134.57
801.000	131.01	127.25	123.33	119.32	115.46
816.000	111.60	107.75	103.91	100.11	96.43
831.000	92.99	89.62	86.31	83.08	79.93
846.000	76.89	74.15	71.48	68.89	66.37
861.000	63.95	61.60	59.34	57.32	55.42
876.000	53.58	51.82	50.11	48.48	46.90
891.000	45.39	43.93	42.54	41.29	40.15
906.000	39.04	37.98	36.95	35.96	35.00

Subsection: Pond Routed Hydrograph (total out)
Label: POND A (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
921.000	34.07	33.16	32.28	31.42	30.58
936.000	29.76	28.97	28.19	27.44	26.79
951.000	26.18	25.58	25.00	24.43	23.88
966.000	23.34	22.82	22.31	21.82	21.34
981.000	20.88	20.43	20.00	19.58	19.17
996.000	18.78	18.40	18.03	17.67	17.33
1,011.000	17.00	16.67	16.36	16.06	15.77
1,026.000	15.49	15.22	14.95	14.73	14.54
1,041.000	14.35	14.16	13.98	13.81	13.63
1,056.000	13.46	13.30	13.13	12.98	12.82
1,071.000	12.67	12.52	12.37	12.23	12.09
1,086.000	11.95	11.82	11.68	11.55	11.42
1,101.000	11.30	11.17	11.04	10.92	10.80
1,116.000	10.67	10.55	10.43	10.31	10.19
1,131.000	10.07	9.95	9.83	9.72	9.61
1,146.000	9.50	9.39	9.28	9.18	9.07
1,161.000	8.97	8.87	8.78	8.68	8.59
1,176.000	8.50	8.41	8.32	8.24	8.15
1,191.000	8.07	7.99	7.91	7.84	7.76
1,206.000	7.69	7.62	7.55	7.48	7.41
1,221.000	7.35	7.28	7.22	7.16	7.09
1,236.000	7.04	6.98	6.92	6.86	6.81
1,251.000	6.75	6.70	6.65	6.60	6.55
1,266.000	6.50	6.45	6.40	6.36	6.31
1,281.000	6.27	6.22	6.18	6.14	6.10
1,296.000	6.06	6.02	5.98	5.94	5.90
1,311.000	5.86	5.82	5.79	5.75	5.72
1,326.000	5.68	5.65	5.61	5.58	5.55
1,341.000	5.52	5.49	5.45	5.42	5.39
1,356.000	5.36	5.33	5.30	5.28	5.25
1,371.000	5.23	5.22	5.20	5.19	5.17
1,386.000	5.15	5.14	5.12	5.11	5.09
1,401.000	5.08	5.06	5.04	5.03	5.01
1,416.000	5.00	4.98	4.97	4.95	4.93
1,431.000	4.92	4.90	4.89	4.87	(N/A)

Subsection: Pond Routed Hydrograph (total out)
 Label: POND A (OUT)
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

Peak Discharge	201.98 ft ³ /s
Time to Peak	777.000 min
Hydrograph Volume	1,958,911.530 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.00	0.00	0.00
297.000	0.01	0.01	0.01	0.01	0.02
312.000	0.02	0.03	0.03	0.04	0.04
327.000	0.05	0.06	0.07	0.08	0.09
342.000	0.10	0.11	0.12	0.13	0.15
357.000	0.16	0.18	0.19	0.21	0.23
372.000	0.25	0.26	0.28	0.31	0.33
387.000	0.35	0.37	0.40	0.43	0.46
402.000	0.48	0.51	0.55	0.58	0.61
417.000	0.65	0.68	0.72	0.76	0.80
432.000	0.84	0.88	0.93	0.97	1.01
447.000	1.06	1.11	1.15	1.20	1.25
462.000	1.31	1.36	1.41	1.46	1.52
477.000	1.58	1.63	1.69	1.75	1.81
492.000	1.88	1.94	2.00	2.07	2.13
507.000	2.20	2.27	2.34	2.41	2.49
522.000	2.56	2.64	2.71	2.79	2.87
537.000	2.95	3.04	3.12	3.21	3.30
552.000	3.39	3.49	3.59	3.69	3.80
567.000	3.92	4.04	4.17	4.30	4.45
582.000	4.60	4.75	4.91	5.08	5.26
597.000	5.59	5.92	6.26	6.60	6.94
612.000	7.29	7.65	8.00	8.37	8.73
627.000	9.11	9.49	9.87	10.27	10.67
642.000	11.09	11.52	11.97	12.44	12.93
657.000	13.45	14.00	14.57	15.28	16.10
672.000	16.96	17.85	18.79	19.78	20.82
687.000	21.90	23.05	24.25	25.52	26.85
702.000	28.45	30.21	32.08	34.09	36.30
717.000	38.78	41.62	45.37	49.84	55.15
732.000	61.74	69.79	79.07	90.07	102.08
747.000	115.03	128.53	141.66	154.62	166.25
762.000	176.83	185.60	192.44	197.55	200.69
777.000	201.98	201.69	200.05	197.29	193.67
792.000	189.45	184.81	179.79	174.49	168.99
807.000	163.48	158.05	152.58	147.13	141.73
822.000	136.68	131.70	126.80	122.00	117.40
837.000	113.08	108.87	104.76	100.76	96.90
852.000	93.39	89.98	86.68	83.48	80.40

Subsection: Pond Routed Hydrograph (total out)
 Label: POND A (OUT)
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
867.000	77.44	74.79	72.26	69.82	67.48
882.000	65.23	63.07	60.99	59.01	57.25
897.000	55.58	53.98	52.44	50.95	49.53
912.000	48.15	46.83	45.55	44.31	43.10
927.000	41.93	40.92	39.92	38.93	37.96
942.000	37.01	36.08	35.17	34.27	33.40
957.000	32.56	31.73	30.94	30.16	29.42
972.000	28.70	28.00	27.33	26.77	26.24
987.000	25.72	25.22	24.73	24.25	23.79
1,002.000	23.35	22.91	22.49	22.09	21.69
1,017.000	21.31	20.94	20.58	20.24	19.90
1,032.000	19.57	19.26	18.95	18.65	18.37
1,047.000	18.09	17.82	17.55	17.30	17.05
1,062.000	16.81	16.58	16.35	16.13	15.92
1,077.000	15.71	15.51	15.32	15.13	14.94
1,092.000	14.78	14.64	14.50	14.36	14.22
1,107.000	14.08	13.93	13.79	13.64	13.50
1,122.000	13.35	13.20	13.06	12.91	12.76
1,137.000	12.62	12.47	12.33	12.19	12.05
1,152.000	11.92	11.78	11.65	11.52	11.39
1,167.000	11.27	11.15	11.03	10.91	10.79
1,182.000	10.68	10.57	10.46	10.36	10.25
1,197.000	10.15	10.05	9.95	9.86	9.76
1,212.000	9.67	9.58	9.50	9.41	9.33
1,227.000	9.24	9.16	9.08	9.01	8.93
1,242.000	8.86	8.78	8.71	8.64	8.57
1,257.000	8.51	8.44	8.37	8.31	8.25
1,272.000	8.19	8.13	8.07	8.01	7.95
1,287.000	7.90	7.84	7.79	7.73	7.68
1,302.000	7.63	7.58	7.53	7.48	7.43
1,317.000	7.39	7.34	7.30	7.25	7.21
1,332.000	7.16	7.12	7.08	7.04	6.99
1,347.000	6.95	6.91	6.87	6.84	6.80
1,362.000	6.76	6.72	6.69	6.65	6.62
1,377.000	6.58	6.55	6.51	6.48	6.45
1,392.000	6.41	6.38	6.35	6.32	6.29
1,407.000	6.26	6.22	6.19	6.17	6.14
1,422.000	6.11	6.08	6.05	6.02	5.99
1,437.000	5.97	5.94	(N/A)	(N/A)	(N/A)

Subsection: Pond Routed Hydrograph (total out)
Label: POND A (OUT)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Peak Discharge	297.87 ft ³ /s
Time to Peak	774.000 min
Hydrograph Volume	2,987,867.198 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
222.000	0.00	0.00	0.00	0.00	0.00
237.000	0.01	0.01	0.01	0.02	0.02
252.000	0.03	0.03	0.04	0.05	0.05
267.000	0.06	0.07	0.09	0.10	0.11
282.000	0.13	0.14	0.16	0.18	0.20
297.000	0.22	0.24	0.26	0.28	0.30
312.000	0.33	0.35	0.38	0.41	0.43
327.000	0.46	0.49	0.53	0.56	0.59
342.000	0.62	0.66	0.69	0.73	0.77
357.000	0.81	0.84	0.88	0.93	0.97
372.000	1.01	1.06	1.10	1.15	1.20
387.000	1.25	1.31	1.36	1.42	1.49
402.000	1.55	1.62	1.69	1.76	1.83
417.000	1.91	1.98	2.06	2.15	2.23
432.000	2.31	2.40	2.49	2.58	2.67
447.000	2.76	2.85	2.95	3.05	3.15
462.000	3.25	3.35	3.45	3.56	3.66
477.000	3.77	3.88	3.99	4.10	4.22
492.000	4.33	4.45	4.57	4.69	4.81
507.000	4.93	5.06	5.19	5.37	5.61
522.000	5.84	6.08	6.31	6.55	6.78
537.000	7.02	7.26	7.49	7.74	7.98
552.000	8.23	8.50	8.77	9.06	9.36
567.000	9.69	10.03	10.40	10.79	11.19
582.000	11.62	12.07	12.54	13.03	13.53
597.000	14.04	14.57	15.18	15.89	16.59
612.000	17.31	18.03	18.75	19.48	20.22
627.000	20.96	21.71	22.48	23.26	24.06
642.000	24.88	25.73	26.62	27.60	28.75
657.000	29.96	31.23	32.57	33.98	35.46
672.000	37.01	38.64	40.34	42.15	44.28
687.000	46.48	48.78	51.17	53.65	56.22
702.000	58.93	62.00	65.21	68.58	72.19
717.000	76.18	80.97	86.67	93.33	101.51
732.000	111.55	123.38	137.30	153.29	170.65
747.000	189.10	208.02	226.26	243.41	258.91
762.000	272.03	282.74	290.56	295.58	297.87
777.000	297.67	295.38	291.36	285.96	279.56
792.000	272.46	264.98	257.14	249.04	241.00

Subsection: Pond Routed Hydrograph (total out)
 Label: POND A (OUT)
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
807.000	232.97	224.95	217.02	209.43	201.96
822.000	194.61	187.58	180.81	174.19	167.71
837.000	161.60	155.71	149.95	144.34	139.03
852.000	134.00	129.12	124.38	119.81	115.59
867.000	111.59	107.73	104.02	100.47	97.07
882.000	94.03	91.10	88.29	85.59	83.01
897.000	80.53	78.16	75.98	73.98	72.04
912.000	70.17	68.36	66.60	64.88	63.19
927.000	61.53	59.89	58.31	56.86	55.42
942.000	53.99	52.58	51.19	49.83	48.49
957.000	47.19	45.92	44.70	43.51	42.36
972.000	41.34	40.39	39.47	38.58	37.71
987.000	36.88	36.07	35.29	34.54	33.82
1,002.000	33.12	32.45	31.80	31.17	30.57
1,017.000	29.99	29.43	28.89	28.37	27.87
1,032.000	27.39	26.98	26.59	26.22	25.86
1,047.000	25.50	25.16	24.82	24.50	24.18
1,062.000	23.87	23.57	23.28	22.99	22.71
1,077.000	22.44	22.18	21.92	21.67	21.42
1,092.000	21.18	20.94	20.69	20.45	20.21
1,107.000	19.97	19.72	19.47	19.22	18.96
1,122.000	18.70	18.45	18.19	17.93	17.67
1,137.000	17.42	17.17	16.92	16.68	16.45
1,152.000	16.21	15.99	15.77	15.55	15.34
1,167.000	15.14	14.94	14.77	14.62	14.47
1,182.000	14.33	14.19	14.05	13.92	13.79
1,197.000	13.66	13.53	13.40	13.28	13.16
1,212.000	13.04	12.93	12.81	12.70	12.59
1,227.000	12.48	12.38	12.28	12.17	12.07
1,242.000	11.98	11.88	11.79	11.69	11.60
1,257.000	11.51	11.43	11.34	11.26	11.17
1,272.000	11.09	11.01	10.94	10.86	10.78
1,287.000	10.71	10.64	10.56	10.49	10.42
1,302.000	10.35	10.29	10.22	10.16	10.09
1,317.000	10.03	9.97	9.90	9.84	9.78
1,332.000	9.73	9.67	9.61	9.56	9.50
1,347.000	9.45	9.39	9.34	9.29	9.24
1,362.000	9.18	9.13	9.08	9.04	8.99
1,377.000	8.94	8.89	8.85	8.80	8.76
1,392.000	8.71	8.67	8.62	8.58	8.54
1,407.000	8.50	8.45	8.41	8.37	8.33
1,422.000	8.29	8.25	8.22	8.18	8.14
1,437.000	8.10	8.07	(N/A)	(N/A)	(N/A)

Subsection: Pond Inflow Summary
Label: POND A (IN)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POND A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA C

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	PROP AREA C	587,792.050	756.000	79.66
Flow (In)	POND A	587,792.050	756.000	79.66

Subsection: Pond Inflow Summary
Label: POND A (IN)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POND A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA C

Node Inflows

Inflow Type	Element	Volume (ft³)	Time to Peak (min)	Flow (Peak) (ft³/s)
Flow (From)	PROP AREA C	1,460,300.375	756.000	214.84
Flow (In)	POND A	1,460,300.375	756.000	214.84

Subsection: Pond Inflow Summary
Label: POND A (IN)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POND A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA C	2,005,600.804	756.000	278.46
Flow (In)	POND A	2,005,600.804	756.000	278.46

Subsection: Pond Inflow Summary
Label: POND A (IN)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POND A'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	PROP AREA C

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	PROP AREA C	3,044,204.099	756.000	384.90
Flow (In)	POND A	3,044,204.099	756.000	384.90

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Peak Discharge	45.09 ft ³ /s
Time to Peak	792.000 min
Hydrograph Volume	567,290.432 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
456.000	0.00	0.00	0.00	0.00	0.01
471.000	0.01	0.02	0.02	0.03	0.03
486.000	0.04	0.05	0.05	0.06	0.06
501.000	0.07	0.07	0.07	0.08	0.08
516.000	0.08	0.09	0.10	0.11	0.12
531.000	0.14	0.16	0.18	0.21	0.23
546.000	0.26	0.28	0.30	0.32	0.34
561.000	0.36	0.37	0.39	0.40	0.42
576.000	0.44	0.46	0.49	0.53	0.57
591.000	0.61	0.65	0.69	0.73	0.77
606.000	0.81	0.85	0.90	0.94	1.00
621.000	1.05	1.11	1.17	1.22	1.28
636.000	1.33	1.39	1.45	1.50	1.56
651.000	1.63	1.69	1.76	1.83	1.91
666.000	1.99	2.08	2.17	2.26	2.37
681.000	2.47	2.58	2.70	2.81	2.93
696.000	3.06	3.20	3.35	3.52	3.70
711.000	3.90	4.14	4.42	4.76	5.17
726.000	6.02	7.10	8.38	9.86	11.54
741.000	13.42	15.63	18.40	21.23	24.05
756.000	26.80	29.82	32.70	35.31	37.58
771.000	39.52	41.13	42.46	43.55	44.33
786.000	44.83	45.07	45.09	44.93	44.59
801.000	44.11	43.51	42.81	42.02	41.27
816.000	40.47	39.64	38.77	37.87	36.93
831.000	35.98	35.02	34.07	33.14	32.23
846.000	31.35	30.51	29.68	28.87	28.07
861.000	27.29	26.63	25.98	25.35	24.75
876.000	24.17	23.62	23.10	22.61	22.13
891.000	21.67	21.21	20.75	20.28	19.80
906.000	19.31	18.82	18.33	17.84	17.37
921.000	16.94	16.54	16.19	15.88	15.60
936.000	15.35	15.11	14.87	14.68	14.48
951.000	14.26	14.02	13.77	13.49	13.21
966.000	12.92	12.64	12.38	12.15	11.95
981.000	11.80	11.68	11.59	11.52	11.46
996.000	11.39	11.32	11.24	11.13	11.01
1,011.000	10.86	10.69	10.49	10.27	10.03
1,026.000	9.78	9.52	9.26	9.00	8.75

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,041.000	8.51	8.30	8.11	7.97	7.86
1,056.000	7.80	7.77	7.78	7.81	7.85
1,071.000	7.89	7.92	7.95	7.95	7.93
1,086.000	7.89	7.82	7.73	7.61	7.48
1,101.000	7.32	7.16	6.99	6.81	6.62
1,116.000	6.43	6.25	6.06	5.87	5.68
1,131.000	5.49	5.31	5.19	5.09	4.99
1,146.000	4.90	4.81	4.74	4.67	4.63
1,161.000	4.61	4.60	4.63	4.67	4.73
1,176.000	4.81	4.89	4.96	5.04	5.11
1,191.000	5.17	5.21	5.24	5.26	5.26
1,206.000	5.24	5.21	5.17	5.12	5.07
1,221.000	5.01	4.94	4.87	4.80	4.73
1,236.000	4.65	4.57	4.49	4.41	4.33
1,251.000	4.25	4.17	4.09	4.01	3.93
1,266.000	3.86	3.78	3.70	3.63	3.55
1,281.000	3.49	3.43	3.38	3.35	3.33
1,296.000	3.34	3.38	3.44	3.52	3.61
1,311.000	3.72	3.82	3.93	4.03	4.11
1,326.000	4.19	4.25	4.30	4.32	4.34
1,341.000	4.33	4.32	4.29	4.26	4.22
1,356.000	4.17	4.12	4.07	4.01	3.95
1,371.000	3.89	3.83	3.76	3.70	3.63
1,386.000	3.57	3.50	3.43	3.37	3.30
1,401.000	3.24	3.17	3.11	3.05	2.98
1,416.000	2.92	2.86	2.80	2.75	2.69
1,431.000	2.63	2.58	2.52	2.47	(N/A)

Subsection: Diverted Hydrograph
 Label: POND A STRUCTURE
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Peak Discharge	147.07 ft ³ /s
Time to Peak	780.000 min
Hydrograph Volume	1,419,892.893 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
336.000	0.00	0.00	0.00	0.00	0.00
351.000	0.01	0.01	0.01	0.01	0.02
366.000	0.02	0.02	0.03	0.03	0.04
381.000	0.05	0.05	0.06	0.07	0.08
396.000	0.09	0.10	0.11	0.12	0.14
411.000	0.15	0.16	0.18	0.20	0.21
426.000	0.23	0.25	0.27	0.29	0.31
441.000	0.33	0.35	0.38	0.40	0.42
456.000	0.45	0.48	0.50	0.53	0.56
471.000	0.59	0.62	0.65	0.68	0.71
486.000	0.75	0.78	0.82	0.85	0.89
501.000	0.93	0.97	1.01	1.05	1.09
516.000	1.13	1.17	1.22	1.26	1.31
531.000	1.36	1.41	1.46	1.51	1.56
546.000	1.61	1.67	1.72	1.78	1.84
561.000	1.91	1.97	2.05	2.12	2.20
576.000	2.28	2.36	2.45	2.55	2.64
591.000	2.74	2.85	2.96	3.07	3.18
606.000	3.30	3.43	3.55	3.68	3.82
621.000	3.95	4.09	4.24	4.39	4.54
636.000	4.70	4.87	5.04	5.22	5.53
651.000	5.88	6.24	6.62	7.01	7.41
666.000	7.84	8.28	8.75	9.24	9.75
681.000	10.30	10.87	11.48	12.12	12.80
696.000	13.52	14.29	15.19	16.33	17.55
711.000	18.87	20.34	22.01	23.93	26.23
726.000	29.26	33.10	37.67	43.10	49.96
741.000	57.59	66.49	75.87	86.01	95.91
756.000	105.88	115.04	123.43	130.77	136.69
771.000	141.22	144.50	146.40	147.07	146.66
786.000	145.36	143.34	140.77	137.85	134.57
801.000	131.01	127.25	123.33	119.32	115.46
816.000	111.60	107.75	103.91	100.11	96.43
831.000	92.99	89.62	86.31	83.08	79.93
846.000	76.89	74.15	71.48	68.89	66.37
861.000	63.95	61.60	59.34	57.32	55.42
876.000	53.58	51.82	50.11	48.48	46.90
891.000	45.39	43.93	42.54	41.29	40.15
906.000	39.04	37.98	36.95	35.96	35.00

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
921.000	34.07	33.16	32.28	31.42	30.58
936.000	29.76	28.97	28.19	27.44	26.79
951.000	26.18	25.58	25.00	24.43	23.88
966.000	23.34	22.82	22.31	21.82	21.34
981.000	20.88	20.43	20.00	19.58	19.17
996.000	18.78	18.40	18.03	17.67	17.33
1,011.000	17.00	16.67	16.36	16.06	15.77
1,026.000	15.49	15.22	14.95	14.73	14.54
1,041.000	14.35	14.16	13.98	13.81	13.63
1,056.000	13.46	13.30	13.13	12.98	12.82
1,071.000	12.67	12.52	12.37	12.23	12.09
1,086.000	11.95	11.82	11.68	11.55	11.42
1,101.000	11.30	11.17	11.04	10.92	10.80
1,116.000	10.67	10.55	10.43	10.31	10.19
1,131.000	10.07	9.95	9.83	9.72	9.61
1,146.000	9.50	9.39	9.28	9.18	9.07
1,161.000	8.97	8.87	8.78	8.68	8.59
1,176.000	8.50	8.41	8.32	8.24	8.15
1,191.000	8.07	7.99	7.91	7.84	7.76
1,206.000	7.69	7.62	7.55	7.48	7.41
1,221.000	7.35	7.28	7.22	7.16	7.09
1,236.000	7.04	6.98	6.92	6.86	6.81
1,251.000	6.75	6.70	6.65	6.60	6.55
1,266.000	6.50	6.45	6.40	6.36	6.31
1,281.000	6.27	6.22	6.18	6.14	6.10
1,296.000	6.06	6.02	5.98	5.94	5.90
1,311.000	5.86	5.82	5.79	5.75	5.72
1,326.000	5.68	5.65	5.61	5.58	5.55
1,341.000	5.52	5.49	5.45	5.42	5.39
1,356.000	5.36	5.33	5.30	5.28	5.25
1,371.000	5.23	5.22	5.20	5.19	5.17
1,386.000	5.15	5.14	5.12	5.11	5.09
1,401.000	5.08	5.06	5.04	5.03	5.01
1,416.000	5.00	4.98	4.97	4.95	4.93
1,431.000	4.92	4.90	4.89	4.87	(N/A)

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Peak Discharge	201.98 ft ³ /s
Time to Peak	777.000 min
Hydrograph Volume	1,958,911.530 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
282.000	0.00	0.00	0.00	0.00	0.00
297.000	0.01	0.01	0.01	0.01	0.02
312.000	0.02	0.03	0.03	0.04	0.04
327.000	0.05	0.06	0.07	0.08	0.09
342.000	0.10	0.11	0.12	0.13	0.15
357.000	0.16	0.18	0.19	0.21	0.23
372.000	0.25	0.26	0.28	0.31	0.33
387.000	0.35	0.37	0.40	0.43	0.46
402.000	0.48	0.51	0.55	0.58	0.61
417.000	0.65	0.68	0.72	0.76	0.80
432.000	0.84	0.88	0.93	0.97	1.01
447.000	1.06	1.11	1.15	1.20	1.25
462.000	1.31	1.36	1.41	1.46	1.52
477.000	1.58	1.63	1.69	1.75	1.81
492.000	1.88	1.94	2.00	2.07	2.13
507.000	2.20	2.27	2.34	2.41	2.49
522.000	2.56	2.64	2.71	2.79	2.87
537.000	2.95	3.04	3.12	3.21	3.30
552.000	3.39	3.49	3.59	3.69	3.80
567.000	3.92	4.04	4.17	4.30	4.45
582.000	4.60	4.75	4.91	5.08	5.26
597.000	5.59	5.92	6.26	6.60	6.94
612.000	7.29	7.65	8.00	8.37	8.73
627.000	9.11	9.49	9.87	10.27	10.67
642.000	11.09	11.52	11.97	12.44	12.93
657.000	13.45	14.00	14.57	15.28	16.10
672.000	16.96	17.85	18.79	19.78	20.82
687.000	21.90	23.05	24.25	25.52	26.85
702.000	28.45	30.21	32.08	34.09	36.30
717.000	38.78	41.62	45.37	49.84	55.15
732.000	61.74	69.79	79.07	90.07	102.08
747.000	115.03	128.53	141.66	154.62	166.25
762.000	176.83	185.60	192.44	197.55	200.69
777.000	201.98	201.69	200.05	197.29	193.67
792.000	189.45	184.81	179.79	174.49	168.99
807.000	163.48	158.05	152.58	147.13	141.73
822.000	136.68	131.70	126.80	122.00	117.40
837.000	113.08	108.87	104.76	100.76	96.90
852.000	93.39	89.98	86.68	83.48	80.40

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
867.000	77.44	74.79	72.26	69.82	67.48
882.000	65.23	63.07	60.99	59.01	57.25
897.000	55.58	53.98	52.44	50.95	49.53
912.000	48.15	46.83	45.55	44.31	43.10
927.000	41.93	40.92	39.92	38.93	37.96
942.000	37.01	36.08	35.17	34.27	33.40
957.000	32.56	31.73	30.94	30.16	29.42
972.000	28.70	28.00	27.33	26.77	26.24
987.000	25.72	25.22	24.73	24.25	23.79
1,002.000	23.35	22.91	22.49	22.09	21.69
1,017.000	21.31	20.94	20.58	20.24	19.90
1,032.000	19.57	19.26	18.95	18.65	18.37
1,047.000	18.09	17.82	17.55	17.30	17.05
1,062.000	16.81	16.58	16.35	16.13	15.92
1,077.000	15.71	15.51	15.32	15.13	14.94
1,092.000	14.78	14.64	14.50	14.36	14.22
1,107.000	14.08	13.93	13.79	13.64	13.50
1,122.000	13.35	13.20	13.06	12.91	12.76
1,137.000	12.62	12.47	12.33	12.19	12.05
1,152.000	11.92	11.78	11.65	11.52	11.39
1,167.000	11.27	11.15	11.03	10.91	10.79
1,182.000	10.68	10.57	10.46	10.36	10.25
1,197.000	10.15	10.05	9.95	9.86	9.76
1,212.000	9.67	9.58	9.50	9.41	9.33
1,227.000	9.24	9.16	9.08	9.01	8.93
1,242.000	8.86	8.78	8.71	8.64	8.57
1,257.000	8.51	8.44	8.37	8.31	8.25
1,272.000	8.19	8.13	8.07	8.01	7.95
1,287.000	7.90	7.84	7.79	7.73	7.68
1,302.000	7.63	7.58	7.53	7.48	7.43
1,317.000	7.39	7.34	7.30	7.25	7.21
1,332.000	7.16	7.12	7.08	7.04	6.99
1,347.000	6.95	6.91	6.87	6.84	6.80
1,362.000	6.76	6.72	6.69	6.65	6.62
1,377.000	6.58	6.55	6.51	6.48	6.45
1,392.000	6.41	6.38	6.35	6.32	6.29
1,407.000	6.26	6.22	6.19	6.17	6.14
1,422.000	6.11	6.08	6.05	6.02	5.99
1,437.000	5.97	5.94	(N/A)	(N/A)	(N/A)

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Peak Discharge	297.87 ft ³ /s
Time to Peak	774.000 min
Hydrograph Volume	2,987,867.198 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
222.000	0.00	0.00	0.00	0.00	0.00
237.000	0.01	0.01	0.01	0.02	0.02
252.000	0.03	0.03	0.04	0.05	0.05
267.000	0.06	0.07	0.09	0.10	0.11
282.000	0.13	0.14	0.16	0.18	0.20
297.000	0.22	0.24	0.26	0.28	0.30
312.000	0.33	0.35	0.38	0.41	0.43
327.000	0.46	0.49	0.53	0.56	0.59
342.000	0.62	0.66	0.69	0.73	0.77
357.000	0.81	0.84	0.88	0.93	0.97
372.000	1.01	1.06	1.10	1.15	1.20
387.000	1.25	1.31	1.36	1.42	1.49
402.000	1.55	1.62	1.69	1.76	1.83
417.000	1.91	1.98	2.06	2.15	2.23
432.000	2.31	2.40	2.49	2.58	2.67
447.000	2.76	2.85	2.95	3.05	3.15
462.000	3.25	3.35	3.45	3.56	3.66
477.000	3.77	3.88	3.99	4.10	4.22
492.000	4.33	4.45	4.57	4.69	4.81
507.000	4.93	5.06	5.19	5.37	5.61
522.000	5.84	6.08	6.31	6.55	6.78
537.000	7.02	7.26	7.49	7.74	7.98
552.000	8.23	8.50	8.77	9.06	9.36
567.000	9.69	10.03	10.40	10.79	11.19
582.000	11.62	12.07	12.54	13.03	13.53
597.000	14.04	14.57	15.18	15.89	16.59
612.000	17.31	18.03	18.75	19.48	20.22
627.000	20.96	21.71	22.48	23.26	24.06
642.000	24.88	25.73	26.62	27.60	28.75
657.000	29.96	31.23	32.57	33.98	35.46
672.000	37.01	38.64	40.34	42.15	44.28
687.000	46.48	48.78	51.17	53.65	56.22
702.000	58.93	62.00	65.21	68.58	72.19
717.000	76.18	80.97	86.67	93.33	101.51
732.000	111.55	123.38	137.30	153.29	170.65
747.000	189.10	208.02	226.26	243.41	258.91
762.000	272.03	282.74	290.56	295.58	297.87
777.000	297.67	295.38	291.36	285.96	279.56
792.000	272.46	264.98	257.14	249.04	241.00

Subsection: Diverted Hydrograph
Label: POND A STRUCTURE
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
807.000	232.97	224.95	217.02	209.43	201.96
822.000	194.61	187.58	180.81	174.19	167.71
837.000	161.60	155.71	149.95	144.34	139.03
852.000	134.00	129.12	124.38	119.81	115.59
867.000	111.59	107.73	104.02	100.47	97.07
882.000	94.03	91.10	88.29	85.59	83.01
897.000	80.53	78.16	75.98	73.98	72.04
912.000	70.17	68.36	66.60	64.88	63.19
927.000	61.53	59.89	58.31	56.86	55.42
942.000	53.99	52.58	51.19	49.83	48.49
957.000	47.19	45.92	44.70	43.51	42.36
972.000	41.34	40.39	39.47	38.58	37.71
987.000	36.88	36.07	35.29	34.54	33.82
1,002.000	33.12	32.45	31.80	31.17	30.57
1,017.000	29.99	29.43	28.89	28.37	27.87
1,032.000	27.39	26.98	26.59	26.22	25.86
1,047.000	25.50	25.16	24.82	24.50	24.18
1,062.000	23.87	23.57	23.28	22.99	22.71
1,077.000	22.44	22.18	21.92	21.67	21.42
1,092.000	21.18	20.94	20.69	20.45	20.21
1,107.000	19.97	19.72	19.47	19.22	18.96
1,122.000	18.70	18.45	18.19	17.93	17.67
1,137.000	17.42	17.17	16.92	16.68	16.45
1,152.000	16.21	15.99	15.77	15.55	15.34
1,167.000	15.14	14.94	14.77	14.62	14.47
1,182.000	14.33	14.19	14.05	13.92	13.79
1,197.000	13.66	13.53	13.40	13.28	13.16
1,212.000	13.04	12.93	12.81	12.70	12.59
1,227.000	12.48	12.38	12.28	12.17	12.07
1,242.000	11.98	11.88	11.79	11.69	11.60
1,257.000	11.51	11.43	11.34	11.26	11.17
1,272.000	11.09	11.01	10.94	10.86	10.78
1,287.000	10.71	10.64	10.56	10.49	10.42
1,302.000	10.35	10.29	10.22	10.16	10.09
1,317.000	10.03	9.97	9.90	9.84	9.78
1,332.000	9.73	9.67	9.61	9.56	9.50
1,347.000	9.45	9.39	9.34	9.29	9.24
1,362.000	9.18	9.13	9.08	9.04	8.99
1,377.000	8.94	8.89	8.85	8.80	8.76
1,392.000	8.71	8.67	8.62	8.58	8.54
1,407.000	8.50	8.45	8.41	8.37	8.33
1,422.000	8.29	8.25	8.22	8.18	8.14
1,437.000	8.10	8.07	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND B
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	580.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
580.00	0.00	0.000	1.060	0.00	0.00	0.00
580.50	3.82	23,086.800	1.060	0.00	3.82	260.34
581.00	10.80	46,173.600	1.060	0.00	10.80	523.84
581.50	19.84	69,260.400	1.060	0.00	19.84	789.40
582.00	30.55	92,347.200	1.060	0.00	30.55	1,056.63
582.50	42.69	115,434.000	1.060	0.00	42.69	1,325.29
583.00	56.12	138,520.800	1.060	0.00	56.12	1,595.24
583.50	70.72	161,607.600	1.060	0.00	70.72	1,866.36
584.00	86.40	184,694.400	1.060	0.00	86.40	2,138.56
584.50	103.10	207,781.200	1.060	0.00	103.10	2,411.78
585.00	120.75	230,868.000	1.060	0.00	120.75	2,685.95
585.50	139.31	253,954.800	1.060	0.00	139.31	2,961.03
586.00	158.73	277,041.600	1.060	0.00	158.73	3,236.97
586.50	178.98	300,128.400	1.060	0.00	178.98	3,513.74
587.00	200.02	323,215.200	1.060	0.00	200.02	3,791.30
587.50	221.83	346,302.000	1.060	0.00	221.83	4,069.63
588.00	244.38	369,388.800	1.060	0.00	244.38	4,348.70
588.50	267.64	392,475.600	1.060	0.00	267.64	4,628.48
589.00	291.60	415,562.400	1.060	0.00	291.60	4,908.96

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND B
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	580.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
580.00	0.00	0.000	1.060	0.00	0.00	0.00
580.50	3.82	23,086.800	1.060	0.00	3.82	260.34
581.00	10.80	46,173.600	1.060	0.00	10.80	523.84
581.50	19.84	69,260.400	1.060	0.00	19.84	789.40
582.00	30.55	92,347.200	1.060	0.00	30.55	1,056.63
582.50	42.69	115,434.000	1.060	0.00	42.69	1,325.29
583.00	56.12	138,520.800	1.060	0.00	56.12	1,595.24
583.50	70.72	161,607.600	1.060	0.00	70.72	1,866.36
584.00	86.40	184,694.400	1.060	0.00	86.40	2,138.56
584.50	103.10	207,781.200	1.060	0.00	103.10	2,411.78
585.00	120.75	230,868.000	1.060	0.00	120.75	2,685.95
585.50	139.31	253,954.800	1.060	0.00	139.31	2,961.03
586.00	158.73	277,041.600	1.060	0.00	158.73	3,236.97
586.50	178.98	300,128.400	1.060	0.00	178.98	3,513.74
587.00	200.02	323,215.200	1.060	0.00	200.02	3,791.30
587.50	221.83	346,302.000	1.060	0.00	221.83	4,069.63
588.00	244.38	369,388.800	1.060	0.00	244.38	4,348.70
588.50	267.64	392,475.600	1.060	0.00	267.64	4,628.48
589.00	291.60	415,562.400	1.060	0.00	291.60	4,908.96

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND B
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	580.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
580.00	0.00	0.000	1.060	0.00	0.00	0.00
580.50	3.82	23,086.800	1.060	0.00	3.82	260.34
581.00	10.80	46,173.600	1.060	0.00	10.80	523.84
581.50	19.84	69,260.400	1.060	0.00	19.84	789.40
582.00	30.55	92,347.200	1.060	0.00	30.55	1,056.63
582.50	42.69	115,434.000	1.060	0.00	42.69	1,325.29
583.00	56.12	138,520.800	1.060	0.00	56.12	1,595.24
583.50	70.72	161,607.600	1.060	0.00	70.72	1,866.36
584.00	86.40	184,694.400	1.060	0.00	86.40	2,138.56
584.50	103.10	207,781.200	1.060	0.00	103.10	2,411.78
585.00	120.75	230,868.000	1.060	0.00	120.75	2,685.95
585.50	139.31	253,954.800	1.060	0.00	139.31	2,961.03
586.00	158.73	277,041.600	1.060	0.00	158.73	3,236.97
586.50	178.98	300,128.400	1.060	0.00	178.98	3,513.74
587.00	200.02	323,215.200	1.060	0.00	200.02	3,791.30
587.50	221.83	346,302.000	1.060	0.00	221.83	4,069.63
588.00	244.38	369,388.800	1.060	0.00	244.38	4,348.70
588.50	267.64	392,475.600	1.060	0.00	267.64	4,628.48
589.00	291.60	415,562.400	1.060	0.00	291.60	4,908.96

Subsection: Elevation-Volume-Flow Table (Pond)
Label: POND B
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Infiltration	
Infiltration Method (Computed)	No Infiltration
Initial Conditions	
Elevation (Water Surface, Initial)	580.00 ft
Volume (Initial)	0.000 ft ³
Flow (Initial Outlet)	0.00 ft ³ /s
Flow (Initial Infiltration)	0.00 ft ³ /s
Flow (Initial, Total)	0.00 ft ³ /s
Time Increment	3.000 min

Elevation (ft)	Outflow (ft ³ /s)	Storage (ft ³)	Area (acres)	Infiltration (ft ³ /s)	Flow (Total) (ft ³ /s)	2S/t + O (ft ³ /s)
580.00	0.00	0.000	1.060	0.00	0.00	0.00
580.50	3.82	23,086.800	1.060	0.00	3.82	260.34
581.00	10.80	46,173.600	1.060	0.00	10.80	523.84
581.50	19.84	69,260.400	1.060	0.00	19.84	789.40
582.00	30.55	92,347.200	1.060	0.00	30.55	1,056.63
582.50	42.69	115,434.000	1.060	0.00	42.69	1,325.29
583.00	56.12	138,520.800	1.060	0.00	56.12	1,595.24
583.50	70.72	161,607.600	1.060	0.00	70.72	1,866.36
584.00	86.40	184,694.400	1.060	0.00	86.40	2,138.56
584.50	103.10	207,781.200	1.060	0.00	103.10	2,411.78
585.00	120.75	230,868.000	1.060	0.00	120.75	2,685.95
585.50	139.31	253,954.800	1.060	0.00	139.31	2,961.03
586.00	158.73	277,041.600	1.060	0.00	158.73	3,236.97
586.50	178.98	300,128.400	1.060	0.00	178.98	3,513.74
587.00	200.02	323,215.200	1.060	0.00	200.02	3,791.30
587.50	221.83	346,302.000	1.060	0.00	221.83	4,069.63
588.00	244.38	369,388.800	1.060	0.00	244.38	4,348.70
588.50	267.64	392,475.600	1.060	0.00	267.64	4,628.48
589.00	291.60	415,562.400	1.060	0.00	291.60	4,908.96

Subsection: Level Pool Pond Routing Summary
Label: POND B (IN)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		580.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		83.62 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		46.56 ft³/s	Time to Peak (Flow, Outlet)
			732.000 min
			753.000 min
Elevation (Water Surface, Peak)		582.64 ft	
Volume (Peak)		122,092.129 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		432,959.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		426,584.000 ft³	
Volume (Retained)		6,188.000 ft³	
Volume (Unrouted)		-187.000 ft³	
Error (Mass Balance)		0.0 %	

Subsection: Level Pool Pond Routing Summary
Label: POND B (IN)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		580.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		228.57 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		142.44 ft³/s	Time to Peak (Flow, Outlet)
			732.000 min
			750.000 min
Elevation (Water Surface, Peak)		585.58 ft	
Volume (Peak)		257,678.995 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		1,035,695.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		1,016,696.000 ft³	
Volume (Retained)		18,441.000 ft³	
Volume (Unrouted)		-557.000 ft³	
Error (Mass Balance)		0.1 %	

Subsection: Level Pool Pond Routing Summary
Label: POND B (IN)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		580.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		292.01 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		192.78 ft³/s	Time to Peak (Flow, Outlet)
			732.000 min
			747.000 min
Elevation (Water Surface, Peak)		586.83 ft	
Volume (Peak)		315,277.032 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		1,410,838.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		1,387,900.000 ft³	
Volume (Retained)		22,265.000 ft³	
Volume (Unrouted)		-673.000 ft³	
Error (Mass Balance)		0.0 %	

Subsection: Level Pool Pond Routing Summary
Label: POND B (IN)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Infiltration			
Infiltration Method (Computed)		No Infiltration	
Initial Conditions			
Elevation (Water Surface, Initial)		580.00 ft	
Volume (Initial)		0.000 ft³	
Flow (Initial Outlet)		0.00 ft³/s	
Flow (Initial Infiltration)		0.00 ft³/s	
Flow (Initial, Total)		0.00 ft³/s	
Time Increment		3.000 min	
Inflow/Outflow Hydrograph Summary			
Flow (Peak In)		398.18 ft³/s	Time to Peak (Flow, In)
Flow (Peak Outlet)		279.57 ft³/s	Time to Peak (Flow, Outlet)
			732.000 min
			747.000 min
Elevation (Water Surface, Peak)		588.75 ft	
Volume (Peak)		403,972.979 ft³	
Mass Balance (ft³)			
Volume (Initial)		0.000 ft³	
Volume (Total Inflow)		2,125,724.000 ft³	
Volume (Total Infiltration)		0.000 ft³	
Volume (Total Outlet Outflow)		2,098,291.000 ft³	
Volume (Retained)		26,534.000 ft³	
Volume (Unrouted)		-899.000 ft³	
Error (Mass Balance)		0.0 %	

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

Peak Discharge	46.56 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	426,584.120 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
240.000	0.00	0.00	0.01	0.03	0.07
255.000	0.11	0.14	0.16	0.17	0.17
270.000	0.17	0.17	0.17	0.16	0.16
285.000	0.15	0.15	0.15	0.14	0.14
300.000	0.13	0.13	0.13	0.12	0.12
315.000	0.12	0.11	0.11	0.11	0.10
330.000	0.10	0.10	0.09	0.09	0.09
345.000	0.09	0.08	0.08	0.08	0.08
360.000	0.07	0.08	0.10	0.17	0.27
375.000	0.37	0.44	0.48	0.50	0.51
390.000	0.51	0.50	0.49	0.48	0.47
405.000	0.45	0.44	0.43	0.41	0.40
420.000	0.39	0.38	0.37	0.36	0.35
435.000	0.34	0.33	0.32	0.31	0.30
450.000	0.29	0.29	0.32	0.41	0.55
465.000	0.68	0.78	0.83	0.85	0.85
480.000	0.85	0.83	0.81	0.79	0.77
495.000	0.75	0.73	0.71	0.69	0.67
510.000	0.65	0.64	0.67	0.77	0.93
525.000	1.08	1.18	1.24	1.25	1.25
540.000	1.24	1.21	1.18	1.15	1.12
555.000	1.09	1.06	1.03	1.02	1.08
570.000	1.21	1.39	1.54	1.64	1.69
585.000	1.70	1.70	1.68	1.65	1.62
600.000	1.63	1.71	1.88	2.07	2.21
615.000	2.29	2.33	2.33	2.31	2.28
630.000	2.24	2.21	2.23	2.34	2.52
645.000	2.70	2.84	2.91	2.93	2.93
660.000	2.96	3.06	3.24	3.44	3.60
675.000	3.69	3.74	3.81	4.09	4.47
690.000	4.82	5.08	5.32	5.65	6.08
705.000	6.55	7.10	7.81	8.77	10.10
720.000	12.15	14.97	18.37	22.69	27.44
735.000	31.99	36.14	39.43	42.05	44.24
750.000	45.83	46.56	46.36	45.43	44.16
765.000	42.87	41.63	40.19	38.53	36.92
780.000	35.60	34.43	33.18	31.72	30.13
795.000	28.57	27.02	25.67	24.67	23.95
810.000	23.30	22.53	21.62	20.60	19.58

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 2

Return Event: 2 years
 Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
825.000	18.64	17.74	17.00	16.58	16.40
840.000	16.28	16.05	15.67	15.17	14.60
855.000	13.97	13.30	12.62	12.01	11.64
870.000	11.59	11.74	11.88	11.87	11.72
885.000	11.45	11.09	10.70	10.33	9.93
900.000	9.52	9.10	8.69	8.36	8.23
915.000	8.34	8.60	8.85	8.98	8.99
930.000	8.91	8.77	8.56	8.30	8.00
945.000	7.69	7.37	7.04	6.72	6.40
960.000	6.09	5.83	5.73	5.86	6.21
975.000	6.60	6.89	7.06	7.12	7.09
990.000	6.99	6.83	6.63	6.40	6.15
1,005.000	5.89	5.63	5.37	5.12	4.87
1,020.000	4.63	4.40	4.18	3.97	3.79
1,035.000	3.68	3.60	3.61	3.75	4.14
1,050.000	4.63	5.02	5.28	5.42	5.48
1,065.000	5.47	5.39	5.26	5.11	4.93
1,080.000	4.74	4.54	4.34	4.14	3.95
1,095.000	3.78	3.68	3.58	3.48	3.38
1,110.000	3.29	3.19	3.10	3.01	2.92
1,125.000	2.84	2.76	2.68	2.60	2.52
1,140.000	2.45	2.38	2.31	2.29	2.37
1,155.000	2.59	2.89	3.19	3.42	3.60
1,170.000	3.71	3.78	3.82	3.81	3.79
1,185.000	3.74	3.68	3.60	3.53	3.44
1,200.000	3.36	3.27	3.19	3.10	3.02
1,215.000	2.93	2.85	2.77	2.69	2.62
1,230.000	2.54	2.47	2.40	2.33	2.26
1,245.000	2.19	2.13	2.07	2.00	1.95
1,260.000	1.89	1.83	1.78	1.73	1.68
1,275.000	1.63	1.58	1.54	1.53	1.63
1,290.000	1.86	2.19	2.51	2.78	2.97
1,305.000	3.11	3.20	3.26	3.27	3.26
1,320.000	3.23	3.18	3.13	3.06	2.99
1,335.000	2.92	2.85	2.78	2.70	2.63
1,350.000	2.56	2.49	2.42	2.35	2.28
1,365.000	2.22	2.15	2.09	2.03	1.97
1,380.000	1.91	1.86	1.80	1.75	1.70
1,395.000	1.65	1.60	1.55	1.51	1.46
1,410.000	1.42	1.38	1.34	1.30	1.26
1,425.000	1.22	1.19	1.15	1.12	1.09
1,440.000	1.05	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Pond Routed Hydrograph (total out)
Label: POND B (OUT)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Peak Discharge	142.44 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	1,016,695.659 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
129.000	0.00	0.00	0.00	0.00	0.00
144.000	0.01	0.01	0.01	0.02	0.02
159.000	0.02	0.03	0.03	0.04	0.04
174.000	0.05	0.05	0.06	0.06	0.07
189.000	0.08	0.08	0.09	0.10	0.11
204.000	0.11	0.12	0.13	0.14	0.15
219.000	0.16	0.16	0.17	0.18	0.19
234.000	0.20	0.21	0.22	0.23	0.24
249.000	0.25	0.26	0.27	0.28	0.29
264.000	0.30	0.32	0.33	0.34	0.35
279.000	0.36	0.37	0.38	0.40	0.41
294.000	0.42	0.43	0.44	0.46	0.47
309.000	0.48	0.49	0.51	0.52	0.53
324.000	0.54	0.56	0.57	0.58	0.60
339.000	0.61	0.62	0.64	0.65	0.67
354.000	0.68	0.69	0.71	0.72	0.74
369.000	0.75	0.77	0.79	0.81	0.82
384.000	0.84	0.86	0.88	0.90	0.92
399.000	0.94	0.96	0.98	1.00	1.02
414.000	1.05	1.07	1.09	1.11	1.13
429.000	1.15	1.18	1.20	1.22	1.25
444.000	1.27	1.30	1.32	1.35	1.38
459.000	1.41	1.43	1.46	1.49	1.52
474.000	1.56	1.59	1.62	1.65	1.69
489.000	1.72	1.76	1.79	1.83	1.87
504.000	1.91	1.95	1.99	2.03	2.07
519.000	2.11	2.16	2.20	2.25	2.29
534.000	2.34	2.39	2.44	2.49	2.55
549.000	2.61	2.68	2.75	2.83	2.92
564.000	3.01	3.11	3.20	3.30	3.40
579.000	3.51	3.61	3.72	3.85	4.05
594.000	4.25	4.46	4.66	4.86	5.07
609.000	5.27	5.48	5.69	5.90	6.12
624.000	6.33	6.56	6.78	7.02	7.26
639.000	7.53	7.83	8.16	8.51	8.87
654.000	9.26	9.66	10.08	10.52	11.04
669.000	11.67	12.35	13.07	13.84	14.64
684.000	15.49	16.38	17.32	18.32	19.39
699.000	20.71	22.24	23.95	25.86	28.10

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
714.000	30.88	34.82	40.04	47.85	59.33
729.000	74.63	91.91	108.55	122.35	132.62
744.000	139.06	142.26	142.44	140.19	136.33
759.000	131.31	125.51	119.35	113.22	107.05
774.000	101.09	95.52	90.16	85.13	80.55
789.000	76.17	72.00	68.22	64.71	61.38
804.000	58.23	55.34	52.78	50.35	48.04
819.000	45.84	43.72	41.79	40.02	38.33
834.000	36.71	35.18	33.72	32.34	31.04
849.000	29.90	28.88	27.90	26.97	26.09
864.000	25.26	24.47	23.72	23.01	22.34
879.000	21.70	21.09	20.52	19.97	19.51
894.000	19.09	18.68	18.29	17.91	17.53
909.000	17.16	16.77	16.38	15.99	15.59
924.000	15.20	14.82	14.44	14.08	13.73
939.000	13.39	13.06	12.75	12.45	12.16
954.000	11.89	11.62	11.37	11.13	10.90
969.000	10.71	10.54	10.38	10.22	10.07
984.000	9.92	9.78	9.64	9.50	9.37
999.000	9.24	9.11	8.99	8.87	8.76
1,014.000	8.65	8.54	8.43	8.32	8.22
1,029.000	8.12	8.03	7.94	7.84	7.75
1,044.000	7.67	7.58	7.50	7.42	7.34
1,059.000	7.26	7.19	7.11	7.04	6.97
1,074.000	6.90	6.84	6.77	6.71	6.64
1,089.000	6.57	6.50	6.43	6.36	6.28
1,104.000	6.21	6.13	6.06	5.99	5.92
1,119.000	5.85	5.78	5.71	5.64	5.58
1,134.000	5.52	5.46	5.40	5.34	5.29
1,149.000	5.23	5.18	5.13	5.08	5.03
1,164.000	4.99	4.94	4.90	4.85	4.81
1,179.000	4.77	4.73	4.69	4.65	4.61
1,194.000	4.58	4.54	4.51	4.47	4.44
1,209.000	4.41	4.37	4.34	4.31	4.28
1,224.000	4.25	4.22	4.20	4.17	4.14
1,239.000	4.11	4.09	4.06	4.03	4.01
1,254.000	3.99	3.96	3.94	3.91	3.89
1,269.000	3.87	3.85	3.83	3.81	3.80
1,284.000	3.79	3.77	3.76	3.75	3.74
1,299.000	3.72	3.71	3.70	3.69	3.68
1,314.000	3.66	3.65	3.64	3.62	3.61
1,329.000	3.60	3.59	3.57	3.56	3.55
1,344.000	3.54	3.52	3.51	3.50	3.48

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,359.000	3.47	3.46	3.45	3.43	3.42
1,374.000	3.41	3.40	3.38	3.37	3.36
1,389.000	3.35	3.33	3.32	3.31	3.30
1,404.000	3.28	3.27	3.26	3.25	3.24
1,419.000	3.22	3.21	3.20	3.19	3.18
1,434.000	3.17	3.15	3.14	(N/A)	(N/A)

Subsection: Pond Routed Hydrograph (total out)
Label: POND B (OUT)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Peak Discharge	192.78 ft ³ /s
Time to Peak	747.000 min
Hydrograph Volume	1,387,900.183 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
108.000	0.00	0.00	0.00	0.01	0.01
123.000	0.01	0.02	0.02	0.02	0.03
138.000	0.03	0.04	0.05	0.05	0.06
153.000	0.07	0.08	0.09	0.10	0.10
168.000	0.11	0.12	0.13	0.15	0.16
183.000	0.17	0.18	0.19	0.20	0.21
198.000	0.23	0.24	0.25	0.26	0.28
213.000	0.29	0.30	0.32	0.33	0.34
228.000	0.36	0.37	0.39	0.40	0.42
243.000	0.43	0.45	0.46	0.48	0.49
258.000	0.51	0.52	0.54	0.55	0.57
273.000	0.58	0.60	0.62	0.63	0.65
288.000	0.67	0.68	0.70	0.72	0.73
303.000	0.75	0.77	0.78	0.80	0.82
318.000	0.83	0.85	0.87	0.89	0.90
333.000	0.92	0.94	0.96	0.98	0.99
348.000	1.01	1.03	1.05	1.07	1.09
363.000	1.11	1.13	1.15	1.17	1.20
378.000	1.23	1.26	1.29	1.32	1.36
393.000	1.39	1.42	1.46	1.50	1.53
408.000	1.57	1.61	1.64	1.68	1.72
423.000	1.76	1.80	1.84	1.89	1.93
438.000	1.97	2.02	2.06	2.11	2.15
453.000	2.20	2.25	2.30	2.35	2.39
468.000	2.45	2.50	2.55	2.60	2.66
483.000	2.71	2.77	2.82	2.88	2.94
498.000	3.00	3.06	3.12	3.18	3.24
513.000	3.31	3.37	3.44	3.51	3.58
528.000	3.65	3.72	3.79	3.90	4.04
543.000	4.17	4.32	4.48	4.66	4.86
558.000	5.07	5.30	5.53	5.77	6.01
573.000	6.26	6.51	6.76	7.01	7.26
588.000	7.51	7.77	8.02	8.28	8.54
603.000	8.81	9.08	9.35	9.62	9.90
618.000	10.19	10.48	10.78	11.16	11.56
633.000	11.96	12.39	12.87	13.40	13.98
648.000	14.61	15.28	15.98	16.70	17.45
663.000	18.23	19.05	19.93	21.01	22.16
678.000	23.36	24.61	25.92	27.29	28.73

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
693.000	30.25	32.02	33.95	36.01	38.26
708.000	40.77	43.80	47.79	52.90	60.04
723.000	70.51	86.21	106.99	130.15	152.07
738.000	169.90	182.47	189.98	192.78	191.58
753.000	187.24	180.72	173.00	164.53	155.81
768.000	147.24	138.79	130.92	123.38	116.42
783.000	109.94	103.82	98.29	93.07	88.13
798.000	83.64	79.50	75.60	71.94	68.66
813.000	65.64	62.77	60.00	57.33	54.85
828.000	52.54	50.32	48.20	46.18	44.26
843.000	42.47	40.92	39.44	38.05	36.73
858.000	35.49	34.31	33.21	32.16	31.18
873.000	30.28	29.50	28.75	28.04	27.36
888.000	26.72	26.10	25.51	24.95	24.41
903.000	23.89	23.37	22.85	22.31	21.75
918.000	21.17	20.60	20.03	19.53	19.06
933.000	18.61	18.16	17.73	17.31	16.91
948.000	16.53	16.16	15.80	15.46	15.14
963.000	14.82	14.52	14.24	13.96	13.70
978.000	13.44	13.20	12.97	12.75	12.53
993.000	12.33	12.13	11.94	11.75	11.58
1,008.000	11.41	11.24	11.09	10.93	10.79
1,023.000	10.68	10.57	10.46	10.36	10.26
1,038.000	10.16	10.06	9.96	9.87	9.77
1,053.000	9.68	9.59	9.50	9.41	9.33
1,068.000	9.25	9.16	9.08	9.00	8.93
1,083.000	8.85	8.77	8.69	8.60	8.50
1,098.000	8.40	8.30	8.20	8.09	7.99
1,113.000	7.88	7.78	7.68	7.58	7.49
1,128.000	7.40	7.31	7.22	7.13	7.05
1,143.000	6.97	6.89	6.81	6.74	6.67
1,158.000	6.60	6.53	6.47	6.40	6.34
1,173.000	6.28	6.22	6.17	6.11	6.06
1,188.000	6.00	5.95	5.90	5.85	5.81
1,203.000	5.76	5.71	5.67	5.62	5.58
1,218.000	5.54	5.50	5.46	5.42	5.38
1,233.000	5.35	5.31	5.27	5.24	5.20
1,248.000	5.17	5.14	5.10	5.07	5.04
1,263.000	5.01	4.98	4.95	4.92	4.89
1,278.000	4.86	4.83	4.81	4.78	4.75
1,293.000	4.73	4.70	4.68	4.65	4.63
1,308.000	4.60	4.58	4.55	4.53	4.51
1,323.000	4.48	4.46	4.44	4.42	4.40

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 25

Return Event: 25 years
 Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,338.000	4.37	4.35	4.33	4.31	4.29
1,353.000	4.27	4.25	4.23	4.21	4.19
1,368.000	4.18	4.16	4.14	4.12	4.10
1,383.000	4.08	4.07	4.05	4.03	4.01
1,398.000	4.00	3.98	3.96	3.95	3.93
1,413.000	3.91	3.90	3.88	3.87	3.85
1,428.000	3.84	3.82	3.81	3.80	3.79

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

Peak Discharge	279.57 ft ³ /s
Time to Peak	747.000 min
Hydrograph Volume	2,098,290.463 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
81.000	0.00	0.00	0.00	0.00	0.01
96.000	0.01	0.02	0.02	0.03	0.04
111.000	0.05	0.05	0.06	0.07	0.09
126.000	0.10	0.11	0.12	0.14	0.15
141.000	0.17	0.18	0.20	0.21	0.23
156.000	0.24	0.26	0.28	0.30	0.32
171.000	0.33	0.35	0.37	0.39	0.41
186.000	0.43	0.45	0.47	0.49	0.51
201.000	0.53	0.55	0.57	0.60	0.62
216.000	0.64	0.66	0.68	0.70	0.73
231.000	0.75	0.77	0.79	0.82	0.84
246.000	0.86	0.89	0.91	0.93	0.95
261.000	0.98	1.00	1.02	1.05	1.07
276.000	1.10	1.12	1.14	1.17	1.19
291.000	1.21	1.24	1.26	1.29	1.32
306.000	1.34	1.37	1.40	1.42	1.45
321.000	1.48	1.51	1.54	1.57	1.60
336.000	1.63	1.67	1.70	1.73	1.77
351.000	1.80	1.84	1.87	1.91	1.95
366.000	1.99	2.03	2.09	2.14	2.20
381.000	2.26	2.33	2.40	2.46	2.53
396.000	2.60	2.67	2.75	2.82	2.89
411.000	2.97	3.04	3.12	3.19	3.27
426.000	3.35	3.43	3.51	3.59	3.67
441.000	3.75	3.84	3.99	4.14	4.29
456.000	4.43	4.57	4.71	4.85	4.99
471.000	5.13	5.27	5.41	5.54	5.68
486.000	5.82	5.95	6.09	6.23	6.36
501.000	6.50	6.64	6.78	6.92	7.06
516.000	7.20	7.35	7.49	7.64	7.79
531.000	7.94	8.09	8.24	8.40	8.56
546.000	8.74	8.96	9.24	9.56	9.91
561.000	10.30	10.70	11.21	11.76	12.30
576.000	12.85	13.39	13.94	14.47	15.01
591.000	15.54	16.07	16.60	17.13	17.65
606.000	18.18	18.71	19.24	19.78	20.41
621.000	21.05	21.70	22.35	23.00	23.68
636.000	24.41	25.24	26.20	27.28	28.46
651.000	29.71	31.09	32.63	34.20	35.80

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
666.000	37.46	39.18	41.00	42.92	45.11
681.000	47.37	49.70	52.11	54.61	57.28
696.000	60.15	63.04	65.95	68.97	72.38
711.000	76.52	81.74	88.79	98.88	114.06
726.000	136.71	166.67	199.60	230.30	254.26
741.000	270.09	278.28	279.57	275.11	266.33
756.000	255.03	242.24	229.17	216.24	203.80
771.000	192.07	181.03	170.92	161.50	152.92
786.000	144.96	137.54	130.76	124.35	118.42
801.000	113.00	107.91	103.14	98.90	94.90
816.000	91.08	87.36	83.85	80.43	77.09
831.000	73.85	70.74	67.96	65.31	62.79
846.000	60.41	58.17	56.07	54.25	52.53
861.000	50.91	49.38	47.93	46.57	45.29
876.000	44.08	42.94	41.94	41.01	40.12
891.000	39.27	38.47	37.69	36.96	36.24
906.000	35.52	34.75	33.92	33.03	32.11
921.000	31.16	30.25	29.41	28.59	27.78
936.000	27.00	26.25	25.53	24.84	24.18
951.000	23.56	22.97	22.40	21.87	21.36
966.000	20.88	20.42	19.99	19.62	19.28
981.000	18.96	18.65	18.35	18.06	17.78
996.000	17.52	17.26	17.01	16.77	16.54
1,011.000	16.31	16.10	15.89	15.68	15.49
1,026.000	15.30	15.11	14.93	14.76	14.59
1,041.000	14.43	14.27	14.11	13.96	13.82
1,056.000	13.67	13.54	13.40	13.27	13.14
1,071.000	13.02	12.90	12.78	12.66	12.55
1,086.000	12.43	12.30	12.15	11.99	11.81
1,101.000	11.63	11.44	11.26	11.07	10.88
1,116.000	10.72	10.59	10.45	10.32	10.19
1,131.000	10.06	9.94	9.82	9.70	9.59
1,146.000	9.48	9.38	9.27	9.17	9.07
1,161.000	8.98	8.89	8.80	8.71	8.63
1,176.000	8.54	8.46	8.38	8.31	8.23
1,191.000	8.16	8.09	8.02	7.96	7.89
1,206.000	7.83	7.77	7.70	7.65	7.59
1,221.000	7.53	7.47	7.42	7.37	7.31
1,236.000	7.26	7.21	7.16	7.12	7.07
1,251.000	7.02	6.98	6.93	6.89	6.85
1,266.000	6.80	6.76	6.72	6.68	6.64
1,281.000	6.60	6.56	6.53	6.49	6.45
1,296.000	6.42	6.38	6.35	6.31	6.28

Subsection: Pond Routed Hydrograph (total out)
 Label: POND B (OUT)
 Scenario: EX 100

Return Event: 100 years
 Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,311.000	6.24	6.21	6.18	6.15	6.12
1,326.000	6.08	6.05	6.02	5.99	5.96
1,341.000	5.93	5.90	5.88	5.85	5.82
1,356.000	5.79	5.77	5.74	5.71	5.69
1,371.000	5.66	5.63	5.61	5.58	5.56
1,386.000	5.53	5.51	5.49	5.46	5.44
1,401.000	5.41	5.39	5.37	5.35	5.32
1,416.000	5.30	5.28	5.26	5.24	5.22
1,431.000	5.20	5.17	5.15	5.13	(N/A)

Subsection: Pond Inflow Summary
Label: POND B (IN)
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Summary for Hydrograph Addition at 'POND B'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA E
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA E	246,761.791	729.000	56.20
Flow (From)	EX AREA D	186,197.006	741.000	33.87
Flow (In)	POND B	432,958.796	732.000	83.62

Subsection: Pond Inflow Summary
Label: POND B (IN)
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

Summary for Hydrograph Addition at 'POND B'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA E
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA E	525,338.518	729.000	140.51
Flow (From)	EX AREA D	510,356.033	741.000	107.11
Flow (In)	POND B	1,035,694.551	732.000	228.57

Subsection: Pond Inflow Summary
Label: POND B (IN)
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Summary for Hydrograph Addition at 'POND B'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA E
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA E	692,892.990	729.000	173.70
Flow (From)	EX AREA D	717,944.967	741.000	141.73
Flow (In)	POND B	1,410,837.957	732.000	292.01

Subsection: Pond Inflow Summary
Label: POND B (IN)
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Summary for Hydrograph Addition at 'POND B'

Upstream Link	Upstream Node
<Catchment to Outflow Node>	EX AREA E
<Catchment to Outflow Node>	EX AREA D

Node Inflows

Inflow Type	Element	Volume (ft ³)	Time to Peak (min)	Flow (Peak) (ft ³ /s)
Flow (From)	EX AREA E	1,007,953.909	729.000	229.03
Flow (From)	EX AREA D	1,117,769.907	738.000	199.85
Flow (In)	POND B	2,125,723.816	732.000	398.18

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

Peak Discharge	46.56 ft ³ /s
Time to Peak	753.000 min
Hydrograph Volume	426,584.120 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
240.000	0.00	0.00	0.01	0.03	0.07
255.000	0.11	0.14	0.16	0.17	0.17
270.000	0.17	0.17	0.17	0.16	0.16
285.000	0.15	0.15	0.15	0.14	0.14
300.000	0.13	0.13	0.13	0.12	0.12
315.000	0.12	0.11	0.11	0.11	0.10
330.000	0.10	0.10	0.09	0.09	0.09
345.000	0.09	0.08	0.08	0.08	0.08
360.000	0.07	0.08	0.10	0.17	0.27
375.000	0.37	0.44	0.48	0.50	0.51
390.000	0.51	0.50	0.49	0.48	0.47
405.000	0.45	0.44	0.43	0.41	0.40
420.000	0.39	0.38	0.37	0.36	0.35
435.000	0.34	0.33	0.32	0.31	0.30
450.000	0.29	0.29	0.32	0.41	0.55
465.000	0.68	0.78	0.83	0.85	0.85
480.000	0.85	0.83	0.81	0.79	0.77
495.000	0.75	0.73	0.71	0.69	0.67
510.000	0.65	0.64	0.67	0.77	0.93
525.000	1.08	1.18	1.24	1.25	1.25
540.000	1.24	1.21	1.18	1.15	1.12
555.000	1.09	1.06	1.03	1.02	1.08
570.000	1.21	1.39	1.54	1.64	1.69
585.000	1.70	1.70	1.68	1.65	1.62
600.000	1.63	1.71	1.88	2.07	2.21
615.000	2.29	2.33	2.33	2.31	2.28
630.000	2.24	2.21	2.23	2.34	2.52
645.000	2.70	2.84	2.91	2.93	2.93
660.000	2.96	3.06	3.24	3.44	3.60
675.000	3.69	3.74	3.81	4.09	4.47
690.000	4.82	5.08	5.32	5.65	6.08
705.000	6.55	7.10	7.81	8.77	10.10
720.000	12.15	14.97	18.37	22.69	27.44
735.000	31.99	36.14	39.43	42.05	44.24
750.000	45.83	46.56	46.36	45.43	44.16
765.000	42.87	41.63	40.19	38.53	36.92
780.000	35.60	34.43	33.18	31.72	30.13
795.000	28.57	27.02	25.67	24.67	23.95
810.000	23.30	22.53	21.62	20.60	19.58

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 2

Return Event: 2 years
Storm Event: 2 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
825.000	18.64	17.74	17.00	16.58	16.40
840.000	16.28	16.05	15.67	15.17	14.60
855.000	13.97	13.30	12.62	12.01	11.64
870.000	11.59	11.74	11.88	11.87	11.72
885.000	11.45	11.09	10.70	10.33	9.93
900.000	9.52	9.10	8.69	8.36	8.23
915.000	8.34	8.60	8.85	8.98	8.99
930.000	8.91	8.77	8.56	8.30	8.00
945.000	7.69	7.37	7.04	6.72	6.40
960.000	6.09	5.83	5.73	5.86	6.21
975.000	6.60	6.89	7.06	7.12	7.09
990.000	6.99	6.83	6.63	6.40	6.15
1,005.000	5.89	5.63	5.37	5.12	4.87
1,020.000	4.63	4.40	4.18	3.97	3.79
1,035.000	3.68	3.60	3.61	3.75	4.14
1,050.000	4.63	5.02	5.28	5.42	5.48
1,065.000	5.47	5.39	5.26	5.11	4.93
1,080.000	4.74	4.54	4.34	4.14	3.95
1,095.000	3.78	3.68	3.58	3.48	3.38
1,110.000	3.29	3.19	3.10	3.01	2.92
1,125.000	2.84	2.76	2.68	2.60	2.52
1,140.000	2.45	2.38	2.31	2.29	2.37
1,155.000	2.59	2.89	3.19	3.42	3.60
1,170.000	3.71	3.78	3.82	3.81	3.79
1,185.000	3.74	3.68	3.60	3.53	3.44
1,200.000	3.36	3.27	3.19	3.10	3.02
1,215.000	2.93	2.85	2.77	2.69	2.62
1,230.000	2.54	2.47	2.40	2.33	2.26
1,245.000	2.19	2.13	2.07	2.00	1.95
1,260.000	1.89	1.83	1.78	1.73	1.68
1,275.000	1.63	1.58	1.54	1.53	1.63
1,290.000	1.86	2.19	2.51	2.78	2.97
1,305.000	3.11	3.20	3.26	3.27	3.26
1,320.000	3.23	3.18	3.13	3.06	2.99
1,335.000	2.92	2.85	2.78	2.70	2.63
1,350.000	2.56	2.49	2.42	2.35	2.28
1,365.000	2.22	2.15	2.09	2.03	1.97
1,380.000	1.91	1.86	1.80	1.75	1.70
1,395.000	1.65	1.60	1.55	1.51	1.46
1,410.000	1.42	1.38	1.34	1.30	1.26
1,425.000	1.22	1.19	1.15	1.12	1.09
1,440.000	1.05	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Diverted Hydrograph
 Label: POND B OUTFALL
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

Peak Discharge	142.44 ft ³ /s
Time to Peak	750.000 min
Hydrograph Volume	1,016,695.659 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
129.000	0.00	0.00	0.00	0.00	0.00
144.000	0.01	0.01	0.01	0.02	0.02
159.000	0.02	0.03	0.03	0.04	0.04
174.000	0.05	0.05	0.06	0.06	0.07
189.000	0.08	0.08	0.09	0.10	0.11
204.000	0.11	0.12	0.13	0.14	0.15
219.000	0.16	0.16	0.17	0.18	0.19
234.000	0.20	0.21	0.22	0.23	0.24
249.000	0.25	0.26	0.27	0.28	0.29
264.000	0.30	0.32	0.33	0.34	0.35
279.000	0.36	0.37	0.38	0.40	0.41
294.000	0.42	0.43	0.44	0.46	0.47
309.000	0.48	0.49	0.51	0.52	0.53
324.000	0.54	0.56	0.57	0.58	0.60
339.000	0.61	0.62	0.64	0.65	0.67
354.000	0.68	0.69	0.71	0.72	0.74
369.000	0.75	0.77	0.79	0.81	0.82
384.000	0.84	0.86	0.88	0.90	0.92
399.000	0.94	0.96	0.98	1.00	1.02
414.000	1.05	1.07	1.09	1.11	1.13
429.000	1.15	1.18	1.20	1.22	1.25
444.000	1.27	1.30	1.32	1.35	1.38
459.000	1.41	1.43	1.46	1.49	1.52
474.000	1.56	1.59	1.62	1.65	1.69
489.000	1.72	1.76	1.79	1.83	1.87
504.000	1.91	1.95	1.99	2.03	2.07
519.000	2.11	2.16	2.20	2.25	2.29
534.000	2.34	2.39	2.44	2.49	2.55
549.000	2.61	2.68	2.75	2.83	2.92
564.000	3.01	3.11	3.20	3.30	3.40
579.000	3.51	3.61	3.72	3.85	4.05
594.000	4.25	4.46	4.66	4.86	5.07
609.000	5.27	5.48	5.69	5.90	6.12
624.000	6.33	6.56	6.78	7.02	7.26
639.000	7.53	7.83	8.16	8.51	8.87
654.000	9.26	9.66	10.08	10.52	11.04
669.000	11.67	12.35	13.07	13.84	14.64
684.000	15.49	16.38	17.32	18.32	19.39
699.000	20.71	22.24	23.95	25.86	28.10

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 10

Return Event: 10 years
Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
714.000	30.88	34.82	40.04	47.85	59.33
729.000	74.63	91.91	108.55	122.35	132.62
744.000	139.06	142.26	142.44	140.19	136.33
759.000	131.31	125.51	119.35	113.22	107.05
774.000	101.09	95.52	90.16	85.13	80.55
789.000	76.17	72.00	68.22	64.71	61.38
804.000	58.23	55.34	52.78	50.35	48.04
819.000	45.84	43.72	41.79	40.02	38.33
834.000	36.71	35.18	33.72	32.34	31.04
849.000	29.90	28.88	27.90	26.97	26.09
864.000	25.26	24.47	23.72	23.01	22.34
879.000	21.70	21.09	20.52	19.97	19.51
894.000	19.09	18.68	18.29	17.91	17.53
909.000	17.16	16.77	16.38	15.99	15.59
924.000	15.20	14.82	14.44	14.08	13.73
939.000	13.39	13.06	12.75	12.45	12.16
954.000	11.89	11.62	11.37	11.13	10.90
969.000	10.71	10.54	10.38	10.22	10.07
984.000	9.92	9.78	9.64	9.50	9.37
999.000	9.24	9.11	8.99	8.87	8.76
1,014.000	8.65	8.54	8.43	8.32	8.22
1,029.000	8.12	8.03	7.94	7.84	7.75
1,044.000	7.67	7.58	7.50	7.42	7.34
1,059.000	7.26	7.19	7.11	7.04	6.97
1,074.000	6.90	6.84	6.77	6.71	6.64
1,089.000	6.57	6.50	6.43	6.36	6.28
1,104.000	6.21	6.13	6.06	5.99	5.92
1,119.000	5.85	5.78	5.71	5.64	5.58
1,134.000	5.52	5.46	5.40	5.34	5.29
1,149.000	5.23	5.18	5.13	5.08	5.03
1,164.000	4.99	4.94	4.90	4.85	4.81
1,179.000	4.77	4.73	4.69	4.65	4.61
1,194.000	4.58	4.54	4.51	4.47	4.44
1,209.000	4.41	4.37	4.34	4.31	4.28
1,224.000	4.25	4.22	4.20	4.17	4.14
1,239.000	4.11	4.09	4.06	4.03	4.01
1,254.000	3.99	3.96	3.94	3.91	3.89
1,269.000	3.87	3.85	3.83	3.81	3.80
1,284.000	3.79	3.77	3.76	3.75	3.74
1,299.000	3.72	3.71	3.70	3.69	3.68
1,314.000	3.66	3.65	3.64	3.62	3.61
1,329.000	3.60	3.59	3.57	3.56	3.55
1,344.000	3.54	3.52	3.51	3.50	3.48

Subsection: Diverted Hydrograph
 Label: POND B OUTFALL
 Scenario: EX 10

Return Event: 10 years
 Storm Event: 10 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,359.000	3.47	3.46	3.45	3.43	3.42
1,374.000	3.41	3.40	3.38	3.37	3.36
1,389.000	3.35	3.33	3.32	3.31	3.30
1,404.000	3.28	3.27	3.26	3.25	3.24
1,419.000	3.22	3.21	3.20	3.19	3.18
1,434.000	3.17	3.15	3.14	(N/A)	(N/A)

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

Peak Discharge	192.78 ft ³ /s
Time to Peak	747.000 min
Hydrograph Volume	1,387,900.183 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
108.000	0.00	0.00	0.00	0.01	0.01
123.000	0.01	0.02	0.02	0.02	0.03
138.000	0.03	0.04	0.05	0.05	0.06
153.000	0.07	0.08	0.09	0.10	0.10
168.000	0.11	0.12	0.13	0.15	0.16
183.000	0.17	0.18	0.19	0.20	0.21
198.000	0.23	0.24	0.25	0.26	0.28
213.000	0.29	0.30	0.32	0.33	0.34
228.000	0.36	0.37	0.39	0.40	0.42
243.000	0.43	0.45	0.46	0.48	0.49
258.000	0.51	0.52	0.54	0.55	0.57
273.000	0.58	0.60	0.62	0.63	0.65
288.000	0.67	0.68	0.70	0.72	0.73
303.000	0.75	0.77	0.78	0.80	0.82
318.000	0.83	0.85	0.87	0.89	0.90
333.000	0.92	0.94	0.96	0.98	0.99
348.000	1.01	1.03	1.05	1.07	1.09
363.000	1.11	1.13	1.15	1.17	1.20
378.000	1.23	1.26	1.29	1.32	1.36
393.000	1.39	1.42	1.46	1.50	1.53
408.000	1.57	1.61	1.64	1.68	1.72
423.000	1.76	1.80	1.84	1.89	1.93
438.000	1.97	2.02	2.06	2.11	2.15
453.000	2.20	2.25	2.30	2.35	2.39
468.000	2.45	2.50	2.55	2.60	2.66
483.000	2.71	2.77	2.82	2.88	2.94
498.000	3.00	3.06	3.12	3.18	3.24
513.000	3.31	3.37	3.44	3.51	3.58
528.000	3.65	3.72	3.79	3.90	4.04
543.000	4.17	4.32	4.48	4.66	4.86
558.000	5.07	5.30	5.53	5.77	6.01
573.000	6.26	6.51	6.76	7.01	7.26
588.000	7.51	7.77	8.02	8.28	8.54
603.000	8.81	9.08	9.35	9.62	9.90
618.000	10.19	10.48	10.78	11.16	11.56
633.000	11.96	12.39	12.87	13.40	13.98
648.000	14.61	15.28	15.98	16.70	17.45
663.000	18.23	19.05	19.93	21.01	22.16
678.000	23.36	24.61	25.92	27.29	28.73

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
693.000	30.25	32.02	33.95	36.01	38.26
708.000	40.77	43.80	47.79	52.90	60.04
723.000	70.51	86.21	106.99	130.15	152.07
738.000	169.90	182.47	189.98	192.78	191.58
753.000	187.24	180.72	173.00	164.53	155.81
768.000	147.24	138.79	130.92	123.38	116.42
783.000	109.94	103.82	98.29	93.07	88.13
798.000	83.64	79.50	75.60	71.94	68.66
813.000	65.64	62.77	60.00	57.33	54.85
828.000	52.54	50.32	48.20	46.18	44.26
843.000	42.47	40.92	39.44	38.05	36.73
858.000	35.49	34.31	33.21	32.16	31.18
873.000	30.28	29.50	28.75	28.04	27.36
888.000	26.72	26.10	25.51	24.95	24.41
903.000	23.89	23.37	22.85	22.31	21.75
918.000	21.17	20.60	20.03	19.53	19.06
933.000	18.61	18.16	17.73	17.31	16.91
948.000	16.53	16.16	15.80	15.46	15.14
963.000	14.82	14.52	14.24	13.96	13.70
978.000	13.44	13.20	12.97	12.75	12.53
993.000	12.33	12.13	11.94	11.75	11.58
1,008.000	11.41	11.24	11.09	10.93	10.79
1,023.000	10.68	10.57	10.46	10.36	10.26
1,038.000	10.16	10.06	9.96	9.87	9.77
1,053.000	9.68	9.59	9.50	9.41	9.33
1,068.000	9.25	9.16	9.08	9.00	8.93
1,083.000	8.85	8.77	8.69	8.60	8.50
1,098.000	8.40	8.30	8.20	8.09	7.99
1,113.000	7.88	7.78	7.68	7.58	7.49
1,128.000	7.40	7.31	7.22	7.13	7.05
1,143.000	6.97	6.89	6.81	6.74	6.67
1,158.000	6.60	6.53	6.47	6.40	6.34
1,173.000	6.28	6.22	6.17	6.11	6.06
1,188.000	6.00	5.95	5.90	5.85	5.81
1,203.000	5.76	5.71	5.67	5.62	5.58
1,218.000	5.54	5.50	5.46	5.42	5.38
1,233.000	5.35	5.31	5.27	5.24	5.20
1,248.000	5.17	5.14	5.10	5.07	5.04
1,263.000	5.01	4.98	4.95	4.92	4.89
1,278.000	4.86	4.83	4.81	4.78	4.75
1,293.000	4.73	4.70	4.68	4.65	4.63
1,308.000	4.60	4.58	4.55	4.53	4.51
1,323.000	4.48	4.46	4.44	4.42	4.40

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 25

Return Event: 25 years
Storm Event: 25 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,338.000	4.37	4.35	4.33	4.31	4.29
1,353.000	4.27	4.25	4.23	4.21	4.19
1,368.000	4.18	4.16	4.14	4.12	4.10
1,383.000	4.08	4.07	4.05	4.03	4.01
1,398.000	4.00	3.98	3.96	3.95	3.93
1,413.000	3.91	3.90	3.88	3.87	3.85
1,428.000	3.84	3.82	3.81	3.80	3.79

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

Peak Discharge	279.57 ft ³ /s
Time to Peak	747.000 min
Hydrograph Volume	2,098,290.463 ft ³

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
81.000	0.00	0.00	0.00	0.00	0.01
96.000	0.01	0.02	0.02	0.03	0.04
111.000	0.05	0.05	0.06	0.07	0.09
126.000	0.10	0.11	0.12	0.14	0.15
141.000	0.17	0.18	0.20	0.21	0.23
156.000	0.24	0.26	0.28	0.30	0.32
171.000	0.33	0.35	0.37	0.39	0.41
186.000	0.43	0.45	0.47	0.49	0.51
201.000	0.53	0.55	0.57	0.60	0.62
216.000	0.64	0.66	0.68	0.70	0.73
231.000	0.75	0.77	0.79	0.82	0.84
246.000	0.86	0.89	0.91	0.93	0.95
261.000	0.98	1.00	1.02	1.05	1.07
276.000	1.10	1.12	1.14	1.17	1.19
291.000	1.21	1.24	1.26	1.29	1.32
306.000	1.34	1.37	1.40	1.42	1.45
321.000	1.48	1.51	1.54	1.57	1.60
336.000	1.63	1.67	1.70	1.73	1.77
351.000	1.80	1.84	1.87	1.91	1.95
366.000	1.99	2.03	2.09	2.14	2.20
381.000	2.26	2.33	2.40	2.46	2.53
396.000	2.60	2.67	2.75	2.82	2.89
411.000	2.97	3.04	3.12	3.19	3.27
426.000	3.35	3.43	3.51	3.59	3.67
441.000	3.75	3.84	3.99	4.14	4.29
456.000	4.43	4.57	4.71	4.85	4.99
471.000	5.13	5.27	5.41	5.54	5.68
486.000	5.82	5.95	6.09	6.23	6.36
501.000	6.50	6.64	6.78	6.92	7.06
516.000	7.20	7.35	7.49	7.64	7.79
531.000	7.94	8.09	8.24	8.40	8.56
546.000	8.74	8.96	9.24	9.56	9.91
561.000	10.30	10.70	11.21	11.76	12.30
576.000	12.85	13.39	13.94	14.47	15.01
591.000	15.54	16.07	16.60	17.13	17.65
606.000	18.18	18.71	19.24	19.78	20.41
621.000	21.05	21.70	22.35	23.00	23.68
636.000	24.41	25.24	26.20	27.28	28.46
651.000	29.71	31.09	32.63	34.20	35.80

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)
Output Time Increment = 3.000 min
Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
666.000	37.46	39.18	41.00	42.92	45.11
681.000	47.37	49.70	52.11	54.61	57.28
696.000	60.15	63.04	65.95	68.97	72.38
711.000	76.52	81.74	88.79	98.88	114.06
726.000	136.71	166.67	199.60	230.30	254.26
741.000	270.09	278.28	279.57	275.11	266.33
756.000	255.03	242.24	229.17	216.24	203.80
771.000	192.07	181.03	170.92	161.50	152.92
786.000	144.96	137.54	130.76	124.35	118.42
801.000	113.00	107.91	103.14	98.90	94.90
816.000	91.08	87.36	83.85	80.43	77.09
831.000	73.85	70.74	67.96	65.31	62.79
846.000	60.41	58.17	56.07	54.25	52.53
861.000	50.91	49.38	47.93	46.57	45.29
876.000	44.08	42.94	41.94	41.01	40.12
891.000	39.27	38.47	37.69	36.96	36.24
906.000	35.52	34.75	33.92	33.03	32.11
921.000	31.16	30.25	29.41	28.59	27.78
936.000	27.00	26.25	25.53	24.84	24.18
951.000	23.56	22.97	22.40	21.87	21.36
966.000	20.88	20.42	19.99	19.62	19.28
981.000	18.96	18.65	18.35	18.06	17.78
996.000	17.52	17.26	17.01	16.77	16.54
1,011.000	16.31	16.10	15.89	15.68	15.49
1,026.000	15.30	15.11	14.93	14.76	14.59
1,041.000	14.43	14.27	14.11	13.96	13.82
1,056.000	13.67	13.54	13.40	13.27	13.14
1,071.000	13.02	12.90	12.78	12.66	12.55
1,086.000	12.43	12.30	12.15	11.99	11.81
1,101.000	11.63	11.44	11.26	11.07	10.88
1,116.000	10.72	10.59	10.45	10.32	10.19
1,131.000	10.06	9.94	9.82	9.70	9.59
1,146.000	9.48	9.38	9.27	9.17	9.07
1,161.000	8.98	8.89	8.80	8.71	8.63
1,176.000	8.54	8.46	8.38	8.31	8.23
1,191.000	8.16	8.09	8.02	7.96	7.89
1,206.000	7.83	7.77	7.70	7.65	7.59
1,221.000	7.53	7.47	7.42	7.37	7.31
1,236.000	7.26	7.21	7.16	7.12	7.07
1,251.000	7.02	6.98	6.93	6.89	6.85
1,266.000	6.80	6.76	6.72	6.68	6.64
1,281.000	6.60	6.56	6.53	6.49	6.45
1,296.000	6.42	6.38	6.35	6.31	6.28

Subsection: Diverted Hydrograph
Label: POND B OUTFALL
Scenario: EX 100

Return Event: 100 years
Storm Event: 100 Yr Storm

HYDROGRAPH ORDINATES (ft³/s)

Output Time Increment = 3.000 min

Time on left represents time for first value in each row.

Time (min)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
1,311.000	6.24	6.21	6.18	6.15	6.12
1,326.000	6.08	6.05	6.02	5.99	5.96
1,341.000	5.93	5.90	5.88	5.85	5.82
1,356.000	5.79	5.77	5.74	5.71	5.69
1,371.000	5.66	5.63	5.61	5.58	5.56
1,386.000	5.53	5.51	5.49	5.46	5.44
1,401.000	5.41	5.39	5.37	5.35	5.32
1,416.000	5.30	5.28	5.26	5.24	5.22
1,431.000	5.20	5.17	5.15	5.13	(N/A)

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Traffic Impact Analysis Taylor Gateway Mixed-Use Retail, Restaurant, and Residential Development



Prepared for
Gateway Oz, LLC.

July 2022
(v1.2, July 2024)

Voigt Associates, Inc.

Professional Traffic Engineers, Texas Registered Firm F-5333
Project 55301 (v1.2)

Taylor Gateway Mixed-Use Retail, Restaurant, Grocery and Residential Development

Traffic Impact Analysis

US Highway 79 at Farm-to-Market Road 973
Taylor, Texas

Prepared for
Gateway Oz, LLC



Voigt Associates, Inc.
F-5333

Prepared by
Voigt Associates, Inc.
Project 53301
Originally Submitted July 2022

v1.3, July 2024

Executive Summary

This report presents the analysis and findings of a Traffic Impact Analysis performed by Voigt Associates, Inc. for a proposed multi-phase mixed-use development on the southeast corner of US 79 and Farm-to-Market Road 973 (FM 973) in Taylor, Texas.

The development is planned to consist of a mix of retail, restaurant, and residential development to include:

- Phase 1:
 - Restaurants
 - Spec Retail
 - Fast-Food/Quick-Service Restaurants
 - Convenience Store/Gas Station
 - Office Space; and
 - Hotels
- Phase 2:
 - Multifamily Mid-Rise Housing; and
 - Single-Family Attached Housing.

The anticipated time frame on development is to begin work on Phase 1 in 2024 with build-out and occupancy in 2025. Phase 2 is anticipated to begin work in 2025 with full build-out assumed for the year 2027 – with both timelines driven by permitting progress and market conditions, but with the desire to accelerate buildout as soon as possible for the entire project given overall market demand in the Taylor area.

The development is proposed to take access directly to US 79 via one driveway on the north side of the tract and with two driveways on FM 973 on the west side of the tract. Options for future access to the south and east are provided and reserved in the conceptual site plan.

Exhibit A1 shows the Site Location Map, Exhibit A2 presents the conceptual land plan, and Exhibit A3 shows the conceptual land plan with the anticipated access locations for Phase1 and Phase 2 conditions. All referenced exhibits in this report and found in Appendix A.

This section of the report summarizes the overall impact of the Taylor Gateway development and includes discussion about recommended site access, traffic control, and other improvements or operations issues that should be addressed as the development is built-out and occupied.

Traffic Operations Analysis

The traffic simulation analysis revealed that the added traffic during the AM and PM peak periods attributed to the proposed development will have marginal impacts at study intersections, with reductions in level of service attributable to background traffic growth in addition to the additional development related trips. Observations from the simulation analysis included:

- The AM peak hour is very much influenced by school-related traffic, with several intersections expected to experience undesirable LOS during that period – these are

discussed in more detail below. The PM peak hour shows LOS D or better at all study intersections for all scenarios and there will less emphasis in discussion for the afternoon peak hour. As of February 2024, the PM peak hour traffic conditions along FM 973 are being influenced by construction-related traffic associated with the Samsung plant west of the study area. The plant construction will have additional roadway capacity and connections to US 79 that will relieve traffic volumes on FM 973 in the study area.

- The signalized intersection of US 79 at FM 973 will begin to see increased delays and degraded levels of service with and without traffic associated with the development, particularly in the AM peak hour:
 - With phase 1 of development, LOS moves from D to E and optimizing the signal timings does not bring LOS back to D. Added capacity would be required to improve the LOS.
 - At the request of the City of Taylor, a scenario with dual left turns northbound on FM 973 at US 79, with addition of a new right turn deceleration/turn lane on the approach, was added to the analysis. This improvement would bring LOS back to D in the buildout year of the development (with associated signal timing adjustments), inclusive of background developments (Castlewood and Phase 1 of Samsung).
 - TxDOT should consider improvements at this intersection based on projected conditions with or without the subject development, including modifications to provide northbound dual left turns from FM 973 to US 79 and adding new pavement for an associated dedicated northbound right turn lane. This improvement should be coordinated with TxDOT to determine responsibility for the improvement (either by the developer, by TxDOT, or a combination of the two parties).
- By others - signalize the intersection of FM 973 at Wesley Miller Lane as this intersection currently meets volume warrants (at least for peak hour Warrant #3) due to school related traffic. This improvement should not be the responsibility of the developer, but TxDOT (or other entity) since it is currently warranted. This improvement lets in January 2025 and should be completed sometime in 2025.
- The site driveway on US 79 is anticipated to have LOS B operation in the AM peak hour and LOS C in the PM peak hour, typically considered acceptable for urban and suburban conditions. No mitigation for LOS or delays is required.
- Proposed Driveway 1B on FM 973 (the easternmost driveway) is required to be right-in/right-out only by TxDOT. At full buildout, this driveway approach will experience LOS C during the peak hours.

- Proposed Driveway 2B on FM 973 (the westernmost driveway) is proposed as full access, but with only stop control would be projected to be LOS F in the AM Peak hour (with D in the PM peak hour). This intersection does meet warrants for signalization (see below). Signalization improves LOS to C in the AM peak hour and LOS B in the PM peak hour.

Compliance With Access Management Guidelines

TxDOT *Access Management Manual* guidelines specify that for highways with posted speed limits equal to or greater than 50 mph (as is the case on US 79 and FM 973) the required access spacing is 425'. The provided spacing at proposed site driveways are:

1. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973;
2. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and
3. Driveway 2B, a full-access driveway on FM 973 located about 520' south of Driveway 1B and 650' north of the Taylor High School north site driveway.

Each of the site driveways meet the TxDOT access spacing standards. No modifications are needed to comply with the Access Management Manual guidelines. However, the requirement for speed change lanes (right turn deceleration lanes) may cause the resulting access separation distances to be greater than those per Table 2-2 of the AMM.

Analysis of Auxiliary Lane Warrants

The proposed driveways to US 79 and FM 973 were examined for the need for right turn speed change lanes using the Texas Department of Transportation (TxDOT) *Access Management Manual* and *Roadway Design Manual* guidelines. In general, a right turn deceleration lane may be needed on roadways with a posted speed limit of 45 mph or higher when right turning volumes exceed 50 vehicles per hour. The speed limit on US 79 is 65 mph and speed limit on FM 973 is 55 mph along the site frontage. If a right turn deceleration lane is warranted is summarized below:

- Driveway 2A (US 79):
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 1B (FM 973):
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 2B (FM 973):
 - Right turn deceleration lane marginally warranted by volume but required per TxDOT based on speed/safety concerns. To be constructed when tied-in to US 79.

Per Table 3-3 of the Roadway Design Manual, the deceleration lane on US 79 at Driveway 2A would have 30' storage and 415' deceleration length. The deceleration lane should be 12' wide with minimum 4' to maximum 12' shoulder (width to be determined with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards. The yield markings and lane edge markings should be refreshed for the northbound FM 973 approach to US 79 as part of the improvement. This configuration retains the current acceleration lane for the right turn movement from northbound on FM 973 to eastbound on US 79.

Per Table 3-3 of the Roadway Design Manual, the right turn deceleration lane on FM 973 at Driveway 1B would be required to have 30' storage and 415' deceleration length. Due to spacing limitations upstream to Driveway 2B, the total length provided would be 421' length with 76' taper. The right turn deceleration lane on FM 973 at Driveway 2B provides the RDM required 445' long deceleration lane with 100' taper. Both deceleration lanes on FM 973 should be 12' wide with minimum 4' shoulder (width to be confirmed with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards.

Traffic Signal Warrant

Driveway 2B on FM 973 was found to meet warrants for signalization at full buildout (and likely earlier) of the site. Per procedures outlined in the Texas Manual on Uniform Traffic Control Devices (TxMUTCD) (3), a traffic signal warrant analysis was conducted using the projected hourly volumes with development at the FM 973 at Driveway 2B intersection. The analysis results and warrant requirements are attached in Appendix E. The analysis showed:

- Warrant 1 (Eight Hour) met for 13 hours (8 required)
- Warrant 2 (Four Hour) met for 12 hours (4 required)
- Warrant 3 (Peak Hour) met for 9 hours (1 required, if applicable)

We would recommend a traffic signal be installed at the developers cost prior to complete build-out of Phase 1.

Access Driveway Recommendations

The development is proposed to have one driveway with access to US 79 and two driveways connecting to FM 973. Proposed site driveways include:

1. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973;
2. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and
3. Driveway 2B, a full-access driveway on FM 973 located about 1100' south of US 79.

The site plan is conceptual, but Driveway 2A on US 79 is right-in/right-out, so one lane in and one lane out will be adequate, with geometry provided to accommodate truck trips to the site to service the convenience store/gas station (refueling truck) and other retail developments within the site at Driveway 2A (likely 35' width with 40' radii). Driveway 1B on FM 973 should provide one lane in and one lanes out, with porkchop divider per TxDOT Austin District requirements. Driveway 2B on FM 973 should provide one lane in and two lanes out, designated as left-only and right-only.

Driveway 1B should also be designed to accommodate service truck access, with geometry adequate to accommodate larger truck turns into and out of the site while minimizing encroachment into opposing lanes without off-tracking.

A standard R6-1R "ONE WAY" (arrow to right) sign should be installed across from Driveway 2A on US 79 at the time the driveway is constructed. A standard R3-2 "NO LEFT TURN" sign should be posted on the southbound approach of FM 973 at and on the driveway approach to FM 973 to enforce the right-in/right-out only movements.

Calculation of Pro-rata Contribution for Traffic Signal at FM 973 at Wesley Miller Lane

The intersection of FM 197 at Wesley Miller Lane warrants signalization under existing conditions, but the Gateway development will add a marginal amount of traffic to the intersection. As of July 1, 2024, TxDOT indicated that the signal would be let for construction in January 2025. Per City of Taylor request, the developer of the Gateway project was asked to make a financial contribution towards construction of the signal based on a pro-rata share of peak hour traffic in the buildout year of the Gateway development. However, if the project is funded and letting in January 2025 to be constructed by TxDOT, the city should confirm if this pro-rata contribution is still required. If this contribution is still required the total was calculated as \$22,119 (amount of contribution paid to City of Taylor by developer, if required).

Summary of Mitigation and Responsibility

The required mitigation, and responsibility thereof, to accommodate the Gateway Development includes:

Mitigation	Trigger	Responsible Party
Add right turn deceleration lane at Driveway 2A on US 79	Upon construction and tie-in of driveway	Developer
Add right turn deceleration lane and directional porkchop at Driveway 1B (east) on FM 973	Upon construction and tie-in of driveway	Developer
Add right turn deceleration lane at Driveway 2B on FM 973	Upon construction and tie-in of driveway	Developer
Add traffic signal at Driveway 2B at FM 973	Prior to substantial completion of Phase 1 of development.	Developer
Add northbound right turn deceleration lane on FM 973 at US 79 (with associated median island and traffic signal modifications)	Prior to substantial completion of Phase 1 of development.	Developer and/or TxDOT (further coordination required)
Pro-rata contribution for traffic signal installation at FM 973 at Wesley Miller Lane	If still required by City of Taylor (projected funded and to be let by TxDOT)	Developer

This report has been prepared to assist in assessing the impacts of traffic generated by the subject development and represents the best estimates and opinion of Voigt Associates, Inc. While the traffic forecasts cannot be precise, they do represent, in our view, a reasonable expectation for the future based on the information available to us as of the date of this report. The estimates contained within this document rely on engineering assumptions and judgments which can be influenced by external circumstances and subject to changes that may materially affect the conclusions drawn herein.

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Exhibit A20-A,B,C.	Conceptual Design Drawings for Driveways and Deceleration Lanes (KHA)

SECTION 1. STUDY PURPOSE AND OBJECTIVES

This report presents the analysis and findings of a Traffic Impact Analysis performed by Voigt Associates, Inc. for a proposed multi-phase mixed-use development on the southeast corner of US 79 and Farm-to-Market Road 973 (FM 973) in Taylor, Texas. The report is the 3rd version produced, documenting slight changes to the land plan and phasing, limiting access to US 79 to only one driveway per TxDOT requirement, turn restrictions at the east FM 973 driveway, and expanding the number of hours of analysis for the traffic signal warrant of the west site driveway to FM 973.

The development is planned to consist of a mix of retail, restaurant, and residential development to include:

- Phase 1:
 - Restaurants
 - Spec Retail
 - Fast-Food/Quick-Service Restaurants
 - Convenience Store/Gas Station
 - Office Space; and
 - Hotels
- Phase 2:
 - Multifamily Mid-Rise Housing; and
 - Single-Family Attached Housing.

The anticipated time frame on development is to begin work on Phase 1 in 2024 with build-out and occupancy in 2025. Phase 2 is anticipated to begin work in 2025 with full build-out assumed for the year 2027 – with both timelines driven by permitting progress and market conditions, but with the desire to accelerate buildout as soon as possible for the entire project given overall market demand in the Taylor area..

The development is proposed to take access directly to US 79 via one driveway on the north side of the tract and with two driveways on FM 973 on the west side of the tract. Options for future access to the south and east are provided and reserved in the conceptual site plan.

Exhibit A1 shows the Site Location Map, Exhibit A2 presents the conceptual land plan, and Exhibit A3 shows the conceptual land plan with the anticipated access locations for Phase1 and Phase 2 conditions. All referenced exhibits in this report and found in Appendix A.

Scope of Study

The scope of this study included:

1. Field visit to the site to assess existing traffic conditions;
2. Conducting video-based vehicle and pedestrian traffic data collection to include weekday AM (7-9 AM) and PM (4-6 PM) time periods at:
 - a. US 79 at FM 973
 - b. US 79 at Crossover West of South Rio Grande Street

- c. US 79 at South Rio Grande Street
 - d. FM 973 at Taylor High School Driveway (in 2022 and updated in late 2023)
 - e. FM 973 at Streamwood Trail; and
 - f. FM 973 at Wesley Miller Lane.
3. Based on the proposed land plan, generate a total estimate of vehicle trips to and from the site for the weekday AM and PM peak hours for Phase 1 and Phase 1+2 of development;
 4. Distribute those trip numbers to the roadway network based on traffic patterns and existing capacity available in the surrounding roadway network;
 5. Capture traffic volumes from area developments (Castlewood subdivision and Samsung Phase 1) into background traffic estimates;
 6. Conduct level of service and capacity analysis for the study intersections and site driveways with projected access conditions with proposed development;
 7. Examine warrants for turn lanes at the study intersections and site driveways;
 8. Conduct traffic signal warrants for one site driveway on FM 973 for projected conditions with development; and
 9. Preparing recommendations for other transportation improvements or mitigation measures resulting from trips to and from the site.

Study Methodology

The study began with site visits to the area in June and July 2022, with an update in February 2024. During the site visits, general observations about the surrounding roadway network were made (existing roadway widths, traffic control, roadway alignment and speed limits). Considering the general nature of the existing roadway network and the portion of the network most likely to be impacted by the proposed developments, the effective study limits for the traffic operations level of service analysis were determined as:

- US 79 from west of FM 973 to South Rio Grande Street; and
- FM 973 from US 79 south to Wesley Miller Lane.

Included in the analysis were six existing intersections:

1. US 79 at FM 973 (signalized);
2. US 79 at Crossover West of South Rio Grande Street (yield);
3. US 79 at South Rio Grande Street (two-way stop);
4. FM 973 at Taylor High School north driveway (one-way stop);
5. FM 973 at Streamwood Trail (one-way stop);
6. FM 973 at Wesley Miller Lane (two-way stop).

Also included were the three proposed site driveways (see Exhibit A3):

1. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973.
2. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and

3. Driveway 2B, a full-access driveway on FM 973 located about 520' south of Driveway 1B and 650' north of the Taylor High School north site driveway.

These intersections were confirmed in a traffic study scoping meeting with Kathy Smith, P.E. (of HDR, representing the City of Taylor) on May 31, 2022.

The next step in the analysis was to estimate the number of trips generated by the proposed development. The Institute of Transportation Engineer's *Trip Generation Handbook, 11th Edition* (1) was used to develop estimates of trips generated by the development. Specific assumptions made in the trip generation analysis are presented in Section 5 of this report. Once trips were generated, internal capture and pass-by rates were applied (as applicable) and the estimated new trips and pass-by trips were then distributed to the roadway network.

Once projected turning movement counts and access driveway volume estimates were finalized, a computerized operational analysis was undertaken using techniques outlined in the *Highway Capacity Manual* (HCM) (2). The computerized traffic simulation software suite Synchro 11.1 was used to complete the operational analysis. The macroscopic tool Synchro was used to report HCM level of service for major and minor intersections and microscopic tool SimTraffic was used to assess queue performance in the network.

SECTION 2. DESCRIPTION OF SITE AND STUDY AREA

The proposed development is located on the southeast quadrant of the intersection of US 79 at FM 973 in Taylor, Texas on about 38 acres of land. The study area generally includes:

- US 79 from west of FM 973 to east of South Rio Grande Street; and
- FM 973 from US 79 south to south of Wesley Miller Lane.

This area includes the Castlewood residential development – a 239-unit single-family housing development on the west side of FM 973 across from Taylor High School. At the time of traffic data collection, crews were working on constructing street tie-ins for access from the development to FM 973 and homesites were being prepped for construction. This analysis provides estimates of trips generated by full buildout of the Castlewood development explicitly included as background traffic. Also included in the background traffic estimates are volumes projected to be attributed to Phase 1 of the Samsung plant. Exhibit A4-A show the locations of these two developments in relation to the subject site.

The development is proposed to have one driveway with access to US 79 and two driveways connecting to FM 973. A depiction of the study area is shown on Exhibit A3, and the site access is shown in Exhibit A4. Proposed site driveways include:

1. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973;
2. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and
3. Driveway 2B, a full-access driveway on FM 973 located about 1100' south of US 79.

The site plan is conceptual, but Driveway 2A on US 79 is a right-in/right-out driveway, so one lane in and one lane out will be adequate, with geometry provided to accommodate truck trips to the

site to service the convenience store/gas station (refueling truck) and other retail tracts within the development. Driveway 1B (east site driveway) will be a right-in/right-out only driveway and will have one lane in and one lane out. Driveway 2B (west site driveway) on FM 973 should provide a minimum of one lane in and two lanes out, designated as left-only and right-only.

Driveway 1B should also be designed to accommodate service truck access, with site engineers ensuring that the driveway design can accommodate larger truck turns into and out of the site.

The existing land use at the site is undeveloped, open land. Areas east of the proposed development are generally undeveloped land and large-lot single family residential. Directly south of the site is Taylor High School. There are new residential developments (Castlewood and Avery Glen) south and west of the high school, with potential for more residential development to the south along FM 973.

Traffic volumes during the AM peak hour in the study area are greatly influenced by Taylor High School related traffic. Projected conditions during the PM peak hour are not as critical because the high school traffic has dissipated by that time of the day. As of February 2024, the PM peak hour traffic conditions along FM 973 are being influenced by construction-related traffic associated with the Samsung plant west of the study area. The plant will have additional roadway capacity and connections to US 79 that will relieve traffic volumes on FM 973 in the study area.

The proposed development is anticipated to be developed in two primary phases which include the following land use:

- Phase 1:
 - Fast-food w/Drive Through – 4,400 SF total;
 - C-Store w/Gas – 5,600 SF;
 - Restaurants – 26,350 SF total;
 - Retail – 11,350 SF total;
 - Multifamily Housing – 45 dwelling units;
 - Live-Work Commercial (Office) – 29,000 SF total; and
 - Hotels (2) – 300 rooms total.
- Phase 2:
 - Multifamily Mid-Rise – 240 dwelling units;
 - Single-Family Attached Housing (townhomes and live/work residential) – 30 units.

The retail and services portion of the development are generally fronting the FM 973 and US 79 frontage, with office/live-work development in the center of the tract. The multifamily and attached-single family housing are located towards the southern end of the development. See Exhibit A2 for the conceptual layout of the site.

Compliance With Access Management Guidelines

TxDOT *Access Management Manual* guidelines specify that for highways with posted speed limits equal to or greater than 50 mph (as is the case on US 79 and FM 973) the required access spacing is 425'. The provided spacing at proposed site driveways are:

1. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973.
2. Driveway 1B, a right-in/right-out driveway on FM 973 located about 570' south of US 79; and
3. Driveway 2B, a full-access driveway on FM 973 located about 520' south of Driveway 1B and 650' north of the Taylor High School north site driveway.

Each of the site driveways meet the TxDOT access spacing standards. No modifications are needed to comply with the Access Management Manual guidelines. However, the requirement for speed change lanes (right turn deceleration lanes) may cause the resulting access separation distances to be greater than those per Table 2-2 of the AMM.

SECTION 3. ROADWAY CONDITIONS

The following roadways were included in the analysis:

- US 79 is an existing four-lane divided asphalt roadway with 12' travel lanes, 8' to 10' shoulders, and 45'-50' center grassy median. There are existing left turn deceleration lanes on both approaches at FM 973; the eastbound approach also includes a right-turn deceleration lane. The posted regulatory speed limit is 65 mph through the study area. US 79 is shown on the City of Taylor's *Comprehensive Plan* Transportation Plan Map as an Enhanced Regional Road and is maintained by TxDOT. US 79 is in generally good condition. A traffic signal controls the intersection with FM 973, with protected eastbound and westbound left turn movements on US 79 and split phasing for FM 973 with the low-volume private driveway on the north side of US 79.
- FM 973 is an existing three-lane undivided asphalt roadway with center two-way left turn lane along the site frontage and transitions to a two-lane roadway south of Wesley Miller Lane. The road is approximately 45'-46' wide with 12' travel lanes. The posted regulatory speed limit is 55 mph along the site frontage, transitioning to 65 mph near Taylor High School. FM 973 is shown on the Transportation Plan Map as an Enhanced Community Boulevard, functioning as an arterial roadway. FM 973 is also in generally good condition in the area.
- South Rio Grande Street is an existing two-lane undivided asphalt roadway located east of the site classified as a Neighborhood Avenue. The road is approximately 22'-24' wide with 11' travel lanes. The regulatory speed is assumed to be 30 mph but is unposted near the US 79 interchange. The South Rio Grande Street approaches to US 79 are stop-controlled with yield-controlled channelized right turns.
- The Taylor High School north driveway was included in the study area. This is a two-lane driveway, stop controlled to FM 973, that provides access to the north side of the high school campus.
- Streamwood Trail is a new two-lane local street stop-controlled to FM 973 providing access to the Castlewood subdivision.

- Wesley Miller Lane is a two-lane asphalt roadway east and west of FM 973. The east side provides access to Taylor High School and the west side to the Castlewood subdivision. The Transportation Plan includes extending Wesley Miller Lane east and west of current termini. This intersection is planned to have a traffic signal installed to accommodate school-related traffic.

Photographs of the roadway facilities included in the study are shown in Appendix F.

Collection of Existing Traffic Data

Vehicle and pedestrian turning movement counts were collected for two-hour peak periods in the weekday AM (7:00-9:00 AM) and weekday PM (4:00-6:00 PM) on Thursday, May 26, 2022 (when school was in session) at the following intersections:

- US 79 at FM 973 (signalized);
- US 79 at Crossover West of South Rio Grande Street (yield);
- FM 973 at Taylor High School north driveway (one-way stop);
- FM 973 at Streamwood Trail (one-way stop);
- FM 973 at Wesley Miller Lane (two-way stop).

After the TIA scoping meeting was conducted, the intersection of US 79 at South Rio Grande Street was added to the study area. This intersection, as well as the crossover on US 79 west of South Rio Grande Street, was collected for the same two-hour peak periods on Tuesday, June 7, 2022. Using the crossover count as a before/after point of comparison with school in and out of session, the US 79 at South Rio Grande Street intersection count was adjusted upwards by 15% in the AM peak hour and 14% in the PM peak hour for use in the analysis.

The intersection of FM 973 at Taylor High School north driveway was recounted on October 23, 2023, as a 24-hour count. This recount was used in a revised traffic signal warrant analysis for the proposed west site driveway (see Appendix B for count results).

Each approach was counted individually and the highest four 15-minute intervals for each period were identified as the peak hour for the intersection. The peak hours identified for analysis for the weekday morning and afternoon were 7:30-8:30 AM and 4:15-5:15 PM, respectively. Peak hour factors were calculated for existing intersections for each peak hour, while a typical peak hour factor of 0.92 was assumed for proposed driveways for use in the simulation analysis.

To determine what impact baseline traffic growth will have on the roadway network, a simple growth factor of 5% per year was used to estimate future traffic volumes on all roadway facilities within the study area for the Phase 1 (2025) and Phase 2 (2027) build-out conditions. Historic traffic growth rates in the area were examined through the TxDOT Stars II database. Table C7 in Appendix C shows a weighted average growth rate over the past 10 years of +2.79% for three count stations nearby. A shorter time period shows a compound annual growth rate of 10.2% determined from a count on FM 973 south of US 79. Using a growth rate this high for a 4-year projection would likely be too aggressive considering that we are explicitly including the

Castlewood subdivision in background traffic estimates, the 5% per year rate was used to project baseline traffic volumes into the 2025 and 2027 build-out years.

All turning movement count data collected is attached to this report in Appendix B. Schematic representations of the existing weekday AM and PM turning movement counts are located on Exhibits A4 and A5 in Appendix A.

SECTION 4. RECORDED OR APPROVED NEARBY DEVELOPMENT

During initial site visits in 2022, there did not appear to be any other significant development (other than the Castlewood subdivision) that would need to be explicitly captured in the background traffic volume projections for this analysis. During site visits in February 2024, it is clear that Samsung Plant construction-related traffic was influencing traffic along FM 973 until connections to US 79 west of Taylor are constructed. Typical development patterns in a suburban area such as where the development lies could be accounted for in the assumed 5% per annum growth rate.

SECTION 5. TRIP GENERATION AND TRIP DISTRIBUTION

If given the expected type of land use and its respective location within a roadway corridor (as well as some idea of access locations), a general estimation of new traffic demand generated by the development on a roadway or at an intersection may be made. This process is typically completed with the assistance of the Institute of Transportation Engineers' *Trip Generation Manual, 11th Edition* (1). The *Trip Generation Manual* is a compilation of thousands of studies of traffic characteristics generated at various land uses. The traffic projected using techniques outlined in Trip Generation must be viewed exclusively as traffic demand estimates, with some variation expected after full build-out and occupancy of the proposed development.

For this study, two analysis phases were examined (as described in Section 2 above):

- Phase 1 (2025): primarily retail/restaurant/office and hotel development; and
- Phase 2 (2027): additional residential.

The estimated trips generated by the development in each build-out phase are shown in Table 1. Detailed calculations are found in Appendix C.

The anticipated time frame on development is to begin work in late 2024 with build-out and occupancy complete of Phase 1 sometime in 2025. Phase 2 is anticipated to begin work in mid-2025 with full build-out complete in 2027. These timelines are influenced by market conditions.

Table 1. Estimated Trips for the Proposed Development.

	Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
	Total Trips	Trips*		Total Trips	Trips*	
		Entering	Exiting		Entering	Exiting
Phase 1 Only						
Total Trips, Pre-Capture/Pass-By:	993	541	451	1,007	517	491
Total Trips, Captured Within Development:	44	22	23	146	72	74
Total Trips, New & Pass-By	949	520	429	862	445	417
Total Trips, Pass-By, Existing on Roadway Network:	339	169	169	419	209	209
Total Trips, New on Roadway Network:	610	350	259	443	235	208
	Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
	Total Trips	Trips*		Total Trips	Trips*	
		Entering	Exiting		Entering	Exiting
Phase 1+2						
Total Trips, Pre-Capture/Pass-By:	1,097	566	531	1,115	582	533
Total Trips, Captured Within Development:	77	40	37	206	105	101
Total Trips, New & Pass-By	1,020	527	493	909	477	432
Total Trips, Pass-By, Existing on Roadway Network:	339	169	169	419	209	209
Total Trips, New on Roadway Network:	681	357	324	491	268	223

Trips generated by a mixed-use development may be adjusted downwards considering internal capture – or trips which occur between land uses on a multi-use site without re-entering public roadways. For this analysis there would be internal capture expected during both Phase 1 and Phase 2 of development. Table C1-A in Appendix C document internal capture calculations for Phase 1 of development. Table C3-A present internal capture calculations for Phase 1+2 of development. The method used to calculate internal capture is consistent with ITE's *Trip Generation Handbook*, and recommended method from NCHRP 684.

The internal capture adjustments applied to the trip generation estimates included:

- Phase 1:
 - AM Peak Hour:
 - Entering: - 4.0%
 - Exiting: - 5.0%
 - PM Peak Hour:
 - Entering: - 14.0%
 - Exiting: - 15.0%
- Phase 1+2:
 - AM Peak Hour:
 - Entering: - 7.0%
 - Exiting: - 7.0%
 - PM Peak Hour:
 - Entering: - 18.0%
 - Exiting: - 19.0%

The internal capture rates are lower for Phase 1 of development due to the lack of residential land use. Once residential land use is introduced in Phase 2, the internal capture rates increase as

more trips are made internal to the site. Trips designated as internal capture were subtracted from the overall trip estimates before application of pass-by trip rates.

Pass-by trips are generated by vehicles visiting the development site on their way to another destination. Pass-by rates per the *Trip Generation Handbook* were applied to land uses expected to generate pass-by trips (primarily retail land uses on the site). The land uses to which pass-by trips were assigned (and their rates – See Appendix H), included:

- 934-Fast-Food Restaurant w/Drive Through: 50% AM and 55% PM peak hours
- 945-Convenience Store/Gas Station: 76% AM and 75% PM peak hours
- 932-High-Turnover Sit-Down Restaurants: 43% PM peak hour only; and
- 822-Strip Retail Plaza: 40% PM peak hour only.

Once estimates of the new trips and pass-by trips generated by the development were found they were then distributed to the roadway network based on existing traffic volumes in the area, anticipated circulation within the development, and engineering judgment based on site visits and review of traffic generators in the area (retail, commercial, schools, shopping, and employment/commuter patterns).

Trip generation and distribution details are found in Appendix C. Trip generation and distribution tables include the following:

- Table C1. Trip Generation Calculations for Phase 1
- Table C1-A. AM & PM Peak Hour Internal Capture Calculations, Phase 1
- Table C2. Trip Distribution, Phase 1 Development
- Table C3. Trip Generation Calculations - Phase 1+2
- Table C3-A. AM & PM Peak Hour Internal Capture Calculations, Phase 1+2
- Table C4. Trip Distribution, Phase 1+2 Development
- Table C5. Trip Generation Calculations, Castlewood Subdivision (Background)
- Table C6. Trip Distribution, Castlewood Subdivision (Background)

Distribution percentages for **new trips** for Phase 1 and 2 were assumed to be:

- 50% To/From US 79, West of FM 973
- 20% To/From US 79, East of South Rio Grande Street
- 18% To/From FM 973, South of Wesley Miller Lane
- 1% To/From South Rio Grande Street, North of US 79
- 2% To/From South Rio Grande Street, South of US 79
- 4% To/From Streamwood Trail, West of FM 973
- 1% To/From Wesley Miller, West of FM 973
- 4% To/From Taylor HS Driveway, East of FM 973.

Exhibits A7 and A8 present the trip distribution percentages for the Phase 1 and Phase 2 build-out scenarios, respectively.

SECTION 6. PROJECTED FUTURE TRAFFIC VOLUMES

Several tables in Appendix C (and corresponding exhibits in Appendix A) present the estimated traffic volumes at the study intersections and proposed access driveways for the development. The tables and exhibits (AM+PM) summarize the following scenarios:

- **Table C8. Existing (2022) Peak Hour Turning Movement Counts (Exhibit A5+A6)**
- Table C9. Projected Peak Hour New Site Trips, Castlewood Subdivision
- **Table C10. Projected 2025 Peak Hour Turning Movement Counts (Phase 1 Build-Out Year without development) (Exhibit A9+A10)**
- **Table C11. Projected 2027 Peak Hour Turning Movement Counts (Phase 2 Build-Out Year without development) (Exhibit A11+A12)**
- Table C12. Projected Peak Hour New Site Trips, Phase 1 of Development
- Table C13. Projected Peak Hour New Site Trips, Phase 1+2 of Development
- Table C14. Projected Peak Hour Pass-By Trips, Phase 1 of Development
- Table C15. Projected Peak Hour Pass-By Trips, Phase 1+2 of Development
- **Table C16. Projected 2024 Peak Hour Turning Movement Counts (w/Phase 1 development) (Exhibit A15+A16)**
- **Table C17. Projected 2026 Peak Hour Turning Movement Counts (w/Phase 1+2 development) (Exhibit A17+A18).**

The tables in **bold text** represent the scenarios simulated, analyzed, and reported in Section 7 below.

SECTION 7. ROADWAY OPERATIONAL ANALYSIS

Synchro 11.1 was used to simulate the roadway network and intersections in the study area. Synchro is a Windows PC based solution for intersection capacity analysis and signal timing optimization. Synchro can model signalized and unsignalized intersections and calculate useful measures of effectiveness such as average vehicle delay and queue lengths. Detailed Synchro output is attached to this report in Appendix D.

Table 2 presents the per-vehicle delay thresholds that define each Highway Capacity Manual level of service (LOS). An explanation of the concept of level of service is that it is like grades in school – A is the best, F the worst. LOS is defined in terms of delay, which is a measure of driver discomfort, frustration, fuel consumption, and lost travel time. LOS is directly related to the control delay value. The LOS thresholds are different for signalized intersections as compared to unsignalized intersections, primarily because drivers expect different levels of performance from distinct types of transportation facilities. The expectation is that a signalized intersection is designed to carry higher traffic volumes than a stop-controlled intersection. Thus, a higher level of control delay is acceptable at a signalized intersection for the same level of service. At a two-way stop-controlled intersection, LOS is defined for each minor movement, but not for the intersection as a whole.

Table 2. LOS Thresholds for Signalized and Unsignalized Intersections.

Level of Service (LOS)	Signalized Intersections Control Delay Per Vehicle (sec/veh)	Unsignalized Intersections Control Delay Per Vehicle (sec/veh)
A	≤10	0-10
B	>10-20	>10-15
C	>20-35	>15-25
D	>35-55	>25-35
E	>55-80	>35-50
F	>80	>50

General access management guidelines state that if the results of the analysis showed that LOS D or worse was caused by the additional traffic generated by a development, the developer (in some cases) may need to make improvements to bring the LOS back up to D or better. If the existing level of service is E or F, and additional traffic lowers the existing level of service, the developer may be required to make improvements to maintain the existing level of service.

All simulations in this study represent the peak 15-minute time periods within any peak hour. In general, the simulation results for the weekday peak periods could be generalized to represent almost an hour of traffic volumes since the volumes should be relatively flat over the weekday peaks for intersections on public roadways. The Synchro results are summarized in Tables 3 and 4.

Simulation was generally initiated with the least amount of traffic control possible (e.g., two-way stop control before signal control). As the simulations were performed, projected operations at each intersection were evaluated as to the potential need for modification, either by additional capacity (turn lanes, added through lanes) or traffic control devices (e.g., traffic signal control) through analysis of warrants, as discussed in Sections 8 and 9. If any modifications appeared to be warranted, they were included in the simulations for build-out conditions.

The signalized intersection of US 79 at FM 973 runs fully actuated and uncoordinated and was assumed to continue fully actuated in future scenarios. Coordination along the FM 973 corridor with additional traffic signal installation may be of benefit, and offsets were optimized for scenarios with signals at Wesley Miller Lane and Driveway 2B. For use in this version of the report, traffic signal timings were obtained from TxDOT and used in the analysis. While the signal runs free, the splits total a maximum cycle length of 160 seconds.

In the scoping meeting it was discussed that a traffic signal at FM 973 at Wesley Miller Lane had been identified as a future improvement based on near-term conditions, which this analysis indicated as needed based on less-than-desirable operation as a two-way stop-controlled intersection. A traffic signal was simulated at FM 973 at Wesley Miller Lane as part of this revised study. The cycle length used at Wesley Miller was half of that at US 79 for all scenarios (based on an indication from Synchro of the optimal cycle length). Information from a July 1, 2024 meeting with TxDOT indicated that the traffic signal construction at Wesley Miller Lane is currently funded

and is scheduled to let in January 2025, so anticipated completion of the signal would be mid- to late 2025.

The traffic simulation analysis revealed that the added traffic during the AM and PM peak periods attributed to the proposed development will have marginal impacts at study intersections, with reductions in level of service attributable to background traffic growth in addition to the additional development related trips. Observations from the simulation analysis included:

- The AM peak hour is very much influenced by school-related traffic, with several intersections expected to experience undesirable LOS during that period – these are discussed in more detail below. The PM peak hour shows LOS D or better at all study intersections for all scenarios and there will less emphasis in discussion for the afternoon peak hour. As of February 2024, the PM peak hour traffic conditions along FM 973 are being influenced by construction-related traffic associated with the Samsung plant west of the study area. The plant construction will have additional roadway capacity and connections to US 79 that will relieve traffic volumes on FM 973 in the study area.
- The signalized intersection of US 79 at FM 973 will begin to see increased delays and degraded levels of service with and without traffic associated with the development, particularly in the AM peak hour:
 - With phase 1 of development, LOS moves from D to E and optimizing the signal timings does not bring LOS back to D. Added capacity would be required to improve the LOS.
 - At the request of the City of Taylor, a scenario with dual left turns northbound on FM 973 at US 79, with addition of a new right turn deceleration/turn lane on the approach, was added to the analysis. This improvement would bring LOS back to D in the buildout year of the development (with associated signal timing adjustments), inclusive of background developments (Castlewood and Phase 1 of Samsung).
 - TxDOT should consider improvements at this intersection based on projected conditions with or without the subject development, including modifications to provide northbound dual left turns from FM 973 to US 79 and adding new pavement for an associated dedicated northbound right turn lane. This improvement should be coordinated with TxDOT to determine responsibility for the improvement (either by the developer, by TxDOT, or a combination of the two parties).
- By others - signalize the intersection of FM 973 at Wesley Miller Lane as this intersection currently meets volume warrants (at least for peak hour Warrant #3) due to school related traffic. This improvement should not be the responsibility of the developer, but TxDOT (or

other entity) since it is currently warranted. This improvement lets in January 2025 and should be completed sometime in 2025.

- The site driveway on US 79 is anticipated to have LOS B operation in the AM peak hour and LOS C in the PM peak hour, typically considered acceptable for urban and suburban conditions. No mitigation for LOS or delays is required.
- Proposed Driveway 1B on FM 973 (the easternmost driveway) is required to be right-in/right-out only by TxDOT. At full buildout, this driveway approach will experience LOS C during the peak hours.
- Proposed Driveway 2B on FM 973 (the westernmost driveway) is proposed as full access, but with only stop control would be projected to be LOS F in the AM Peak hour (with D in the PM peak hour). This intersection does meet warrants for signalization (see below). Signalization improves LOS to C in the AM peak hour and LOS B in the PM peak hour.

Table 3. Weekday AM Peak Hour Simulation Results.

Intersection/Scenario	Analysis Method Reported	Traffic Control /Cycle Length (s)	AM Peak Hour										Overall Intersection Delay (s/v)	Overall Intersection LOS
			Eastbound		Westbound		Northbound		Southbound		Overall Intersection Delay (s/v)	Overall Intersection LOS		
			Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS				
1. US 79 at FM 973														
2022 Existing Conditions	HCM 2000	Free/160	35.5	C	19.9	B	46.2	D	49.2	D	32.2	C		
2025 Projected w/o Development	HCM 2000	Free/160	42.2	D	59.5	E	52.5	D	59.6	E	50.9	D		
2025 Projected w/Phase 1 Dev.	HCM 2000	Free/160	48.1	D	118.8	F	55.1	E	70.8	E	74.7	E		
2025 Projected w/Phase 1 Dev. w/opt. timings	HCM 2000	Free/140	66.0	E	44.3	D	84.5	F	69.1	E	62.6	E		
2025 Projected w/Phase 1 Dev. w/NB Dual Left	HCM 2000	Free/160	35.7	D	49.9	D	49.8	E	56.7	E	44.0	D		
2027 Projected w/o Development	HCM 2000	Free/160	48.1	D	92.2	F	52.5	D	64.6	E	65.4	E		
2027 Projected w/Phase 2 Dev.	HCM 2000	Free/160	50.5	D	138.0	F	77.6	E	72.1	E	87.8	F		
2027 Projected w/Phase 2 Dev. w/opt. timings	HCM 2000	Free/150	91.6	F	52.8	D	103.9	F	74.6	E	80.7	F		
2027 Projected w/Phase 2 Dev. w/NB Dual Left	HCM 2000	Free/160	38.8	D	70.8	E	52.4	D	59.8	E	53.3	D		
3. US 79 at Driveway 2A (Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	TWSC	---	---	---	---	11.7	B	---	---	11.7	B		
2027 Projected w/Phase 2 Dev.	HCM 6TH	TWSC	---	---	---	---	12.5	B	---	---	12.5	B		
4. US 79 at Crossover West of South Rio Grande Street														
2022 Existing Conditions	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2025 Projected w/o Development	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2025 Projected w/Phase 1 Dev.	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2027 Projected w/o Development	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2027 Projected w/Phase 2 Dev.	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
5. US 79 at South Rio Grande Street														
2022 Existing Conditions	HCM 2000	TWSC	---	---	---	---	15.1	C	15.6	C	15.6	C		
2025 Projected w/o Development	HCM 2000	TWSC	---	---	---	---	18.8	C	20.8	C	20.8	C		
2025 Projected w/Phase 1 Dev.	HCM 2000	TWSC	---	---	---	---	20.8	C	18.9	C	20.8	C		
2027 Projected w/o Development	HCM 2000	TWSC	---	---	---	---	20.3	C	22.5	C	22.5	C		
2027 Projected w/Phase 2 Dev.	HCM 2000	TWSC	---	---	---	---	23.0	C	20.4	C	23.0	C		
6. FM 973 at Driveway 1B (East Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	14.5	B	---	---	---	---	14.5	B		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	15.8	C	---	---	---	---	15.8	C		
7. FM 973 at Driveway 2B (West Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	131.9	F	---	---	---	---	131.9	F		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	209.4	F	---	---	---	---	209.4	F		
2027 Projected w/Phase 2 Dev.-signalized	HCM 6TH	80	---	---	37.9	D	18.0	B	30.1	C	26.7	C		
8. FM 973 at Taylor High School Driveway														
2022 Existing Conditions	HCM 6TH	OWSC	---	---	12.7	B	---	---	---	---	12.7	B		
2025 Projected w/o Development	HCM 6TH	OWSC	---	---	18.1	C	---	---	---	---	18.1	C		
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	21.2	C	---	---	---	---	21.2	C		
2027 Projected w/o Development	HCM 6TH	OWSC	---	---	19.9	C	---	---	---	---	19.9	C		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	24.2	C	---	---	---	---	24.2	C		
9. FM 973 at Streamwood Trail														
2022 Existing Conditions	HCM 6TH	OWSC	15.3	C	---	---	---	---	---	---	15.3	C		
2025 Projected w/o Development	HCM 6TH	OWSC	34.6	D	---	---	---	---	---	---	34.6	D		
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	45.4	E	---	---	---	---	---	---	45.4	E		
2027 Projected w/o Development	HCM 6TH	OWSC	40.4	E	---	---	---	---	---	---	40.4	E		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	57.3	F	---	---	---	---	---	---	57.3	F		
10. FM 973 at Wesley Miller Lane														
2022 Existing Conditions	HCM 2000	TWSC	10.4	B	14.0	B	---	---	---	---	14.0	B		
2025 Projected w/o Development	HCM 2000	TWSC	217.6	F	22.0	C	---	---	---	---	217.6	F		
2025 Projected w/Phase 1 Dev.	HCM 2000	TWSC	446.3	F	25.8	D	---	---	---	---	446.3	F		
2025 Projected w/Phase 1 Dev.-signalized	HCM 2000	80	31.4	C	32.6	C	31.7	C	13.9	B	20.7	C		
2027 Projected w/o Development	HCM 2000	TWSC	366.5	F	31.0	D	---	---	---	---	366.5	F		
2027 Projected w/Phase 2 Dev.	HCM 2000	TWSC	784.2	F	40.3	E	---	---	---	---	784.2	F		
2025 Projected w/Phase 1 Dev.-signalized	HCM 2000	80	31.2	C	32.6	C	37.7	D	19.7	B	25.8	C		
Signalized	TWSC - two-way stop control AWSC - all-way stop control													
Unsignalized	OWSC - one-way stop control													

Table 4. Weekday PM Peak Hour Simulation Results.

Intersection/Scenario	Analysis Method Reported	Traffic Control /Cycle Length (s)	PM Peak Hour										Overall Intersection Delay (s/v)	Overall Intersection LOS
			Eastbound		Westbound		Northbound		Southbound					
			Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS	Approach Delay (s/v)	LOS				
1. US 79 at FM 973														
2022 Existing Conditions	HCM 2000	Free/160	24.4	C	15.7	B	27.7	C	0.0	A	22.9	C		
2025 Projected w/o Development	HCM 2000	Free/160	30.1	C	20.3	C	35.1	D	0.0	A	28.9	C		
2025 Projected w/Phase 1 Dev.	HCM 2000	Free/160	36.2	D	21.9	C	54.0	D	0.0	A	38.5	D		
2025 Projected w/Phase 1 Dev. w/NB Dual Left	HCM 2000	Free/160	28.5	C	18.3	B	38.0	D	0.0	A	29.0	C		
2027 Projected w/o Development	HCM 2000	Free/160	37.0	D	33.2	C	46.1	D	0.0	A	38.3	D		
2027 Projected w/Phase 2 Dev.	HCM 2000	Free/160	41.6	D	58.9	E	52.5	D	0.0	A	49.5	D		
2027 Projected w/Phase 2 Dev. w/NB Dual Left	HCM 2000	Free/160	35.4	D	24.6	C	51.7	D	0.0	A	36.4	D		
3. US 79 at Driveway 2A (Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	TWSC	---	---	---	---	10.7	B	---	---	10.7	B		
2027 Projected w/Phase 2 Dev.	HCM 6TH	TWSC	---	---	---	---	18.5	C	---	---	18.5	C		
4. US 79 at Crossover West of South Rio Grande Street														
2022 Existing Conditions	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2025 Projected w/o Development	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2025 Projected w/Phase 1 Dev.	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2027 Projected w/o Development	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
2027 Projected w/Phase 2 Dev.	HCM 2000	OWSC	---	---	---	---	---	---	0.0	A	0.0	A		
5. US 79 at South Rio Grande Street														
2022 Existing Conditions	HCM 2000	TWSC	---	---	---	---	18.9	C	17.3	C	18.9	C		
2025 Projected w/o Development	HCM 2000	TWSC	---	---	---	---	22.5	C	20.0	C	22.5	C		
2025 Projected w/Phase 1 Dev.	HCM 2000	TWSC	---	---	---	---	29.6	D	21.2	C	29.6	D		
2027 Projected w/o Development	HCM 2000	TWSC	---	---	---	---	26.5	D	23.3	C	26.5	D		
2027 Projected w/Phase 2 Dev.	HCM 2000	TWSC	---	---	---	---	30.2	D	21.7	C	30.2	D		
6. FM 973 at Driveway 1B (East Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	16.9	C	---	---	---	---	16.9	C		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	15.0	C	---	---	---	---	15.0	C		
7. FM 973 at Driveway 2B (West Site Driveway)														
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	26.4	C	---	---	---	---	26.4	C		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	30.4	D	---	---	---	---	30.4	D		
2027 Projected w/Phase 2 Dev.-signalized	HCM 6TH	80	---	---	37.5	D	14.0	B	8.9	A	13.7	B		
8. FM 973 at Taylor High School Driveway														
2022 Existing Conditions	HCM 6TH	OWSC	---	---	11.5	B	---	---	---	---	11.5	B		
2025 Projected w/o Development	HCM 6TH	OWSC	---	---	13.6	B	---	---	---	---	13.6	B		
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	---	---	17.0	C	---	---	---	---	17.0	C		
2027 Projected w/o Development	HCM 6TH	OWSC	---	---	15.2	C	---	---	---	---	15.2	C		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	---	---	16.1	C	---	---	---	---	16.1	C		
9. FM 973 at Streamwood Trail														
2022 Existing Conditions	HCM 6TH	OWSC	0.0	A	---	---	---	---	---	---	0.0	A		
2025 Projected w/o Development	HCM 6TH	OWSC	15.9	C	---	---	---	---	---	---	15.9	C		
2025 Projected w/Phase 1 Dev.	HCM 6TH	OWSC	20.5	C	---	---	---	---	---	---	20.5	C		
2027 Projected w/o Development	HCM 6TH	OWSC	20.7	C	---	---	---	---	---	---	20.7	C		
2027 Projected w/Phase 2 Dev.	HCM 6TH	OWSC	23.7	C	---	---	---	---	---	---	23.7	C		
10. FM 973 at Wesley Miller Lane														
2022 Existing Conditions	HCM 2000	TWSC	10.2	B	11.7	B	---	---	---	---	11.7	B		
2025 Projected w/o Development	HCM 2000	TWSC	13.2	B	13.0	B	---	---	---	---	13.2	B		
2025 Projected w/Phase 1 Dev.	HCM 2000	TWSC	15.8	C	16.0	C	---	---	---	---	16.0	C		
2025 Projected w/Phase 1 Dev.-signalized	HCM 2000	80	36.9	D	37.0	D	8.0	A	5.2	A	7.7	A		
2027 Projected w/o Development	HCM 2000	TWSC	15.9	C	14.4	B	---	---	---	---	15.9	C		
2027 Projected w/Phase 2 Dev.	HCM 2000	TWSC	17.1	C	15.1	C	---	---	---	---	17.1	C		
2025 Projected w/Phase 1 Dev.-signalized	HCM 2000	80	36.9	D	37.0	D	6.8	A	7.0	A	7.6	A		
Signalized	TWSC - two-way stop control AWSC - all-way stop control													
Unsignalized	OWSC - one-way stop control													

SECTION 8. TRAFFIC SIGNAL WARRANT

The site driveway on US 79 is right-in/right-out only due to the divided highway configuration of US 79 and there is no indication that a full access with a crossover would be considered or allowed by TxDOT for this site driveway. Access to US 79 was not considered for any configuration other than right-in/right-out.

Driveway 1B on FM 973, to be constructed during Phase 1 of development will be restricted to right-in/right-out access and would not be a candidate for signalization.

Driveway 2B on FM 973, anticipated to be constructed later in Phase 1 or in the early portion of Phase 2 of development is a good candidate for signalization as it is located about 1100' south of US 79 and would be a logical location for a signal-controlled intersection providing access to the subject tract as well as to the commercially zoned tract west of FM 973 across from the site.

Per procedures outlined in the Texas Manual on Uniform Traffic Control Devices (TxMUTCD) (3), a traffic signal warrant analysis was conducted using the projected hourly volumes with full buildout of the Gateway development at the FM 973 at Driveway 2B intersection. The peak period trip estimates were added to the peak-hour traffic volume counts that were conducted on US 79 at FM 973, projected by 5% per annum to reflect 2027 conditions. This assessment also assumes that Driveway 1B is right-in/right-out only and all left turn demand to/from FM 973 occurs at Driveway 2B.

The analysis results and warrant requirements are attached in Appendix E.

A traffic signal would be warranted for the FM 973 at Driveway 2B intersection for projected conditions. The analysis showed that:

- Warrant 1 (Eight Hour) met for 13 hours (8 required)
- Warrant 2 (Four Hour) met for 12 hours (4 required)
- Warrant 3 (Peak Hour) met for 9 hours (1 required, if applicable)

We would recommend a traffic signal be installed at the developers cost prior to complete build-out of Phase 1 if TxDOT requires right-in/right-out only restrictions at Driveway 1B, but construction of the signal during Phase 2 of development regardless of access restrictions at Driveway 1B. When a traffic signal is installed at Driveway 2B pavement marking modifications to designate a southbound left turn lane would be required.

SECTION 9. ANALYSIS OF AUXILIARY LANE WARRANTS

The proposed driveways to US 79 and FM 973 were examined for the need for right turn speed change lanes using the Texas Department of Transportation (TxDOT) *Access Management Manual* and *Roadway Design Manual* guidelines. In general, a right turn deceleration lane may be needed on roadways with a posted speed limit of 45 mph or higher when right turning volumes exceed 50 vehicles per hour. The speed limit on US 79 is 65 mph and speed limit on FM 973 is 55 mph along the site frontage.

The projected peak hour right turn movements at each driveway, and if a right turn deceleration lane is warranted is summarized below:

- Driveway 2A (US 79):
 - AM Peak Hour: 284 right turns at full buildout
 - PM Peak Hour: 267 right turns at full buildout
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 1B (FM 973):
 - AM Peak Hour: 105 right turns at full buildout
 - PM Peak Hour: 116 right turns at full buildout
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 2B (FM 973):
 - AM Peak Hour: 26 right turns at full buildout
 - PM Peak Hour: 19 right turns at full buildout
 - Right turn deceleration lane not warranted by volume but required per TxDOT based on speed/safety concerns. To be constructed when tied-in to US 79.

Per Table 3-3 of the Roadway Design Manual, the deceleration lane on US 79 at Driveway 2A would have 30' storage and 415' deceleration length. The deceleration lane should be 12' wide with minimum 4' to maximum 12' shoulder (width to be determined with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards. The yield markings and lane edge markings should be refreshed for the northbound FM 973 approach to US 79 as part of the improvement. This configuration retains the current acceleration lane for the right turn movement from northbound on FM 973 to eastbound on US 79.

Per Table 3-3 of the Roadway Design Manual, the right turn deceleration lane on FM 973 at Driveway 1B would be required to have 30' storage and 415' deceleration length. Due to spacing limitations upstream to Driveway 2B, the total length provided would be 421' length with 76' taper. The right turn deceleration lane on FM 973 at Driveway 2B provides the RDM required 445' long deceleration lane with 100' taper.

Both deceleration lanes on FM 973 should be 12' wide with minimum 4' shoulder (width to be confirmed with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards.

SECTION 10. FINDINGS AND RECOMMENDATIONS

This section of the report summarizes the overall impact of the Taylor Gateway development and includes discussion about recommended site access, traffic control, and other improvements or operations issues that should be addressed as the development is built-out and occupied.

Traffic Operations Analysis

The traffic simulation analysis revealed that the added traffic during the AM and PM peak periods attributed to the proposed development will have marginal impacts at study intersections, with

reductions in level of service attributable to background traffic growth in addition to the additional development related trips. Observations from the simulation analysis included:

- The AM peak hour is very much influenced by school-related traffic, with several intersections expected to experience undesirable LOS during that period – these are discussed in more detail below. The PM peak hour shows LOS D or better at all study intersections for all scenarios and there will be less emphasis in discussion for the afternoon peak hour. As of February 2024, the PM peak hour traffic conditions along FM 973 are being influenced by construction-related traffic associated with the Samsung plant west of the study area. The plant construction will have additional roadway capacity and connections to US 79 that will relieve traffic volumes on FM 973 in the study area.
- The signalized intersection of US 79 at FM 973 will begin to see increased delays and degraded levels of service with and without traffic associated with the development, particularly in the AM peak hour:
 - With phase 1 of development, LOS moves from D to E and optimizing the signal timings does not bring LOS back to D. Added capacity would be required to improve the LOS.
 - At the request of the City of Taylor, a scenario with dual left turns northbound on FM 973 at US 79, with addition of a new right turn deceleration/turn lane on the approach, was added to the analysis. This improvement would bring LOS back to D in the buildout year of the development (with associated signal timing adjustments), inclusive of background developments (Castlewood and Phase 1 of Samsung).
 - TxDOT should consider improvements at this intersection based on projected conditions with or without the subject development, including modifications to provide northbound dual left turns from FM 973 to US 79 and adding new pavement for an associated dedicated northbound right turn lane. This improvement should be coordinated with TxDOT to determine responsibility for the improvement (either by the developer, by TxDOT, or a combination of the two parties).
- By others - signalize the intersection of FM 973 at Wesley Miller Lane as this intersection currently meets volume warrants (at least for peak hour Warrant #3) due to school related traffic. This improvement should not be the responsibility of the developer, but TxDOT (or other entity) since it is currently warranted. This improvement lets in January 2025 and should be completed sometime in 2025.

- The site driveway on US 79 is anticipated to have LOS B operation in the AM peak hour and LOS C in the PM peak hour, typically considered acceptable for urban and suburban conditions. No mitigation for LOS or delays is required.
- Proposed Driveway 1B on FM 973 (the easternmost driveway) is required to be right-in/right-out only by TxDOT. At full buildout, this driveway approach will experience LOS C during the peak hours.
- Proposed Driveway 2B on FM 973 (the westernmost driveway) is proposed as full access, but with only stop control would be projected to be LOS F in the AM Peak hour (with D in the PM peak hour). This intersection does meet warrants for signalization (see below). Signalization improves LOS to C in the AM peak hour and LOS B in the PM peak hour.

Compliance With Access Management Guidelines

TxDOT *Access Management Manual* guidelines specify that for highways with posted speed limits equal to or greater than 50 mph (as is the case on US 79 and FM 973) the required access spacing is 425'. The provided spacing at proposed site driveways are:

4. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973;
5. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and
6. Driveway 2B, a full-access driveway on FM 973 located about 520' south of Driveway 1B and 650' north of the Taylor High School north site driveway.

Each of the site driveways meet the TxDOT access spacing standards. No modifications are needed to comply with the Access Management Manual guidelines. However, the requirement for speed change lanes (right turn deceleration lanes) may cause the resulting access separation distances to be greater than those per Table 2-2 of the AMM.

Analysis of Auxiliary Lane Warrants

The proposed driveways to US 79 and FM 973 were examined for the need for right turn speed change lanes using the Texas Department of Transportation (TxDOT) *Access Management Manual* and *Roadway Design Manual* guidelines. In general, a right turn deceleration lane may be needed on roadways with a posted speed limit of 45 mph or higher when right turning volumes exceed 50 vehicles per hour. The speed limit on US 79 is 65 mph and speed limit on FM 973 is 55 mph along the site frontage. If a right turn deceleration lane is warranted is summarized below:

- Driveway 2A (US 79):
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 1B (FM 973):
 - Right turn deceleration lane warranted, constructed when tied-in to US 79
- Driveway 2B (FM 973):
 - Right turn deceleration lane marginally warranted by volume but required per TxDOT based on speed/safety concerns. To be constructed when tied-in to US 79.

Per Table 3-3 of the Roadway Design Manual, the deceleration lane on US 79 at Driveway 2A would have 30' storage and 415' deceleration length. The deceleration lane should be 12' wide with minimum 4' to maximum 12' shoulder (width to be determined with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards. The yield markings and lane edge markings should be refreshed for the northbound FM 973 approach to US 79 as part of the improvement. This configuration retains the current acceleration lane for the right turn movement from northbound on FM 973 to eastbound on US 79.

Per Table 3-3 of the Roadway Design Manual, the right turn deceleration lane on FM 973 at Driveway 1B would be required to have 30' storage and 415' deceleration length. Due to spacing limitations upstream to Driveway 2B, the total length provided would be 421' length with 76' taper. The right turn deceleration lane on FM 973 at Driveway 2B provides the RDM required 445' long deceleration lane with 100' taper. Both deceleration lanes on FM 973 should be 12' wide with minimum 4' shoulder (width to be confirmed with TxDOT coordination) and be designated with pavement markings and signing to TxDOT standards.

Traffic Signal Warrant

Driveway 2B on FM 973 was found to meet warrants for signalization at full buildout (and likely earlier) of the site. Per procedures outlined in the Texas Manual on Uniform Traffic Control Devices (TxMUTCD) (3), a traffic signal warrant analysis was conducted using the projected hourly volumes with development at the FM 973 at Driveway 2B intersection. The analysis results and warrant requirements are attached in Appendix E. The analysis showed:

- Warrant 1 (Eight Hour) met for 13 hours (8 required)
- Warrant 2 (Four Hour) met for 12 hours (4 required)
- Warrant 3 (Peak Hour) met for 9 hours (1 required, if applicable)

We would recommend a traffic signal be installed at the developers cost prior to complete build-out of Phase 1.

Access Driveway Recommendations

The development is proposed to have one driveway with access to US 79 and two driveways connecting to FM 973. Proposed site driveways include:

4. Driveway 2A, a right-in/right-out driveway on US 79 located about 1070' east of FM 973;
5. Driveway 1B, a full-access driveway on FM 973 located about 570' south of US 79; and
6. Driveway 2B, a full-access driveway on FM 973 located about 1100' south of US 79.

The site plan is conceptual, but Driveway 2A on US 79 is right-in/right-out, so one lane in and one lane out will be adequate, with geometry provided to accommodate truck trips to the site to service the convenience store/gas station (refueling truck) and other retail developments within the site at Driveway 2A (likely 35' width with 40' radii). Driveway 1B on FM 973 should provide one lane in and one lanes out, with porkchop divider per TxDOT Austin District requirements. Driveway 2B on FM 973 should provide one lane in and two lanes out, designated as left-only and right-only.

Driveway 1B should also be designed to accommodate service truck access, with geometry adequate to accommodate larger truck turns into and out of the site while minimizing encroachment into opposing lanes without off-tracking.

A standard R6-1R “ONE WAY” (arrow to right) sign should be installed across from Driveway 2A on US 79 at the time the driveway is constructed. A standard R3-2 “NO LEFT TURN” sign should be posted on the southbound approach of FM 973 at and on the driveway approach to FM 973 to enforce the right-in/right-out only movements.

Calculation of Pro-rata Contribution for Traffic Signal at FM 973 at Wesley Miller Lane

The intersection of FM 197 at Wesley Miller Lane warrants signalization under existing conditions, but the Gateway development will add a marginal amount of traffic to the intersection. As of July 1, 2024, TxDOT indicated that the signal would be let for construction in January 2025. Per City of Taylor request, the developer of the Gateway project was asked to make a financial contribution towards construction of the signal based on a pro-rata share of peak hour traffic in the buildout year of the Gateway development. However, if the project is funded and letting in January 2025 to be constructed by TxDOT, the city should confirm if this pro-rata contribution is still required.

If this contribution is still required, the calculation is based on the total of AM and PM peak hour traffic in the Gateway buildout year with development traffic versus the buildout year traffic estimates without the Gateway development. These values come from Tables C11 (without development) and C17 (with development) in Appendix C. With a \$300,000 estimated cost of construction of a signal at the intersection, the calculation is as follows:

Percentage Increase =

$$[(\text{AM+PM intersection volumes with development}) - (\text{AM+PM intersection volumes without development})] / [(\text{AM+PM intersection volumes without development}) \times 100].$$

$$[(3243 - 3020) / 3020] \times 100 = 7.37\%$$

Calculation of pro-rata calculation:

$$7.37\% \times \$300,000 = \$22,119 \text{ (amount of contribution paid to City of Taylor by developer, if required).}$$

Summary of Mitigation Required

The required mitigation, and responsibility thereof, to accommodate the Gateway Development includes:

Mitigation	Trigger	Responsible Party
Add right turn deceleration lane at Driveway 2A on US 79	Upon construction and tie-in of driveway	Developer
Add right turn deceleration lane and directional porkchop at Driveway 1B (east) on FM 973	Upon construction and tie-in of driveway	Developer
Add right turn deceleration lane at Driveway 2B on FM 973	Upon construction and tie-in of driveway	Developer
Add traffic signal at Driveway 2B at FM 973	Prior to substantial completion of Phase 1 of development.	Developer
Add northbound right turn deceleration lane on FM 973 at US 79 (with associated median island and traffic signal modifications)	Prior to substantial completion of Phase 1 of development.	Developer and/or TxDOT (further coordination required)
Pro-rata contribution for traffic signal installation at FM 973 at Wesley Miller Lane	If still required by City of Taylor (projected funded and to be let by TxDOT)	Developer

REFERENCES

1. Trip Generation Handbook Trip Generation, 11th Edition, Institute of Transportation Engineers, Washington, DC, via website.
2. Highway Capacity Manual, 6th Edition Transportation Research Board, National Research Council, Washington, DC, 2016.
3. Texas Manual of Uniform Traffic Control Devices, Texas Department of Transportation, 2011.

Appendix A – Exhibits



Exhibit A1. Site Location Map.
North to top of page, Not to Scale.



Lot	Land Use	Bldg. Sq. Ft. / Units	Parking Ratio	Required Parking	Provided Parking
1	Retail/Restaurant	4,400 Sq. Ft.	1/200	22 spaces	*41 spaces
2,3	Gas Station/Convenience Store	5,200 Sq. Ft.	1/100	52 spaces	*69 spaces
4,5	Retail/Restaurant	22,700 Sq. Ft.	1/200	114 spaces	*112 spaces
6	Hotel	150 rooms	0.89/room	134 spaces	*134 spaces
7	Hotel	150 rooms	0.89/room	134 spaces	35 on street spaces 128 parking lot spaces *120 spaces total
8,9	Retail/Restaurant	15,000 Sq. Ft.	1/200	75 spaces	
10	Multi-Family Residential	16 units	1.7/unit	27 spaces	
	Live-Work Residential	12 units	2/unit	24 spaces	*72 spaces total
11	Live-Work Commercial	12,000 Sq. Ft.	3/1000	36 spaces	
	Live-Work Residential	17 units	2/unit	34 spaces	*91 spaces total
12	Live-Work Commercial	17,000 Sq. Ft.	3/1000	51 spaces	
	Multi-Family Residential	24 units	1.7/unit	41 spaces	*52 spaces
13	Multi-Family Residential	144 units	1.7/unit	245 spaces	*243 spaces
14,15	Multi-Family Residential	72 units	1.7/unit	122 spaces	*128 spaces
16,17	Townhome	30 units	2/unit	60 spaces	*60 spaces
				1171 spaces	1285 spaces

* Conceptual parking count only, site plans are needed to verify.



SEC Planning, LLC

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MARKETING EXHIBIT

GATEWAY

TAYLOR, TEXAS

Exhibit A2. Proposed Site Plan.



0 100' 200' 400'

North

Scale: 1" = 200'

Date: October 05, 2023

SHEET FILE: I:\210152-GAOZ\Cadfiles\PLANNING\Exhibits\Concept C Marketing Exhibit.dwg

Base mapping compiled from best available information. All map data should be considered as preliminary, in need of verification, and subject to change. This land plan is conceptual in nature and does not represent any regulatory approval. Plan is subject to change.



Exhibit A3. Conceptual Site Layout with Preliminary Access Locations.
North to top of page, not to scale.



Exhibit A4. Study Area and Intersection Control Status.
North to top of page, not to scale.

○ Study Intersections

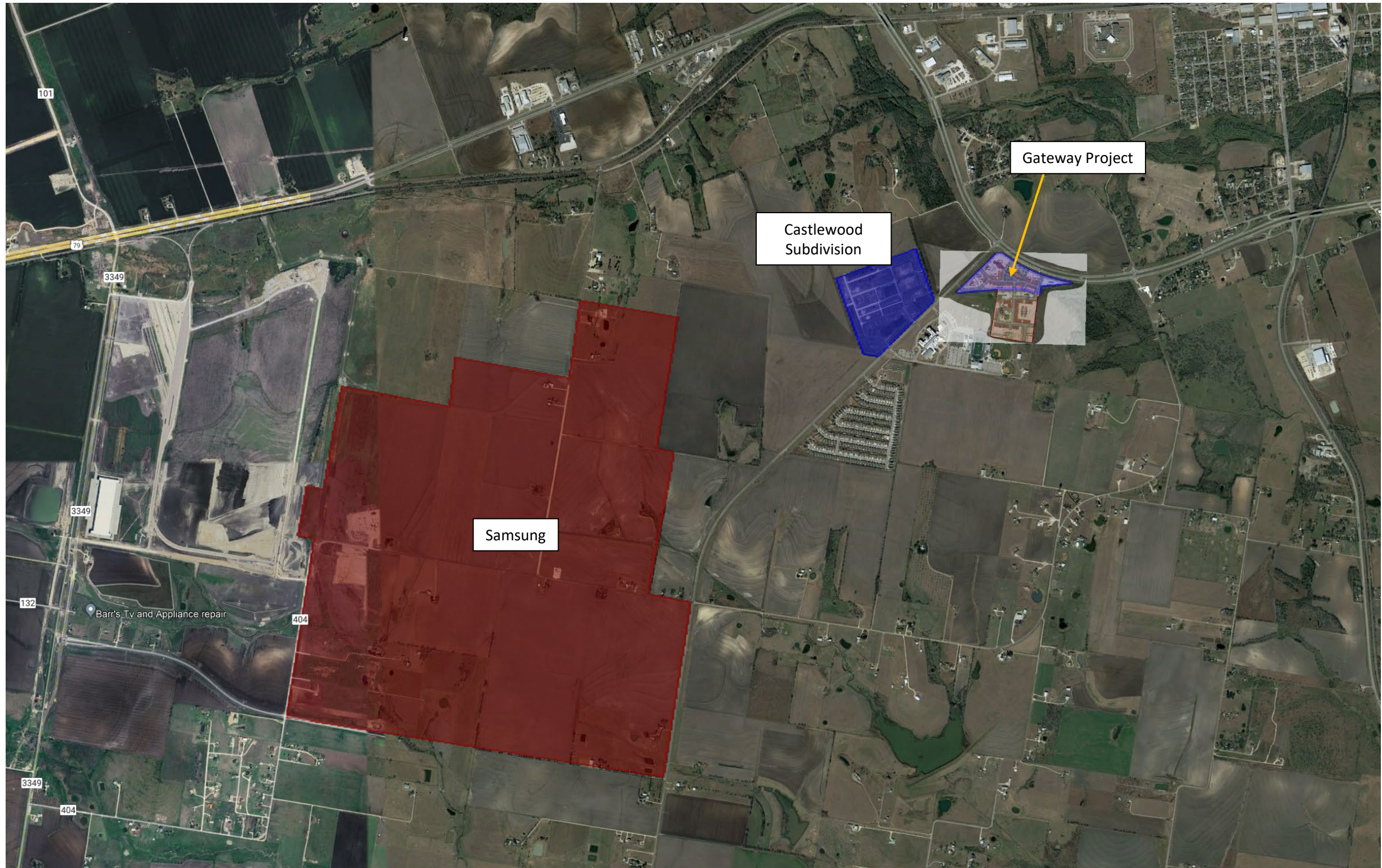
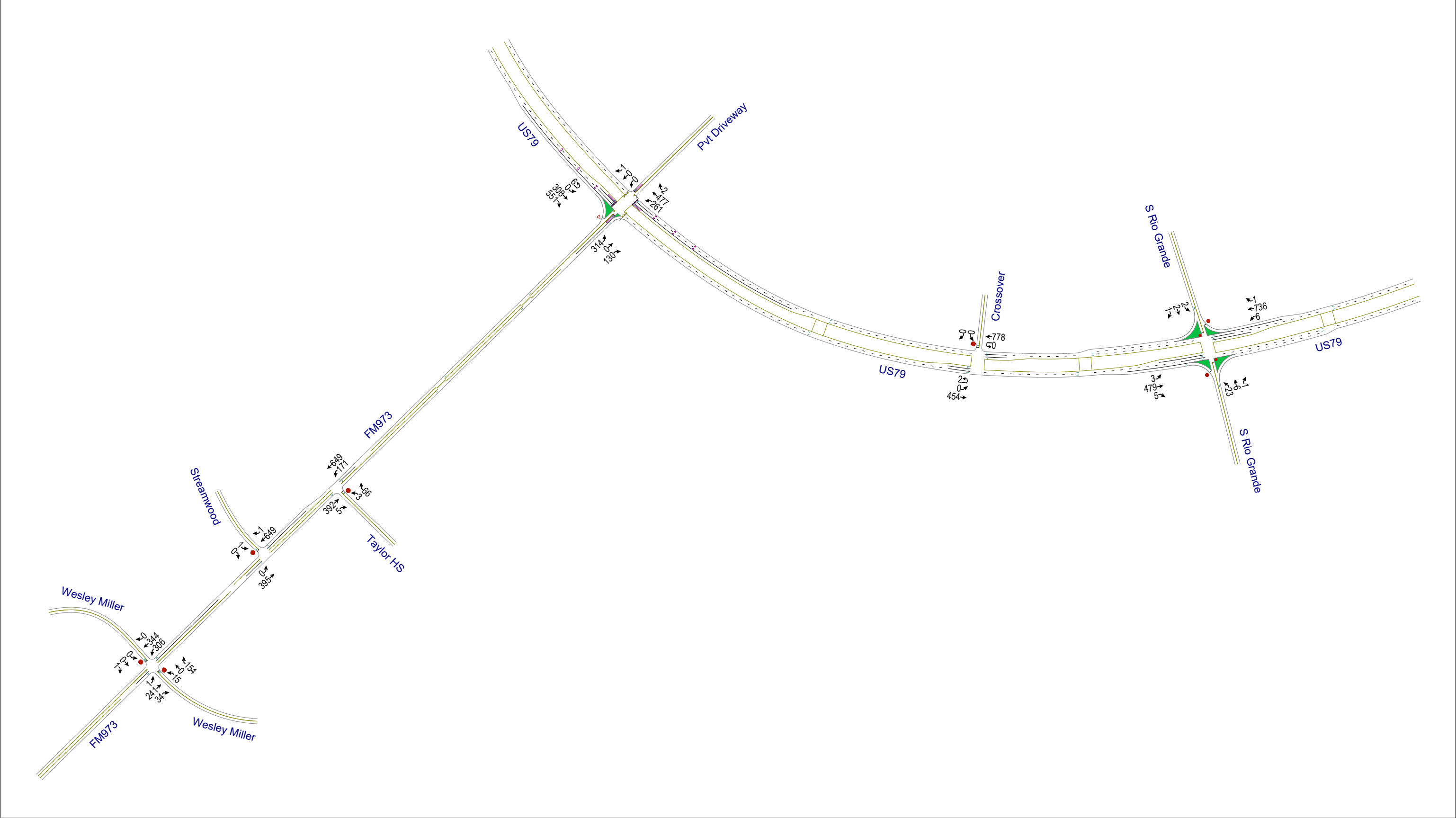
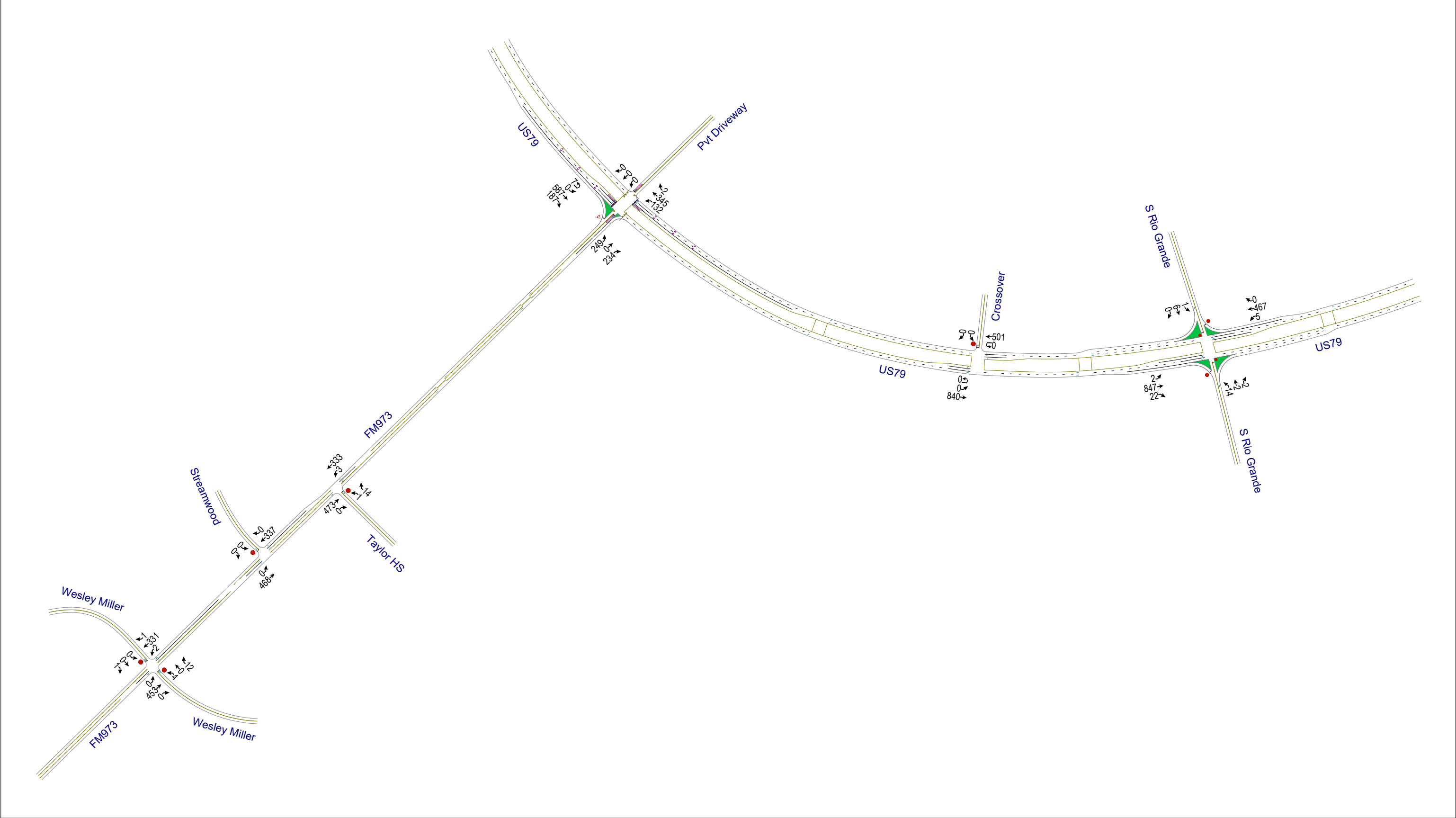


Exhibit A4-A. Background Developments In The Study Area
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Not To Scale
North to Top of Page



Not To Scale
North to Top of Page

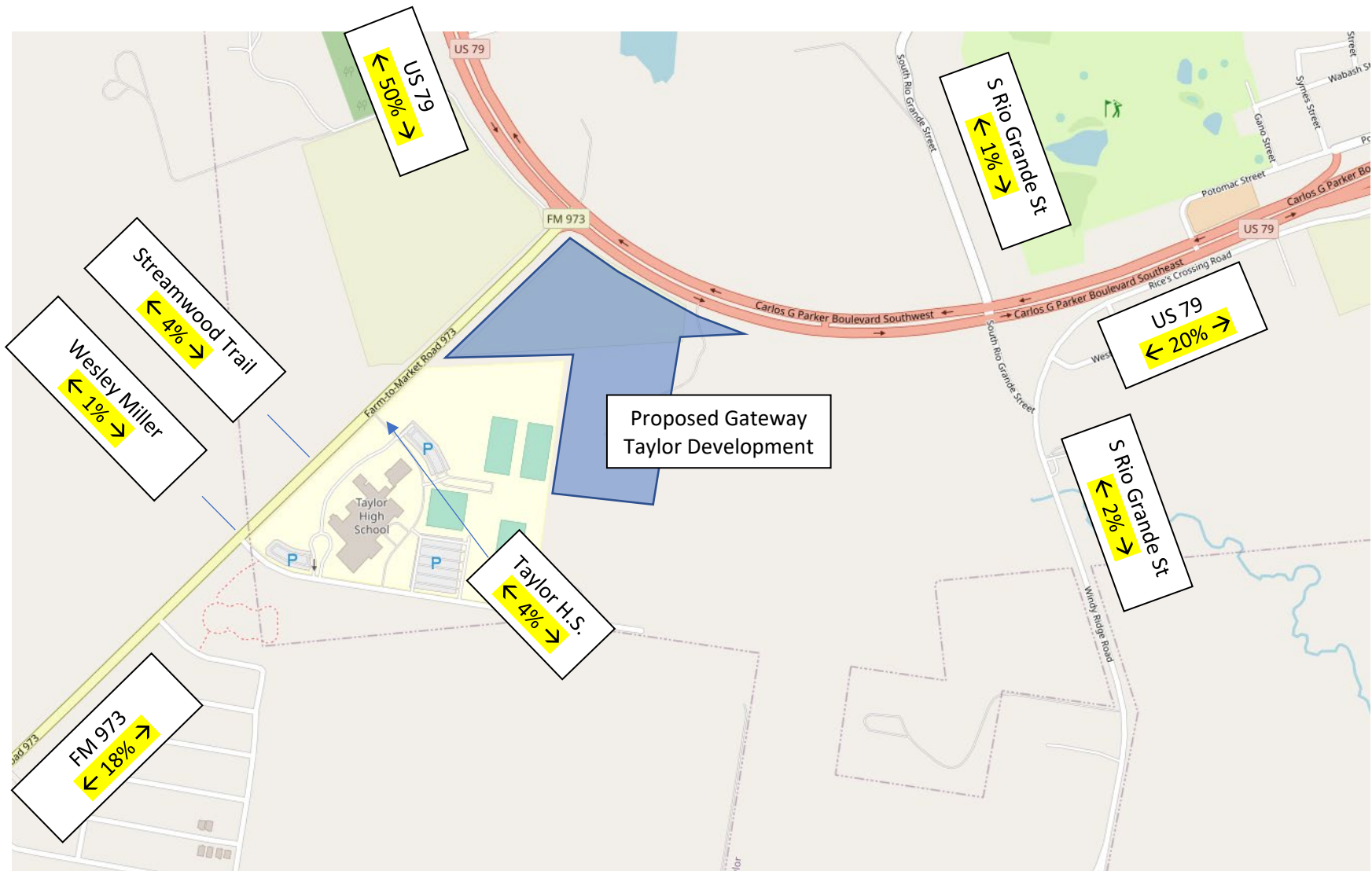


Exhibit A7. Trip Distribution: AM & PM Peak Origins/Destinations (Phase 1 Build-Out).

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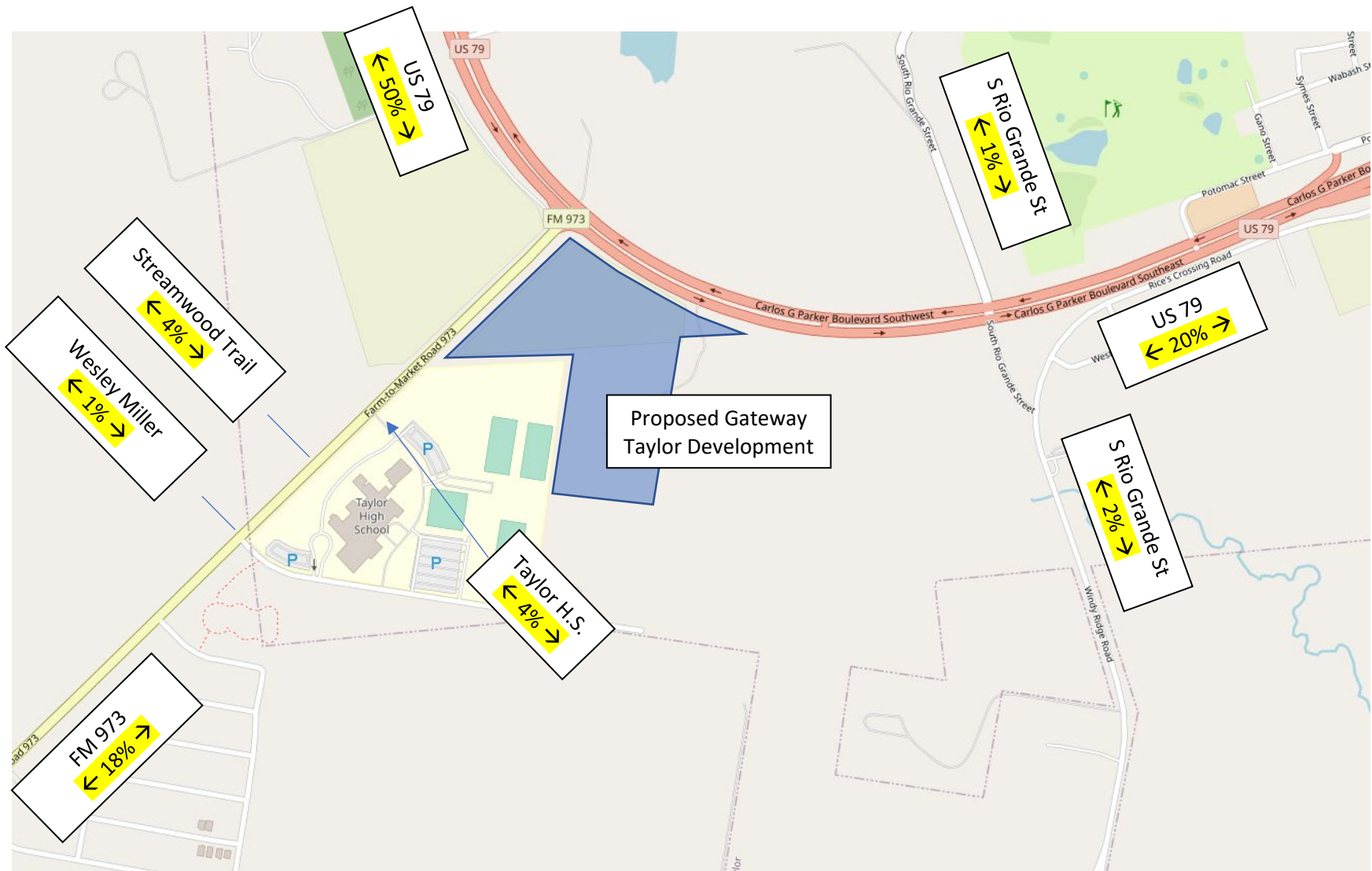
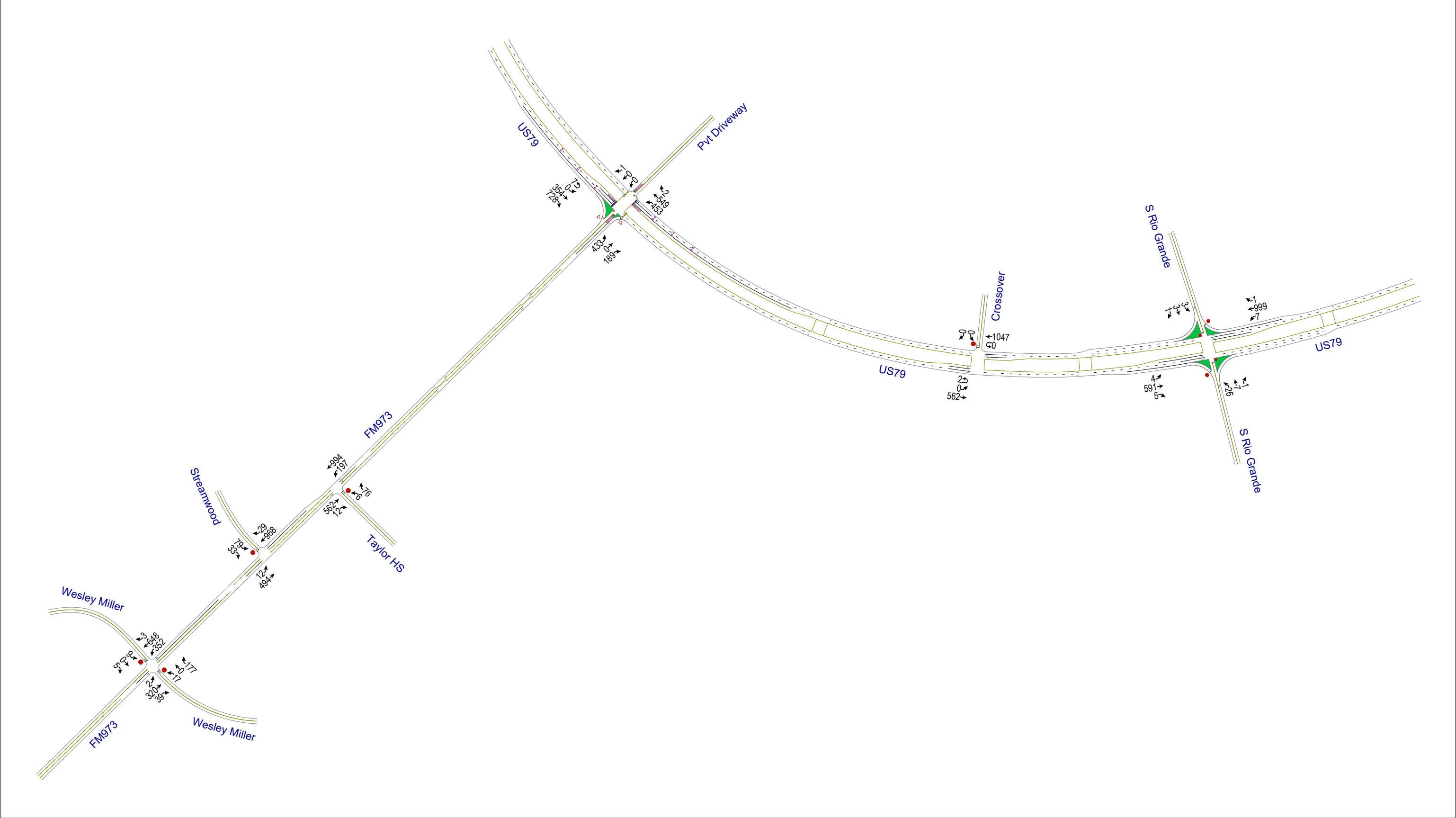
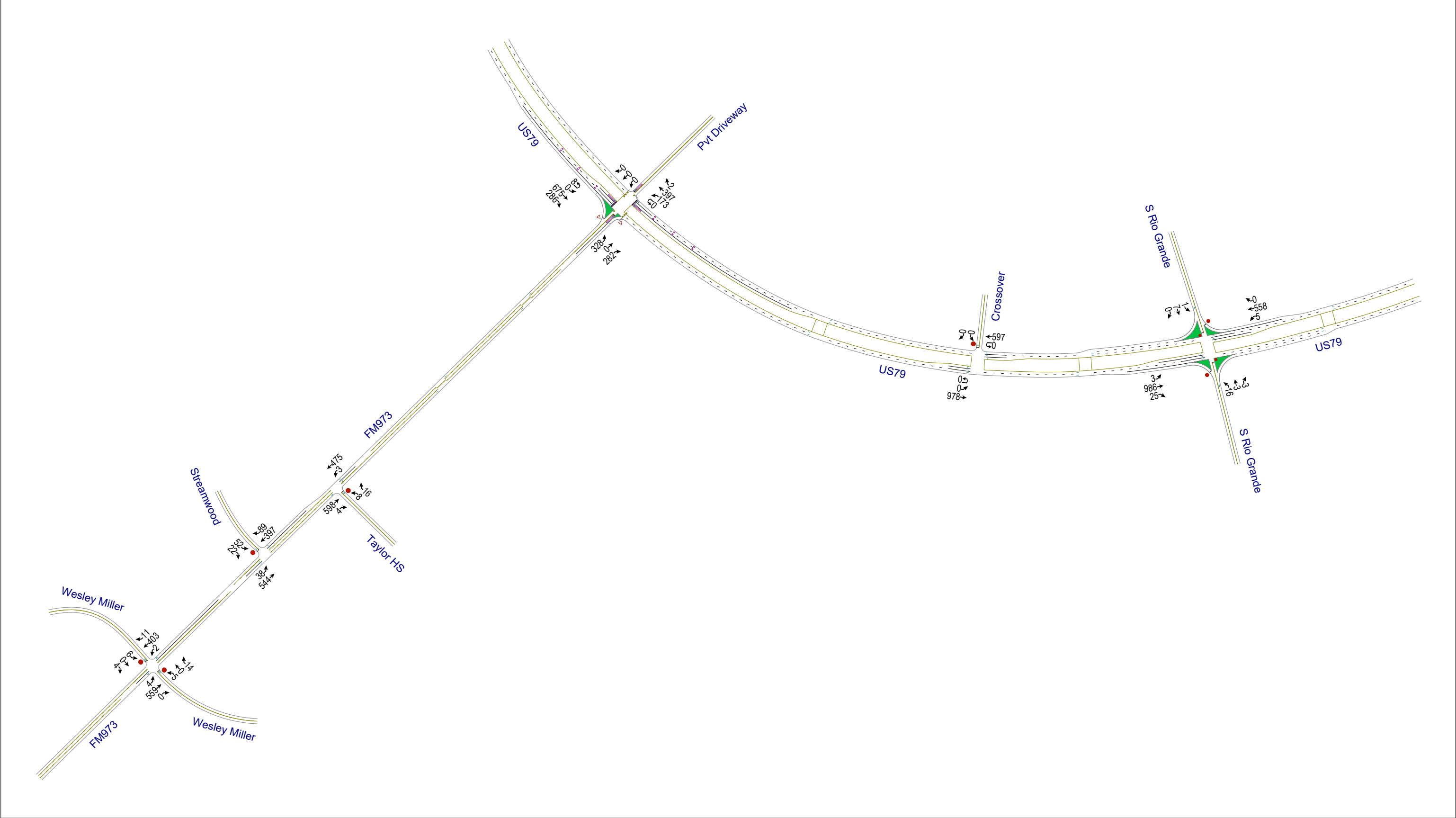


Exhibit A8. Trip Distribution: AM & PM Peak Origins/Destinations (Phase 2 Build-Out).

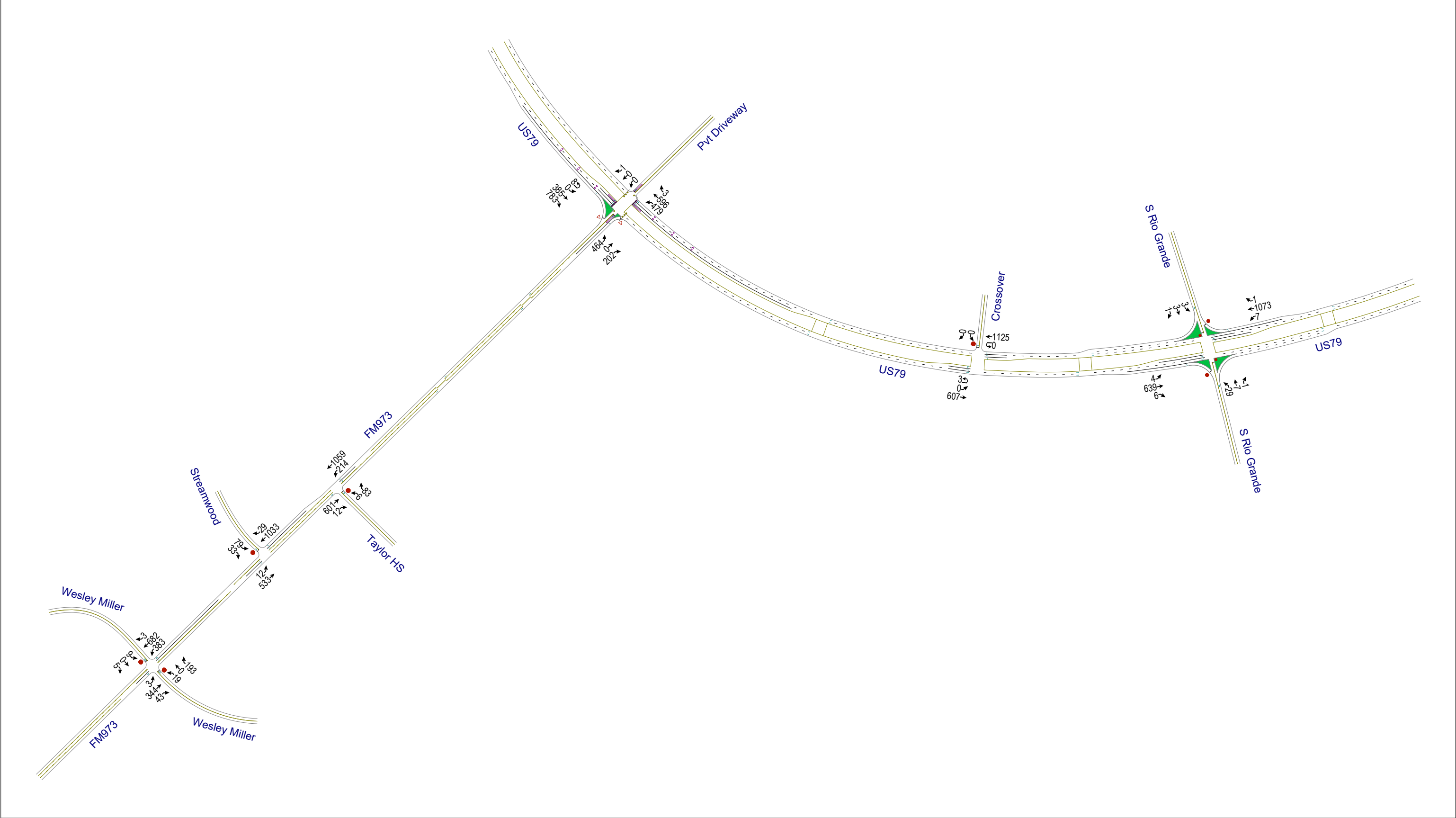
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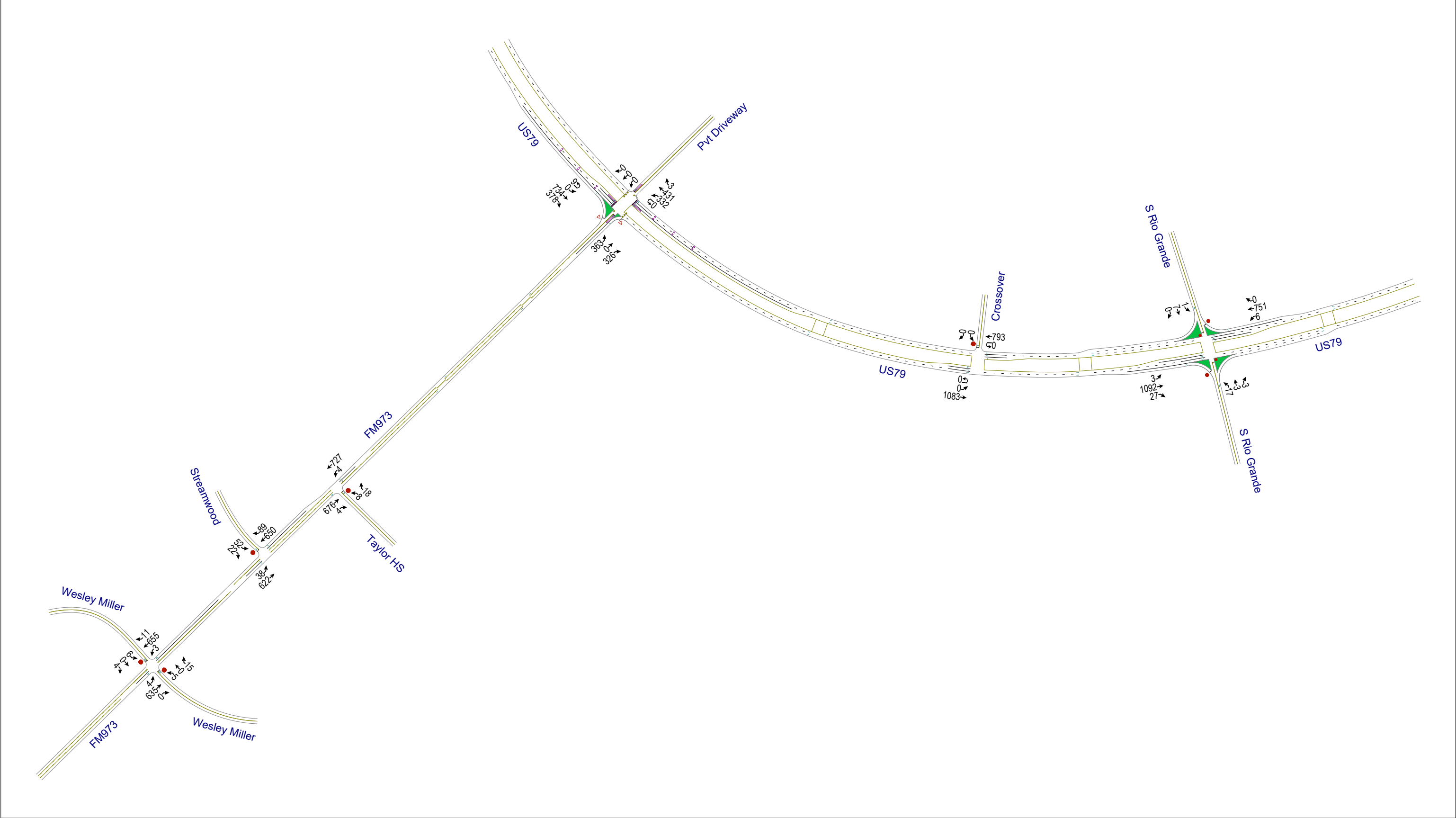
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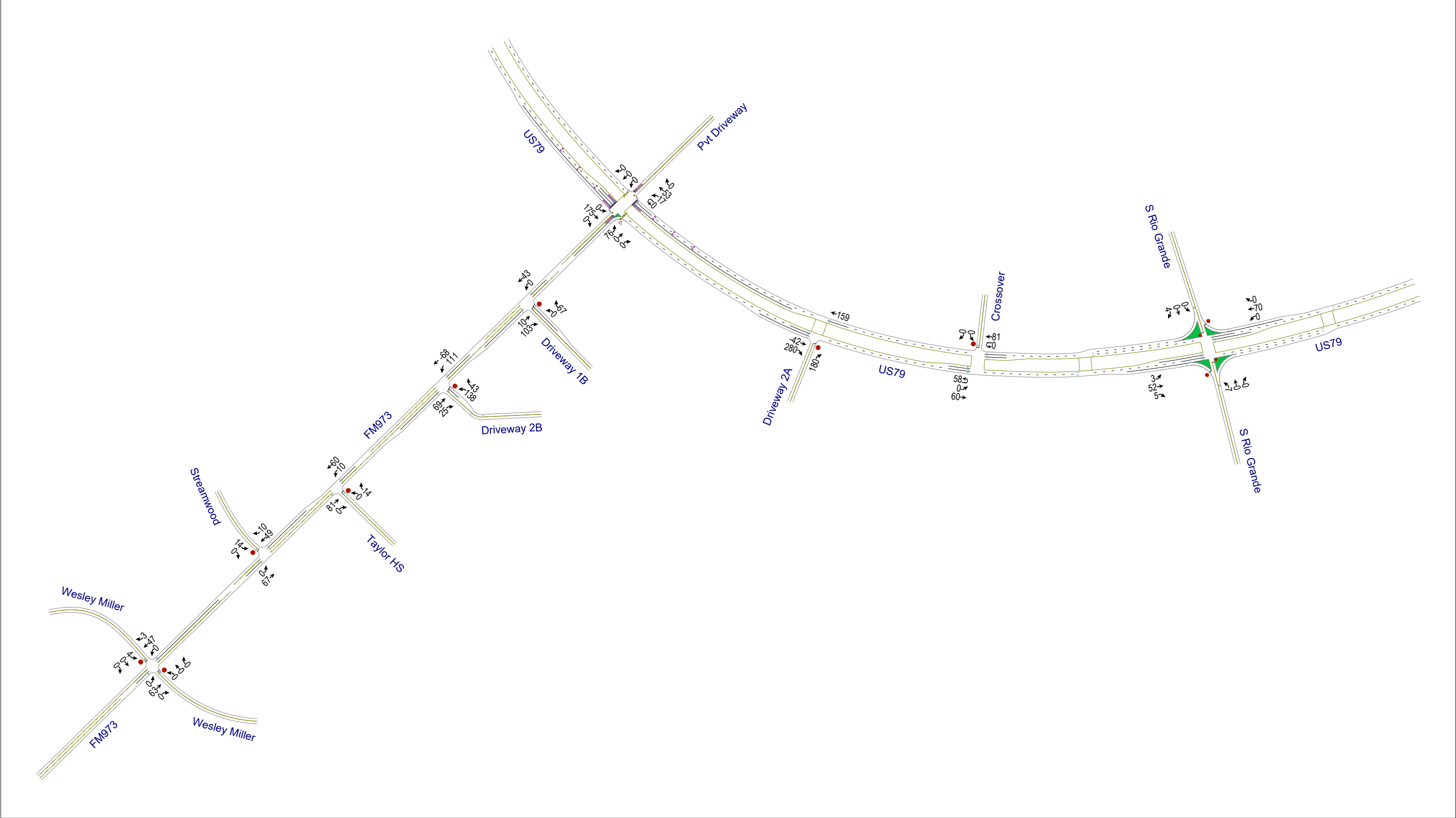
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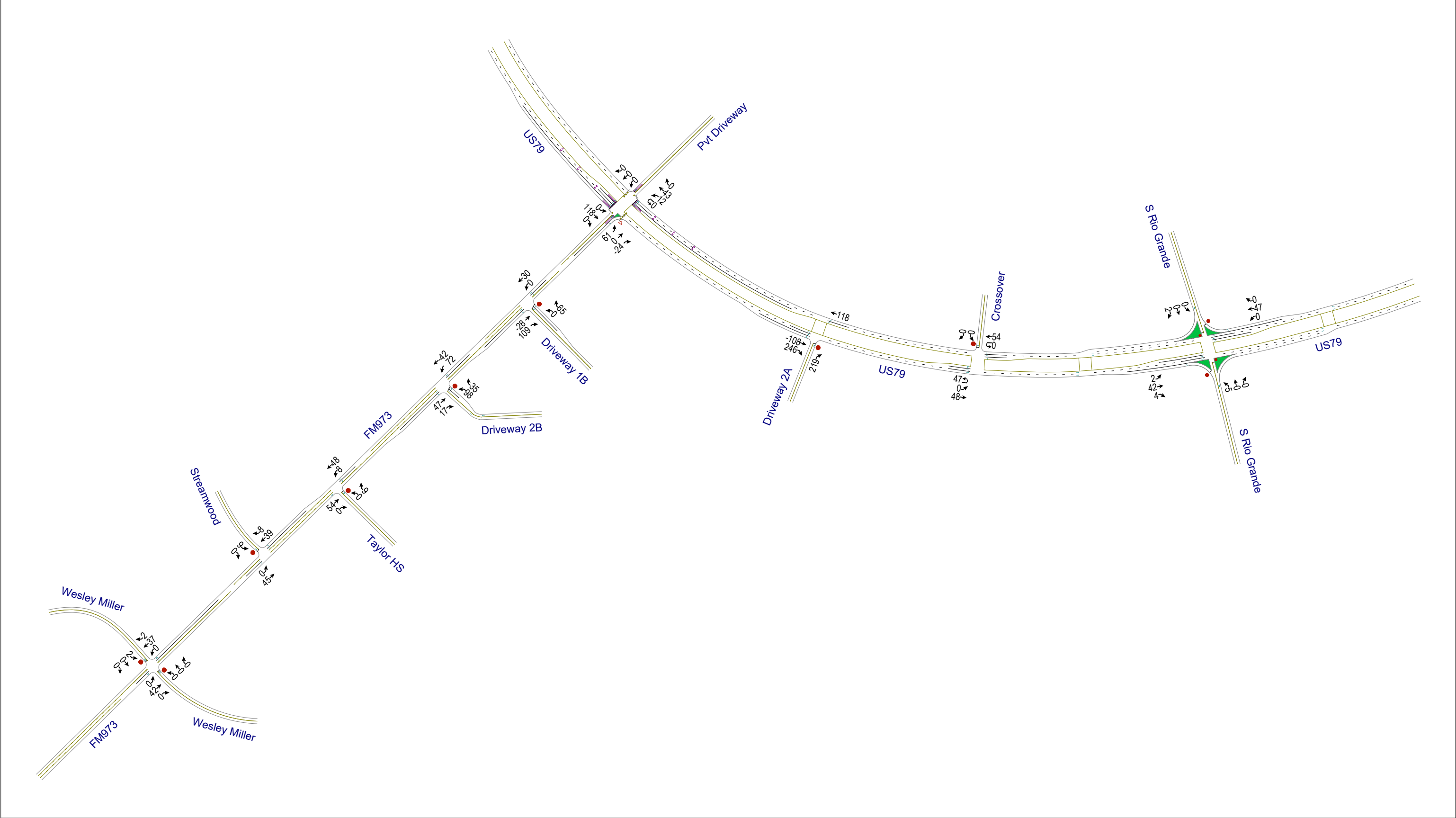
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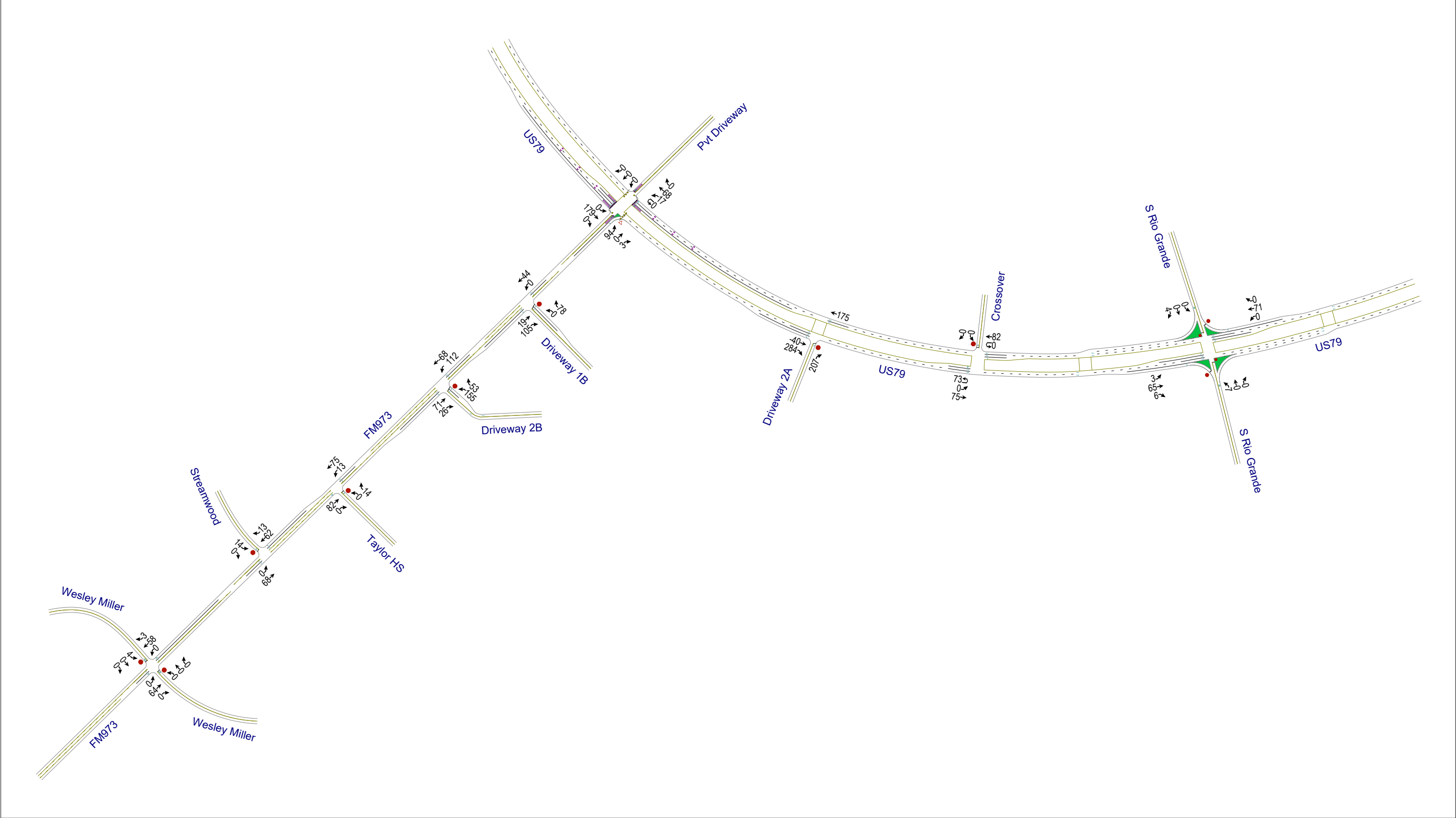
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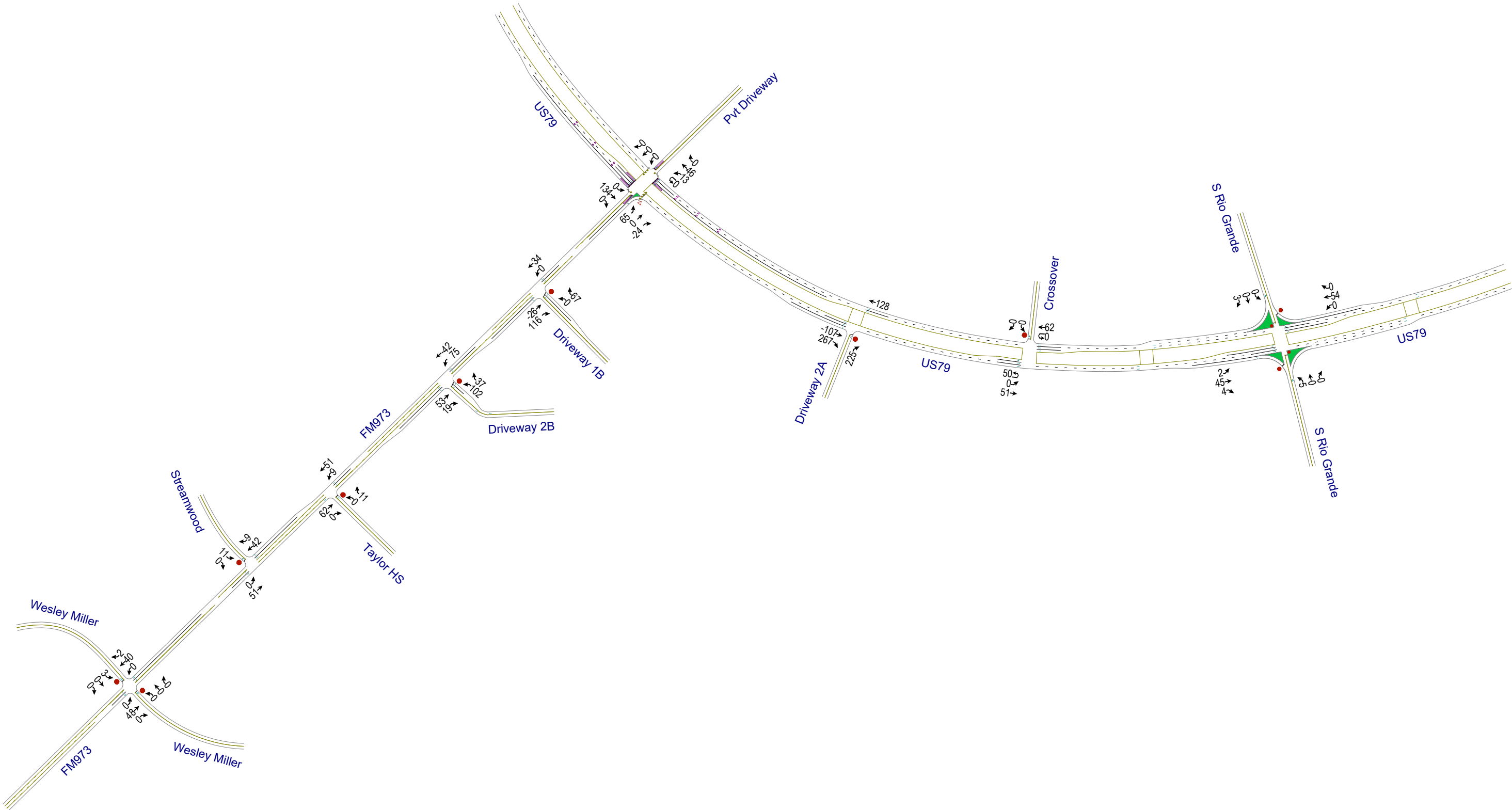
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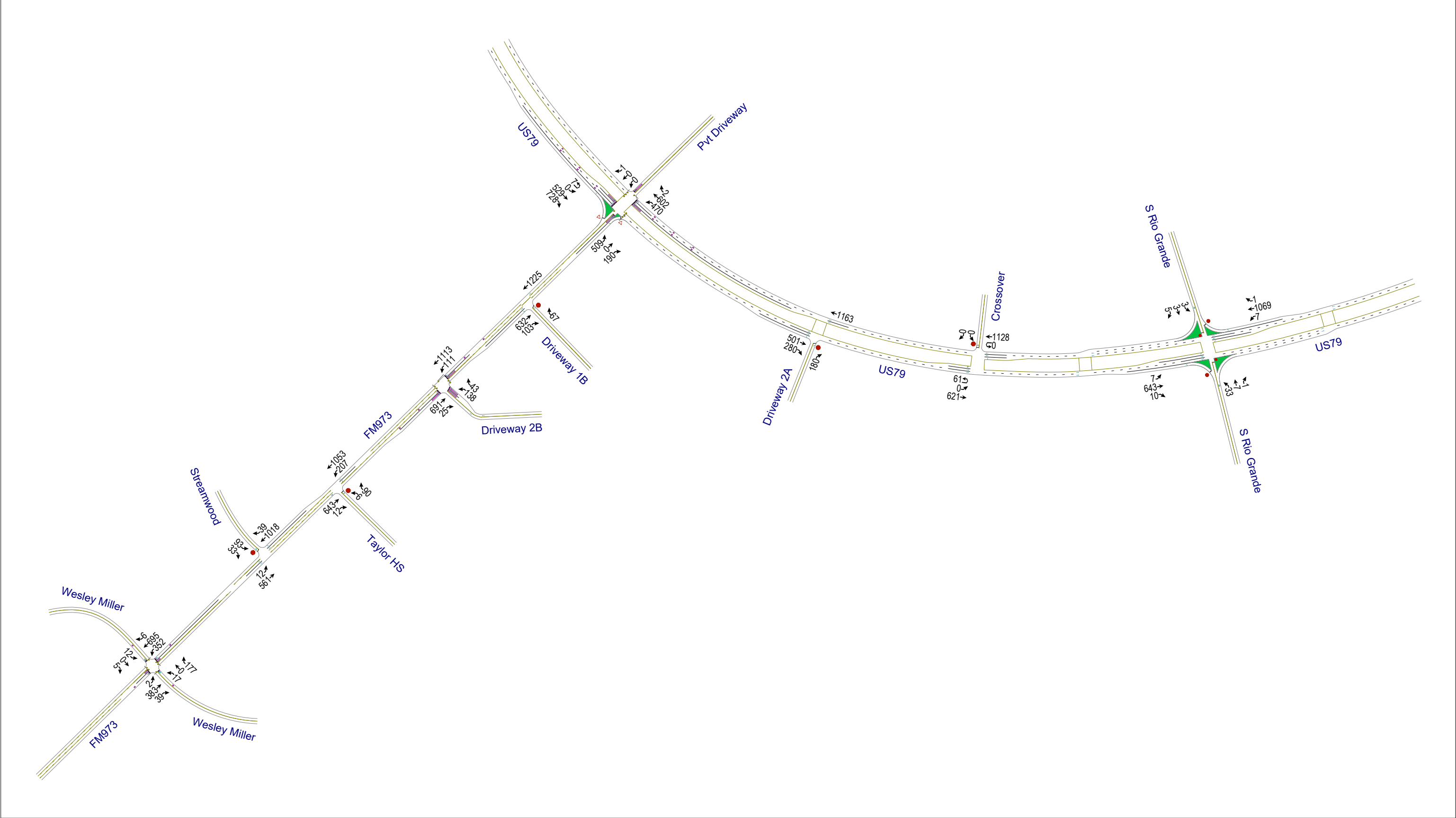
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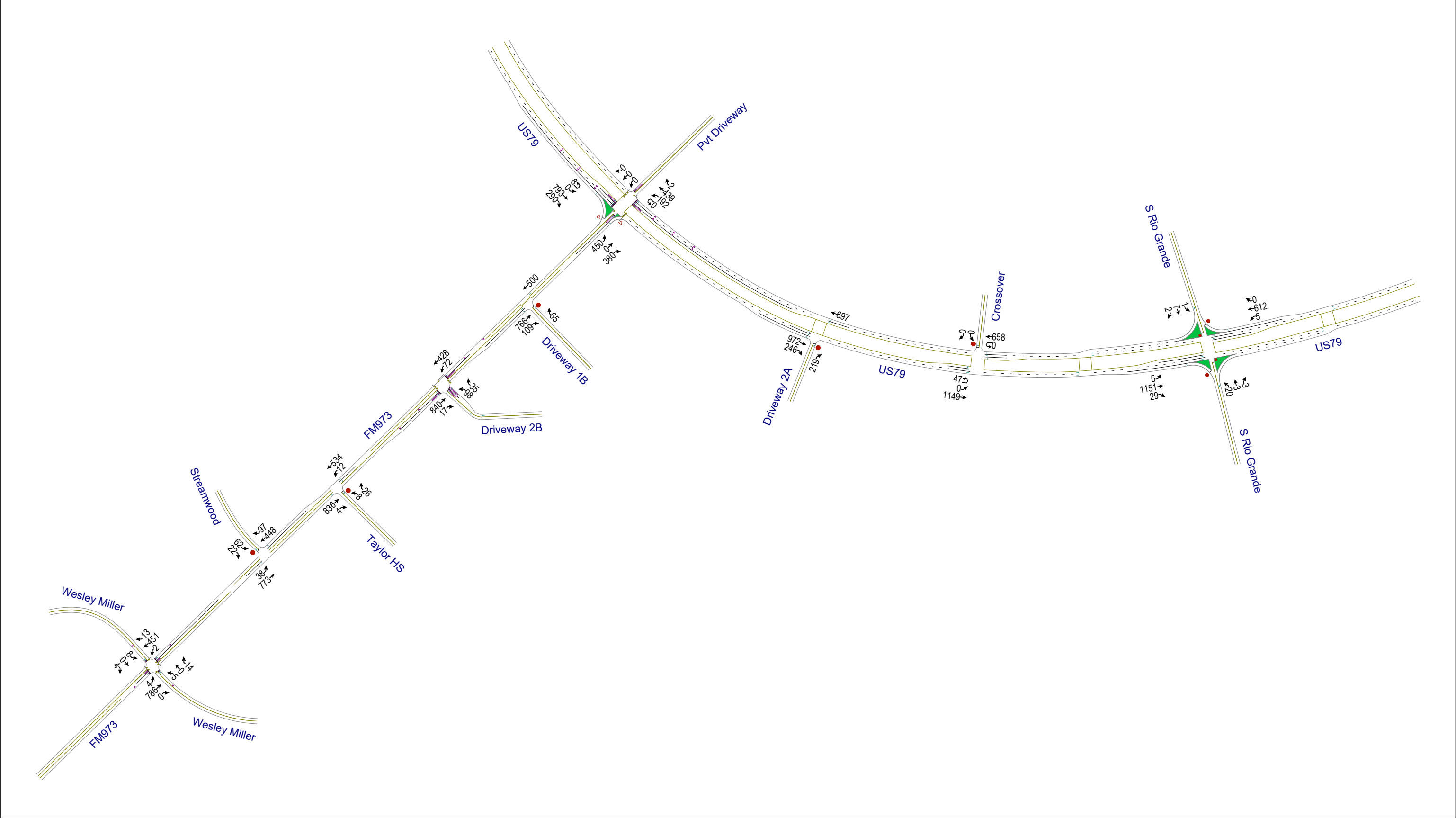
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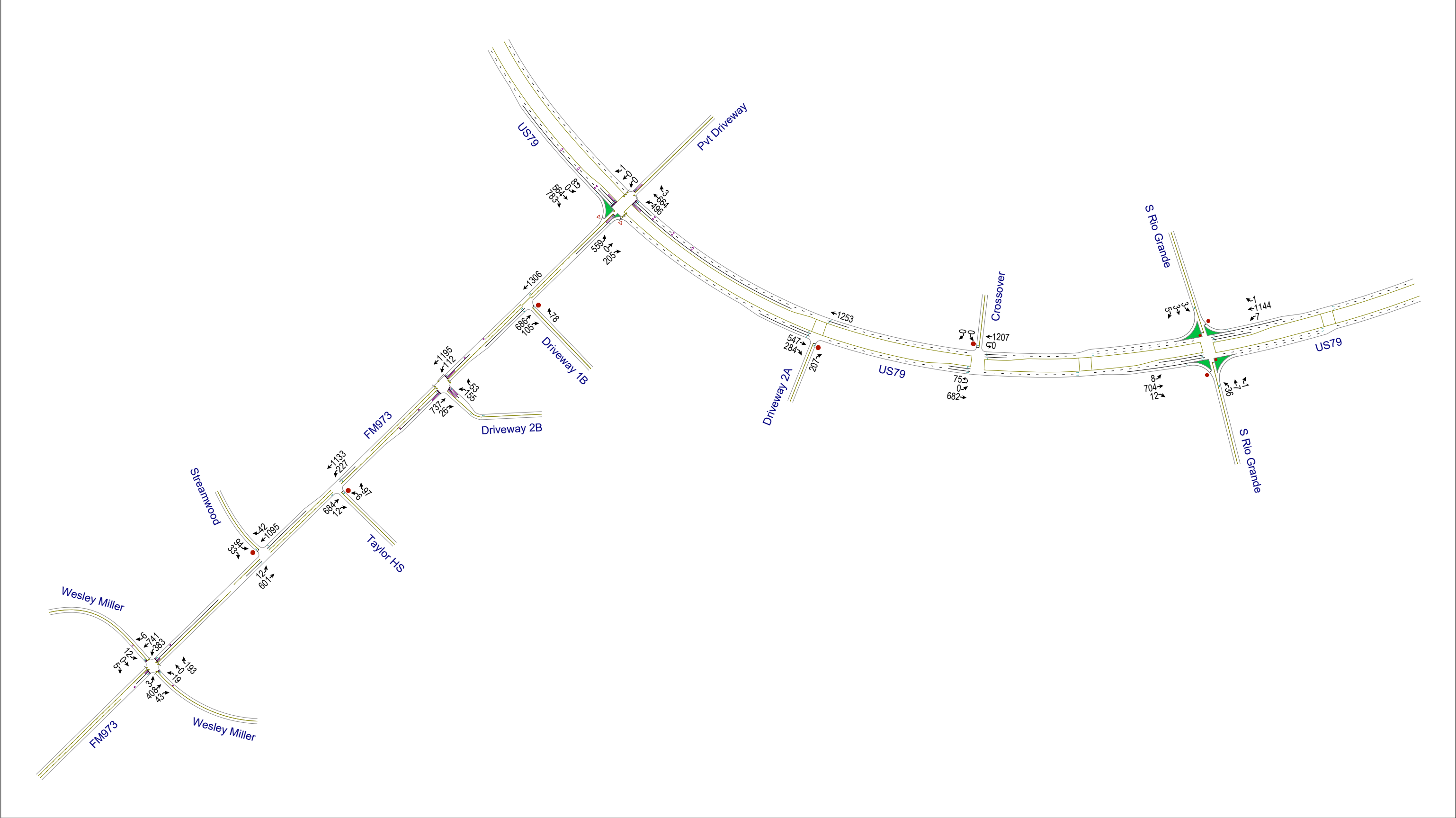
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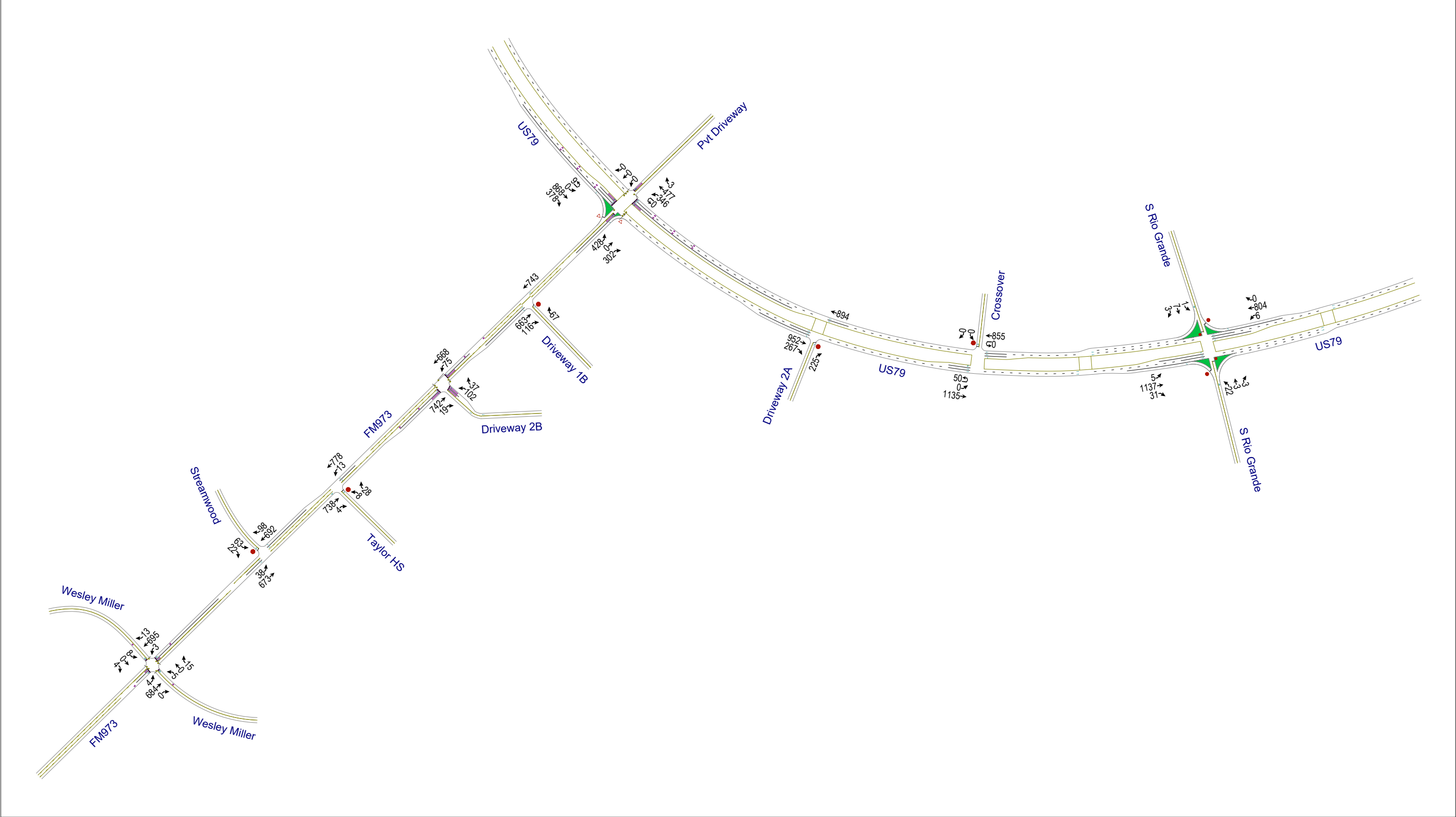
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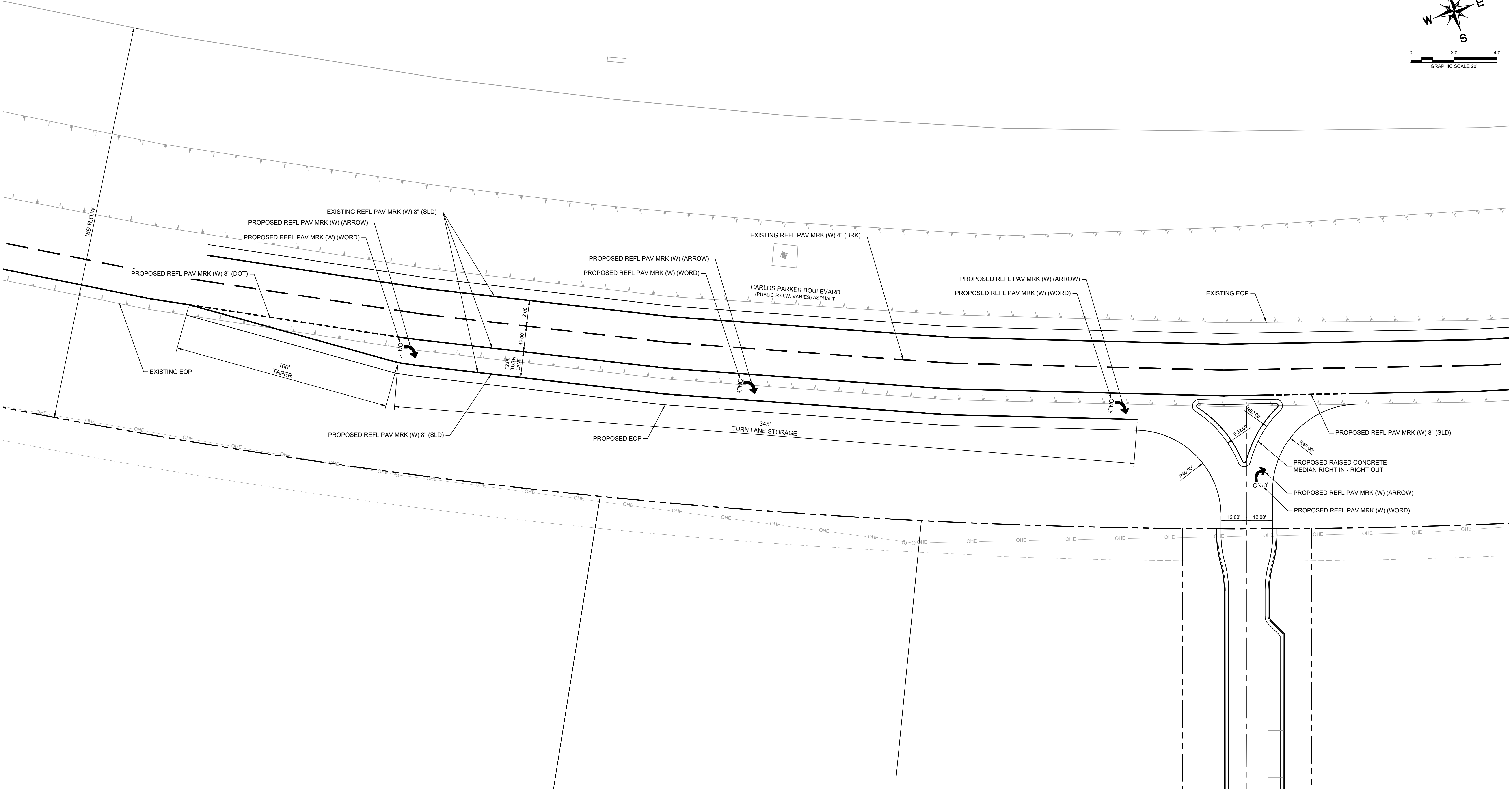
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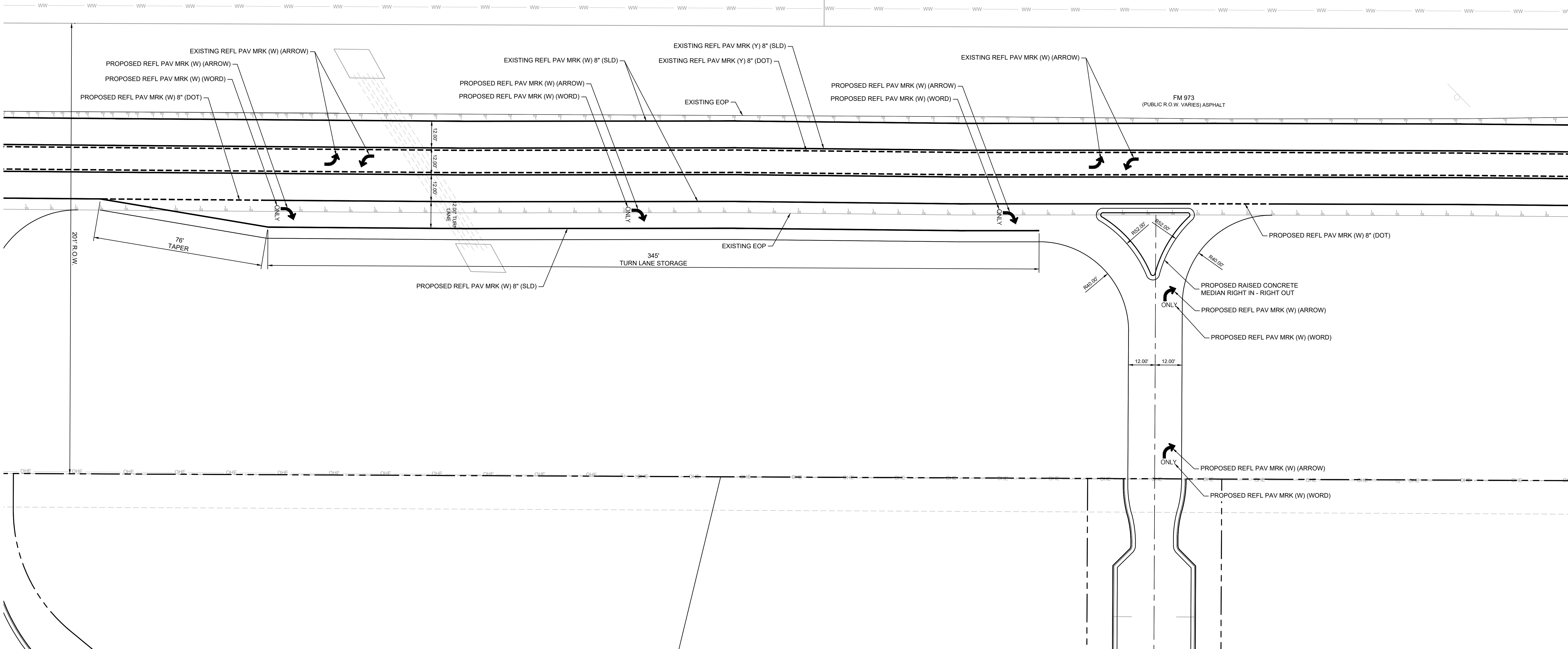


○ Study Intersections



Gateway Driveway 1A

Page 1011 of 1693



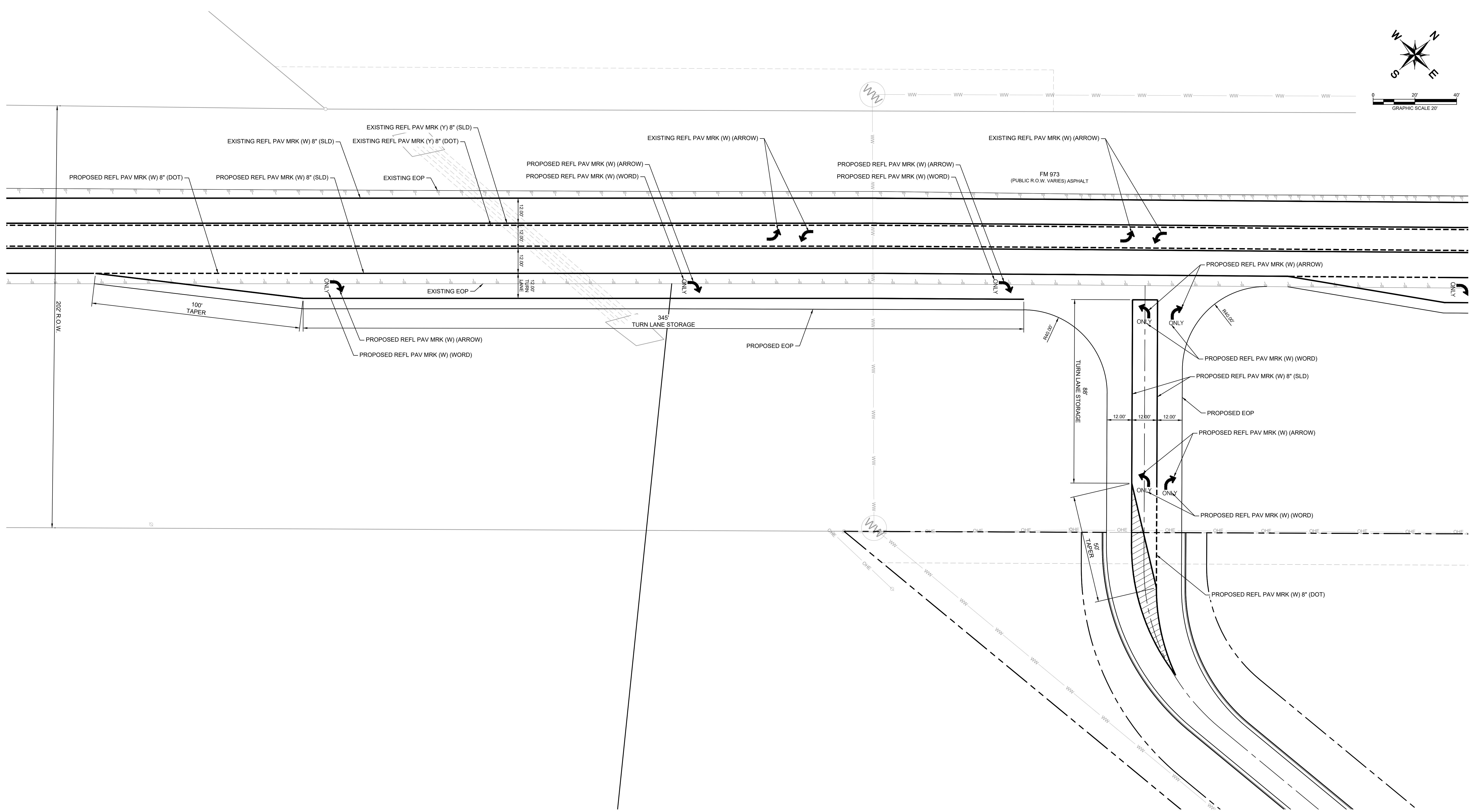
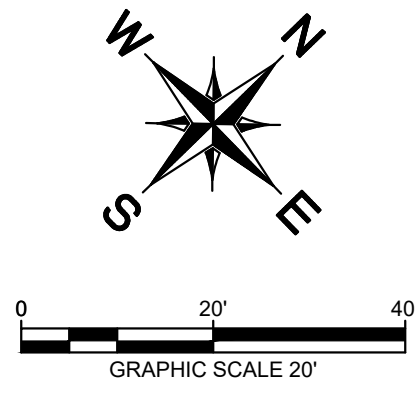


Exhibit A20-C

Gateway Driveway 2B

Taylor, Texas
July 2024

Kimley»Horn
10814 Jollyville Road
Campus IV, Suite 200
Austin, TX 78759
512-418-1771
State of Texas Registration No. F-928

Appendix B – Traffic Count Data

Thursday, May 26, 2022

[illegible]

Peds						
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW
0	0	0	0	0	0	0
0	0	0	0	0	0	0

0.92

	Eastbound				Westbound				Northbound				Southbound				
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
AM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	0%	0%	11%	0%	0%
PM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	0%	0%	11%	0%	0%

Vehicle/Pedestrian Turning Movement Count
FM 973 at Streamwood Trail (Under Construction)
Thursday, May 26, 2022

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:00	0		0	0					0	59		0		80	0	0	139
7:15	0		0	0					0	83		1		108	0	0	192
7:30	1		0	0					0	84		0		148	0	0	233
7:45	0		0	0					0	88		1		158	0	0	247
8:00	0		0	0					0	95		0		181	0	0	276
8:15	0		0	0					0	128		0		162	1	0	291
8:30	0		0	0					0	69		0		80	0	0	149
8:45	0		0	0					0	58		0		62	0	0	120
16:00	0		0	0					0	123		0		57	0	0	180
16:15	0		0	0					0	116		0		74	0	0	190
16:30	0		0	0					0	115		0		97	0	0	212
16:45	0		0	0					0	108		0		76	0	0	184
17:00	0		0	0					0	129		0		90	0	0	219
17:15	0		1	0					0	106		0		64	0	0	171
17:30	0		0	0					0	109		0		72	0	0	181
17:45	0		0	0					0	117		0		73	0	0	190
4-Hour Total	1	0	1	0	0	0	0	0	0	1587	0	2	0	1582	1	0	3174

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

Peak Hour Total	Eastbound				Westbound				Northbound				Southbound				Hour Vehicle
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:30-8:30	1	0	0	0	0	0	0	0	0	395	0	1	0	649	1	0	1047
16:15-17:15	0	0	0	0	0	0	0	0	0	468	0	0	0	337	0	0	805

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

PHF:
0.90
0.92

Peak Period, Percent Trucks, By Movement

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	8%	0%	0%	0%	5%	0%	0%
PM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	0%	12%	0%	0%

Vehicle/Pedestrian Turning Movement Count
 FM 973 at Taylor High School Driveway
 Thursday, May 26, 2022

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:00					0		1	0		57	0	0	7	82		0	147
7:15					1		9	0		82	0	0	22	111		0	225
7:30					0		10	0		85	0	0	19	151		0	265
7:45					1		15	0		88	1	0	29	162		0	296
8:00					1		17	0		93	1	0	50	183		1	346
8:15					1		24	0		126	3	0	73	153		0	380
8:30					0		16	0		63	1	0	23	78		0	181
8:45					0		2	0		63	0	1	3	64		0	133
16:00					0		6	0		113	0	0	1	58		0	178
16:15					0		3	0		120	0	0	2	87		0	212
16:30					1		5	0		119	0	0	0	85		0	210
16:45					0		2	0		112	0	0	1	82		0	197
17:00					0		4	0		122	0	0	0	79		0	205
17:15					0		0	0		111	0	0	0	67		0	178
17:30					0		1	0		115	0	0	0	78		0	194
17:45					0		1	0		119	0	0	0	70		1	191
4-Hour Total	0	0	0	0	5	0	116	0	0	1588	6	1	230	1590	0	2	3538

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

Peak Hour Total	Eastbound				Westbound				Northbound				Southbound				Hour Vehicle Total
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:30-8:30	0	0	0	0	3	0	66	0	0	392	5	0	171	649	0	1	1287
16:15-17:15	0	0	0	0	1	0	14	0	0	473	0	0	3	333	0	0	824

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

PHF:
0.85
0.97

Peak Period, Percent Trucks, By Movement

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	8%	0%	0%	0%	6%	0%	0%
PM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	0%	11%	0%	0%

Vehicle/Pedestrian Turning Movement Count
FM 973 at Taylor High School Driveway
Wednesday, October 25, 2023

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total	
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
0:00					v	0	0	0		11	0			0	0			11
0:15					0		0	0		3	0			0	0			3
0:30					0		0	0		6	0			0	0			6
0:45					0		0	0		3	0			0	0			3
1:00					0		0	0		9	0			0	5			14
1:15					0		0	0		2	0			0	1			3
1:30					0		0	0		9	0			0	10			19
1:45					0		0	0		22	0			0	4			26
2:00					0		0	0		23	0			0	4			27
2:15					0		0	0		8	0			0	6			14
2:30					0		0	0		11	0			0	5			16
2:45					0		0	0		18	0			0	4			22
3:00					0		0	0		15	0			0	4			19
3:15					0		0	0		13	0			0	16			29
3:30					0		0	0		8	0			0	19			27
3:45					0		0	0		5	0			0	24			29
4:00					0		0	0		16	0			0	20			36
4:15					0		0	0		36	0			0	41			77
4:30					0		0	0		41	0			0	71			112
4:45					0		1	0		36	0			1	135			173
5:00					0		0	0		45	0			4	252			301
5:15					0		1	0		34	0			4	368			407
5:30					0		3	0		47	0			9	402			461
5:45					0		2	0		79	0			9	402			492
6:00					0		2	0		54	0			3	390			449
6:15					0		3	0		55	0			13	365			436
6:30					0		3	0		85	0			4	362			454
6:45					0		1	0		61	0			1	222			285
7:00					0		2	0		77	0			2	185			266
7:15					1		4	0		63	1			17	183			269
7:30					0		7	0		100	0			21	235			363
7:45					0		18	0		88	0			24	244			374
8:00					0		39	0		98	1			38	253			429
8:15					0		44	0		121	2			51	181			399
8:30					0		15	0		65	0			6	128			214
8:45					0		5	0		50	0			8	119			182
9:00					0		5	0		58	0			4	88			155
9:15					0		2	0		53	0			6	82			143
9:30					0		3	0		53	1			2	70			129
9:45					0		1	0		46	0			2	72			121
10:00					1		7	0		61	0			2	58			129
10:15					0		2	0		59	0			2	51			114
10:30					1		4	0		50	1			1	51			108
10:45					0		3	0		89	0			3	74			169
11:00					0		3	0		113	0			3	83			202
11:15					0		0	0		151	0			1	93			245
11:30					1		13	0		160	0			4	87			265
11:45					0		10	0		161	1			7	114			293
12:00					1		11	0		154	0			11	142			319
12:15					0		23	0		109	1			14	170			317
12:30					0		10	0		82	0			26	189			307
12:45					1		68	0		150	1			26	176			422
13:00					1		32	0		129	1			22	138			323
13:15					0		10	0		71	0			1	101			183
13:30					0		13	0		78	0			5	72			168
13:45					1		2	0		50	0			3	82			138
14:00					0		7	0		49	0			4	70			130
14:15					0		4	0		91	0			3	68			166
14:30					0		1	0		83	0			1	76			161
14:45					0		2	0		103	0			3	72			180
15:00					0		6	0		111	0			5	105			227
15:15					0		6	0		144	0			2	79			231
15:30					2		10	0		147	0			1	102			262
15:45					1		7	0		154	0			3	100			265
16:00					0		16	0		200	0			0	134			350
16:15					0		7	0		218	0			0	115			340
16:30					0		11	0		265	0			0	125			401
16:45					1		2	0		247	1			0	115			366
17:00					0		2	0		242	0			0	105			349
17:15					0		1	0		223	1			1	94			320
17:30					0		1	0		283	1			0	103			388
17:45					0		1	0		255	0			0	82			338
18:00					0		0	0		286	0			0	75			361
18:15					1		0	0		262	0			0	67			330
18:30					0		1	0		151	0			0	82			234
18:45					0		0	0		150	0			1	59			210
19:00					1		1	0		141	2			0	63			208
19:15					1		4	0		100	1			0	44			150
19:30					0		0	0		67	0			1	57			125
19:45					0		0	0		59	0			0	40			99
20:00					0		0	0		42	0			0	32			74
20:15					0		0	0		37	0			0	10			47
20:30					0		0	0		33	0			0	30			63
20:45					0		0	0		29	0			0	31			60
21:00					0		0	0		41	0			0	30			71
21:15					0		2	0		28	0			0	32			62
21:30					0		1	0		22	0			0	28			51
21:45					0		0	0		20	0			0	19			39
22:00					0		0	0		28	0			0	33			61
22:15					0		0	0		19	0			0	26			45
22:30					0		0	0		15	0			0	19			34
22:45					0		0	0		22	0			0	9			31
23:00					0		0	0		15	0			0	1			16
23:15					0		0	0		15	0			0	0			15
23:30					0		0	0		5	0			0	0			5
23:45					0		0	0		7	0			0	0			7
24-Hour Total	0	0	0	0	15	0	465	0	0	7703	16	0	385	8985	0	0	0	17569

Peak Hour Total	Eastbound				Westbound				Northbound				Southbound				Hour Vehicle Total
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:30-8:30	0	0	0	0	0	0	108	0	0	407	3	0	134	913	0	0	1565
16:15-17:15	0	0	0	0	1	0	22	0	0	972	1	0	0	460	0	0	1456

PHF:
0.91
0.91

Peak Period, Percent Trucks, By Movement

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>								Taylor High School Driveway				FM 973			
									FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	0%
PM Peak Period	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	3%	0%	0%

Thursday, May 26, 2022

[illegible]

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

Peak Period, Percent Trucks, By Movement

Taylor Texas Gateway Traffic Impact Analysis
Prepared for Gateway Oz, LLC
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Vehicle/Pedestrian Turning Movement Count
US 79 at Crossover East of FM 973 (School in Session)
Thursday, May 26, 2022

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:00		83		0		177		0									260
7:15		90		0		195		0									285
7:30		134		0		204		0									338
7:45		106		1		196		0									303
8:00		106		0		185		0									291
8:15		108		1		193		0									302
8:30		113		0		127		0									240
8:45		106		0		129		0									235
16:00		157		0		109		0									266
16:15		193		0		116		0									309
16:30		220		0		136		0									356
16:45		195		0		122		0									317
17:00		232		0		127		0									359
17:15		207		0		120		0									327
17:30		204		0		112		0									316
17:45		174		0		131		0									305
4-Hour Total	0	2428	0	2	0	2379	0	0	0	0	0	0	0	0	0	0	4809

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

Peak Hour Total	Eastbound				Westbound				Northbound				Southbound				Hour Vehicle
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:30-8:30	0	454	0	2	0	778	0	0	0	0	0	0	0	0	0	0	1234
16:15-17:15	0	840	0	0	0	501	0	0	0	0	0	0	0	0	0	0	1341

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

PHF:
0.91
0.93

Peak Period, Percent Trucks, By Movement

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak Period	0%	10%	0%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
PM Peak Period	0%	4%	0%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Vehicle/Pedestrian Turning Movement Count
US 79 at Crossover East of FM 973 (School not in Session)
Tuesday, June 7, 2022

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>				
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:00		67		0		182		0									249
7:15		73		1		233		0									307
7:30		115		0		185		0									300
7:45		100		0		192		0									292
8:00		90		0		155		0									245
8:15		105		0		134		0									239
8:30		86		0		127		0									213
8:45		93		0		96		0									189
16:00		192		0		111		0									303
16:15		191		0		91		0									282
16:30		181		0		107		0									288
16:45		172		0		99		0									271
17:00		213		0		124		0									337
17:15		219		0		118		0									337
17:30		181		0		94		0									275
17:45		198		0		83		0									281
4-Hour Total	0	2276	0	1	0	2131	0	0	0	0	0	0	0	0	0	0	4408

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

Peak Hour Total	Eastbound				Westbound				Northbound				Southbound				Hour Vehicle
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
7:30-8:30	0	410	0	0	0	666	0	0	0	0	0	0	0	0	0	0	1076
16:15-17:15	0	757	0	0	0	421	0	0	0	0	0	0	0	0	0	0	1178

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

PHF:
0.90
0.87

Peak Period, Percent Trucks, By Movement

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak Period	0%	12%	0%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
PM Peak Period	0%	2%	0%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Vehicle/Pedestrian Turning Movement Count
US 79 at South Rio Grande Street
Tuesday, June 7, 2022

Time Begin	Eastbound				Westbound				Northbound				Southbound				15-min Vehicle Total	
	US 79				US 79				South Rio Grande Street				South Rio Grande Street					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00	0	67	2	0	0	176	0	0	4	0	0	0	0	0	0	0	0	249
7:15	0	76	0	0	0	221	0	0	8	0	1	0	0	0	0	0	0	306
7:30	0	123	2	0	1	185	0	0	4	1	0	0	0	0	0	1	0	317
7:45	2	97	0	0	1	186	0	0	4	2	0	0	0	0	1	0	0	293
8:00	1	101	0	0	3	139	1	0	7	2	0	0	1	1	0	0	0	256
8:15	0	97	2	0	0	132	0	0	5	0	1	0	1	0	0	0	0	238
8:30	0	100	3	0	0	112	0	0	9	0	0	0	0	0	0	0	0	224
8:45	1	87	1	0	0	97	0	0	6	0	0	0	0	0	0	0	0	192
16:00	1	182	9	0	1	111	0	0	3	0	0	0	2	1	0	0	0	310
16:15	1	190	7	0	1	84	0	0	3	1	0	0	0	1	0	0	0	288
16:30	0	176	2	0	1	110	0	0	4	0	0	0	0	1	0	0	0	294
16:45	1	166	4	0	1	98	0	0	4	1	1	0	0	1	0	0	0	277
17:00	0	212	6	0	1	118	0	0	1	0	1	0	1	2	0	0	0	342
17:15	2	212	6	0	1	110	0	0	6	1	0	0	0	2	0	0	0	340
17:30	3	191	7	0	0	87	0	0	6	2	1	0	0	1	0	0	0	298
17:45	1	175	6	0	1	79	0	0	3	0	0	0	0	1	0	0	0	266
4-Hour Total	13	2252	57	0	12	2045	1	0	77	10	5	0	5	12	1	0	0	4490

[illegible]

Peak Hour Total	Eastbound					Westbound					Northbound				Southbound				Hour Vehicle Total		
	US 79					US 79					South Rio Grande Street				South Rio Grande Street						
	Left	Thru	Right	U		Left	Thru	Right	U		Left	Thru	Right	U		Left	Thru	Right		U	
7:30-8:30	3	418	4	0		5	642	1	0		20	5	1	0		2	2	1	0		1104
16:15-17:15	2	744	19	0		4	410	0	0		12	2	2	0		1	5	0	0		1201

Peds							
EB-CW	EB-CCW	WB-CW	WB-CCW	NB-CW	NB-CCW	SB-CW	SB-CCW
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0

PHF:
0.87
0.88

Peak Period, Percent Trucks, By Movement

[illegible]

Appendix C – Trip Generation & Distribution Analysis Details and Projected Turning Movement Counts

Table C1. Trip Generation Calculations - Phase 1

Trip Rates															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Rate*	Percent		Trip Rate*	Percent		Trip Rate*	Percent	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Fast-food w/Drive Through (Lot 1)	934	Fast-Food Restaurant w/Drive Through	1000 SF	4,400	24/AM/PM	---	467.48	50%	50%	44.61	51%	49%	33.03	52%	48%
C-Store w/Gas (Lot 2+3)	945	Convenience Store/Gas Station	1000 SF	5,600	AM/PM	24	700.43	50%	50%	56.52	50%	50%	54.52	50%	50%
Restaurant (Lots 4+5+8+9)	932	High-Turnover Sit Down Restaurants	1000 SF	26,350	24/AM/PM	---	107.20	50%	50%	9.57	55%	45%	9.05	61%	39%
Retail (Lots 4+5)	822	Strip Retail Plaza	1000 SF	11,350	24/AM/PM	---	54.45	50%	50%	2.36	60%	40%	6.59	50%	50%
Multifamily (Lots 8+9+10+11)	221	Multifamily Mid-Rise	Dwelling Unit	45	---	24/AM/PM	4.54	50%	50%	0.37	23%	77%	0.39	61%	39%
Live-Work Commercial (Office) (Lots 10+11)	710	General Office Building	1000 SF	29,000	---	24/AM/PM	10.84	50%	50%	1.52	88%	12%	1.44	17%	83%
Hotel 1 (Lot 6)	310	Hotel	Rooms	150	---	24/AM/PM	7.99	50%	50%	0.46	56%	44%	0.59	51%	49%
Hotel 2 (Lot 7)	310	Hotel	Rooms	150	---	24/AM/PM	7.99	50%	50%	0.46	56%	44%	0.59	51%	49%
Trip End Calculations															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Fast-food w/Drive Through (Lot 1)	934	Fast-Food Restaurant w/Drive Through	1000 SF	4,400	24/AM/PM	---	2,057	1,028	1,028	196	100	96	145	76	70
C-Store w/Gas (Lot 2+3)	945	Convenience Store/Gas Station	1000 SF	5,600	AM/PM	24	3,690	1,845	1,845	317	158	158	305	153	153
Restaurant (Lots 4+5+8+9)	932	High-Turnover Sit Down Restaurants	1000 SF	26,350	24/AM/PM	---	2,825	1,412	1,412	252	139	113	238	145	93
Retail (Lots 4+5)	822	Strip Retail Plaza	1000 SF	11,350	24/AM/PM	---	618	309	309	27	16	11	75	37	37
Multifamily (Lots 8+9+10+11)	221	Multifamily Mid-Rise	Dwelling Unit	45	---	24/AM/PM	168	84	84	8	2	6	18	11	7
Live-Work Commercial (Office) (Lots 10+11)	710	General Office Building	1000 SF	29,000	---	24/AM/PM	395	198	198	58	51	7	59	10	49
Hotel 1 (Lot 6)	310	Hotel	Rooms	150	---	24/AM/PM	1,202	601	601	68	38	30	83	42	41
Hotel 2 (Lot 7)	310	Hotel	Rooms	150	---	24/AM/PM	1,202	601	601	68	38	30	83	42	41
Trip End Totals															
Trip Totals							Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
							12,158	6,079	6,079	993	541	451	1,007	517	491

*trip rates shown for information only, fitted curves used for trip generation

Trip Adjustment Factors		
Adjustment Factor	Time Period	Factor
Internal Capture Rates:		
	Weekday	-
Weekday AM Peak Hour of Adjacent Roadway Entering		4.0%
Weekday AM Peak Hour of Adjacent Roadway Exiting		5.0%
Weekday PM Peak Hour of Adjacent Roadway Entering		14.0%
Weekday PM Peak Hour of Adjacent Roadway Exiting		15.0%
Pass-By Trips:		
	Weekday	-
Weekday AM Peak Hour of Adjacent Roadway		50.0%
Weekday PM Peak Hour of Adjacent Roadway		55.0%
Weekday AM Peak Hour of Adjacent Roadway		76.0%
Weekday PM Peak Hour of Adjacent Roadway		75.0%
Weekday AM Peak Hour of Adjacent Roadway		0.0%
Weekday PM Peak Hour of Adjacent Roadway		43.0%
Weekday AM Peak Hour of Adjacent Roadway		0.0%
Weekday PM Peak Hour of Adjacent Roadway		40.0%

LU 934	(98)	
LU 934		(80)
LU 945	(241)	
LU 945		(229)
LU 932	-	
LU 932		(103)
LU 822 (Using LU 821)	-	
LU 822 (Using LU 821)		(7)

Trip Totals													
Trip Type		Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway								
		Total Trips	Trips*		Total Trips	Trips*							
			Entering	Exiting		Entering	Exiting						
Total Trips, Pre-Capture/Pass-By:		993	541	451	1,007	517	491						
Total Trips, Captured Within Development:		44	22	23	146	72	74						
Total Trips, New & Pass-By		949	520	429	862	445	417						
Total Trips, Pass-By, Existing on Roadway Network:		339	169	169	419	209	209						
Total Trips, New on Roadway Network:		610	350	259	443	235	208						

*trip estimates subject to roundoff error

Table C1-A. Internal Capture Calculations (Phase 1)

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Taylor Gateway			Organization:	Voigt Associates, Inc.
Project Location:	US 79 at FM 973, Taylor TX			Performed By:	Anthony Voigt, P.E., PTOE
Scenario Description:	Phase 2 Buildout (AM Peak Hour)			Date:	02/22/2024
Analysis Year:	2027			Checked By:	
Analysis Period:	AM Street Peak Hour			Date:	

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	29,000	1000 SF	58	51	7
Retail	822	11,350	1000 SF	27	16	11
Restaurant	932+934	30,750	1000 SF	448	239	210
Cinema/Entertainment				0		
Residential	215+221	45	Dwelling Unit	8	2	6
Hotel	310	300	Rooms	135	76	59
All Other Land Uses ²	945	5,600	1000 SF	317	158	158
				993	541	451

Table 2-A: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office	1.00	0%	0%	1.00	0%	0%
Retail	1.50	0%	0%	1.50	0%	0%
Restaurant	1.50	0%	0%	1.50	0%	0%
Cinema/Entertainment	1.50	0%	0%	1.50	0%	0%
Residential	1.00	0%	0%	1.00	0%	0%
Hotel	1.00	0%	0%	1.00	0%	0%
All Other Land Uses ²	1.50	0%	0%	1.50	0%	0%

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-A: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		2	4	0	0	0
Retail	2		2	0	0	0
Restaurant	7	2		0	0	3
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	1	0		0
Hotel	2	1	5	0	0	

Table 5-A: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	1,387	748	639
Internal Capture Percentage	4%	4%	5%
External Vehicle-Trips ⁵	941	517	424
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-A: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	22%	86%
Retail	21%	25%
Restaurant	3%	4%
Cinema/Entertainment	N/A	N/A
Residential	0%	17%
Hotel	4%	14%

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Table C1-A. Internal Capture Calculations (Phase 1)

Project Name:	Taylor Gateway
Analysis Period:	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-A (D): Entering Trips			Table 7-A (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	50.80735067	51	1.00	6.928275091	7
Retail	1.50	16.0716	24	1.50	10.7144	16
Restaurant	1.50	238.798065	358	1.50	209.655435	314
Cinema/Entertainment	1.50	0	0	1.50	0	0
Residential	1.00	1.8837	2	1.00	6.3063	6
Hotel	1.00	75.656	76	1.00	59.444	59

Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		2	4	0	0	0
Retail	5		2	0	2	0
Restaurant	97	44		0	13	9
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	1	0		0
Hotel	44	8	5	0	0	

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		8	82	0	0	0
Retail	2		179	0	0	0
Restaurant	7	2		0	0	3
Cinema/Entertainment	0	0	0		0	0
Residential	2	4	72	0		0
Hotel	2	1	21	0	0	

Table 9-A (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	11	40	51	40	0	0
Retail	5	19	24	13	0	0
Restaurant	12	346	358	231	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	0	2	2	2	0	0
Hotel	3	73	76	73	0	0
All Other Land Uses ³	0	237	237	158	0	0

Table 9-A (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	6	1	7	1	0	0
Retail	4	12	16	8	0	0
Restaurant	12	302	314	201	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	1	5	6	5	0	0
Hotel	8	51	59	51	0	0
All Other Land Uses ³	0	237	237	158	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

Table C1-A. Internal Capture Calculations (Phase 1)

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Taylor Gateway			Organization:	Voigt Associates, Inc.
Project Location:	US 79 at FM 973, Taylor TX			Performed By:	Anthony Voigt, P.E., PTOE
Scenario Description:	Phase 2 Buildout (AM Peak Hour)			Date:	02/22/2024
Analysis Year:	2027			Checked By:	
Analysis Period:	PM Street Peak Hour			Date:	

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	29,000	1000 SF	59	10	49
Retail	822	11,350	1000 SF	75	37	37
Restaurant	932+934	30,750	1000 SF	384	221	163
Cinema/Entertainment				0		
Residential	215+221	315	Dwelling Unit	18	11	7
Hotel	310	300	Rooms	166	85	81
All Other Land Uses ²	945	5,600	1000 SF	305	153	153
				1,007	517	491

Table 2-P: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office	1.00	0%	0%	1.00	0%	0%
Retail	1.50	0%	0%	1.50	0%	0%
Restaurant	1.50	0%	0%	1.50	0%	0%
Cinema/Entertainment	1.50	0%	0%	1.50	0%	0%
Residential	1.00	0%	0%	1.00	0%	0%
Hotel	1.00	0%	0%	1.00	0%	0%
All Other Land Uses ²	1.50	0%	0%	1.50	0%	0%

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		800	800		600	
Retail					700	
Restaurant					700	
Cinema/Entertainment					700	
Residential		800	800			
Hotel					650	

Table 4-P: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		4	2	0	0	0
Retail	1		16	0	5	3
Restaurant	3	28		0	2	17
Cinema/Entertainment	0	0	0		0	0
Residential	0	2	1	0		0
Hotel	0	1	17	0	0	

Table 5-P: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	1,389	723	666
Internal Capture Percentage	15%	14%	15%
External Vehicle-Trips ⁵	852	439	413
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-P: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	40%	12%
Retail	63%	45%
Restaurant	11%	20%
Cinema/Entertainment	N/A	N/A
Residential	64%	43%
Hotel	24%	22%

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Table C1-A. Internal Capture Calculations (Phase 1)

Project Name:	Taylor Gateway
Analysis Period:	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-P (D): Entering Trips			Table 7-P (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	10.10366972	10	1.00	49.3296816	49
Retail	1.50	37.39825	56	1.50	37.39825	56
Restaurant	1.50	221.037815	332	1.50	162.761685	244
Cinema/Entertainment	1.50	0	0	1.50	0	0
Residential	1.00	10.9129	11	1.00	6.9771	7
Hotel	1.00	84.7722	85	1.00	81.4478	81

Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		8	2	0	1	0
Retail	1		16	2	15	3
Restaurant	7	100		20	44	17
Cinema/Entertainment	0	0	0		0	0
Residential	0	2	1	0		0
Hotel	0	13	55	0	2	

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		4	5	0	0	0
Retail	3		96	0	5	14
Restaurant	3	28		0	2	60
Cinema/Entertainment	1	2	10		0	1
Residential	6	5	38	0		10
Hotel	0	1	17	0	0	

Table 9-P (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	4	6	10	6	0	0
Retail	35	21	56	14	0	0
Restaurant	36	296	332	197	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	7	4	11	4	0	0
Hotel	20	65	85	65	0	0
All Other Land Uses ³	0	229	229	153	0	0

Table 9-P (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	6	43	49	43	0	0
Retail	25	31	56	21	0	0
Restaurant	50	194	244	129	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	3	4	7	4	0	0
Hotel	18	63	81	63	0	0
All Other Land Uses ³	0	229	229	153	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

Table C1-A. Internal Capture Calculations (Phase 1)

Table 7.1a Adjusted Internal Trip Capture Rates for Trip Origins within a Multi-Use Development			
Land Use Pairs		Weekday	
		AM Peak Hour	PM Peak Hour
From OFFICE	To Office	0.0%	0.0%
	To Retail	28.0%	16.4%
	To Restaurant	63.0%	3.3%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	1.0%	2.0%
	To Hotel	0.0%	0.0%
From RETAIL	To Office	29.0%	2.0%
	To Retail	0.0%	0.0%
	To Restaurant	13.0%	29.0%
	To Cinema/Entertainment	0.0%	4.0%
	To Residential	14.0%	26.0%
	To Hotel	0.0%	5.0%
From RESTAURANT	To Office	31.0%	3.0%
	To Retail	14.0%	41.0%
	To Restaurant	0.0%	0.0%
	To Cinema/Entertainment	0.0%	8.0%
	To Residential	4.0%	18.0%
	To Hotel	3.0%	7.0%
From CINEMA/ENTERTAINMENT	To Office	0.0%	2.0%
	To Retail	0.0%	21.0%
	To Restaurant	0.0%	31.0%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	8.0%
	To Hotel	0.0%	2.0%
From RESIDENTIAL	To Office	2.0%	4.0%
	To Retail	1.0%	34.4%
	To Restaurant	20.0%	17.2%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	0.0%
	To Hotel	0.0%	3.0%
From HOTEL	To Office	75.0%	0.0%
	To Retail	14.0%	16.0%
	To Restaurant	9.0%	68.0%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	2.0%
	To Hotel	0.0%	0.0%

Table C1-A. Internal Capture Calculations (Phase 1)

Table 7.2a Adjusted Internal Trip Capture Rates for Trip Destinations within a Multi-Use Development			
Land Use Pairs		Weekday	
		AM Peak Hour	PM Peak Hour
To OFFICE	From Office	0.0%	0.0%
	From Retail	4.0%	31.0%
	From Restaurant	14.0%	30.0%
	From Cinema/Entertainment	0.0%	6.0%
	From Residential	3.0%	57.0%
	From Hotel	3.0%	0.0%
To RETAIL	From Office	32.0%	6.6%
	From Retail	0.0%	0.0%
	From Restaurant	8.0%	50.0%
	From Cinema/Entertainment	0.0%	4.0%
	From Residential	17.0%	8.2%
	From Hotel	4.0%	2.0%
To RESTAURANT	From Office	23.0%	1.6%
	From Retail	50.0%	29.0%
	From Restaurant	0.0%	0.0%
	From Cinema/Entertainment	0.0%	3.0%
	From Residential	20.0%	11.5%
	From Hotel	6.0%	5.0%
To CINEMA/ENTERTAINMENT	From Office	0.0%	1.0%
	From Retail	0.0%	26.0%
	From Restaurant	0.0%	32.0%
	From Cinema/Entertainment	0.0%	0.0%
	From Residential	0.0%	0.0%
	From Hotel	0.0%	0.0%
To RESIDENTIAL	From Office	0.0%	4.0%
	From Retail	2.0%	46.0%
	From Restaurant	5.0%	16.0%
	From Cinema/Entertainment	0.0%	4.0%
	From Residential	0.0%	0.0%
	From Hotel	0.0%	0.0%
To HOTEL	From Office	0.0%	0.0%
	From Retail	0.0%	17.0%
	From Restaurant	4.0%	71.0%
	From Cinema/Entertainment	0.0%	1.0%
	From Residential	0.0%	12.0%
	From Hotel	0.0%	0.0%

Table C2. Trip Distribution & Assignment, Phase 1 Development

GLOBAL ORIGINS/DESTINATIONS								
Origins+Peak Hour Volume			Destinations+Peak Hour Volume					
	AM	PM		AM	PM			
From US 79, West of FM 973	865	781	To US 79, West of FM 973	792	594			
From US 79, East of South Rio Grande Street	743	471	To US 79, East of South Rio Grande Street	487	871			
From FM 973, South of Wesley Miller Lane	276	453	To FM 973, South of Wesley Miller Lane	360	336			
From South Rio Grande Street, North of US 79	5	6	To South Rio Grande Street, North of US 79	9	4			
From South Rio Grande Street, South of US 79	26	16	To South Rio Grande Street, South of US 79	11	28			
From Streamwood Trail, West of FM 973	111	75	To Streamwood Trail, West of FM 973	39	127			
From Wesley Miller, West of FM 973	12	8	To Wesley Miller, West of FM 973	4	14			
From Taylor HS Driveway, East of FM 973	69	15	To Taylor HS Driveway, East of FM 973	176	3			
TRIP DISTRIBUTION (NEW TRIPS)								
FROM (ENTERING DEVELOPMENT)	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
From US 79, West of FM 973	865	50.0%	41.0%	175	781	50.0%	42.8%	118
From US 79, East of South Rio Grande Street	743	20.0%	35.3%	70	471	20.0%	25.8%	47
From FM 973, South of Wesley Miller Lane	276	18.0%	13.1%	63	453	18.0%	24.8%	42
From South Rio Grande Street, North of US 79	5	1.0%	0.2%	4	6	1.0%	0.3%	2
From South Rio Grande Street, South of US 79	26	2.0%	1.2%	7	16	2.0%	0.9%	5
From Streamwood Trail, West of FM 973	111	4.0%	5.3%	14	75	4.0%	4.1%	9
From Wesley Miller, West of FM 973	12	1.0%	0.6%	4	8	1.0%	0.5%	2
From Taylor HS Driveway, East of FM 973	69	4.0%	3.3%	14	15	4.0%	0.8%	9
Total	2108	100.0%	100.0%	350	1825	100.0%	100.0%	235
TO (EXITING DEVELOPMENT)								
	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
To US 79, West of FM 973	792	50.0%	42.2%	130	594	50.0%	30.0%	104
To US 79, East of South Rio Grande Street	487	20.0%	25.9%	52	871	20.0%	44.0%	42
To FM 973, South of Wesley Miller Lane	360	18.0%	19.2%	47	336	18.0%	17.0%	37
To South Rio Grande Street, North of US 79	9	1.0%	0.5%	4	4	1.0%	0.2%	2
To South Rio Grande Street, South of US 79	11	2.0%	0.6%	5	28	2.0%	1.4%	4
To Streamwood Trail, West of FM 973	39	4.0%	2.1%	10	127	4.0%	6.4%	8
To Wesley Miller, West of FM 973	4	1.0%	0.2%	3	14	1.0%	0.7%	2
To Taylor HS Driveway, East of FM 973	176	4.0%	9.4%	10	3	4.0%	0.2%	8
Total	1879	100.0%	100.0%	259	1977	100.0%	100.0%	208
*Development volumes and sum totals rounded to whole vehicles								
TRIP DISTRIBUTION (PASS-BY/DIVERTED LINK TRIPS)								
PASS-BY TRIP PATH	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
EB to EB US 79	308	25.0%	15.1%	42	587	40.0%	33.9%	84
WB to WB US 79	477	15.0%	23.4%	25	345	10.0%	19.9%	21
NB FM 973 to EB US 79	130	5.0%	6.4%	8	234	15.0%	13.5%	31
NB FM 973 to WB US 79	314	15.0%	15.4%	25	249	15.0%	14.4%	31
WB US 79 to SB FM 973	261	10.0%	12.8%	17	132	10.0%	7.6%	21
EB US 79 to SB FM 973	551	30.0%	27.0%	51	187	10.0%	10.8%	21
Total	2041	100.0%	100.0%	169	1734	100.0%	100.0%	209
*Development volumes and sum totals rounded to whole vehicle								

TRIP DISTRIBUTION DETAILS - NEW TRIPS																	
FROM (ENTERING DEVELOPMENT)	ROUTE (ENTERING DEVELOPMENT)										AM PEAK HOUR	PM PEAK HOUR	AM PEAK HOUR	PM PEAK HOUR			
	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/RD)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller	Vehicles	Vehicles	Percent of Entering Vehicles	Percent of Entering Vehicles	
From US 79, West of FM 973																	
via US 79 Driveway 2A	85.0%	85.0%	EBT	EBT	EBR								149	100	42.50%	42.50%	
via FM 973 Driveway 2B	15.0%	15.0%	EBT					SBT	SBL				26	18	7.50%	7.50%	
From US 79, East of South Rio Grande Street																	
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	WBT						60	40	17.00%	17.00%	
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	WBT	SBT	SBL				11	7	3.00%	3.00%	
From FM 973, South of Wesley Miller Lane																	
via FM 973 Driveway 1B	80.0%	80.0%						NBR	NBT	NBT	NBT	NBT	50	34	14.40%	14.40%	
via FM 973 Driveway 2B	20.0%	20.0%							NBR	NBT	NBT	NBT	13	8	3.60%	3.60%	
From South Rio Grande Street, North of US 79																	
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	SBR						3	2	0.85%	0.85%	
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	SBR	SBT	SBL				1	0	0.15%	0.15%	
From South Rio Grande Street, South of US 79																	
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	NBL						6	4	1.70%	1.70%	
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	NBL	SBT	SBL				1	1	0.30%	0.30%	
From Streamwood Trail, West of FM 973																	
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	NBT	EBL		8	6	2.40%	2.40%	
via FM 973 Driveway 2B	40.0%	40.0%							NBR	NBT	EBL		6	4	1.60%	1.60%	
From Wesley Miller, West of FM 973																	
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	NBT	NBT	EBL	2	1	0.60%	0.60%	
via FM 973 Driveway 2B	40.0%	40.0%							NBR	NBT	EBL		1	1	0.40%	0.40%	
From Taylor HS Driveway, East of FM 973																	
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	WBR			8	6	2.40%	2.40%	
via FM 973 Driveway 2B	40.0%	40.0%							NBR	WBR			6	4	1.60%	1.60%	
TO (EXITING DEVELOPMENT)	ROUTE (EXITING DEVELOPMENT)										AM PEAK HOUR	PM PEAK HOUR	AM PEAK HOUR	PM PEAK HOUR			
	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/RD)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller	Vehicles	Vehicles	Percent of Exiting Vehicles	Percent of Exiting Vehicles	
To US 79, West of FM 973																	
via US 79 Driveway 2A	45.0%	45.0%	WBT	WBT	NBR WBT	EBU							58	47	22.50%	22.50%	
via FM 973 Driveway 1B	30.0%	30.0%	NBL					WBR					39	31	15.00%	15.00%	
via FM 973 Driveway 2B	25.0%	25.0%	NBL						NBT	WBR			32	26	12.50%	12.50%	
To US 79, East of South Rio Grande Street																	
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBT						44	35	17.00%	17.00%	
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	EBT	EBT	EBT	WBR					3	2	1.00%	1.00%	
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBT	NBT	WBR				5	4	2.00%	2.00%	
To FM 973, South of Wesley Miller Lane																	
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBT	SBT	47	37	18.00%	18.00%	
To South Rio Grande Street, North of US 79																	
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBL						2	2	0.85%	0.85%	
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	EBT	EBT	EBL	WBR					0	0	0.05%	0.05%	
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBL	NBT	WBR				0	0	0.10%	0.10%	
To South Rio Grande Street, South of US 79																	
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBR						4	4	1.70%	1.70%	
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	EBT	EBT	EBR	WBR					0	0	0.10%	0.10%	
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBR	NBT	WBR				1	0	0.20%	0.20%	
To Streamwood Trail, West of FM 973																	
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBR		10	8	4.00%	4.00%	
To Wesley Miller, West of FM 973																	
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBT	SBR	3	2	1.00%	1.00%	
To Taylor HS Driveway, East of FM 973																	
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBL			10	8	4.00%	4.00%	

PASS-BY TRIPS	ROUTE (EXITING DEVELOPMENT)										AM PEAK HOUR Vehicles	PM PEAK HOUR Vehicles
	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/RD)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller
EB to EB US 79	100.0%	100.0%										
via US 79 Driveway 2A	100.0%	100.0%			+EBR,-EBT,+NBR							
WB to WB US 79	100.0%	100.0%										
via US 79 Driveway 2A	80.0%	80.0%			+WBU							
via FM 973 Driveway 2B	20.0%	20.0%			+WBL,-WBT,-NBL							
NB FM 973 to EB US 79												
via FM 973 Driveway 1B	100.0%	100.0%			-EBT,-NBR							
NB FM 973 to WB US 79												
via FM 973 Driveway 1B	100.0%	100.0%										
WB US 79 to SB FM 973												
via FM 973 Driveway 2B	100.0%	100.0%										
EB US 79 to SB FM 973												
via FM 973 Driveway 2B	100.0%	100.0%										

Table C3. Trip Generation Calculations - Phase 1+2

Trip Rates															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Rate*	Percent		Trip Rate*	Percent		Trip Rate*	Percent	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Fast-food w/Drive Through (Lot 1)	934	Fast-Food Restaurant w/Drive Through	1000 SF	4.400	24/AM/PM	---	467.48	50%	50%	44.61	51%	49%	33.03	52%	48%
C-Store w/Gas (Lot 2+3)	945	Convenience Store/Gas Station	1000 SF	5.600	AM/PM	24	700.43	50%	50%	56.52	50%	50%	54.52	50%	50%
Restaurant (Lots 4+5+8+9)	932	High-Turnover Sit Down Restaurants	1000 SF	26.350	24/AM/PM	---	107.20	50%	50%	9.57	55%	45%	9.05	61%	39%
Retail (Lots 4+5)	822	Strip Retail Plaza	1000 SF	11.350	24/AM/PM	---	54.45	50%	50%	2.36	60%	40%	6.59	50%	50%
Multifamily (Lots 8+9+10+11)	221	Multifamily Mid-Rise	Dwelling Unit	45	---	24/AM/PM	4.54	50%	50%	0.37	23%	77%	0.39	61%	39%
Live-Work Commercial (Office) (Lots 10+11)	710	General Office Building	1000 SF	29.000	---	24/AM/PM	10.84	50%	50%	1.52	88%	12%	1.44	17%	83%
Hotel 1 (Lot 6)	310	Hotel	Rooms	150	---	24/AM/PM	7.99	50%	50%	0.46	56%	44%	0.59	51%	49%
Hotel 2 (Lot 7)	310	Hotel	Rooms	150	---	24/AM/PM	7.99	50%	50%	0.46	56%	44%	0.59	51%	49%
Multifamily (Lots 12,13,14,15)	221	Multifamily Mid-Rise	Dwelling Unit	240	---	24/AM/PM	4.54	50%	50%	0.37	23%	77%	0.39	61%	39%
Townhouse (Lots 16+17)	215	Single-Family Attached Housing	Dwelling Unit	30	---	24/AM/PM	7.20	50%	50%	0.48	31%	69%	0.57	57%	43%
Trip End Calculations															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Fast-food w/Drive Through (Lot 1)	934	Fast-Food Restaurant w/Drive Through	1000 SF	4.400	24/AM/PM	---	2,057	1,028	1,028	196	100	96	145	76	70
C-Store w/Gas (Lot 2+3)	945	Convenience Store/Gas Station	1000 SF	5.600	AM/PM	24	3,690	1,845	1,845	317	158	158	305	153	153
Restaurant (Lots 4+5+8+9)	932	High-Turnover Sit Down Restaurants	1000 SF	26.350	24/AM/PM	---	2,825	1,412	1,412	252	139	113	238	145	93
Retail (Lots 4+5)	822	Strip Retail Plaza	1000 SF	11.350	24/AM/PM	---	618	309	309	27	16	11	75	37	37
Multifamily (Lots 8+9+10+11)	221	Multifamily Mid-Rise	Dwelling Unit	45	---	24/AM/PM	168	84	84	8	2	6	18	11	7
Live-Work Commercial (Office) (Lots 10+11)	710	General Office Building	1000 SF	29.000	---	24/AM/PM	395	198	198	58	51	7	59	10	49
Hotel 1 (Lot 6)	310	Hotel	Rooms	150	---	24/AM/PM	1,202	601	601	68	38	30	83	42	41
Hotel 2 (Lot 7)	310	Hotel	Rooms	150	---	24/AM/PM	1,202	601	601	68	38	30	83	42	41
Multifamily (Lots 12,13,14,15)	221	Multifamily Mid-Rise	Dwelling Unit	240	---	24/AM/PM	1,098	549	549	94	22	72	94	57	37
Townhouse (Lots 16+17)	215	Single-Family Attached Housing	Dwelling Unit	30	---	24/AM/PM	178	89	89	10	3	7	14	8	6
Trip End Totals															
							Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Trip Totals							13,434	6,717	6,717	1,097	566	531	1,115	582	533
*trip rates shown for information only, fitted curves used for trip generation															
Trip Adjustment Factors															
Adjustment Factor		Time Period		Factor		Net Phase 2 Trips:									
Internal Capture Rates:						1,276 638 638 104 25 79 108 65 43									
Weekday						-									
Weekday AM Peak Hour of Adjacent Roadway Entering						7.0%									
Weekday AM Peak Hour of Adjacent Roadway Exiting						7.0%									
Weekday PM Peak Hour of Adjacent Roadway Entering						18.0%									
Weekday PM Peak Hour of Adjacent Roadway Exiting						19.0%									
Pass-By Trips:						-									
Weekday						-									
Weekday AM Peak Hour of Adjacent Roadway						50.0% LU 934 (98)									
Weekday PM Peak Hour of Adjacent Roadway						55.0% LU 934 (80)									
Weekday AM Peak Hour of Adjacent Roadway						76.0% LU 945 (241)									
Weekday PM Peak Hour of Adjacent Roadway						75.0% LU 945 (229)									
Weekday AM Peak Hour of Adjacent Roadway						0.0% LU 932 -									
Weekday PM Peak Hour of Adjacent Roadway						43.0% LU 932 (103)									
Weekday AM Peak Hour of Adjacent Roadway						0.0% LU 822 (Using LU 821) -									
Weekday PM Peak Hour of Adjacent Roadway						40.0% LU 822 (Using LU 821) (7)									
Trip Totals															
Trip Type										Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
										Total Trips	Trips*		Total Trips	Trips*	
											Entering	Exiting		Entering	Exiting
Total Trips, Pre-Capture/Pass-By:										1,097	566	531	1,115	582	533
Total Trips, Captured Within Development:										77	40	37	206	105	101
Total Trips, New & Pass-By										1,020	527	493	909	477	432
Total Trips, Pass-By, Existing on Roadway Network:										339	169	169	419	209	209
Total Trips, New on Roadway Network:										681	357	324	491	268	223
*trip estimates subject to roundoff error															

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Taylor Gateway			Organization:	Voigt Associates, Inc.
Project Location:	US 79 at FM 973, Taylor TX			Performed By:	Anthony Voigt, P.E., PTOE
Scenario Description:	Phase 2 Buildout (AM Peak Hour)			Date:	02/22/2024
Analysis Year:	2027			Checked By:	
Analysis Period:	AM Street Peak Hour			Date:	

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	29,000	1000 SF	58	51	7
Retail	822	11,350	1000 SF	27	16	11
Restaurant	932+934	30,750	1000 SF	448	239	210
Cinema/Entertainment				0		
Residential	215+221	315	Dwelling Unit	112	27	86
Hotel	310	300	Rooms	135	76	59
All Other Land Uses ²	945	5,600	1000 SF	317	158	158
				1,097	566	531

Table 2-A: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office	1.00	0%	0%	1.00	0%	0%
Retail	1.50	0%	0%	1.50	0%	0%
Restaurant	1.50	0%	0%	1.50	0%	0%
Cinema/Entertainment	1.50	0%	0%	1.50	0%	0%
Residential	1.00	0%	0%	1.00	0%	0%
Hotel	1.00	0%	0%	1.00	0%	0%
All Other Land Uses ²	1.50	0%	0%	1.50	0%	0%

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-A: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		2	4	0	0	0
Retail	2		2	0	1	0
Restaurant	7	2		0	1	3
Cinema/Entertainment	0	0	0		0	0
Residential	2	1	17	0		0
Hotel	2	1	5	0	0	

Table 5-A: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	1,492	773	719
Internal Capture Percentage	7%	7%	7%
External Vehicle-Trips ⁵	1,010	526	484
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-A: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	25%	86%
Retail	25%	31%
Restaurant	8%	4%
Cinema/Entertainment	N/A	N/A
Residential	7%	23%
Hotel	4%	14%

¹ Land Use Codes (LUCs) from <i>Trip Generation Manual</i> , published by the Institute of Transportation Engineers.
² Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.
³ Enter trips assuming no transit or non-motorized trips (as assumed in ITE <i>Trip Generation Manual</i>).
⁴ Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.
⁵ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.
⁶ Person-Trips
*Indicates computation that has been rounded to the nearest whole number.
Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Table C3-A. Phase 1+2 Internal
Capture Calculations

Project Name:	Taylor Gateway
Analysis Period:	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-A (D): Entering Trips			Table 7-A (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	50.80735067	51	1.00	6.928275091	7
Retail	1.50	16.0716	24	1.50	10.7144	16
Restaurant	1.50	238.798065	358	1.50	209.655435	314
Cinema/Entertainment	1.50	0	0	1.50	0	0
Residential	1.00	26.5704	27	1.00	85.5096	86
Hotel	1.00	75.656	76	1.00	59.444	59

Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		2	4	0	0	0
Retail	5		2	0	2	0
Restaurant	97	44		0	13	9
Cinema/Entertainment	0	0	0		0	0
Residential	2	1	17	0		0
Hotel	44	8	5	0	0	

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		8	82	0	0	0
Retail	2		179	0	1	0
Restaurant	7	2		0	1	3
Cinema/Entertainment	0	0	0		0	0
Residential	2	4	72	0		0
Hotel	2	1	21	0	0	

Table 9-A (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	13	38	51	38	0	0
Retail	6	18	24	12	0	0
Restaurant	28	330	358	220	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	2	25	27	25	0	0
Hotel	3	73	76	73	0	0
All Other Land Uses ³	0	237	237	158	0	0

Table 9-A (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	6	1	7	1	0	0
Retail	5	11	16	7	0	0
Restaurant	13	301	314	201	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	20	66	86	66	0	0
Hotel	8	51	59	51	0	0
All Other Land Uses ³	0	237	237	158	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

Table C3-A. Phase 1+2 Internal
Capture Calculations

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Taylor Gateway			Organization:	Voigt Associates, Inc.
Project Location:	US 79 at FM 973, Taylor TX			Performed By:	Anthony Voigt, P.E., PTOE
Scenario Description:	Phase 2 Buildout (PM Peak Hour)			Date:	02/22/2024
Analysis Year:	2027			Checked By:	
Analysis Period:	PM Street Peak Hour			Date:	

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	29,000	1000 SF	59	10	49
Retail	822	11,350	1000 SF	75	37	37
Restaurant	932+934	30,750	1000 SF	384	221	163
Cinema/Entertainment				0		
Residential	215+221	315	Dwelling Unit	126	76	50
Hotel	310	300	Rooms	166	85	81
All Other Land Uses ²	945	5,600	1000 SF	305	153	153
				1,115	582	533

Table 2-P: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office	1.00	0%	0%	1.00	0%	0%
Retail	1.50	0%	0%	1.50	0%	0%
Restaurant	1.50	0%	0%	1.50	0%	0%
Cinema/Entertainment	1.50	0%	0%	1.50	0%	0%
Residential	1.00	0%	0%	1.00	0%	0%
Hotel	1.00	0%	0%	1.00	0%	0%
All Other Land Uses ²	1.50	0%	0%	1.50	0%	0%

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		800	800		600	
Retail					700	
Restaurant					700	
Cinema/Entertainment					700	
Residential		800	800			
Hotel					650	

Table 4-P: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		4	2	0	1	0
Retail	1		16	0	15	3
Restaurant	3	28		0	12	17
Cinema/Entertainment	0	0	0		0	0
Residential	2	5	9	0		2
Hotel	0	1	17	0	0	

Table 5-P: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	1,497	788	709
Internal Capture Percentage	18%	18%	19%
External Vehicle-Trips ⁵	899	472	427
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-P: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	60%	14%
Retail	68%	63%
Restaurant	13%	25%
Cinema/Entertainment	N/A	N/A
Residential	37%	36%
Hotel	26%	22%

¹ Land Use Codes (LUCs) from <i>Trip Generation Manual</i> , published by the Institute of Transportation Engineers.
² Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.
³ Enter trips assuming no transit or non-motorized trips (as assumed in ITE <i>Trip Generation Manual</i>).
⁴ Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be
⁵ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.
⁶ Person-Trips
*Indicates computation that has been rounded to the nearest whole number.
Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Table C3-A. Phase 1+2 Internal
Capture Calculations

Project Name:	Taylor Gateway
Analysis Period:	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-P (D): Entering Trips			Table 7-P (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	10.10366972	10	1.00	49.3296816	49
Retail	1.50	37.39825	56	1.50	37.39825	56
Restaurant	1.50	221.037815	332	1.50	162.761685	244
Cinema/Entertainment	1.50	0	0	1.50	0	0
Residential	1.00	76.2362	76	1.00	49.6638	50
Hotel	1.00	84.7722	85	1.00	81.4478	81

Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		8	2	0	1	0
Retail	1		16	2	15	3
Restaurant	7	100		20	44	17
Cinema/Entertainment	0	0	0		0	0
Residential	2	17	9	0		2
Hotel	0	13	55	0	2	

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		4	5	0	3	0
Retail	3		96	0	35	14
Restaurant	3	28		0	12	60
Cinema/Entertainment	1	2	10		3	1
Residential	6	5	38	0		10
Hotel	0	1	17	0	0	

Table 9-P (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	6	4	10	4	0	0
Retail	38	18	56	12	0	0
Restaurant	44	288	332	192	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	28	48	76	48	0	0
Hotel	22	63	85	63	0	0
All Other Land Uses ³	0	229	229	153	0	0

Table 9-P (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	7	42	49	42	0	0
Retail	35	21	56	14	0	0
Restaurant	60	184	244	123	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	18	32	50	32	0	0
Hotel	18	63	81	63	0	0
All Other Land Uses ³	0	229	229	153	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

Table C3-A. Phase 1+2 Internal

Table 7.1a Adjusted Internal Trip Capture Rates for Trip Origins within a Multi-Use Development			
Land Use Pairs		Weekday	
		AM Peak Hour	PM Peak Hour
From OFFICE	To Office	0.0%	0.0%
	To Retail	28.0%	16.4%
	To Restaurant	63.0%	3.3%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	1.0%	2.0%
	To Hotel	0.0%	0.0%
From RETAIL	To Office	29.0%	2.0%
	To Retail	0.0%	0.0%
	To Restaurant	13.0%	29.0%
	To Cinema/Entertainment	0.0%	4.0%
	To Residential	14.0%	26.0%
	To Hotel	0.0%	5.0%
From RESTAURANT	To Office	31.0%	3.0%
	To Retail	14.0%	41.0%
	To Restaurant	0.0%	0.0%
	To Cinema/Entertainment	0.0%	8.0%
	To Residential	4.0%	18.0%
	To Hotel	3.0%	7.0%
From CINEMA/ENTERTAINMENT	To Office	0.0%	2.0%
	To Retail	0.0%	21.0%
	To Restaurant	0.0%	31.0%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	8.0%
	To Hotel	0.0%	2.0%
From RESIDENTIAL	To Office	2.0%	4.0%
	To Retail	1.0%	34.4%
	To Restaurant	20.0%	17.2%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	0.0%
	To Hotel	0.0%	3.0%
From HOTEL	To Office	75.0%	0.0%
	To Retail	14.0%	16.0%
	To Restaurant	9.0%	68.0%
	To Cinema/Entertainment	0.0%	0.0%
	To Residential	0.0%	2.0%
	To Hotel	0.0%	0.0%

Table C3-A. Phase 1+2 Internal

Table 7.2a Adjusted Internal Trip Capture Rates for Trip Destinations within a Multi-Use Development			
Land Use Pairs		Weekday	
		AM Peak Hour	PM Peak Hour
To OFFICE	From Office	0.0%	0.0%
	From Retail	4.0%	31.0%
	From Restaurant	14.0%	30.0%
	From Cinema/Entertainment	0.0%	6.0%
	From Residential	3.0%	57.0%
	From Hotel	3.0%	0.0%
To RETAIL	From Office	32.0%	6.6%
	From Retail	0.0%	0.0%
	From Restaurant	8.0%	50.0%
	From Cinema/Entertainment	0.0%	4.0%
	From Residential	17.0%	8.2%
	From Hotel	4.0%	2.0%
To RESTAURANT	From Office	23.0%	1.6%
	From Retail	50.0%	29.0%
	From Restaurant	0.0%	0.0%
	From Cinema/Entertainment	0.0%	3.0%
	From Residential	20.0%	11.5%
	From Hotel	6.0%	5.0%
To CINEMA/ENTERTAINMENT	From Office	0.0%	1.0%
	From Retail	0.0%	26.0%
	From Restaurant	0.0%	32.0%
	From Cinema/Entertainment	0.0%	0.0%
	From Residential	0.0%	0.0%
	From Hotel	0.0%	0.0%
To RESIDENTIAL	From Office	0.0%	4.0%
	From Retail	2.0%	46.0%
	From Restaurant	5.0%	16.0%
	From Cinema/Entertainment	0.0%	4.0%
	From Residential	0.0%	0.0%
	From Hotel	0.0%	0.0%
To HOTEL	From Office	0.0%	0.0%
	From Retail	0.0%	17.0%
	From Restaurant	4.0%	71.0%
	From Cinema/Entertainment	0.0%	1.0%
	From Residential	0.0%	12.0%
	From Hotel	0.0%	0.0%

Table C4. Trip Distribution & Assignment, Phase 1+2 Development

GLOBAL ORIGINS/DESTINATIONS								
Origins-Peak Hour Volume			Destinations-Peak Hour Volume					
	AM	PM		AM	PM		AM	PM
From US 79, West of FM 973	865	781	To US 79, West of FM 973	792	594			
From US 79, East of South Rio Grande Street	743	471	To US 79, East of South Rio Grande Street	487	871			
From FM 973, South of Wesley Miller Lane	276	453	To FM 973, South of Wesley Miller Lane	360	336			
From South Rio Grande Street, North of US 79	5	6	To South Rio Grande Street, North of US 79	9	4			
From South Rio Grande Street, South of US 79	26	16	To South Rio Grande Street, South of US 79	11	28			
From Streamwood Trail, West of FM 973	111	75	To Streamwood Trail, West of FM 973	39	127			
From Wesley Miller, West of FM 973	12	8	To Wesley Miller, West of FM 973	4	14			
From Taylor HS Driveway, East of FM 973	69	15	To Taylor HS Driveway, East of FM 973	176	3			
TRIP DISTRIBUTION (NEW TRIPS)								
FROM (ENTERING DEVELOPMENT)	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
From US 79, West of FM 973	865	50.0%	41.0%	179	781	50.0%	42.8%	134
From US 79, East of South Rio Grande Street	743	20.0%	35.3%	71	471	20.0%	25.8%	54
From FM 973, South of Wesley Miller Lane	276	18.0%	13.1%	64	453	18.0%	24.8%	48
From South Rio Grande Street, North of US 79	5	1.0%	0.2%	4	6	1.0%	0.3%	3
From South Rio Grande Street, South of US 79	26	2.0%	1.2%	7	16	2.0%	0.9%	5
From Streamwood Trail, West of FM 973	111	4.0%	5.3%	14	75	4.0%	4.1%	11
From Wesley Miller, West of FM 973	12	1.0%	0.6%	4	8	1.0%	0.5%	3
From Taylor HS Driveway, East of FM 973	69	4.0%	3.3%	14	15	4.0%	0.8%	11
Total	2108	100.0%	100.0%	357	1825	100.0%	100.0%	288
TRIP DISTRIBUTION (PASS-BY/DIVERTED LINK TRIPS)								
PASS-BY TRIP PATH	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
EB to EB US 79	308	25.0%	15.1%	42	587	40.0%	33.9%	84
WB to WB US 79	477	15.0%	23.4%	25	345	10.0%	19.9%	21
NB FM 973 to EB US 79	130	5.0%	6.4%	8	234	15.0%	13.5%	31
NB FM 973 to WB US 79	314	15.0%	15.4%	25	249	15.0%	14.4%	31
WB US 79 to SB FM 973	261	10.0%	12.8%	17	132	10.0%	7.6%	21
EB US 79 to SB FM 973	551	30.0%	27.0%	51	187	10.0%	10.8%	21
Total	2041	100.0%	100.0%	169	1734	100.0%	100.0%	209

TRIP DISTRIBUTION DETAILS - NEW TRIPS													
ROUTE (ENTERING DEVELOPMENT)												AM PEAK HOUR Vehicles	PM PEAK HOUR Vehicles
FROM (ENTERING DEVELOPMENT)	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/R0)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller	AM PEAK HOUR Percent of Entering Vehicles
From US 79, West of FM 973													
via US 79 Driveway 2A	85.0%	85.0%	EBT	EBT	EBR								152
via FM 973 Driveway 2B	15.0%	15.0%	EBT					SBT	SBL				27
From US 79, East of South Rio Grande Street													114
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	WBT						61
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	WBT	SBT	SBL				46
From FM 973, South of Wesley Miller Lane													8
via FM 973 Driveway 1B	80.0%	80.0%						NBR	NBT	NBT	NBT	NBT	51
via FM 973 Driveway 2B	20.0%	20.0%						NBR	NBT	NBT	NBT	NBT	13
From South Rio Grande Street, North of US 79													39
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	SBR						6
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	SBR	SBT	SBL				5
From South Rio Grande Street, South of US 79													2
via US 79 Driveway 2A	85.0%	85.0%		WBT EBT	WBT EBR	WBT	NBL						3
via FM 973 Driveway 2B	15.0%	15.0%	WBL	WBT	WBT	WBT	NBL	SBT	SBL				1
From Streamwood Trail, West of FM 973													1
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	NBT	EBL		9
via FM 973 Driveway 2B	40.0%	40.0%						NBR	NBT	NBT	EBL		6
From Wesley Miller, West of FM 973													6
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	NBT	NBT	EBL	2
via FM 973 Driveway 2B	40.0%	40.0%						NBR	NBT	NBT	NBT	EBL	1
From Taylor HS Driveway, East of FM 973													2
via FM 973 Driveway 1B	60.0%	60.0%						NBR	NBT	WBR			9
via FM 973 Driveway 2B	40.0%	40.0%						NBR	NBT	WBR			6
ROUTE (EXITING DEVELOPMENT)													
TO (EXITING DEVELOPMENT)	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/R0)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller	AM PEAK HOUR Percent of Exiting Vehicles
To US 79, West of FM 973													
via US 79 Driveway 2A	45.0%	45.0%	WBT	WBT	NBR WBT	EBU							73
via FM 973 Driveway 1B	30.0%	30.0%	NBL					WBR					49
via FM 973 Driveway 2B	25.0%	25.0%	NBL					NBT	WBR				33
To US 79, East of South Rio Grande Street													28
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBT						55
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	NBR	EBT	EBT	WBR					3
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBT	NBT	WBR				2
To FM 973, South of Wesley Miller Lane													4
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBT	SBT	6
To South Rio Grande Street, North of US 79													40
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBL						3
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	EBT	EBT	EBL	WBR					0
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBL	NBT	WBR				0
To South Rio Grande Street, South of US 79													0
via US 79 Driveway 2A	85.0%	85.0%			NBR	EBT	EBR						6
via FM 973 Driveway 1B	5.0%	5.0%	NBR	EBT	EBT	EBT	EBR	WBR					0
via FM 973 Driveway 2B	10.0%	10.0%	NBR	EBT	EBT	EBT	EBR	NBT	WBR				0
To Streamwood Trail, West of FM 973													4
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBR		8
To Wesley Miller, West of FM 973													2
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBT	SBT	SBR	13
To Taylor HS Driveway, East of FM 973													9
via FM 973 Driveway 2B	100.0%	100.0%							WBL	SBL			3

PASS-BY TRIPS													
ROUTE (EXITING DEVELOPMENT)												AM PEAK HOUR Vehicles	PM PEAK HOUR Vehicles
PASS-BY TRIPS	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B (R/R0)	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller	
EB to EB US 79													42
via US 79 Driveway 2A	100.0%	100.0%				+EBR,EBT,+NBR							84
WB to WB US 79													42
via US 79 Driveway 2A													25
via FM 973 Driveway 2B	80.0%	80.0%	+WBU		+EBR,+NBR,+WBT	+EBU							17
NB FM 973 to EB US 79	20.0%	20.0%	+WBL,+WBT,+NBL					+SBT,+NBT	+SBL,+WBR				20
via FM 973 Driveway 1B													5
via FM 973 Driveway 2B													4
NB FM 973 to WB US 79													8
via FM 973 Driveway 1B	100.0%	100.0%	+NBR		+EBT,+NBR			+NBR,+NBT					31
via FM 973 Driveway 2B													8
NB FM 973 to WB US 79													25
via FM 973 Driveway 1B	100.0%	100.0%						+NBR,+NBT,+WBR					31
via FM 973 Driveway 2B													25
WB US 79 to SB FM 973													17
via FM 973 Driveway 2B	100.0%	100.0%							+SBL,+SBT,+WBL				21
EB US 79 to SB FM 973													17
via FM 973 Driveway 2B	100.0%	100.0%											51
via FM 973 Driveway 2B	100.0%	100.0%							+SBL,+SBT,+WBL				21

Table C5. Trip Generation Calculations, Castlewood Subdivision (Background)

Trip Rates															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Rate*	Percent		Trip Rate*	Percent		Trip Rate*	Percent	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Castlewood Subdivision	210	Single-Family Detached Housing	Dwelling Unit	239	--	24/AM/PM	9.43	50%	50%	0.70	26%	74%	0.94	63%	37%
Trip End Calculations															
Development Description	ITE Trip Generation Land Use Number	Trip Generation Land Use	Independent Variable	Value	Use Rate	Use Curve	Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
Castlewood Subdivision	210	Single-Family Detached Housing	Dwelling Unit	239	--	24/AM/PM	2,254	1,127	1,127	167	43	124	225	142	83
Trip End Totals															
Trip Totals							Weekday			Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
							24-Hr Trip Ends	Trips		Peak Hour Trips	Trips		Peak Hour Trips	Trips	
								Entering	Exiting		Entering	Exiting		Entering	Exiting
							2,254	1,127	1,127	167	43	124	225	142	83

*trip rates shown for information only, fitted curves used for trip generation

Trip Adjustment Factors		
Adjustment Factor	Time Period	Factor
Internal Capture Rates:		
Weekday		
Weekday AM Peak Hour of Adjacent Roadway		0.0%
Weekday PM Peak Hour of Adjacent Roadway		0.0%
Pass-By Trips:		
Weekday		
Weekday AM Peak Hour of Adjacent Roadway		0.0%
Weekday PM Peak Hour of Adjacent Roadway		0.0%

Trip Totals						
Trip Type	Weekday AM Peak Hour of Adjacent Roadway			Weekday PM Peak Hour of Adjacent Roadway		
	Total Trips	Trips*		Total Trips	Trips*	
		Entering	Exiting		Entering	Exiting
Total Trips, Pre-Capture/Pass-By:	167	43	124	225	142	83
Total Trips, Captured Within Development:	-	-	-	-	-	-
Total Trips, New & Pass-By	167	43	124	225	142	83
Total Trips, Pass-By, Existing on Roadway Network:	-	-	-	-	-	-
Total Trips, New on Roadway Network:	167	43	124	225	142	83

*trip estimates subject to roundoff error

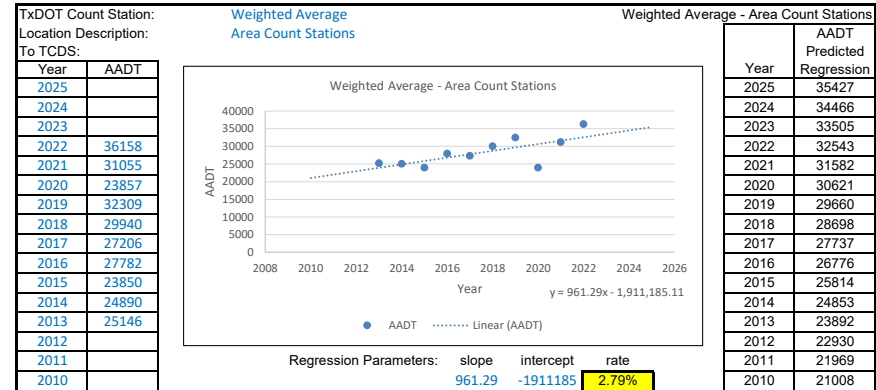
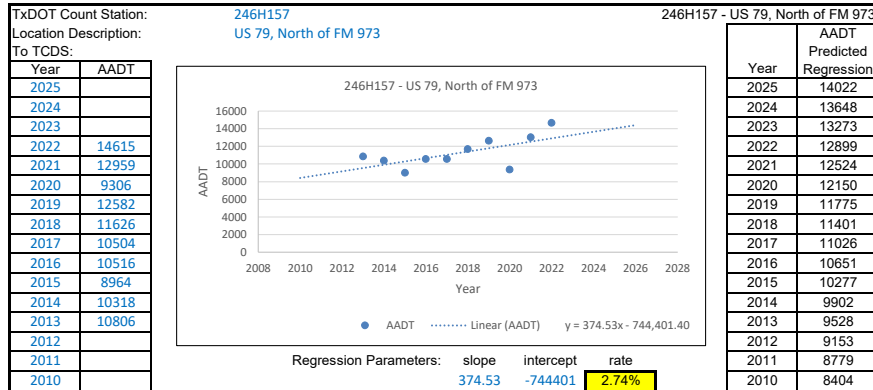
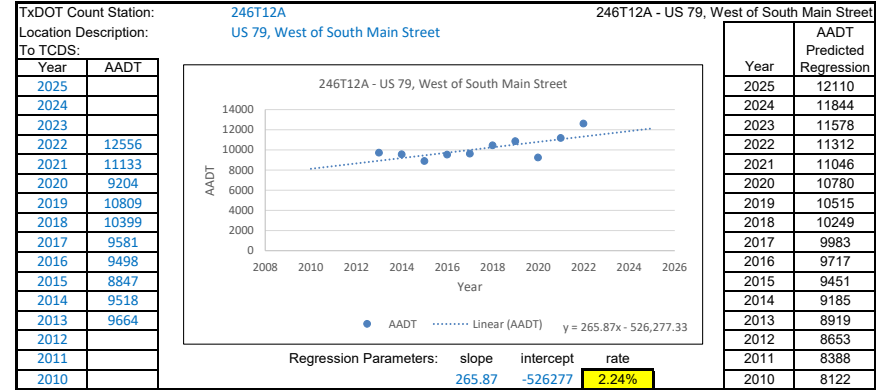
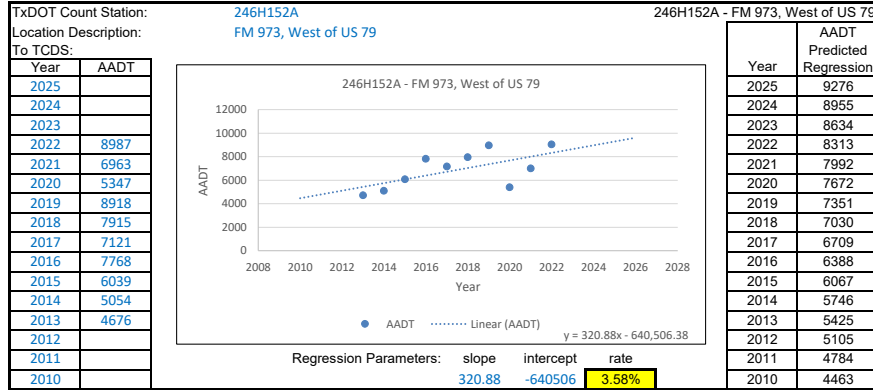
Table C6. Trip Distribution & Assignment, Castlewood Subdivision

GLOBAL ORIGINS/DESTINATIONS								
Origins-Peak Hour Volume			Destinations-Peak Hour Volume					
	AM	PM		AM	PM			
From US 79, West of FM 973	865	781	To US 79, West of FM 973	792	594			
From US 79, East of South Rio Grande Street	743	471	To US 79, East of South Rio Grande Street	487	871			
From FM 973, South of Wesley Miller Lane	276	453	To FM 973, South of Wesley Miller Lane	360	336			
From South Rio Grande Street, North of US 79	5	6	To South Rio Grande Street, North of US 79	9	4			
From South Rio Grande Street, South of US 79	26	16	To South Rio Grande Street, South of US 79	11	28			
From Streamwood Trail, West of FM 973	111	75	To Streamwood Trail, West of FM 973	39	127			
From Wesley Miller, West of FM 973	12	8	To Wesley Miller, West of FM 973	4	14			
From Taylor HS Driveway, East of FM 973	69	15	To Taylor HS Driveway, East of FM 973	176	3			
TRIP DISTRIBUTION (NEW TRIPS)								
FROM (ENTERING DEVELOPMENT)	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
From US 79, West of FM 973	865	50.0%	41.0%	22	781	50.0%	42.8%	71
From US 79, East of South Rio Grande Street	743	15.0%	35.3%	7	471	15.0%	25.8%	21
From FM 973, South of Wesley Miller Lane	276	30.0%	13.1%	13	453	30.0%	24.8%	42
From Taylor HS Driveway, East of FM 973	69	5.0%	3.3%	2	15	5.0%	0.8%	7
Total	2108	100.0%	100.0%	43	1825	100.0%	100.0%	142
TO (EXITING DEVELOPMENT)	Existing Total Volume AM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	AM Peak Development Volume*	Existing Total Volume PM Peak	Manually Estimated Trip Distribution (%)	Volume Based Trip Distribution Percentage	PM Peak Development Volume*
To US 79, West of FM 973	792	50.0%	42.2%	62	594	50.0%	30.0%	42
To US 79, East of South Rio Grande Street	487	15.0%	25.9%	19	871	15.0%	44.0%	12
To FM 973, South of Wesley Miller Lane	360	30.0%	19.2%	37	336	30.0%	17.0%	25
To Taylor HS Driveway, East of FM 973	176	5.0%	9.4%	6	3	5.0%	0.2%	4
Total	1879	100.0%	100.0%	124	1977	100.0%	100.0%	83

*Development volumes and sum totals rounded to whole vehicles

TRIP DISTRIBUTION DETAILS - NEW TRIPS															
ROUTE (ENTERING DEVELOPMENT)												AM PEAK HOUR Vehicles	PM PEAK HOUR Vehicles	AM PEAK HOUR Percent of Entering Vehicles	PM PEAK HOUR Percent of Entering Vehicles
FROM (ENTERING DEVELOPMENT)	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller			
From US 79, West of FM 973															
via Streamwood Trail	90.0%	90.0%	EBR					SBT	SBT	SBT	SBR		20	64	45.00%
via Wesley Miller	10.0%	10.0%	EBR					SBT	SBT	SBT	SBT	SBR	2	7	5.00%
From US 79, East of South Rio Grande Street															
via Streamwood Trail	90.0%	90.0%	WBL	WBT	WBT	WBT	WBT	SBT	SBT	SBT	SBR		6	19	13.50%
via Wesley Miller	10.0%	10.0%	WBL	WBT	WBT	WBT	WBT	SBT	SBT	SBT	SBT	SBR	1	2	1.50%
From FM 973, South of Wesley Miller Lane															
via Streamwood Trail	90.0%	90.0%									NBL	NBT	12	38	27.00%
via Wesley Miller	10.0%	10.0%									NBL	NBT	1	4	3.00%
From Taylor HS Driveway, East of FM 973															
via Streamwood Trail	90.0%	90.0%								WBL	SBR		2	6	4.50%
via Wesley Miller	10.0%	10.0%								WBL	SBT	SBR	0	1	0.50%
ROUTE (EXITING DEVELOPMENT)												AM PEAK HOUR Vehicles	PM PEAK HOUR Vehicles	AM PEAK HOUR Percent of Exiting Vehicles	PM PEAK HOUR Percent of Exiting Vehicles
TO (EXITING DEVELOPMENT)	AM Percent Dist.	PM Percent Dist.	Int01: US 79 at FM 973	Int02: US 79 at Driveway 1A	Int03: US 79 at Driveway 2A	Int04: US 79 at Crossover	Int05: US 79 at S Rio Grande St	Int06: FM973 at Driveway 1B	Int07: FM973 at Driveway 2B	Int08: FM973 at Taylor HS Driveway	Int09: FM973 at Streamwood	Int10: FM973 at Wesley Miller			
To US 79, West of FM 973															
via Streamwood Trail	90.0%	90.0%	NBL					NBT	NBT	NBT	EBL		56	37	45.00%
via Wesley Miller	10.0%	10.0%	NBL					NBT	NBT	NBT	NBT	EBL	6	4	5.00%
To US 79, East of South Rio Grande Street															
via Streamwood Trail	90.0%	90.0%	NBR	EBT	EBT	EBT	EBT	NBT	NBT	NBT	EBL		17	11	13.50%
via Wesley Miller	10.0%	10.0%	NBR	EBT	EBT	EBT	EBT	NBT	NBT	NBT	NBT	EBL	2	1	1.50%
To FM 973, South of Wesley Miller Lane															
via Streamwood Trail	90.0%	90.0%									EBR	SBT	33	22	27.00%
via Wesley Miller	10.0%	10.0%									EBR	EBR	4	2	3.00%
To Taylor HS Driveway, East of FM 973															
via Streamwood Trail	90.0%	90.0%								NBR	EBL		6	4	4.50%
via Wesley Miller	10.0%	10.0%								NBR	NBT	EBL	1	0	0.50%

Table C7. Background Traffic Growth Rate Calculations



Overall Average:	2.86%
Weighted Average:	2.79%
Use in Analysis:	5.00%

Table C8. Existing (2022) Peak Hour Turning Movement Counts

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	308	551	6	261	477	2	0	314	0	130	0	0	0	1	0
PM Peak	0	587	187	7	132	345	2	0	249	0	234	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		438				740										
PM Peak		821				479										

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		438				740										
PM Peak		821				479										

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	454	0	2	0	778	0	0	0	0	0	0	0	0	0	0
PM Peak	0	840	0	0	0	501	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

Seasonal Adjustment AM Pe 1.147 PM Pe 1.138

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	3	479	5	0	6	736	1	0	23	6	1	0	2	2	1	0
PM Peak	2	847	22	0	5	467	0	0	14	2	2	0	1	6	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak										444				812		
PM Peak										483				319		

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak										444				812		
PM Peak										483				319		

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	3	0	66	0	0	392	5	0	171	649	0	1
PM Peak	0	0	0	0	1	0	14	0	0	473	0	0	3	333	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	1	0	0	0	0	0	0	0	0	395	0	1	0	649	1	0
PM Peak	0	0	0	0	0	0	0	0	0	468	0	0	0	337	0	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	1	0	15	0	154	0	1	241	34	0	306	344	0	0
PM Peak	0	0	1	0	4	0	12	0	0	453	0	0	2	331	1	0

Table C9-A. Projected Peak Hour New Site Trips, Castlewood Subdivision

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	22	0	7	0	0	0	62	0	19	0	0	0	0	0
PM Peak	0	0	71	0	21	0	0	0	42	0	12	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	19	0	0	0	7	0	0	0	0	0	0	0	0	0	0
PM Peak	0	12	0	0	0	21	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	19	0	0	0	7	0	0	0	0	0	0	0	0	0	0
PM Peak	0	12	0	0	0	21	0	0	0	0	0	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	19	0	0	0	7	0	0	0	0	0	0	0	0	0	0
PM Peak	0	12	0	0	0	21	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	19	0	0	0	7	0	0	0	0	0	0	0	0	0	0
PM Peak	0	12	0	0	0	21	0	0	0	0	0	0	0	0	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	80	0	0	0	28	0	0
PM Peak	0	0	0	0	0	0	0	0	0	54	0	0	0	92	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	80	0	0	0	28	0	0
PM Peak	0	0	0	0	0	0	0	0	0	54	0	0	0	92	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	2	0	0	0	0	80	6	0	0	28	0	0
PM Peak	0	0	0	0	7	0	0	0	0	54	4	0	0	92	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	78	0	33	0	0	0	0	0	12	9	0	0	0	3	27	0
PM Peak	52	0	22	0	0	0	0	0	38	6	0	0	0	10	89	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	9	0	4	0	0	0	0	0	1	12	0	0	0	33	3	0
PM Peak	6	0	2	0	0	0	0	0	4	38	0	0	0	22	10	0

Table C9-B. Projected Peak Hour New Site Trips, Samsung Plant (Phase 1)
As provided by City of Taylor
1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak			73		146				10		21					
PM Peak			4		7				61		123					

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		21			146											
PM Peak		123			7											

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		21			146											
PM Peak		123			7											

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		21			146											
PM Peak		123			7											

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak		21			146											
PM Peak		123			7											

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak									31				219			
PM Peak									184				11			

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak									31				219			
PM Peak									184				11			

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak									31				219			
PM Peak									184				11			

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak									31				219			
PM Peak									184				11			

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak									31				219			
PM Peak									184				11			

Table C10. Projected 2025 Peak Hour Turning Movement Counts (Phase 1 Build-Out Year without development)

1. US 79 at FM 973

Adjustment Factor: 1.150

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	354	728	7	453	549	2	0	433	0	189	0	0	0	1	0
PM Peak	0	675	290	8	180	397	2	0	389	0	405	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	543	0	0	0	1004	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1080	0	0	0	579	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	543	0	0	0	1004	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1080	0	0	0	579	0	0	0	0	0	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	562	0	2	0	1047	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1101	0	0	0	604	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	4	591	5	0	7	999	1	0	26	7	1	0	3	3	1	0
PM Peak	3	1109	25	0	5	565	0	0	16	3	3	0	1	7	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	622	0	0	0	1181	0	0
PM Peak	0	0	0	0	0	0	0	0	0	793	0	0	0	470	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	622	0	0	0	1181	0	0
PM Peak	0	0	0	0	0	0	0	0	0	793	0	0	0	470	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	6	0	76	0	0	562	12	0	197	994	0	1
PM Peak	0	0	0	0	8	0	16	0	0	782	4	0	3	486	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	79	0	33	0	0	0	0	0	12	494	0	1	0	968	29	0
PM Peak	52	0	22	0	0	0	0	0	38	728	0	0	0	408	89	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	9	0	5	0	17	0	177	0	2	320	39	0	352	648	3	0
PM Peak	6	0	4	0	5	0	14	0	4	743	0	0	2	414	11	0

Table C11. Projected 2027 Peak Hour Turning Movement Counts (Phase 2 Build-Out Year without development)

1. US 79 at FM 973

Adjustment Factor: 1.250

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	385	783	8	479	596	3	0	464	0	202	0	0	0	1	0
PM Peak	0	734	378	9	332	431	3	0	363	0	326	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	587	0	0	0	1078	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1060	0	0	0	766	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	587	0	0	0	1078	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1060	0	0	0	766	0	0	0	0	0	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	607	0	3	0	1125	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1083	0	0	0	793	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	4	639	6	0	7	1073	1	0	29	7	1	0	3	3	1	0
PM Peak	3	1092	27	0	6	751	0	0	17	3	3	0	1	7	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	666	0	0	0	1262	0	0
PM Peak	0	0	0	0	0	0	0	0	0	689	0	0	0	710	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	666	0	0	0	1262	0	0
PM Peak	0	0	0	0	0	0	0	0	0	689	0	0	0	710	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	6	0	83	0	0	601	12	0	214	1059	0	1
PM Peak	0	0	0	0	8	0	18	0	0	676	4	0	4	727	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	79	0	33	0	0	0	0	0	12	533	0	1	0	1033	29	0
PM Peak	52	0	22	0	0	0	0	0	38	622	0	0	0	650	89	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	9	0	5	0	19	0	193	0	3	344	43	0	383	682	3	0
PM Peak	6	0	4	0	5	0	15	0	4	635	0	0	3	655	11	0

Table C12. Projected Peak Hour New Site Trips, Phase 1 of Development

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	175	0	0	12	58	0	0	71	0	9	0	0	0	0	0
PM Peak	0	118	0	0	8	47	0	0	57	0	7	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	226	0	0	0	139	0	0	0	0	0	0	0	0	0	0
PM Peak	0	153	0	0	0	101	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	9	217	0	0	139	0	0	0	0	109	0	0	0	0	0
PM Peak	0	7	146	0	0	101	0	0	0	0	87	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	60	0	58	0	81	0	0	0	0	0	0	0	0	0	0
PM Peak	0	48	0	47	0	54	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	3	52	5	0	0	70	0	0	7	0	0	0	0	0	4	0
PM Peak	2	42	4	0	0	47	0	0	5	0	0	0	0	0	2	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	42	0	0	38	69	0	0	38	0	0
PM Peak	0	0	0	0	0	0	34	0	0	31	47	0	0	26	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	70	0	38	0	0	69	25	0	38	0	0	0
PM Peak	0	0	0	0	56	0	31	0	0	47	17	0	26	0	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	14	0	0	81	0	0	10	60	0	0
PM Peak	0	0	0	0	0	0	9	0	0	54	0	0	8	48	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	14	0	0	0	0	0	0	0	0	67	0	0	0	49	10	0
PM Peak	9	0	0	0	0	0	0	0	0	45	0	0	0	39	8	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	4	0	0	0	0	0	0	0	0	63	0	0	0	47	3	0
PM Peak	2	0	0	0	0	0	0	0	0	42	0	0	0	37	2	0

Table C13. Projected Peak Hour New Site Trips, Phase 1+2 of Development

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	179	0	0	12	73	0	0	89	0	11	0	0	0	0	0
PM Peak	0	134	0	0	9	50	0	0	61	0	8	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	233	0	0	0	155	0	0	0	0	0	0	0	0	0	0
PM Peak	0	174	0	0	0	112	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	11	222	0	0	155	0	0	0	0	136	0	0	0	0	0
PM Peak	0	8	166	0	0	112	0	0	0	0	94	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	75	0	73	0	82	0	0	0	0	0	0	0	0	0	0
PM Peak	0	51	0	50	0	62	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	3	65	6	0	0	71	0	0	7	0	0	0	0	0	4	0
PM Peak	2	45	4	0	0	54	0	0	5	0	0	0	0	0	3	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	52	0	0	48	71	0	0	39	0	0
PM Peak	0	0	0	0	0	0	36	0	0	33	53	0	0	29	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	87	0	48	0	0	71	26	0	39	0	0	0
PM Peak	0	0	0	0	60	0	33	0	0	53	19	0	29	0	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	14	0	0	82	0	0	13	75	0	0
PM Peak	0	0	0	0	0	0	11	0	0	62	0	0	9	51	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	14	0	0	0	0	0	0	0	0	68	0	0	0	62	13	0
PM Peak	11	0	0	0	0	0	0	0	0	51	0	0	0	42	9	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	4	0	0	0	0	0	0	0	0	64	0	0	0	58	3	0
PM Peak	3	0	0	0	0	0	0	0	0	48	0	0	0	40	2	0

Table C14. Projected Peak Hour Pass-By Trips, Phase 1 of Development

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	5	-5	0	0	5	0	-8	0	0	0	0	0
PM Peak	0	0	0	0	4	-4	0	0	4	0	-31	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	-51	63	0	0	20	0	0	0	0	71	0	0	0	0	0
PM Peak	0	-115	100	0	0	17	0	0	0	0	132	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	25	0	0	-29	34	0	0	5	0	0
PM Peak	0	0	0	0	0	0	31	0	0	-59	63	0	0	4	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	68	0	5	0	0	0	0	0	73	-68	0	0
PM Peak	0	0	0	0	42	0	4	0	0	0	0	0	46	-42	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Table C15. Projected Peak Hour Pass-By Trips, Phase 1+2 of Development

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	5	-5	0	0	5	0	-8	0	0	0	0	0
PM Peak	0	0	0	0	4	-4	0	0	4	0	-31	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	-51	63	0	0	20	0	0	0	0	71	0	0	0	0	0
PM Peak	0	-115	100	0	0	17	0	0	0	0	132	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	25	0	0	-29	34	0	0	5	0	0
PM Peak	0	0	0	0	0	0	31	0	0	-59	63	0	0	4	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	68	0	5	0	0	0	0	0	73	-68	0	0
PM Peak	0	0	0	0	42	0	4	0	0	0	0	0	46	-42	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM Peak	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Table C16. Projected 2025 Peak Hour Turning Movement Counts (w/Phase 1 development)

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	529	728	7	470	602	2	0	509	0	190	0	0	0	1	0
PM Peak	0	793	290	8	192	439	2	0	450	0	380	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	770	0	0	0	1142	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1233	0	0	0	680	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	501	280	0	0	1163	0	0	0	0	180	0	0	0	0	0
PM Peak	0	972	246	0	0	697	0	0	0	0	219	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	621	0	61	0	1128	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1149	0	47	0	658	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	7	643	10	0	7	1069	1	0	33	7	1	0	3	3	5	0
PM Peak	5	1151	29	0	5	612	0	0	20	3	3	0	1	7	2	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	67	0	0	632	103	0	0	1225	0	0
PM Peak	0	0	0	0	0	0	65	0	0	766	109	0	0	500	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	138	0	43	0	0	691	25	0	111	1113	0	0
PM Peak	0	0	0	0	98	0	35	0	0	840	17	0	72	428	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	6	0	90	0	0	643	12	0	207	1053	0	1
PM Peak	0	0	0	0	8	0	26	0	0	836	4	0	12	534	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	93	0	33	0	0	0	0	0	12	561	0	1	0	1018	39	0
PM Peak	62	0	22	0	0	0	0	0	38	773	0	0	0	448	97	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	12	0	5	0	17	0	177	0	2	383	39	0	352	695	6	0
PM Peak	8	0	4	0	5	0	14	0	4	786	0	0	2	451	13	0

Table C17. Projected 2027 Peak Hour Turning Movement Counts (w/Phase 1+2 development)

1. US 79 at FM 973

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				FM 973				Private Driveway			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	564	783	8	496	664	3	0	559	0	205	0	0	0	1	0
PM Peak	0	868	378	9	346	477	3	0	428	0	302	0	0	0	0	0

2. US 79 at Driveway 1A (REMOVED)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 1A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	820	0	0	0	1233	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1234	0	0	0	878	0	0	0	0	0	0	0	0	0	0

3. US 79 at Driveway 2A (Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				Driveway 2A				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	547	284	0	0	1253	0	0	0	0	207	0	0	0	0	0
PM Peak	0	952	267	0	0	894	0	0	0	0	225	0	0	0	0	0

4. US 79 at Crossover West of South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				<<NO APPROACH>>				<<NO APPROACH>>			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	682	0	75	0	1207	0	0	0	0	0	0	0	0	0	0
PM Peak	0	1135	0	50	0	855	0	0	0	0	0	0	0	0	0	0

5. US 79 at South Rio Grande Street

	Eastbound				Westbound				Northbound				Southbound			
	US 79				US 79				South Rio Grande Street				South Rio Grande Street			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	8	704	12	0	7	1144	1	0	36	7	1	0	3	3	5	0
PM Peak	5	1137	31	0	6	804	0	0	22	3	3	0	1	7	3	0

6. FM 973 at Driveway 1B (East Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 1B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	0	0	78	0	0	686	105	0	0	1306	0	0
PM Peak	0	0	0	0	0	0	67	0	0	663	116	0	0	743	0	0

7. FM 973 at Driveway 2B (West Site Driveway)

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Driveway 2B				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	155	0	53	0	0	737	26	0	112	1195	0	0
PM Peak	0	0	0	0	102	0	37	0	0	742	19	0	75	668	0	0

8. FM 973 at Taylor High School Driveway

	Eastbound				Westbound				Northbound				Southbound			
	<<NO APPROACH>>				Taylor High School Driveway				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	0	0	0	0	6	0	97	0	0	684	12	0	227	1133	0	1
PM Peak	0	0	0	0	8	0	28	0	0	738	4	0	13	778	0	0

9. FM 973 at Streamwood Trail

	Eastbound				Westbound				Northbound				Southbound			
	Streamwood Trail				<<NO APPROACH>>				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	94	0	33	0	0	0	0	0	12	601	0	1	0	1095	42	0
PM Peak	63	0	22	0	0	0	0	0	38	673	0	0	0	692	98	0

10. FM 973 at Wesley Miller Lane

	Eastbound				Westbound				Northbound				Southbound			
	Wesley Miller Lane				Wesley Miller Lane				FM 973				FM 973			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U
AM Peak	12	0	5	0	19	0	193	0	3	408	43	0	383	741	6	0
PM Peak	8	0	4	0	5	0	15	0	4	684	0	0	3	695	13	0

Appendix D – Traffic Simulation Output

Appendix D. Traffic Simulation Output

Existing Conditions (2022, October), AM & PM

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2022 AM Peak Hour
Existing Conditions

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	6	0	308	551	261	477	2	314	0	130	0	0
Future Volume (vph)	6	0	308	551	261	477	2	314	0	130	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				586						143		381
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	6	0	328	586	278	507	2	334	0	138	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	6	328	586	278	509	0	0	334	138	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2022 AM Peak Hour
Existing Conditions

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.3	19.6	19.6	25.8	49.2			23.4	23.4		5.2
Actuated g/C Ratio		0.11	0.22	0.22	0.29	0.55			0.26	0.26		0.06
v/c Ratio		0.03	0.46	0.74	0.57	0.28			0.77	0.27		0.00
Control Delay		45.0	33.9	9.3	36.8	14.6			44.3	6.1		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		45.0	33.9	9.3	36.8	14.6			44.3	6.1		0.0
LOS		D	C	A	D	B			D	A		A
Approach Delay			18.3			22.5			33.1			
Approach LOS			B			C			C			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 90

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 23.0

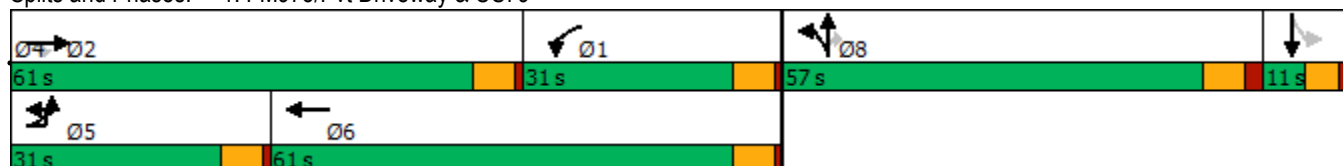
Intersection LOS: C

Intersection Capacity Utilization 67.7%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2022 AM Peak Hour
Existing Conditions

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	31.1	31.1	25.0	46.1	37.8	37.8	5.0
90th %ile Term Code	Min	Gap	Gap	Max	Hold	Gap	Gap	Max
70th %ile Green (s)	0.0	21.3	21.3	25.0	52.3	26.2	26.2	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
50th %ile Green (s)	0.0	17.3	17.3	25.0	48.3	22.1	22.1	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	15.0	15.0	24.6	45.6	18.8	18.8	0.0
30th %ile Term Code	Skip	Min	Min	Gap	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	15.0	15.0	25.0	46.0	14.9	14.9	0.0
10th %ile Term Code	Skip	Min	Min	Max	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 90								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 123.9								
70th %ile Actuated Cycle: 91.5								
50th %ile Actuated Cycle: 83.4								
30th %ile Actuated Cycle: 77.4								
10th %ile Actuated Cycle: 73.9								

Queues
1: FM973/Pvt Driveway & US79

2022 AM Peak Hour
Existing Conditions

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	6	328	586	278	509	334	138	1
v/c Ratio	0.03	0.46	0.74	0.57	0.28	0.77	0.27	0.00
Control Delay	45.0	33.9	9.3	36.8	14.6	44.3	6.1	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	45.0	33.9	9.3	36.8	14.6	44.3	6.1	0.0
Queue Length 50th (ft)	3	80	0	121	63	160	0	0
Queue Length 95th (ft)	19	161	107	#349	206	338	44	0
Internal Link Dist (ft)		854			990	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	518	2092	1197	493	2130	959	961	453
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.16	0.49	0.56	0.24	0.35	0.14	0.00

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2022 AM Peak Hour

Existing Conditions

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	6	0	308	551	261	477	2	314	0	130	0	0
Future Volume (vph)	6	0	308	551	261	477	2	314	0	130	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	6	0	328	586	278	507	2	334	0	138	0	0
RTOR Reduction (vph)	0	0	0	471	0	0	0	0	0	106	0	1
Lane Group Flow (vph)	0	6	328	115	278	509	0	0	334	32	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.5	19.6	19.6	31.1	49.2			23.4	23.4		0.7
Effective Green, g (s)		1.5	19.6	19.6	31.1	49.2			23.4	23.4		0.7
Actuated g/C Ratio		0.02	0.20	0.20	0.31	0.49			0.23	0.23		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		27	650	304	535	1662			391	367		11
v/s Ratio Prot		0.00	c0.10		c0.16	0.15			c0.20			c0.00
v/s Ratio Perm				0.07						0.02		
v/c Ratio		0.22	0.50	0.38	0.52	0.31			0.85	0.09		0.00
Uniform Delay, d1		48.6	35.8	34.8	28.2	15.1			36.6	29.9		49.2
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00
Incremental Delay, d2		1.5	0.2	0.3	0.4	0.0			16.4	0.1		0.0
Delay (s)		50.1	36.0	35.1	28.6	15.1			53.0	30.0		49.2
Level of Service		D	D	D	C	B			D	C		D
Approach Delay (s)			35.5			19.9			46.2			49.2
Approach LOS			D			B			D			D
Intersection Summary												
HCM 2000 Control Delay			32.2			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.61									
Actuated Cycle Length (s)			99.8			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			67.7%			ICU Level of Service			C			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2022 AM Peak Hour
Existing Conditions

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2022 AM Peak Hour
Existing Conditions

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	2	0	454	0	778	0	0	0
Future Volume (vph)	2	0	454	0	778	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	2	0	499	0	855	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	2	499	0	855	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	24.8%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2022 AM Peak Hour
Existing Conditions

								
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (veh/h)	2	0	454	0	778	0	0	0
Future Volume (Veh/h)	2	0	454	0	778	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	499	0	855	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage (veh)			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	855		0			1104	428
vC1, stage 1 conf vol							855	
vC2, stage 2 conf vol							250	
vCu, unblocked vol	0	855		0			1104	428
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	793		0			362	581
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	250	250	428	428	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.15	0.15	0.25	0.25	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			24.8%		ICU Level of Service		A	
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	2	0	454	0	778	0	0	0
Future Vol, veh/h	2	0	454	0	778	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	2	0	499	0	855	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	855	855	0	499	-	0	1109	428
Stage 1	-	-	-	-	-	-	855	-
Stage 2	-	-	-	-	-	-	254	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	418	793	-	702	-	0	207	581
Stage 1	-	-	-	-	-	0	382	-
Stage 2	-	-	-	-	-	0	771	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	418	418	-	702	-	-	206	581
Mov Cap-2 Maneuver	-	-	-	-	-	-	206	-
Stage 1	-	-	-	-	-	-	380	-
Stage 2	-	-	-	-	-	-	771	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.1		0		0			
HCM LOS	A							
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	418	-	702	-	-			
HCM Lane V/C Ratio	0.005	-	-	-	-			
HCM Control Delay (s)	13.7	-	0	-	0			
HCM Lane LOS	B	-	A	-	A			
HCM 95th %tile Q(veh)	0	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2022 AM Peak Hour
Existing Conditions

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	479	5	6	736	1	23	6	1	2	2	1
Future Volume (vph)	3	479	5	6	736	1	23	6	1	2	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.996			0.973	
Flt Protected	0.950			0.950				0.963			0.980	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1822	0	0	1812	0
Flt Permitted	0.950			0.950				0.963			0.980	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1822	0	0	1812	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	3	544	6	7	836	1	26	7	1	2	2	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	544	6	7	836	1	0	34	0	0	5	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	30.3%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2022 AM Peak Hour
Existing Conditions

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (veh/h)	3	479	5	6	736	1	23	6	1	2	2	1
Future Volume (Veh/h)	3	479	5	6	736	1	23	6	1	2	2	1
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	3	544	6	7	836	1	26	7	1	2	2	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	836			544			983	1400	272	1132	1400	418
vC1, stage 1 conf vol							550	550		850	850	
vC2, stage 2 conf vol							433	850		282	550	
vCu, unblocked vol	836			544			983	1400	272	1132	1400	418
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			94	98	100	99	99	100
cM capacity (veh/h)	807			1035			406	321	732	304	322	589
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	3	272	272	6	7	418	418	1	34	5		
Volume Left	3	0	0	0	7	0	0	0	26	2		
Volume Right	0	0	0	6	0	0	0	1	1	1		
cSH	807	1700	1700	1700	1035	1700	1700	1700	390	345		
Volume to Capacity	0.00	0.16	0.16	0.00	0.01	0.25	0.25	0.00	0.09	0.01		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	7	1		
Control Delay (s)	9.5	0.0	0.0	0.0	8.5	0.0	0.0	0.0	15.1	15.6		
Lane LOS	A				A				C	C		
Approach Delay (s)	0.1				0.1				15.1	15.6		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.5											
Intersection Capacity Utilization	30.3%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	3	479	5	6	736	1	23	6	1	2	2	1
Future Vol, veh/h	3	479	5	6	736	1	23	6	1	2	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	3	544	6	7	836	1	26	7	1	2	2	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	836	0	0	544	0	0	983	1400	-	1132	1400	-
Stage 1	-	-	-	-	-	-	550	550	-	850	850	-
Stage 2	-	-	-	-	-	-	433	850	-	282	550	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	807	-	-	1035	-	-	206	142	0	160	142	0
Stage 1	-	-	-	-	-	-	492	519	0	326	380	0
Stage 2	-	-	-	-	-	-	577	380	0	707	519	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	807	-	-	1035	-	-	202	140	-	153	140	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	202	140	-	153	140	-
Stage 1	-	-	-	-	-	-	490	517	-	325	377	-
Stage 2	-	-	-	-	-	-	570	377	-	695	517	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	807	-	-	1035	-	-	-
HCM Lane V/C Ratio	-	0.004	-	-	0.007	-	-	-
HCM Control Delay (s)	-	9.5	-	-	8.5	-	-	-
HCM Lane LOS	-	A	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2022 AM Peak Hour
Existing Conditions

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	3	66	392	5	171	649
Future Volume (vph)	3	66	392	5	171	649
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.872		0.998			
Flt Protected	0.998				0.950	
Satd. Flow (prot)	1653	0	1757	0	1805	1792
Flt Permitted	0.998				0.950	
Satd. Flow (perm)	1653	0	1757	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	4	78	461	6	201	764
Shared Lane Traffic (%)						
Lane Group Flow (vph)	82	0	467	0	201	764
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	45.1%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2022 AM Peak Hour
Existing Conditions

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	3	66	392	5	171	649
Future Volume (Veh/h)	3	66	392	5	171	649
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	4	78	461	6	201	764
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh)			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1630	464			467	
vC1, stage 1 conf vol	464					
vC2, stage 2 conf vol	1166					
vCu, unblocked vol	1630	464			467	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	87			82	
cM capacity (veh/h)	230	602			1105	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	82	467	201	764		
Volume Left	4	0	201	0		
Volume Right	78	6	0	0		
cSH	558	1700	1105	1700		
Volume to Capacity	0.15	0.27	0.18	0.45		
Queue Length 95th (ft)	13	0	17	0		
Control Delay (s)	12.6	0.0	9.0	0.0		
Lane LOS	B		A			
Approach Delay (s)	12.6	0.0	1.9			
Approach LOS	B					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilization			45.1%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	66	392	5	171	649
Future Vol, veh/h	3	66	392	5	171	649
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	4	78	461	6	201	764
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1630	464	0	0	467	0
Stage 1	464	-	-	-	-	-
Stage 2	1166	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	113	602	-	-	1105	-
Stage 1	637	-	-	-	-	-
Stage 2	299	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	92	602	-	-	1105	-
Mov Cap-2 Maneuver	193	-	-	-	-	-
Stage 1	637	-	-	-	-	-
Stage 2	245	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	12.7	0		1.9		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 551		1105	-	
HCM Lane V/C Ratio	-	- 0.147		0.182	-	
HCM Control Delay (s)	-	- 12.7		9	-	
HCM Lane LOS	-	- B		A	-	
HCM 95th %tile Q(veh)	-	- 0.5		0.7	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2022 AM Peak Hour
Existing Conditions

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	1	0	0	395	649	1
Future Volume (vph)	1	0	0	395	649	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						0.850
Flt Protected	0.950					
Satd. Flow (prot)	1805	0	1900	1759	1810	1615
Flt Permitted	0.950					
Satd. Flow (perm)	1805	0	1900	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	1	0	0	439	721	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	0	439	721	1
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	44.2%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2022 AM Peak Hour
Existing Conditions

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	1	0	0	395	649	1
Future Volume (Veh/h)	1	0	0	395	649	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	1	0	0	439	721	1
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1160	721	722			
vC1, stage 1 conf vol	721					
vC2, stage 2 conf vol	439					
vCu, unblocked vol	1160	721	722			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	424	431	889			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	1	0	439	721	1	
Volume Left	1	0	0	0	0	
Volume Right	0	0	0	0	1	
cSH	424	1700	1700	1700	1700	
Volume to Capacity	0.00	0.00	0.26	0.42	0.00	
Queue Length 95th (ft)	0	0	0	0	0	
Control Delay (s)	13.5	0.0	0.0	0.0	0.0	
Lane LOS	B					
Approach Delay (s)	13.5	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			44.2%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	1	0	0	395	649	1
Future Vol, veh/h	1	0	0	395	649	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	1	0	0	439	721	1
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1160	721	722	0	-	0
Stage 1	721	-	-	-	-	-
Stage 2	439	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	218	431	889	-	-	-
Stage 1	485	-	-	-	-	-
Stage 2	654	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	218	431	889	-	-	-
Mov Cap-2 Maneuver	350	-	-	-	-	-
Stage 1	485	-	-	-	-	-
Stage 2	654	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	15.3	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	889	-	350	-	-	
HCM Lane V/C Ratio	-	-	0.003	-	-	
HCM Control Delay (s)	0	-	15.3	-	-	
HCM Lane LOS	A	-	C	-	-	
HCM 95th %tile Q(veh)	0	-	0	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2022 AM Peak Hour
Existing Conditions

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	1	15	0	154	1	241	34	306	344	0
Future Volume (vph)	0	0	1	15	0	154	1	241	34	306	344	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865			0.877			0.981				
Flt Protected					0.996		0.950			0.950		
Satd. Flow (prot)	0	1644	0	0	1660	0	1805	1673	0	1805	1712	0
Flt Permitted					0.996		0.950			0.950		
Satd. Flow (perm)	0	1644	0	0	1660	0	1805	1673	0	1805	1712	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	0	0	1	17	0	175	1	274	39	348	391	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	192	0	1	313	0	348	391	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	58.7%						ICU Level of Service B					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2022 AM Peak Hour
Existing Conditions

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	1	15	0	154	1	241	34	306	344	0
Future Volume (Veh/h)	0	0	1	15	0	154	1	241	34	306	344	0
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	0	0	1	17	0	175	1	274	39	348	391	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLTL						TWLTL					
Median storage veh	2						2					
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1538	1402	391	1384	1382	294	391	313				
vC1, stage 1 conf vol	1087	1087	296		296							
vC2, stage 2 conf vol	451	315	1088		1087							
vCu, unblocked vol	1538	1402	391	1384	1382	294	391	313				
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1				
tC, 2 stage (s)	6.1	5.5	6.1		5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2				
p0 queue free %	100	100	100	91	100	77	100	72				
cM capacity (veh/h)	99	191	662	183	204	751	1179	1259				
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	1	192	1	313	348	391						
Volume Left	0	17	1	0	348	0						
Volume Right	1	175	0	39	0	0						
cSH	662	589	1179	1700	1259	1700						
Volume to Capacity	0.00	0.33	0.00	0.18	0.28	0.23						
Queue Length 95th (ft)	0	35	0	0	28	0						
Control Delay (s)	10.4	14.0	8.1	0.0	8.9	0.0						
Lane LOS	B	B	A	A								
Approach Delay (s)	10.4	14.0	0.0	4.2								
Approach LOS	B	B										
Intersection Summary												
Average Delay	4.7											
Intersection Capacity Utilization	58.7%			ICU Level of Service			B					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	5.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔		↔	↔		↔	↔	
Traffic Vol, veh/h	0	0	1	15	0	154	1	241	34	306	344	0
Future Vol, veh/h	0	0	1	15	0	154	1	241	34	306	344	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	11	0
Mvmt Flow	0	0	1	17	0	175	1	274	39	348	391	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1470	1402	391	1384	1383	294	391	0	0	313	0	0
Stage 1	1087	1087	-	296	296	-	-	-	-	-	-	-
Stage 2	383	315	-	1088	1087	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	106	141	662	122	145	750	1179	-	-	1259	-	-
Stage 1	264	295	-	717	672	-	-	-	-	-	-	-
Stage 2	644	659	-	264	295	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	64	102	662	96	105	750	1179	-	-	1259	-	-
Mov Cap-2 Maneuver	64	102	-	96	105	-	-	-	-	-	-	-
Stage 1	264	214	-	716	671	-	-	-	-	-	-	-
Stage 2	493	658	-	191	214	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.4	18	0	4.2
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1179	-	-	662	467	1259	-
HCM Lane V/C Ratio	0.001	-	-	0.002	0.411	0.276	-
HCM Control Delay (s)	8.1	-	-	10.4	18	8.9	-
HCM Lane LOS	A	-	-	B	C	A	-
HCM 95th %tile Q(veh)	0	-	-	0	2	1.1	-

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2022 PM Peak Hour
Existing Conditions

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	587	187	132	345	2	249	0	234	0	0
Future Volume (vph)	7	0	587	187	132	345	2	249	0	234	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				205						257		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	8	0	645	205	145	379	2	274	0	257	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	8	645	205	145	381	0	0	274	257	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2022 PM Peak Hour
Existing Conditions

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.6	24.1	24.1	12.3	40.5			19.9	19.9		
Actuated g/C Ratio		0.14	0.32	0.32	0.16	0.53			0.26	0.26		
v/c Ratio		0.03	0.59	0.34	0.53	0.22			0.62	0.43		
Control Delay		38.7	25.4	5.3	40.1	11.7			33.4	6.1		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		38.7	25.4	5.3	40.1	11.7			33.4	6.1		
LOS		D	C	A	D	B			C	A		
Approach Delay			20.7			19.6			20.2			
Approach LOS			C			B			C			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 76.4

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 20.3

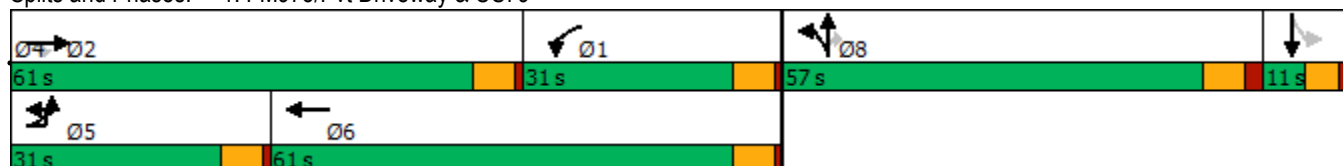
Intersection LOS: C

Intersection Capacity Utilization 53.2%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2022 PM Peak Hour
Existing Conditions



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	36.6	36.6	19.5	46.1	31.7	31.7	0.0
90th %ile Term Code	Min	Gap	Gap	Gap	Hold	Gap	Gap	Skip
70th %ile Green (s)	0.0	28.0	28.0	14.6	48.6	23.3	23.3	0.0
70th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
50th %ile Green (s)	0.0	23.4	23.4	12.0	41.4	19.2	19.2	0.0
50th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	19.4	19.4	9.7	35.1	15.7	15.7	0.0
30th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	15.0	15.0	7.2	28.2	11.8	11.8	0.0
10th %ile Term Code	Skip	Min	Min	Gap	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 76.4								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 106.8								
70th %ile Actuated Cycle: 84.9								
50th %ile Actuated Cycle: 73.6								
30th %ile Actuated Cycle: 63.8								
10th %ile Actuated Cycle: 53								

Queues
1: FM973/Pvt Driveway & US79

2022 PM Peak Hour
Existing Conditions

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	8	645	205	145	381	274	257
v/c Ratio	0.03	0.59	0.34	0.53	0.22	0.62	0.43
Control Delay	38.7	25.4	5.3	40.1	11.7	33.4	6.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.7	25.4	5.3	40.1	11.7	33.4	6.1
Queue Length 50th (ft)	3	127	0	61	41	110	0
Queue Length 95th (ft)	21	243	50	153	120	242	58
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	625	2618	1148	595	2520	1179	1175
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.25	0.18	0.24	0.15	0.23	0.22
Intersection Summary							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2022 PM Peak Hour

Existing Conditions

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	587	187	132	345	2	249	0	234	0	0
Future Volume (vph)	7	0	587	187	132	345	2	249	0	234	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	8	0	645	205	145	379	2	274	0	257	0	0
RTOR Reduction (vph)	0	0	0	144	0	0	0	0	0	194	0	0
Lane Group Flow (vph)	0	8	645	61	145	381	0	0	274	63	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	24.1	24.1	17.8	40.5			19.9	19.9		
Effective Green, g (s)		1.4	24.1	24.1	17.8	40.5			19.9	19.9		
Actuated g/C Ratio		0.02	0.30	0.30	0.22	0.50			0.25	0.25		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		31	1035	433	378	1674			419	389		
v/s Ratio Prot		0.00	c0.19		c0.08	0.11			c0.16			
v/s Ratio Perm				0.04						0.04		
v/c Ratio		0.26	0.62	0.14	0.38	0.23			0.65	0.16		
Uniform Delay, d1		39.2	24.4	20.8	26.8	11.3			27.4	23.9		
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		
Incremental Delay, d2		1.6	0.8	0.1	0.2	0.0			3.6	0.2		
Delay (s)		40.8	25.3	20.8	27.1	11.4			31.0	24.1		
Level of Service		D	C	C	C	B			C	C		
Approach Delay (s)			24.4			15.7			27.7			0.0
Approach LOS			C			B			C			A
Intersection Summary												
HCM 2000 Control Delay			22.9			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.62									
Actuated Cycle Length (s)			80.8			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			53.2%			ICU Level of Service			A			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2022 PM Peak Hour
Existing Conditions

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2022 PM Peak Hour
Existing Conditions

							
Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	0	840	0	501	0	0	0
Future Volume (vph)	0	840	0	501	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		100		0	0	0
Storage Lanes	1		1		0	1	0
Taper Length (ft)	100		100			100	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt							
Flt Protected							
Satd. Flow (prot)	1900	3471	1900	3374	0	1900	0
Flt Permitted							
Satd. Flow (perm)	1900	3471	1900	3374	0	1900	0
Link Speed (mph)		65		65		30	
Link Distance (ft)		739		504		317	
Travel Time (s)		7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	0	923	0	551	0	0	0
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	923	0	551	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)		60		60		12	
Link Offset(ft)		0		0		0	
Crosswalk Width(ft)		16		16		16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9		9	15	9
Sign Control		Free		Free		Stop	
Intersection Summary							
Area Type:	Other						
Control Type:	Unsignalized						
Intersection Capacity Utilization	26.6%			ICU Level of Service A			
Analysis Period (min)	15						

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2022 PM Peak Hour
Existing Conditions

							
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (veh/h)	0	840	0	501	0	0	0
Future Volume (Veh/h)	0	840	0	501	0	0	0
Sign Control		Free		Free		Stop	
Grade		0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	923	0	551	0	0	0
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		Raised		Raised			
Median storage (veh)		2		2			
Upstream signal (ft)							
pX, platoon unblocked			0.00				
vC, conflicting volume	551		0			1012	276
vC1, stage 1 conf vol						551	
vC2, stage 2 conf vol						462	
vCu, unblocked vol	551		0			1012	276
tC, single (s)	4.1		0.0			6.8	6.9
tC, 2 stage (s)						5.8	
tF (s)	2.2		0.0			3.5	3.3
p0 queue free %	100		0			100	100
cM capacity (veh/h)	1029		0			451	728
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1
Volume Total	0	462	462	276	276	0	0
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	0
cSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.00	0.27	0.27	0.16	0.16	0.00	0.00
Queue Length 95th (ft)	0	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							A
Approach Delay (s)	0.0			0.0			0.0
Approach LOS							A
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			26.6%		ICU Level of Service		A
Analysis Period (min)			15				

Intersection							
Int Delay, s/veh	0						
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Vol, veh/h	0	840	0	501	0	0	0
Future Vol, veh/h	0	840	0	501	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	-	None	-	None
Storage Length	100	-	100	-	-	0	-
Veh in Median Storage, #	-	0	-	0	-	0	-
Grade, %	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91
Heavy Vehicles, %	0	4	0	7	0	0	0
Mvmt Flow	0	923	0	551	0	0	0
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	551	0	923	-	0	1013	276
Stage 1	-	-	-	-	-	551	-
Stage 2	-	-	-	-	-	462	-
Critical Hdwy	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	1029	-	378	-	0	239	727
Stage 1	-	-	-	-	0	547	-
Stage 2	-	-	-	-	0	607	-
Platoon blocked, %		-		-			
Mov Cap-1 Maneuver	1029	-	378	-	-	239	727
Mov Cap-2 Maneuver	-	-	-	-	-	239	-
Stage 1	-	-	-	-	-	547	-
Stage 2	-	-	-	-	-	607	-
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		0		
HCM LOS	A						
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1		
Capacity (veh/h)	1029	-	378	-	-		
HCM Lane V/C Ratio	-	-	-	-	-		
HCM Control Delay (s)	0	-	0	-	0		
HCM Lane LOS	A	-	A	-	A		
HCM 95th %tile Q(veh)	0	-	0	-	-		

Lanes, Volumes, Timings
5: S Rio Grande & US79

2022 PM Peak Hour
Existing Conditions

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	2	847	22	5	467	0	14	2	2	1	6	0
Future Volume (vph)	2	847	22	5	467	0	14	2	2	1	6	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.986				
Flt Protected	0.950			0.950				0.962			0.994	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1889	0
Flt Permitted	0.950			0.950				0.962			0.994	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1889	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	2	963	25	6	531	0	16	2	2	1	7	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	2	963	25	6	531	0	0	20	0	0	8	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	33.9%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2022 PM Peak Hour
Existing Conditions

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	2	847	22	5	467	0	14	2	2	1	6	0
Future Volume (Veh/h)	2	847	22	5	467	0	14	2	2	1	6	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	2	962	25	6	531	0	16	2	2	1	7	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	531			962			1247	1509	481	1029	1509	266
vC1, stage 1 conf vol							966	966		543	543	
vC2, stage 2 conf vol							281	543		486	966	
vCu, unblocked vol	531			962			1247	1509	481	1029	1509	266
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			94	99	100	100	98	100
cM capacity (veh/h)	1047			724			263	293	537	391	290	739
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	2	481	481	25	6	266	266	0	20	8		
Volume Left	2	0	0	0	6	0	0	0	16	1		
Volume Right	0	0	0	25	0	0	0	0	2	0		
cSH	1047	1700	1700	1700	724	1700	1700	1700	280	300		
Volume to Capacity	0.00	0.28	0.28	0.01	0.01	0.16	0.16	0.00	0.07	0.03		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	6	2		
Control Delay (s)	8.4	0.0	0.0	0.0	10.0	0.0	0.0	0.0	18.9	17.3		
Lane LOS	A				B				C	C		
Approach Delay (s)	0.0				0.1				18.9	17.3		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	33.9%			ICU Level of Service			A					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	2	847	22	5	467	0	14	2	2	1	6	0
Future Vol, veh/h	2	847	22	5	467	0	14	2	2	1	6	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	2	963	25	6	531	0	16	2	2	1	7	0
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	531	0	0	963	0	0	1248	1510	-	1030	1510	-
Stage 1	-	-	-	-	-	-	967	967	-	543	543	-
Stage 2	-	-	-	-	-	-	281	543	-	487	967	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	1047	-	-	723	-	-	132	122	0	190	122	0
Stage 1	-	-	-	-	-	-	277	335	0	497	523	0
Stage 2	-	-	-	-	-	-	708	523	0	536	335	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1047	-	-	723	-	-	125	121	-	186	121	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	125	121	-	186	121	-
Stage 1	-	-	-	-	-	-	276	334	-	496	519	-
Stage 2	-	-	-	-	-	-	693	519	-	531	334	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.1								
HCM LOS							-			-		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	-	1047	-	-	723	-	-	-				
HCM Lane V/C Ratio	-	0.002	-	-	0.008	-	-	-				
HCM Control Delay (s)	-	8.4	-	-	10	-	-	-				
HCM Lane LOS	-	A	-	-	B	-	-	-				
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-				

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2022 PM Peak Hour
Existing Conditions

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	1	14	473	0	3	333
Future Volume (vph)	1	14	473	0	3	333
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.874					
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1656	0	1827	0	1805	1712
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1656	0	1827	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	1	14	488	0	3	343
Shared Lane Traffic (%)						
Lane Group Flow (vph)	15	0	488	0	3	343
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	34.9%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2022 PM Peak Hour
Existing Conditions

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	1	14	473	0	3	333
Future Volume (Veh/h)	1	14	473	0	3	333
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1	14	488	0	3	343
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT			TWLT
Median storage veh			2			2
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	837	488			488	
vC1, stage 1 conf vol	488					
vC2, stage 2 conf vol	349					
vCu, unblocked vol	837	488			488	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	98			100	
cM capacity (veh/h)	540	584			1086	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	15	488	3	343		
Volume Left	1	0	3	0		
Volume Right	14	0	0	0		
cSH	581	1700	1086	1700		
Volume to Capacity	0.03	0.29	0.00	0.20		
Queue Length 95th (ft)	2	0	0	0		
Control Delay (s)	11.4	0.0	8.3	0.0		
Lane LOS	B		A			
Approach Delay (s)	11.4	0.0	0.1			
Approach LOS	B					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			34.9%	ICU Level of Service	A	
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	1	14	473	0	3	333
Future Vol, veh/h	1	14	473	0	3	333
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	1	14	488	0	3	343
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	837	488	0	0	488	0
Stage 1	488	-	-	-	-	-
Stage 2	349	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	339	584	-	-	1086	-
Stage 1	621	-	-	-	-	-
Stage 2	719	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	338	584	-	-	1086	-
Mov Cap-2 Maneuver	456	-	-	-	-	-
Stage 1	621	-	-	-	-	-
Stage 2	717	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	11.5	0		0.1		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	573	1086	-	
HCM Lane V/C Ratio	-	-	0.027	0.003	-	
HCM Control Delay (s)	-	-	11.5	8.3	-	
HCM Lane LOS	-	-	B	A	-	
HCM 95th %tile Q(veh)	-	-	0.1	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2022 PM Peak Hour
Existing Conditions

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	468	337	0
Future Volume (vph)	0	0	0	468	337	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1900	0	1900	1827	1696	1900
Flt Permitted						
Satd. Flow (perm)	1900	0	1900	1827	1696	1900
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	0	0	0	509	366	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	509	366	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	28.0%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2022 PM Peak Hour
Existing Conditions

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	0	468	337	0
Future Volume (Veh/h)	0	0	0	468	337	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	509	366	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage (veh)				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	875	366	366			
vC1, stage 1 conf vol	366					
vC2, stage 2 conf vol	509					
vCu, unblocked vol	875	366	366			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	526	684	1204			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	0	0	509	366	0	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.00	0.30	0.22	0.00	
Queue Length 95th (ft)	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	
Lane LOS	A					
Approach Delay (s)	0.0	0.0		0.0		
Approach LOS	A					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			28.0%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	468	337	0
Future Vol, veh/h	0	0	0	468	337	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	0	0	0	509	366	0
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	875	366	366	0	-	0
Stage 1	366	-	-	-	-	-
Stage 2	509	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	322	684	1204	-	-	-
Stage 1	706	-	-	-	-	-
Stage 2	608	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	322	684	1204	-	-	-
Mov Cap-2 Maneuver	444	-	-	-	-	-
Stage 1	706	-	-	-	-	-
Stage 2	608	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	0	0		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1204	-	-	-	-	
HCM Lane V/C Ratio	-	-	-	-	-	
HCM Control Delay (s)	0	-	0	-	-	
HCM Lane LOS	A	-	A	-	-	
HCM 95th %tile Q(veh)	0	-	-	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2022 PM Peak Hour
Existing Conditions

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	1	4	0	12	0	453	0	2	331	1
Future Volume (vph)	0	0	1	4	0	12	0	453	0	2	331	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865			0.897							
Flt Protected					0.988					0.950		
Satd. Flow (prot)	0	1644	0	0	1684	0	1900	1827	0	1805	1712	0
Flt Permitted					0.988					0.950		
Satd. Flow (perm)	0	1644	0	0	1684	0	1900	1827	0	1805	1712	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	0	0	1	4	0	13	0	492	0	2	360	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	17	0	0	492	0	2	361	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	35.2%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2022 PM Peak Hour
Existing Conditions

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	1	4	0	12	0	453	0	2	331	1
Future Volume (Veh/h)	0	0	1	4	0	12	0	453	0	2	331	1
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	1	4	0	13	0	492	0	2	360	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLTL						TWLTL					
Median storage veh)	2						2					
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	870	856	360	857	857	492	361	492				
vC1, stage 1 conf vol	364	364		492	492							
vC2, stage 2 conf vol	505	492		365	365							
vCu, unblocked vol	870	856	360	857	857	492	361	492				
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1				
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2				
p0 queue free %	100	100	100	99	100	98	100	100				
cM capacity (veh/h)	467	474	689	481	475	581	1209	1082				
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	1	17	0	492	2	361						
Volume Left	0	4	0	0	2	0						
Volume Right	1	13	0	0	0	1						
cSH	689	554	1700	1700	1082	1700						
Volume to Capacity	0.00	0.03	0.00	0.29	0.00	0.21						
Queue Length 95th (ft)	0	2	0	0	0	0						
Control Delay (s)	10.2	11.7	0.0	0.0	8.3	0.0						
Lane LOS	B	B			A							
Approach Delay (s)	10.2	11.7	0.0	0.0								
Approach LOS	B	B										
Intersection Summary												
Average Delay	0.3											
Intersection Capacity Utilization	35.2%			ICU Level of Service					A			
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	0	1	4	0	12	0	453	0	2	331	1
Future Vol, veh/h	0	0	1	4	0	12	0	453	0	2	331	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	11	0
Mvmt Flow	0	0	1	4	0	13	0	492	0	2	360	1
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	864	857	361	857	857	492	361	0	0	492	0	0
Stage 1	365	365	-	492	492	-	-	-	-	-	-	-
Stage 2	499	492	-	365	365	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	277	297	688	280	297	581	1209	-	-	1082	-	-
Stage 1	658	627	-	562	551	-	-	-	-	-	-	-
Stage 2	557	551	-	658	627	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	270	296	688	279	296	581	1209	-	-	1082	-	-
Mov Cap-2 Maneuver	270	296	-	279	296	-	-	-	-	-	-	-
Stage 1	658	626	-	562	551	-	-	-	-	-	-	-
Stage 2	544	551	-	656	626	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	10.2		13.2		0		0					
HCM LOS	B		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	1209	-	-	688	457	1082	-	-				
HCM Lane V/C Ratio	-	-	-	0.002	0.038	0.002	-	-				
HCM Control Delay (s)	0	-	-	10.2	13.2	8.3	-	-				
HCM Lane LOS	A	-	-	B	B	A	-	-				
HCM 95th %tile Q(veh)	0	-	-	0	0.1	0	-	-				

Appendix D. Traffic Simulation Output

Phase 1 Buildout Year (2025) Projected Conditions without Development, AM & PM

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	354	728	453	549	2	433	0	189	0	0
Future Volume (vph)	7	0	354	728	453	549	2	433	0	189	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				755						157		316
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	7	0	377	774	482	584	2	461	0	201	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	7	377	774	482	586	0	0	461	201	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.5	26.9	26.9	26.2	57.0			36.4	36.4		5.2
Actuated g/C Ratio		0.09	0.24	0.24	0.24	0.51			0.33	0.33		0.05
v/c Ratio		0.04	0.47	0.82	1.19	0.34			0.84	0.32		0.00
Control Delay		58.6	38.1	11.2	145.6	19.0			50.9	10.0		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		58.6	38.1	11.2	145.6	19.0			50.9	10.0		0.0
LOS		E	D	B	F	B			D	A		A
Approach Delay			20.3			76.2			38.5			
Approach LOS			C			E			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 110.8

Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.19

Intersection Signal Delay: 45.1

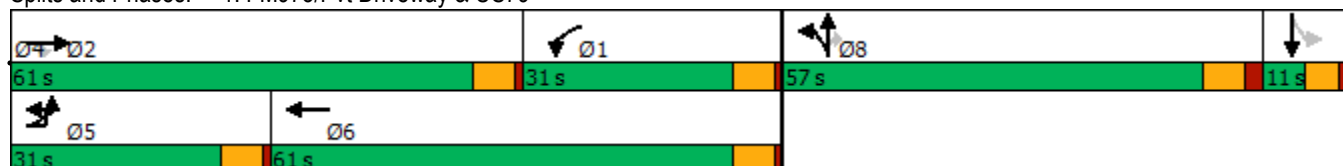
Intersection LOS: D

Intersection Capacity Utilization 89.3%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	50.6	50.6	25.0	65.6	50.0	50.0	5.0
90th %ile Term Code	Min	Gap	Gap	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	28.7	28.7	25.0	59.7	43.0	43.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
50th %ile Green (s)	0.0	24.8	24.8	25.0	55.8	35.9	35.9	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	20.5	20.5	25.0	51.5	30.3	30.3	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	15.4	15.4	25.0	46.4	23.7	23.7	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 110.8								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 155.6								
70th %ile Actuated Cycle: 115.7								
50th %ile Actuated Cycle: 104.7								
30th %ile Actuated Cycle: 94.8								
10th %ile Actuated Cycle: 83.1								

Queues

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour

Phase 1 Build-Out Year w/o Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	7	377	774	482	586	461	201	1
v/c Ratio	0.04	0.47	0.82	1.19	0.34	0.84	0.32	0.00
Control Delay	58.6	38.1	11.2	145.6	19.0	50.9	10.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.6	38.1	11.2	145.6	19.0	50.9	10.0	0.0
Queue Length 50th (ft)	4	114	10	~387	111	278	20	0
Queue Length 95th (ft)	25	202	154	#952	271	#631	97	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	426	1719	1169	405	1828	788	822	378
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.22	0.66	1.19	0.32	0.59	0.24	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	354	728	453	549	2	433	0	189	0	0
Future Volume (vph)	7	0	354	728	453	549	2	433	0	189	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	0	377	774	482	584	2	461	0	201	0	0
RTOR Reduction (vph)	0	0	0	587	0	0	0	0	0	110	0	1
Lane Group Flow (vph)	0	7	377	187	482	586	0	0	461	91	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	26.8	26.8	31.6	57.0			36.4	36.4		0.7
Effective Green, g (s)		1.4	26.8	26.8	31.6	57.0			36.4	36.4		0.7
Actuated g/C Ratio		0.01	0.22	0.22	0.26	0.47			0.30	0.30		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		20	736	345	450	1595			504	473		9
v/s Ratio Prot		0.00	0.11		c0.28	0.17			c0.28			c0.00
v/s Ratio Perm				c0.12						0.06		
v/c Ratio		0.35	0.51	0.54	1.07	0.37			0.91	0.19		0.00
Uniform Delay, d1		59.1	41.1	41.4	44.5	20.3			40.6	31.2		59.6
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00
Incremental Delay, d2		3.8	0.3	0.9	62.7	0.1			21.1	0.2		0.0
Delay (s)		62.9	41.4	42.4	107.2	20.3			61.7	31.4		59.6
Level of Service		E	D	D	F	C			E	C		E
Approach Delay (s)			42.2			59.5			52.5			59.6
Approach LOS			D			E			D			E
Intersection Summary												
HCM 2000 Control Delay			50.9		HCM 2000 Level of Service				D			
HCM 2000 Volume to Capacity ratio			0.85									
Actuated Cycle Length (s)			120.5		Sum of lost time (s)				25.0			
Intersection Capacity Utilization			89.3%		ICU Level of Service				E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	2	0	562	0	1047	0	0	0
Future Volume (vph)	2	0	562	0	1047	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	2	0	618	0	1151	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	2	618	0	1151	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	32.3%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

								
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (veh/h)	2	0	562	0	1047	0	0	0
Future Volume (Veh/h)	2	0	562	0	1047	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	618	0	1151	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1151		0			1460	576
vC1, stage 1 conf vol							1151	
vC2, stage 2 conf vol							309	
vCu, unblocked vol	0	1151		0			1460	576
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	614		0			255	466
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	309	309	576	576	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.18	0.18	0.34	0.34	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			32.3%		ICU Level of Service		A	
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	2	0	562	0	1047	0	0	0
Future Vol, veh/h	2	0	562	0	1047	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	2	0	618	0	1151	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	1151	1151	0	618	-	0	1464	576
Stage 1	-	-	-	-	-	-	1151	-
Stage 2	-	-	-	-	-	-	313	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	270	614	-	590	-	0	121	466
Stage 1	-	-	-	-	-	0	268	-
Stage 2	-	-	-	-	-	0	721	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	270	270	-	590	-	-	120	466
Mov Cap-2 Maneuver	-	-	-	-	-	-	120	-
Stage 1	-	-	-	-	-	-	266	-
Stage 2	-	-	-	-	-	-	721	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.1		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	270	-	590	-	-			
HCM Lane V/C Ratio	0.008	-	-	-	-			
HCM Control Delay (s)	18.4	-	0	-	0			
HCM Lane LOS	C	-	A	-	A			
HCM 95th %tile Q(veh)	0	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	4	591	5	7	999	1	26	7	1	3	3	1
Future Volume (vph)	4	591	5	7	999	1	26	7	1	3	3	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.981	
Flt Protected	0.950			0.950				0.963			0.979	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1824	0	0	1825	0
Flt Permitted	0.950			0.950				0.963			0.979	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1824	0	0	1825	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	5	672	6	8	1135	1	30	8	1	3	3	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	5	672	6	8	1135	1	0	39	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	37.6%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	4	591	5	7	999	1	26	7	1	3	3	1
Future Volume (Veh/h)	4	591	5	7	999	1	26	7	1	3	3	1
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	5	672	6	8	1135	1	30	8	1	3	3	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1135			672			1267	1833	336	1501	1833	568
vC1, stage 1 conf vol							682	682		1151	1151	
vC2, stage 2 conf vol							585	1151		350	682	
vCu, unblocked vol	1135			672			1267	1833	336	1501	1833	568
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			91	97	100	99	99	100
cM capacity (veh/h)	623			928			319	233	666	201	236	471
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	5	336	336	6	8	568	568	1	39	7		
Volume Left	5	0	0	0	8	0	0	0	30	3		
Volume Right	0	0	0	6	0	0	0	1	1	1		
cSH	623	1700	1700	1700	928	1700	1700	1700	301	235		
Volume to Capacity	0.01	0.20	0.20	0.00	0.01	0.33	0.33	0.00	0.13	0.03		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	11	2		
Control Delay (s)	10.8	0.0	0.0	0.0	8.9	0.0	0.0	0.0	18.8	20.8		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				18.8	20.8		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.5											
Intersection Capacity Utilization	37.6%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	4	591	5	7	999	1	26	7	1	3	3	1
Future Vol, veh/h	4	591	5	7	999	1	26	7	1	3	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	5	672	6	8	1135	1	30	8	1	3	3	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1135	0	0	672	0	0	1267	1833	-	1501	1833	-
Stage 1	-	-	-	-	-	-	682	682	-	1151	1151	-
Stage 2	-	-	-	-	-	-	585	1151	-	350	682	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	623	-	-	928	-	-	128	77	0	86	77	0
Stage 1	-	-	-	-	-	-	411	453	0	214	275	0
Stage 2	-	-	-	-	-	-	469	275	0	645	453	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	623	-	-	928	-	-	122	76	-	78	76	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	122	76	-	78	76	-
Stage 1	-	-	-	-	-	-	408	449	-	212	273	-
Stage 2	-	-	-	-	-	-	459	273	-	628	449	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	623	-	-	928	-	-	-
HCM Lane V/C Ratio	-	0.007	-	-	0.009	-	-	-
HCM Control Delay (s)	-	10.8	-	-	8.9	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	76	562	12	197	994
Future Volume (vph)	6	76	562	12	197	994
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.875		0.997			
Flt Protected	0.996				0.950	
Satd. Flow (prot)	1656	0	1757	0	1805	1792
Flt Permitted	0.996				0.950	
Satd. Flow (perm)	1656	0	1757	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	89	661	14	232	1169
Shared Lane Traffic (%)						
Lane Group Flow (vph)	96	0	675	0	232	1169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	64.0%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	76	562	12	197	994
Future Volume (Veh/h)	6	76	562	12	197	994
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	89	661	14	232	1169
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2301	668			675	
vC1, stage 1 conf vol	668					
vC2, stage 2 conf vol	1633					
vCu, unblocked vol	2301	668			675	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	81			75	
cM capacity (veh/h)	126	462			926	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	96	675	232	1169		
Volume Left	7	0	232	0		
Volume Right	89	14	0	0		
cSH	387	1700	926	1700		
Volume to Capacity	0.25	0.40	0.25	0.69		
Queue Length 95th (ft)	24	0	25	0		
Control Delay (s)	17.4	0.0	10.2	0.0		
Lane LOS	C		B			
Approach Delay (s)	17.4	0.0	1.7			
Approach LOS	C					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilization			64.0%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	76	562	12	197	994
Future Vol, veh/h	6	76	562	12	197	994
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	89	661	14	232	1169
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2301	668	0	0	675	0
Stage 1	668	-	-	-	-	-
Stage 2	1633	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	43	462	-	-	926	-
Stage 1	513	-	-	-	-	-
Stage 2	177	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	32	462	-	-	926	-
Mov Cap-2 Maneuver	105	-	-	-	-	-
Stage 1	513	-	-	-	-	-
Stage 2	133	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	18.1	0		1.7		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	370	926	-	
HCM Lane V/C Ratio	-	-	0.261	0.25	-	
HCM Control Delay (s)	-	-	18.1	10.2	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1	1	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	79	33	12	494	968	29
Future Volume (vph)	79	33	12	494	968	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.960					0.850
Flt Protected	0.966		0.950			
Satd. Flow (prot)	1762	0	1805	1759	1810	1615
Flt Permitted	0.966		0.950			
Satd. Flow (perm)	1762	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	88	37	13	549	1076	32
Shared Lane Traffic (%)						
Lane Group Flow (vph)	125	0	13	549	1076	32
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	64.0%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	79	33	12	494	968	29
Future Volume (Veh/h)	79	33	12	494	968	29
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	88	37	13	549	1076	32
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage veh				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1651	1076	1108			
vC1, stage 1 conf vol	1076					
vC2, stage 2 conf vol	575					
vCu, unblocked vol	1651	1076	1108			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	70	86	98			
cM capacity (veh/h)	292	269	638			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	125	13	549	1076	32	
Volume Left	88	13	0	0	0	
Volume Right	37	0	0	0	32	
cSH	284	638	1700	1700	1700	
Volume to Capacity	0.44	0.02	0.32	0.63	0.02	
Queue Length 95th (ft)	53	2	0	0	0	
Control Delay (s)	27.2	10.8	0.0	0.0	0.0	
Lane LOS	D	B				
Approach Delay (s)	27.2	0.2		0.0		
Approach LOS	D					
Intersection Summary						
Average Delay		2.0				
Intersection Capacity Utilization		64.0%		ICU Level of Service		C
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	79	33	12	494	968	29
Future Vol, veh/h	79	33	12	494	968	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	88	37	13	549	1076	32

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1651	1076	1108	0	-	0
Stage 1	1076	-	-	-	-	-
Stage 2	575	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	110	269	638	-	-	-
Stage 1	330	-	-	-	-	-
Stage 2	567	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	108	269	638	-	-	-
Mov Cap-2 Maneuver	232	-	-	-	-	-
Stage 1	323	-	-	-	-	-
Stage 2	567	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	34.6	0.3	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	638	-	242	-	-
HCM Lane V/C Ratio	0.021	-	0.514	-	-
HCM Control Delay (s)	10.8	-	34.6	-	-
HCM Lane LOS	B	-	D	-	-
HCM 95th %tile Q(veh)	0.1	-	2.7	-	-

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	9	0	5	17	0	177	2	320	39	352	648	3
Future Volume (vph)	9	0	5	17	0	177	2	320	39	352	648	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.949			0.877			0.984			0.999	
Flt Protected		0.970			0.996		0.950			0.950		
Satd. Flow (prot)	0	1749	0	0	1660	0	1805	1675	0	1805	1711	0
Flt Permitted		0.970			0.996		0.950			0.950		
Satd. Flow (perm)	0	1749	0	0	1660	0	1805	1675	0	1805	1711	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	10	0	6	19	0	201	2	364	44	400	736	3
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	220	0	2	408	0	400	739	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	60.4%						ICU Level of Service B					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 AM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	9	0	5	17	0	177	2	320	39	352	648	3
Future Volume (Veh/h)	9	0	5	17	0	177	2	320	39	352	648	3
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	10	0	6	19	0	201	2	364	44	400	736	3
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLTL						TWLTL					
Median storage veh	2						2					
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2106	1950	738	1932	1929	386	739			408		
vC1, stage 1 conf vol	1538	1538		390	390							
vC2, stage 2 conf vol	569	412		1542	1539							
vCu, unblocked vol	2106	1950	738	1932	1929	386	739			408		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	48	100	99	79	100	70	100			66		
cM capacity (veh/h)	19	105	421	89	112	666	876			1162		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	16	220	2	408	400	739						
Volume Left	10	19	2	0	400	0						
Volume Right	6	201	0	44	0	3						
cSH	30	428	876	1700	1162	1700						
Volume to Capacity	0.53	0.51	0.00	0.24	0.34	0.43						
Queue Length 95th (ft)	43	71	0	0	39	0						
Control Delay (s)	217.6	22.0	9.1	0.0	9.7	0.0						
Lane LOS	F	C	A		A							
Approach Delay (s)	217.6	22.0	0.0		3.4							
Approach LOS	F	C										
Intersection Summary												
Average Delay	6.8											
Intersection Capacity Utilization	60.4%			ICU Level of Service					B			
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	11.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	9	0	5	17	0	177	2	320	39	352	648	3
Future Vol, veh/h	9	0	5	17	0	177	2	320	39	352	648	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	11	0
Mvmt Flow	10	0	6	19	0	201	2	364	44	400	736	3

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2029	1950	738	1931	1929	386	739	0	0	408	0	0
Stage 1	1538	1538	-	390	390	-	-	-	-	-	-	-
Stage 2	491	412	-	1541	1539	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	43	65	421	51	67	666	876	-	-	1162	-	-
Stage 1	146	179	-	638	611	-	-	-	-	-	-	-
Stage 2	563	598	-	146	179	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	22	43	421	37	44	666	876	-	-	1162	-	-
Mov Cap-2 Maneuver	22	43	-	37	44	-	-	-	-	-	-	-
Stage 1	146	117	-	637	610	-	-	-	-	-	-	-
Stage 2	392	597	-	94	117	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	190.6	59.5	0.1	3.4
HCM LOS	F	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	876	-	-	33	268	1162	-
HCM Lane V/C Ratio	0.003	-	-	0.482	0.823	0.344	-
HCM Control Delay (s)	9.1	-	-	190.6	59.5	9.7	-
HCM Lane LOS	A	-	-	F	F	A	-
HCM 95th %tile Q(veh)	0	-	-	1.6	6.6	1.6	-

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	675	286	173	397	2	328	0	282	0	0
Future Volume (vph)	8	0	675	286	173	397	2	328	0	282	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				314						310		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	9	0	742	314	190	436	2	360	0	310	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	742	314	190	438	0	0	360	310	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.7	32.8	32.8	16.8	53.8			28.7	28.7		
Actuated g/C Ratio		0.11	0.33	0.33	0.17	0.55			0.29	0.29		
v/c Ratio		0.05	0.64	0.45	0.65	0.24			0.73	0.46		
Control Delay		53.5	32.0	5.5	53.5	14.0			42.8	5.9		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		53.5	32.0	5.5	53.5	14.0			42.8	5.9		
LOS		D	C	A	D	B			D	A		
Approach Delay			24.4			25.9			25.7			
Approach LOS			C			C			C			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 98.7

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 25.2

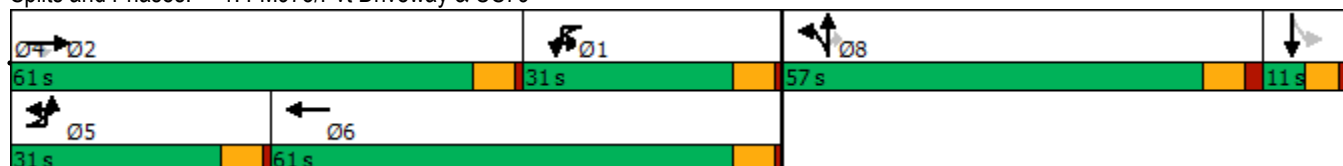
Intersection LOS: C

Intersection Capacity Utilization 62.2%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	49.6	49.6	25.0	64.6	45.1	45.1	0.0
90th %ile Term Code	Min	Gap	Gap	Max	Hold	Gap	Gap	Skip
70th %ile Green (s)	0.0	39.3	39.3	20.9	66.2	35.2	35.2	0.0
70th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
50th %ile Green (s)	0.0	31.9	31.9	16.6	54.5	27.7	27.7	0.0
50th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	26.5	26.5	13.4	45.9	22.4	22.4	0.0
30th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	19.3	19.3	9.5	34.8	16.2	16.2	0.0
10th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 98.7								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 138.7								
70th %ile Actuated Cycle: 114.4								
50th %ile Actuated Cycle: 95.2								
30th %ile Actuated Cycle: 81.3								
10th %ile Actuated Cycle: 64								

Queues
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	9	742	314	190	438	360	310
v/c Ratio	0.05	0.64	0.45	0.65	0.24	0.73	0.46
Control Delay	53.5	32.0	5.5	53.5	14.0	42.8	5.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.5	32.0	5.5	53.5	14.0	42.8	5.9
Queue Length 50th (ft)	5	197	0	108	64	195	0
Queue Length 95th (ft)	27	354	65	245	164	391	66
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	491	2076	996	467	2109	926	1002
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.36	0.32	0.41	0.21	0.39	0.31
Intersection Summary							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	675	286	173	397	2	328	0	282	0	0
Future Volume (vph)	8	0	675	286	173	397	2	328	0	282	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	9	0	742	314	190	436	2	360	0	310	0	0
RTOR Reduction (vph)	0	0	0	214	0	0	0	0	0	224	0	0
Lane Group Flow (vph)	0	9	742	100	190	438	0	0	360	86	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	32.8	32.8	22.4	53.8			28.7	28.7		
Effective Green, g (s)		1.4	32.8	32.8	22.4	53.8			28.7	28.7		
Actuated g/C Ratio		0.01	0.32	0.32	0.22	0.52			0.28	0.28		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		24	1106	463	374	1746			474	441		
v/s Ratio Prot		0.00	c0.21		c0.11	0.13			c0.21			
v/s Ratio Perm				0.07						0.05		
v/c Ratio		0.38	0.67	0.22	0.51	0.25			0.76	0.20		
Uniform Delay, d1		50.3	30.4	25.6	35.4	13.5			33.9	28.3		
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		
Incremental Delay, d2		3.6	1.3	0.1	0.4	0.0			6.9	0.2		
Delay (s)		53.9	31.6	25.7	35.8	13.5			40.8	28.5		
Level of Service		D	C	C	D	B			D	C		
Approach Delay (s)			30.1			20.3			35.1			0.0
Approach LOS			C			C			D			A
Intersection Summary												
HCM 2000 Control Delay			28.9			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.71									
Actuated Cycle Length (s)			102.9			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			62.2%			ICU Level of Service			B			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

							
Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	0	978	0	597	0	0	0
Future Volume (vph)	0	978	0	597	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		100		0	0	0
Storage Lanes	1		1		0	1	0
Taper Length (ft)	100		100			100	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt							
Flt Protected							
Satd. Flow (prot)	1900	3471	1900	3374	0	1900	0
Flt Permitted							
Satd. Flow (perm)	1900	3471	1900	3374	0	1900	0
Link Speed (mph)		65		65		30	
Link Distance (ft)		739		504		317	
Travel Time (s)		7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	0	1052	0	642	0	0	0
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	1052	0	642	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)		60		60		12	
Link Offset(ft)		0		0		0	
Crosswalk Width(ft)		16		16		16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9		9	15	9
Sign Control		Free		Free		Stop	
Intersection Summary							
Area Type:	Other						
Control Type:	Unsignalized						
Intersection Capacity Utilization	30.4%			ICU Level of Service A			
Analysis Period (min)	15						

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

							
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (veh/h)	0	978	0	597	0	0	0
Future Volume (Veh/h)	0	978	0	597	0	0	0
Sign Control		Free		Free		Stop	
Grade		0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	1052	0	642	0	0	0
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		Raised		Raised			
Median storage (veh)		2		2			
Upstream signal (ft)							
pX, platoon unblocked			0.00				
vC, conflicting volume	642		0			1168	321
vC1, stage 1 conf vol						642	
vC2, stage 2 conf vol						526	
vCu, unblocked vol	642		0			1168	321
tC, single (s)	4.1		0.0			6.8	6.9
tC, 2 stage (s)						5.8	
tF (s)	2.2		0.0			3.5	3.3
p0 queue free %	100		0			100	100
cM capacity (veh/h)	952		0			402	681
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1
Volume Total	0	526	526	321	321	0	0
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	0
cSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.00	0.31	0.31	0.19	0.19	0.00	0.00
Queue Length 95th (ft)	0	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							A
Approach Delay (s)	0.0			0.0			0.0
Approach LOS							A
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			30.4%		ICU Level of Service		A
Analysis Period (min)			15				

Intersection							
Int Delay, s/veh	0						
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Vol, veh/h	0	978	0	597	0	0	0
Future Vol, veh/h	0	978	0	597	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	-	None	-	None
Storage Length	100	-	100	-	-	0	-
Veh in Median Storage, #	-	0	-	0	-	0	-
Grade, %	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93
Heavy Vehicles, %	0	4	0	7	0	0	0
Mvmt Flow	0	1052	0	642	0	0	0
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	642	0	1052	-	0	1168	321
Stage 1	-	-	-	-	-	642	-
Stage 2	-	-	-	-	-	526	-
Critical Hdwy	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	952	-	313	-	0	189	681
Stage 1	-	-	-	-	0	492	-
Stage 2	-	-	-	-	0	563	-
Platoon blocked, %		-		-			
Mov Cap-1 Maneuver	952	-	313	-	-	189	681
Mov Cap-2 Maneuver	-	-	-	-	-	189	-
Stage 1	-	-	-	-	-	492	-
Stage 2	-	-	-	-	-	563	-
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		0		
HCM LOS					A		
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1		
Capacity (veh/h)	952	-	313	-	-		
HCM Lane V/C Ratio	-	-	-	-	-		
HCM Control Delay (s)	0	-	0	-	0		
HCM Lane LOS	A	-	A	-	A		
HCM 95th %tile Q(veh)	0	-	0	-	-		

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	986	25	5	558	0	16	3	3	1	7	0
Future Volume (vph)	3	986	25	5	558	0	16	3	3	1	7	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.983				
Flt Protected	0.950			0.950				0.964			0.994	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1800	0	0	1889	0
Flt Permitted	0.950			0.950				0.964			0.994	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1800	0	0	1889	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	3	1120	28	6	634	0	18	3	3	1	8	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	1120	28	6	634	0	0	24	0	0	9	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	38.8%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	3	986	25	5	558	0	16	3	3	1	7	0
Future Volume (Veh/h)	3	986	25	5	558	0	16	3	3	1	7	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	3	1120	28	6	634	0	18	3	3	1	8	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh)	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	634			1120			1459	1772	560	1214	1772	317
vC1, stage 1 conf vol							1126	1126		646	646	
vC2, stage 2 conf vol							333	646		568	1126	
vCu, unblocked vol	634			1120			1459	1772	560	1214	1772	317
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			91	99	99	100	97	100
cM capacity (veh/h)	959			631			210	245	477	334	242	685
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	3	560	560	28	6	317	317	0	24	9		
Volume Left	3	0	0	0	6	0	0	0	18	1		
Volume Right	0	0	0	28	0	0	0	0	3	0		
cSH	959	1700	1700	1700	631	1700	1700	1700	230	249		
Volume to Capacity	0.00	0.33	0.33	0.02	0.01	0.19	0.19	0.00	0.10	0.04		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	9	3		
Control Delay (s)	8.8	0.0	0.0	0.0	10.8	0.0	0.0	0.0	22.5	20.0		
Lane LOS	A				B				C	C		
Approach Delay (s)	0.0				0.1				22.5	20.0		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	38.8%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	3	986	25	5	558	0	16	3	3	1	7	0
Future Vol, veh/h	3	986	25	5	558	0	16	3	3	1	7	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	3	1120	28	6	634	0	18	3	3	1	8	0

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	634	0	0	1120	0	0	1459	1772	-	1214	1772	-
Stage 1	-	-	-	-	-	-	1126	1126	-	646	646	-
Stage 2	-	-	-	-	-	-	333	646	-	568	1126	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	959	-	-	631	-	-	92	84	0	140	84	0
Stage 1	-	-	-	-	-	-	222	282	0	431	470	0
Stage 2	-	-	-	-	-	-	660	470	0	480	282	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	959	-	-	631	-	-	84	83	-	134	83	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	84	83	-	134	83	-
Stage 1	-	-	-	-	-	-	221	281	-	430	465	-
Stage 2	-	-	-	-	-	-	643	465	-	473	281	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	959	-	-	631	-	-	-
HCM Lane V/C Ratio	-	0.004	-	-	0.009	-	-	-
HCM Control Delay (s)	-	8.8	-	-	10.8	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	16	598	4	3	475
Future Volume (vph)	8	16	598	4	3	475
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.910		0.999			
Flt Protected	0.984				0.950	
Satd. Flow (prot)	1701	0	1826	0	1805	1712
Flt Permitted	0.984				0.950	
Satd. Flow (perm)	1701	0	1826	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	16	616	4	3	490
Shared Lane Traffic (%)						
Lane Group Flow (vph)	24	0	620	0	3	490
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	41.7%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 8: FM973 & Taylor HS

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	16	598	4	3	475
Future Volume (Veh/h)	8	16	598	4	3	475
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	16	616	4	3	490
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh)			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1114	618			620	
vC1, stage 1 conf vol	618					
vC2, stage 2 conf vol	496					
vCu, unblocked vol	1114	618			620	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	97			100	
cM capacity (veh/h)	448	493			970	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	24	620	3	490		
Volume Left	8	0	3	0		
Volume Right	16	4	0	0		
cSH	477	1700	970	1700		
Volume to Capacity	0.05	0.36	0.00	0.29		
Queue Length 95th (ft)	4	0	0	0		
Control Delay (s)	12.9	0.0	8.7	0.0		
Lane LOS	B		A			
Approach Delay (s)	12.9	0.0	0.1			
Approach LOS	B					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			41.7%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	16	598	4	3	475
Future Vol, veh/h	8	16	598	4	3	475
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	16	616	4	3	490
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1114	618	0	0	620	0
Stage 1	618	-	-	-	-	-
Stage 2	496	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	232	493	-	-	970	-
Stage 1	542	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	231	493	-	-	970	-
Mov Cap-2 Maneuver	368	-	-	-	-	-
Stage 1	542	-	-	-	-	-
Stage 2	614	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	13.6	0		0.1		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	443	970	-	
HCM Lane V/C Ratio	-	-	0.056	0.003	-	
HCM Control Delay (s)	-	-	13.6	8.7	-	
HCM Lane LOS	-	-	B	A	-	
HCM 95th %tile Q(veh)	-	-	0.2	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	52	22	38	544	397	89
Future Volume (vph)	52	22	38	544	397	89
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.960					0.850
Flt Protected	0.966		0.950			
Satd. Flow (prot)	1762	0	1805	1827	1696	1615
Flt Permitted	0.966		0.950			
Satd. Flow (perm)	1762	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	57	24	41	591	432	97
Shared Lane Traffic (%)						
Lane Group Flow (vph)	81	0	41	591	432	97
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	39.5%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	52	22	38	544	397	89
Future Volume (Veh/h)	52	22	38	544	397	89
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	57	24	41	591	432	97
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1105	432	529			
vC1, stage 1 conf vol	432					
vC2, stage 2 conf vol	673					
vCu, unblocked vol	1105	432	529			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	87	96	96			
cM capacity (veh/h)	430	628	1048			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	81	41	591	432	97	
Volume Left	57	41	0	0	0	
Volume Right	24	0	0	0	97	
cSH	475	1048	1700	1700	1700	
Volume to Capacity	0.17	0.04	0.35	0.25	0.06	
Queue Length 95th (ft)	15	3	0	0	0	
Control Delay (s)	14.1	8.6	0.0	0.0	0.0	
Lane LOS	B	A				
Approach Delay (s)	14.1	0.6		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			39.5%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	52	22	38	544	397	89
Future Vol, veh/h	52	22	38	544	397	89
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	57	24	41	591	432	97

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1105	432	529	0	-	0
Stage 1	432	-	-	-	-	-
Stage 2	673	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	235	628	1048	-	-	-
Stage 1	659	-	-	-	-	-
Stage 2	511	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	226	628	1048	-	-	-
Mov Cap-2 Maneuver	359	-	-	-	-	-
Stage 1	633	-	-	-	-	-
Stage 2	511	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	15.9	0.6	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1048	-	411	-	-
HCM Lane V/C Ratio	0.039	-	0.196	-	-
HCM Control Delay (s)	8.6	-	15.9	-	-
HCM Lane LOS	A	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.7	-	-

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	6	0	4	5	0	14	4	559	0	2	403	11
Future Volume (vph)	6	0	4	5	0	14	4	559	0	2	403	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.951			0.899						0.996	
Flt Protected		0.969			0.988		0.950			0.950		
Satd. Flow (prot)	0	1751	0	0	1688	0	1805	1827	0	1805	1709	0
Flt Permitted		0.969			0.988		0.950			0.950		
Satd. Flow (perm)	0	1751	0	0	1688	0	1805	1827	0	1805	1709	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	7	0	4	5	0	15	4	608	0	2	438	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	11	0	0	20	0	4	608	0	2	450	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	39.4%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 PM Peak Hour
Phase 1 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	6	0	4	5	0	14	4	559	0	2	403	11
Future Volume (Veh/h)	6	0	4	5	0	14	4	559	0	2	403	11
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	0	4	5	0	15	4	608	0	2	438	12
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLTL						TWLTL					
Median storage veh	2						2					
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1079	1064	444	1062	1070	608	450	608				
vC1, stage 1 conf vol	448	448		616	616							
vC2, stage 2 conf vol	631	616		446	454							
vCu, unblocked vol	1079	1064	444	1062	1070	608	450	608				
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1				
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2				
p0 queue free %	98	100	99	99	100	97	100	100				
cM capacity (veh/h)	391	410	618	405	409	499	1121	980				
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	11	20	4	608	2	450						
Volume Left	7	5	4	0	2	0						
Volume Right	4	15	0	0	0	12						
cSH	451	472	1121	1700	980	1700						
Volume to Capacity	0.02	0.04	0.00	0.36	0.00	0.26						
Queue Length 95th (ft)	2	3	0	0	0	0						
Control Delay (s)	13.2	13.0	8.2	0.0	8.7	0.0						
Lane LOS	B	B	A		A							
Approach Delay (s)	13.2	13.0	0.1		0.0							
Approach LOS	B	B										
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	39.4%			ICU Level of Service					A			
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	6	0	4	5	0	14	4	559	0	2	403	11
Future Vol, veh/h	6	0	4	5	0	14	4	559	0	2	403	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	11	0
Mvmt Flow	7	0	4	5	0	15	4	608	0	2	438	12

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1072	1064	444	1066	1070	608	450	0	0	608	0	0
Stage 1	448	448	-	616	616	-	-	-	-	-	-	-
Stage 2	624	616	-	450	454	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	200	225	618	202	223	499	1121	-	-	980	-	-
Stage 1	594	576	-	481	485	-	-	-	-	-	-	-
Stage 2	477	485	-	592	573	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	193	224	618	200	222	499	1121	-	-	980	-	-
Mov Cap-2 Maneuver	193	224	-	200	222	-	-	-	-	-	-	-
Stage 1	592	575	-	479	483	-	-	-	-	-	-	-
Stage 2	461	483	-	587	572	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	19.1		15.7		0.1		0	
HCM LOS	C		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1121	-	-	266 358	980	-	-
HCM Lane V/C Ratio	0.004	-	-	0.041 0.058	0.002	-	-
HCM Control Delay (s)	8.2	-	-	19.1 15.7	8.7	-	-
HCM Lane LOS	A	-	-	C C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1 0.2	0	-	-

Appendix D. Traffic Simulation Output

- Phase 1 Buildout Year (2025) Projected Conditions with Development, AM & PM
 - No Mitigation (Existing Timings at FM 973 at US 79)

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850						0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				754						143		289
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	7	563	774	500	642	0	0	541	202	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

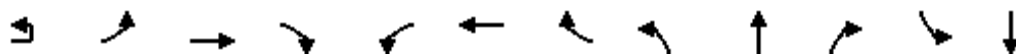
Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases					2					8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.1	36.4	36.4	25.3	65.0			50.5	50.5		5.1
Actuated g/C Ratio		0.08	0.27	0.27	0.19	0.49			0.38	0.38		0.04
v/c Ratio		0.05	0.62	0.80	1.54	0.39			0.85	0.30		0.00
Control Delay		65.3	45.2	10.1	292.2	22.7			53.8	12.0		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		65.3	45.2	10.1	292.2	22.7			53.8	12.0		0.0
LOS		E	D	B	F	C			D	B		A
Approach Delay			25.1			140.7			42.4			
Approach LOS			C			F			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 133.2

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.54

Intersection Signal Delay: 69.9

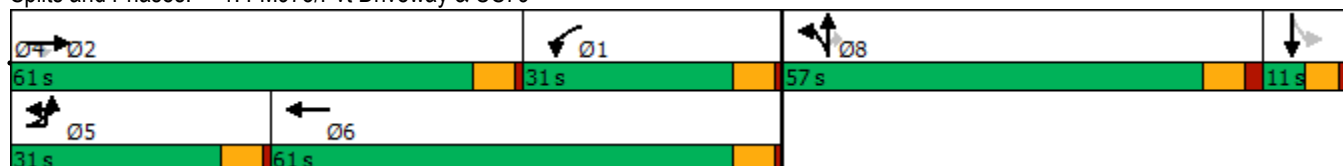
Intersection LOS: E

Intersection Capacity Utilization 91.4%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	40.8	40.8	25.0	71.8	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	35.2	35.2	25.0	66.2	50.0	50.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	29.8	29.8	25.0	60.8	50.0	50.0	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	24.4	24.4	25.0	55.4	50.0	50.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 133.2								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 160								
70th %ile Actuated Cycle: 134.8								
50th %ile Actuated Cycle: 129.2								
30th %ile Actuated Cycle: 123.8								
10th %ile Actuated Cycle: 118.4								

Queues
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	7	563	774	500	642	541	202	1
v/c Ratio	0.05	0.62	0.80	1.54	0.39	0.85	0.30	0.00
Control Delay	65.3	45.2	10.1	292.2	22.7	53.8	12.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.3	45.2	10.1	292.2	22.7	53.8	12.0	0.0
Queue Length 50th (ft)	6	218	12	~586	160	407	31	0
Queue Length 95th (ft)	25	305	150	#1034	290	#843	115	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	342	1381	1087	325	1648	633	683	340
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.41	0.71	1.54	0.39	0.85	0.30	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
RTOR Reduction (vph)	0	0	0	562	0	0	0	0	0	93	0	1
Lane Group Flow (vph)	0	7	563	212	500	642	0	0	541	109	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.7	36.4	36.4	30.4	65.1			50.5	50.5		0.8
Effective Green, g (s)		1.7	36.4	36.4	30.4	65.1			50.5	50.5		0.8
Actuated g/C Ratio		0.01	0.25	0.25	0.21	0.45			0.35	0.35		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		21	842	395	365	1534			589	553		9
v/s Ratio Prot		0.00	c0.17		c0.29	0.19			c0.32			c0.00
v/s Ratio Perm				0.14						0.07		
v/c Ratio		0.33	0.67	0.54	1.37	0.42			0.92	0.20		0.00
Uniform Delay, d1		70.1	47.9	46.1	56.3	26.3			44.3	32.2		70.8
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		1.00
Incremental Delay, d2		3.4	1.6	0.7	183.0	0.1			19.3	0.2		0.0
Delay (s)		73.5	49.5	46.8	238.4	25.7			63.6	32.4		70.8
Level of Service		E	D	D	F	C			E	C		E
Approach Delay (s)			48.1			118.8			55.1			70.8
Approach LOS			D			F			E			E
Intersection Summary												
HCM 2000 Control Delay			74.7		HCM 2000 Level of Service				E			
HCM 2000 Volume to Capacity ratio			0.95									
Actuated Cycle Length (s)			143.1		Sum of lost time (s)				25.0			
Intersection Capacity Utilization			91.4%		ICU Level of Service				F			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	501	280	0	1163	0	180
Future Volume (vph)	501	280	0	1163	0	180
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3282	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3282	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1072			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	0%	0%	7%	0%	0%
Adj. Flow (vph)	545	304	0	1264	0	196
Shared Lane Traffic (%)						
Lane Group Flow (vph)	545	304	0	1264	0	196
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	35.5%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	501	280	0	1163	0	180
Future Volume (Veh/h)	501	280	0	1163	0	180
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	545	304	0	1264	0	196
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1072					
pX, platoon unblocked			0.91		0.91	0.91
vC, conflicting volume			849		1177	272
vC1, stage 1 conf vol					545	
vC2, stage 2 conf vol					632	
vCu, unblocked vol			642		1002	10
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	80
cM capacity (veh/h)			869		433	981
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	272	272	304	632	632	196
Volume Left	0	0	0	0	0	0
Volume Right	0	0	304	0	0	196
cSH	1700	1700	1700	1700	1700	981
Volume to Capacity	0.16	0.16	0.18	0.37	0.37	0.20
Queue Length 95th (ft)	0	0	0	0	0	19
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.6
Lane LOS						A
Approach Delay (s)	0.0				0.0	9.6
Approach LOS						A
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			35.5%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	501	280	0	1163	0	180
Future Vol, veh/h	501	280	0	1163	0	180
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	0	0	7	0	0
Mvmt Flow	545	304	0	1264	0	196
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	273
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	731
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	731
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.7	
HCM LOS					B	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	731	-	-	-		
HCM Lane V/C Ratio	0.268	-	-	-		
HCM Control Delay (s)	11.7	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	1.1	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	61	0	621	0	1128	0	0	0
Future Volume (vph)	61	0	621	0	1128	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	67	0	682	0	1240	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	67	682	0	1240	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	41.2%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	61	0	621	0	1128	0	0	0
Future Volume (Veh/h)	61	0	621	0	1128	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	682	0	1240	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1240		0			1581	620
vC1, stage 1 conf vol							1240	
vC2, stage 2 conf vol							341	
vCu, unblocked vol	0	1240		0			1581	620
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	569		0			229	436
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	341	341	620	620	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.20	0.20	0.36	0.36	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.2%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.9							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	61	0	621	0	1128	0	0	0
Future Vol, veh/h	61	0	621	0	1128	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	67	0	682	0	1240	0	0	0
Major/Minor	Major1		Major2			Minor2		
Conflicting Flow All	1240	1240	0	682	-	0	1715	620
Stage 1	-	-	-	-	-	-	1240	-
Stage 2	-	-	-	-	-	-	475	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	237	569	-	538	-	0	83	436
Stage 1	-	-	-	-	-	0	240	-
Stage 2	-	-	-	-	-	0	597	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	237	237	-	538	-	-	60	436
Mov Cap-2 Maneuver	-	-	-	-	-	-	60	-
Stage 1	-	-	-	-	-	-	172	-
Stage 2	-	-	-	-	-	-	597	-
Approach	EB		WB			SB		
HCM Control Delay, s	2.3		0			0		
HCM LOS						A		
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	237	-	538	-	-			
HCM Lane V/C Ratio	0.283	-	-	-	-			
HCM Control Delay (s)	26.1	-	0	-	0			
HCM Lane LOS	D	-	A	-	A			
HCM 95th %tile Q(veh)	1.1	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.932	
Flt Protected	0.950			0.950				0.961			0.988	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Flt Permitted	0.950			0.950				0.961			0.988	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	8	731	11	8	1215	1	0	47	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	41.8%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (Veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1215			731			1372	1978	366	1616	1978	608
vC1, stage 1 conf vol							747	747		1231	1231	
vC2, stage 2 conf vol							625	1231		386	747	
vCu, unblocked vol	1215			731			1372	1978	366	1616	1978	608
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			87	96	100	98	99	99
cM capacity (veh/h)	581			883			289	210	637	180	214	444
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	8	366	366	11	8	608	608	1	47	12		
Volume Left	8	0	0	0	8	0	0	0	38	3		
Volume Right	0	0	0	11	0	0	0	1	1	6		
cSH	581	1700	1700	1700	883	1700	1700	1700	275	271		
Volume to Capacity	0.01	0.21	0.21	0.01	0.01	0.36	0.36	0.00	0.17	0.04		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	15	3		
Control Delay (s)	11.3	0.0	0.0	0.0	9.1	0.0	0.0	0.0	20.8	18.9		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				20.8	18.9		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.7											
Intersection Capacity Utilization	41.8%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Future Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	8	731	11	8	1215	1	38	8	1	3	3	6

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	1215	0	0	731	0	0	1372	1978	-	1617	1978	-
Stage 1	-	-	-	-	-	-	747	747	-	1231	1231	-
Stage 2	-	-	-	-	-	-	625	1231	-	386	747	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	581	-	-	883	-	-	107	63	0	70	63	0
Stage 1	-	-	-	-	-	-	376	423	0	191	252	0
Stage 2	-	-	-	-	-	-	444	252	0	614	423	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	581	-	-	883	-	-	101	62	-	62	62	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	101	62	-	62	62	-
Stage 1	-	-	-	-	-	-	371	417	-	188	250	-
Stage 2	-	-	-	-	-	-	434	250	-	594	417	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	581	-	-	883	-	-	-
HCM Lane V/C Ratio	-	0.014	-	-	0.009	-	-	-
HCM Control Delay (s)	-	11.3	-	-	9.1	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	67	632	103	0	1225
Future Volume (vph)	0	67	632	103	0	1225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1759	1615	0	1792
Flt Permitted						
Satd. Flow (perm)	0	1644	1759	1615	0	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	0	73	687	112	0	1332
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	73	687	112	0	1332
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.8%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

6: FM973 & Driveway 1B

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	67	632	103	0	1225
Future Volume (Veh/h)	0	67	632	103	0	1225
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	73	687	112	0	1332
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)					636	
pX, platoon unblocked						
vC, conflicting volume	2019	687			799	
vC1, stage 1 conf vol	687					
vC2, stage 2 conf vol	1332					
vCu, unblocked vol	2019	687			799	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	84			100	
cM capacity (veh/h)	223	450			833	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	73	687	112	1332		
Volume Left	0	0	0	0		
Volume Right	73	0	112	0		
cSH	450	1700	1700	1700		
Volume to Capacity	0.16	0.40	0.07	0.78		
Queue Length 95th (ft)	14	0	0	0		
Control Delay (s)	14.5	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	14.5	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			67.8%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	67	632	103	0	1225
Future Vol, veh/h	0	67	632	103	0	1225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	0	73	687	112	0	1332
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	687	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	450	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	450	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	14.5	0		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	450		-		
HCM Lane V/C Ratio	-	0.162		-		
HCM Control Delay (s)	-	14.5		-		
HCM Lane LOS	-	B		-		
HCM 95th %tile Q(veh)	-	0.6		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	138	43	691	25	111	1113
Future Volume (vph)	138	43	691	25	111	1113
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	150	47	751	27	121	1210
Shared Lane Traffic (%)						
Lane Group Flow (vph)	150	47	751	27	121	1210
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	72.9%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	138	43	691	25	111	1113
Future Volume (Veh/h)	138	43	691	25	111	1113
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	150	47	751	27	121	1210
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		4				
Median type			TWLT			TWLT
Median storage veh)			2			2
Upstream signal (ft)						1181
pX, platoon unblocked						
vC, conflicting volume	2203	751			778	
vC1, stage 1 conf vol	751					
vC2, stage 2 conf vol	1452					
vCu, unblocked vol	2203	751			778	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	12	89			86	
cM capacity (veh/h)	171	414			848	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	197	751	27	121	1210	
Volume Left	150	0	0	121	0	
Volume Right	47	0	27	0	0	
cSH	219	1700	1700	848	1700	
Volume to Capacity	0.90	0.44	0.02	0.14	0.71	
Queue Length 95th (ft)	183	0	0	12	0	
Control Delay (s)	83.0	0.0	0.0	10.0	0.0	
Lane LOS	F			A		
Approach Delay (s)	83.0	0.0		0.9		
Approach LOS	F					
Intersection Summary						
Average Delay			7.6			
Intersection Capacity Utilization			72.9%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	11.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	138	43	691	25	111	1113
Future Vol, veh/h	138	43	691	25	111	1113
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	250	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	150	47	751	27	121	1210
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2203	751	0	0	778	0
Stage 1	751	-	-	-	-	-
Stage 2	1452	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	~ 50	414	-	-	848	-
Stage 1	470	-	-	-	-	-
Stage 2	217	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	~ 43	414	-	-	848	-
Mov Cap-2 Maneuver	~ 137	-	-	-	-	-
Stage 1	470	-	-	-	-	-
Stage 2	186	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	131.9	0		0.9		
HCM LOS	F					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	137	414	848	-
HCM Lane V/C Ratio	-	-	1.095	0.113	0.142	-
HCM Control Delay (s)	-	-	168.4	14.8	9.9	-
HCM Lane LOS	-	-	F	B	A	-
HCM 95th %tile Q(veh)	-	-	8.4	0.4	0.5	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	90	643	12	207	1053
Future Volume (vph)	6	90	643	12	207	1053
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.998			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1654	0	1758	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1654	0	1758	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	106	756	14	244	1239
Shared Lane Traffic (%)						
Lane Group Flow (vph)	113	0	770	0	244	1239
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	68.0%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	90	643	12	207	1053
Future Volume (Veh/h)	6	90	643	12	207	1053
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	106	756	14	244	1239
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT			TWLT
Median storage veh			2			2
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2490	763			770	
vC1, stage 1 conf vol	763					
vC2, stage 2 conf vol	1727					
vCu, unblocked vol	2490	763			770	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	74			71	
cM capacity (veh/h)	108	408			854	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	113	770	244	1239		
Volume Left	7	0	244	0		
Volume Right	106	14	0	0		
cSH	348	1700	854	1700		
Volume to Capacity	0.32	0.45	0.29	0.73		
Queue Length 95th (ft)	34	0	30	0		
Control Delay (s)	20.3	0.0	10.9	0.0		
Lane LOS	C		B			
Approach Delay (s)	20.3	0.0	1.8			
Approach LOS	C					
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utilization			68.0%	ICU Level of Service	C	
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	90	643	12	207	1053
Future Vol, veh/h	6	90	643	12	207	1053
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	106	756	14	244	1239
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2490	763	0	0	770	0
Stage 1	763	-	-	-	-	-
Stage 2	1727	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	33	408	-	-	854	-
Stage 1	464	-	-	-	-	-
Stage 2	159	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	24	408	-	-	854	-
Mov Cap-2 Maneuver	90	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	114	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	21.2	0		1.8		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	-	334	854	-	
HCM Lane V/C Ratio	-	-	0.338	0.285	-	
HCM Control Delay (s)	-	-	21.2	10.9	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.5	1.2	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	93	33	12	561	1018	39
Future Volume (vph)	93	33	12	561	1018	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964					0.850
Flt Protected	0.965		0.950			
Satd. Flow (prot)	1767	0	1805	1759	1810	1615
Flt Permitted	0.965		0.950			
Satd. Flow (perm)	1767	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	103	37	13	623	1131	43
Shared Lane Traffic (%)						
Lane Group Flow (vph)	140	0	13	623	1131	43
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.4%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	93	33	12	561	1018	39
Future Volume (Veh/h)	93	33	12	561	1018	39
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	103	37	13	623	1131	43
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1780	1131	1174			
vC1, stage 1 conf vol	1131					
vC2, stage 2 conf vol	649					
vCu, unblocked vol	1780	1131	1174			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	62	85	98			
cM capacity (veh/h)	271	250	602			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	140	13	623	1131	43	
Volume Left	103	13	0	0	0	
Volume Right	37	0	0	0	43	
cSH	265	602	1700	1700	1700	
Volume to Capacity	0.53	0.02	0.37	0.67	0.03	
Queue Length 95th (ft)	71	2	0	0	0	
Control Delay (s)	32.9	11.1	0.0	0.0	0.0	
Lane LOS	D	B				
Approach Delay (s)	32.9	0.2		0.0		
Approach LOS	D					
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization			67.4%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	93	33	12	561	1018	39
Future Vol, veh/h	93	33	12	561	1018	39
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	103	37	13	623	1131	43
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1780	1131	1174	0	-	0
Stage 1	1131	-	-	-	-	-
Stage 2	649	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 91	250	602	-	-	-
Stage 1	311	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 89	250	602	-	-	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	304	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	45.4		0.2		0	
HCM LOS	E					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	602	-	222	-	-	
HCM Lane V/C Ratio	0.022	-	0.631	-	-	
HCM Control Delay (s)	11.1	-	45.4	-	-	
HCM Lane LOS	B	-	E	-	-	
HCM 95th %tile Q(veh)	0.1	-	3.8	-	-	
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Future Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.959			0.877			0.986			0.999	
Flt Protected		0.966			0.996		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1660	0	1805	1676	0	1805	1711	0
Flt Permitted		0.966			0.996		0.950			0.950		
Satd. Flow (perm)	0	1760	0	0	1660	0	1805	1676	0	1805	1711	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	14	0	6	19	0	201	2	435	44	400	790	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	220	0	2	479	0	400	797	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	63.7%						ICU Level of Service B					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 AM Peak Hour
Phase 1 Build-Out Year w/Development

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (veh/h)	12	0	5	17	0	177	2	383	39	352	695	6	
Future Volume (Veh/h)	12	0	5	17	0	177	2	383	39	352	695	6	
Sign Control		Stop			Stop			Free			Free		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Hourly flow rate (vph)	14	0	6	19	0	201	2	435	44	400	790	7	
Pedestrians													
Lane Width (ft)													
Walking Speed (ft/s)													
Percent Blockage													
Right turn flare (veh)													
Median type								TWLTL					TWLTL
Median storage veh								2					2
Upstream signal (ft)													
pX, platoon unblocked													
vC, conflicting volume	2234	2076	794	2057	2058	457	797					479	
vC1, stage 1 conf vol	1594	1594		461	461								
vC2, stage 2 conf vol	640	483		1596	1597								
vCu, unblocked vol	2234	2076	794	2057	2058	457	797					479	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1					4.1	
tC, 2 stage (s)	6.1	5.5		6.1	5.5								
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2					2.2	
p0 queue free %	5	100	98	76	100	67	100					63	
cM capacity (veh/h)	15	91	391	80	101	608	834					1094	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	20	220	2	479	400	797							
Volume Left	14	19	2	0	400	0							
Volume Right	6	201	0	44	0	7							
cSH	21	388	834	1700	1094	1700							
Volume to Capacity	0.97	0.57	0.00	0.28	0.37	0.47							
Queue Length 95th (ft)	67	85	0	0	42	0							
Control Delay (s)	446.3	25.8	9.3	0.0	10.2	0.0							
Lane LOS	F	D	A		B								
Approach Delay (s)	446.3	25.8	0.0		3.4								
Approach LOS	F	D											
Intersection Summary													
Average Delay			9.7										
Intersection Capacity Utilization			63.7%	ICU Level of Service				B					
Analysis Period (min)			15										

Intersection												
Int Delay, s/veh	17.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	12	0	5	17	0	177	2	383	39	352	695	6
Future Vol, veh/h	12	0	5	17	0	177	2	383	39	352	695	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	11	0
Mvmt Flow	14	0	6	19	0	201	2	435	44	400	790	7

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2156	2077	794	2058	2058	457	797	0	0	479	0	0
Stage 1	1594	1594	-	461	461	-	-	-	-	-	-	-
Stage 2	562	483	-	1597	1597	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	35	54	391	41	56	608	834	-	-	1094	-	-
Stage 1	136	168	-	584	569	-	-	-	-	-	-	-
Stage 2	515	556	-	135	168	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	17	34	391	29	35	608	834	-	-	1094	-	-
Mov Cap-2 Maneuver	17	34	-	29	35	-	-	-	-	-	-	-
Stage 1	136	107	-	583	568	-	-	-	-	-	-	-
Stage 2	344	555	-	84	107	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	348.3	106.2	0	3.4
HCM LOS	F	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	834	-	-	24 221	1094	-	-
HCM Lane V/C Ratio	0.003	-	-	0.805 0.998	0.366	-	-
HCM Control Delay (s)	9.3	-	-	\$ 348.3 106.2	10.2	-	-
HCM Lane LOS	A	-	-	F F	B	-	-
HCM 95th %tile Q(veh)	0	-	-	2.4 9.1	1.7	-	-

Notes			
~: Volume exceeds capacity	\$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				319						367		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	871	319	211	484	0	0	495	418	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

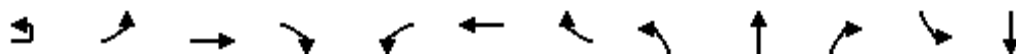
Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases					2						8	4
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	7.0	7.0	10.0	10.0	7.0	10.0		8.0	8.0	8.0	8.0	8.0
Minimum Split (s)	13.5	13.5	24.5	24.5	13.5	24.5		25.0	25.0	25.0	25.0	25.0
Total Split (s)	13.5	13.5	36.0	36.0	23.0	45.5		36.0	36.0	36.0	25.0	25.0
Total Split (%)	11.3%	11.3%	30.0%	30.0%	19.2%	37.9%		30.0%	30.0%	30.0%	20.8%	20.8%
Maximum Green (s)	7.0	7.0	29.5	29.5	16.5	39.0		29.0	29.0	29.0	18.0	18.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)	6.5		6.5	6.5	6.5	6.5		7.0		7.0	7.0	
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)	7.0		29.5	29.5	15.9	49.2		29.0		29.0		
Actuated g/C Ratio	0.07		0.31	0.31	0.17	0.52		0.31		0.31		
v/c Ratio	0.07		0.80	0.47	0.73	0.28		0.95		0.56		
Control Delay	42.2		36.7	5.5	53.1	14.2		62.1		7.9		
Queue Delay	0.0		0.0	0.0	0.0	0.0		0.0		0.0		
Total Delay	42.2		36.7	5.5	53.1	14.2		62.1		7.9		
LOS	D		D	A	D	B		E		A		
Approach Delay			28.5			26.0		37.3				
Approach LOS			C			C		D				

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 94.4

Natural Cycle: 130

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.95

Intersection Signal Delay: 30.7

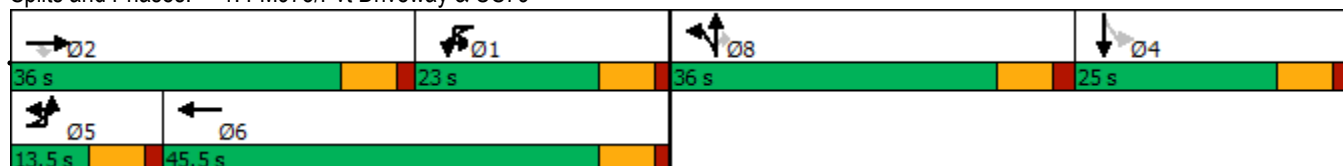
Intersection LOS: C

Intersection Capacity Utilization 74.2%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	7.0	10.0	10.0	7.0	10.0	8.0	8.0	8.0
Minimum Split (s)	13.5	24.5	24.5	13.5	24.5	25.0	25.0	25.0
Total Split (s)	13.5	36.0	36.0	23.0	45.5	36.0	36.0	25.0
Total Split (%)	11.3%	30.0%	30.0%	19.2%	37.9%	30.0%	30.0%	21%
Maximum Green (s)	7.0	29.5	29.5	16.5	39.0	29.0	29.0	18.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	7.0	29.5	29.5	16.5	39.0	29.0	29.0	0.0
90th %ile Term Code	Max	Max	Max	Max	Hold	Max	Max	Skip
70th %ile Green (s)	0.0	29.5	29.5	16.5	52.5	29.0	29.0	0.0
70th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	29.5	29.5	16.5	52.5	29.0	29.0	0.0
50th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	29.5	29.5	16.5	52.5	29.0	29.0	0.0
30th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	29.5	29.5	13.5	49.5	29.0	29.0	0.0
10th %ile Term Code	Skip	Max	Max	Gap	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 120								
Actuated Cycle Length: 94.4								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 95								
70th %ile Actuated Cycle: 95								
50th %ile Actuated Cycle: 95								
30th %ile Actuated Cycle: 95								
10th %ile Actuated Cycle: 92								

Queues
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	9	871	319	211	484	495	418
v/c Ratio	0.07	0.80	0.47	0.73	0.28	0.95	0.56
Control Delay	42.2	36.7	5.5	53.1	14.2	62.1	7.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.2	36.7	5.5	53.1	14.2	62.1	7.9
Queue Length 50th (ft)	5	253	0	119	74	291	22
Queue Length 95th (ft)	20	328	60	m#216	m142	#491	103
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	134	1084	673	300	1741	522	740
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.80	0.47	0.70	0.28	0.95	0.56
Intersection Summary							
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.							
m Volume for 95th percentile queue is metered by upstream signal.							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.5	6.5	6.5	6.5	6.5			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3342			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3342			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
RTOR Reduction (vph)	0	0	0	225	0	0	0	0	0	260	0	0
Lane Group Flow (vph)	0	9	871	94	211	484	0	0	495	158	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	29.5	29.5	21.1	49.2			29.0	29.0		
Effective Green, g (s)		1.4	29.5	29.5	21.1	49.2			29.0	29.0		
Actuated g/C Ratio		0.01	0.30	0.30	0.21	0.49			0.29	0.29		
Clearance Time (s)		6.5	6.5	6.5	6.5	6.5			7.0	7.0		
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		25	1028	430	364	1650			495	460		
v/s Ratio Prot		0.00	c0.25		c0.12	0.14			c0.29			
v/s Ratio Perm				0.06						0.10		
v/c Ratio		0.36	0.85	0.22	0.58	0.29			1.00	0.34		
Uniform Delay, d1		48.7	32.9	26.4	35.3	14.9			35.3	27.8		
Progression Factor		1.00	1.00	1.00	1.00	1.01			1.00	1.00		
Incremental Delay, d2		8.7	6.6	0.3	2.2	0.1			40.5	0.4		
Delay (s)		57.3	39.5	26.6	37.4	15.2			75.8	28.2		
Level of Service		E	D	C	D	B			E	C		
Approach Delay (s)			36.2			21.9			54.0			0.0
Approach LOS			D			C			D			A
Intersection Summary												
HCM 2000 Control Delay			38.5			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.91									
Actuated Cycle Length (s)			99.6			Sum of lost time (s)			27.0			
Intersection Capacity Utilization			74.2%			ICU Level of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	972	246	0	697	0	219
Future Volume (vph)	972	246	0	697	0	219
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3471	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3471	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1070			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	0%	7%	0%	0%
Adj. Flow (vph)	1057	267	0	758	0	238
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1057	267	0	758	0	238
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	47.1%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	972	246	0	697	0	219
Future Volume (Veh/h)	972	246	0	697	0	219
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1057	267	0	758	0	238
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1070					
pX, platoon unblocked			0.79		0.79	0.79
vC, conflicting volume			1324		1436	528
vC1, stage 1 conf vol					1057	
vC2, stage 2 conf vol					379	
vCu, unblocked vol			884		1025	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	72
cM capacity (veh/h)			613		388	864
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	528	528	267	379	379	238
Volume Left	0	0	0	0	0	0
Volume Right	0	0	267	0	0	238
cSH	1700	1700	1700	1700	1700	864
Volume to Capacity	0.31	0.31	0.16	0.22	0.22	0.28
Queue Length 95th (ft)	0	0	0	0	0	28
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	10.7
Lane LOS						B
Approach Delay (s)	0.0			0.0		
Approach LOS						B
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			47.1%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	972	246	0	697	0	219
Future Vol, veh/h	972	246	0	697	0	219
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	0	7	0	0
Mvmt Flow	1057	267	0	758	0	238
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	529
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	499
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	499
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		18.6	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	499	-	-	-		
HCM Lane V/C Ratio	0.477	-	-	-		
HCM Control Delay (s)	18.6	-	-	-		
HCM Lane LOS	C	-	-	-		
HCM 95th %tile Q(veh)	2.5	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	47	0	1149	0	658	0	0	0
Future Volume (vph)	47	0	1149	0	658	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3471	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3471	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	51	0	1235	0	708	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	51	1235	0	708	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	41.8%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↕↕	↕	↕↕		↕↕	
Traffic Volume (veh/h)	47	0	1149	0	658	0	0	0
Future Volume (Veh/h)	47	0	1149	0	658	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	1235	0	708	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	708		0			1326	354
vC1, stage 1 conf vol							708	
vC2, stage 2 conf vol							618	
vCu, unblocked vol	0	708		0			1326	354
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	900		0			360	648
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	618	618	354	354	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.36	0.36	0.21	0.21	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.8%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.3							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	47	0	1149	0	658	0	0	0
Future Vol, veh/h	47	0	1149	0	658	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	0	7	0	0	0
Mvmt Flow	51	0	1235	0	708	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	708	708	0	1235	-	0	1428	354
Stage 1	-	-	-	-	-	-	708	-
Stage 2	-	-	-	-	-	-	720	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	518	900	-	239	-	0	128	648
Stage 1	-	-	-	-	-	0	455	-
Stage 2	-	-	-	-	-	0	448	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	518	518	-	239	-	-	115	648
Mov Cap-2 Maneuver	-	-	-	-	-	-	115	-
Stage 1	-	-	-	-	-	-	410	-
Stage 2	-	-	-	-	-	-	448	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.5		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	518	-	239	-	-			
HCM Lane V/C Ratio	0.098	-	-	-	-			
HCM Control Delay (s)	12.7	-	0	-	0			
HCM Lane LOS	B	-	A	-	A			
HCM 95th %tile Q(veh)	0.3	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	5	1151	29	5	612	0	20	3	3	1	7	2
Future Volume (vph)	5	1151	29	5	612	0	20	3	3	1	7	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.986			0.975	
Flt Protected	0.950			0.950				0.962			0.995	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1843	0
Flt Permitted	0.950			0.950				0.962			0.995	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1843	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	6	1308	33	6	695	0	23	3	3	1	8	2
Shared Lane Traffic (%)												
Lane Group Flow (vph)	6	1308	33	6	695	0	0	29	0	0	11	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.4%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	1151	29	5	612	0	20	3	3	1	7	2
Future Volume (Veh/h)	5	1151	29	5	612	0	20	3	3	1	7	2
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	1308	33	6	695	0	23	3	3	1	8	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh)	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	695			1308			1684	2027	654	1374	2027	348
vC1, stage 1 conf vol							1320	1320		707	707	
vC2, stage 2 conf vol							364	707		668	1320	
vCu, unblocked vol	695			1308			1684	2027	654	1374	2027	348
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			86	99	99	100	96	100
cM capacity (veh/h)	910			536			160	201	414	292	197	654
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	6	654	654	33	6	348	348	0	29	11		
Volume Left	6	0	0	0	6	0	0	0	23	1		
Volume Right	0	0	0	33	0	0	0	0	3	2		
cSH	910	1700	1700	1700	536	1700	1700	1700	175	234		
Volume to Capacity	0.01	0.38	0.38	0.02	0.01	0.20	0.20	0.00	0.17	0.05		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	14	4		
Control Delay (s)	9.0	0.0	0.0	0.0	11.8	0.0	0.0	0.0	29.6	21.2		
Lane LOS	A				B				D	C		
Approach Delay (s)	0.0				0.1				29.6	21.2		
Approach LOS									D	C		
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	45.4%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	5	1151	29	5	612	0	20	3	3	1	7	2
Future Vol, veh/h	5	1151	29	5	612	0	20	3	3	1	7	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	6	1308	33	6	695	0	23	3	3	1	8	2

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	695	0	0	1308	0	0	1684	2027	-	1375	2027	-
Stage 1	-	-	-	-	-	-	1320	1320	-	707	707	-
Stage 2	-	-	-	-	-	-	364	707	-	668	1320	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	910	-	-	536	-	-	63	58	0	106	58	0
Stage 1	-	-	-	-	-	-	168	228	0	397	441	0
Stage 2	-	-	-	-	-	-	633	441	0	419	228	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	910	-	-	536	-	-	56	57	-	100	57	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	56	57	-	100	57	-
Stage 1	-	-	-	-	-	-	167	226	-	394	436	-
Stage 2	-	-	-	-	-	-	614	436	-	410	226	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	910	-	-	536	-	-	-
HCM Lane V/C Ratio	-	0.006	-	-	0.011	-	-	-
HCM Control Delay (s)	-	9	-	-	11.8	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	65	766	109	0	500
Future Volume (vph)	0	65	766	109	0	500
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1827	1615	0	1712
Flt Permitted						
Satd. Flow (perm)	0	1644	1827	1615	0	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	0	71	833	118	0	543
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	71	833	118	0	543
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	51.0%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

6: FM973 & Driveway 1B

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	65	766	109	0	500
Future Volume (Veh/h)	0	65	766	109	0	500
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	71	833	118	0	543
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)					636	
pX, platoon unblocked						
vC, conflicting volume	1376	833			951	
vC1, stage 1 conf vol	833					
vC2, stage 2 conf vol	543					
vCu, unblocked vol	1376	833			951	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	81			100	
cM capacity (veh/h)	368	372			730	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	71	833	118	543		
Volume Left	0	0	0	0		
Volume Right	71	0	118	0		
cSH	372	1700	1700	1700		
Volume to Capacity	0.19	0.49	0.07	0.32		
Queue Length 95th (ft)	17	0	0	0		
Control Delay (s)	17.0	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	17.0	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			51.0%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	65	766	109	0	500
Future Vol, veh/h	0	65	766	109	0	500
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	0	71	833	118	0	543
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	-	833	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	372	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	372	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	16.9	0	0			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBT			
Capacity (veh/h)	-	-	372	-		
HCM Lane V/C Ratio	-	-	0.19	-		
HCM Control Delay (s)	-	-	16.9	-		
HCM Lane LOS	-	-	C	-		
HCM 95th %tile Q(veh)	-	-	0.7	-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	98	35	840	17	72	428
Future Volume (vph)	98	35	840	17	72	428
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	107	38	913	18	78	465
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	38	913	18	78	465
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	63.6%			ICU Level of Service B		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	98	35	840	17	72	428
Future Volume (Veh/h)	98	35	840	17	72	428
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	107	38	913	18	78	465
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	4					
Median type	TWLTL			TWLTL		
Median storage veh	2			2		
Upstream signal (ft)	1181					
pX, platoon unblocked						
vC, conflicting volume	1534	913			931	
vC1, stage 1 conf vol	913					
vC2, stage 2 conf vol	621					
vCu, unblocked vol	1534	913			931	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	66	89			90	
cM capacity (veh/h)	318	334			743	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	145	913	18	78	465	
Volume Left	107	0	0	78	0	
Volume Right	38	0	18	0	0	
cSH	431	1700	1700	743	1700	
Volume to Capacity	0.34	0.54	0.01	0.10	0.27	
Queue Length 95th (ft)	37	0	0	9	0	
Control Delay (s)	20.7	0.0	0.0	10.4	0.0	
Lane LOS	C			B		
Approach Delay (s)	20.7	0.0		1.5		
Approach LOS	C					
Intersection Summary						
Average Delay	2.4					
Intersection Capacity Utilization	63.6%			ICU Level of Service		B
Analysis Period (min)	15					

Intersection						
Int Delay, s/veh	2.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	98	35	840	17	72	428
Future Vol, veh/h	98	35	840	17	72	428
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	250	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	107	38	913	18	78	465
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1534	913	0	0	931	0
Stage 1	913	-	-	-	-	-
Stage 2	621	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	129	334	-	-	743	-
Stage 1	395	-	-	-	-	-
Stage 2	540	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	115	334	-	-	743	-
Mov Cap-2 Maneuver	250	-	-	-	-	-
Stage 1	395	-	-	-	-	-
Stage 2	483	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	26.4	0	1.5			
HCM LOS	D					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 250 334	743	-		
HCM Lane V/C Ratio	-	- 0.426 0.114	0.105	-		
HCM Control Delay (s)	-	- 29.7 17.2	10.4	-		
HCM Lane LOS	-	- D C	B	-		
HCM 95th %tile Q(veh)	-	- 2 0.4	0.4	-		

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	26	836	4	12	534
Future Volume (vph)	8	26	836	4	12	534
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.896		0.999			
Flt Protected	0.989				0.950	
Satd. Flow (prot)	1684	0	1825	0	1805	1712
Flt Permitted	0.989				0.950	
Satd. Flow (perm)	1684	0	1825	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	27	862	4	12	551
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	866	0	12	551
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	54.2%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	26	836	4	12	534
Future Volume (Veh/h)	8	26	836	4	12	534
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	27	862	4	12	551
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1439	864			866	
vC1, stage 1 conf vol	864					
vC2, stage 2 conf vol	575					
vCu, unblocked vol	1439	864			866	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	92			98	
cM capacity (veh/h)	352	357			786	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	35	866	12	551		
Volume Left	8	0	12	0		
Volume Right	27	4	0	0		
cSH	356	1700	786	1700		
Volume to Capacity	0.10	0.51	0.02	0.32		
Queue Length 95th (ft)	8	0	1	0		
Control Delay (s)	16.2	0.0	9.7	0.0		
Lane LOS	C		A			
Approach Delay (s)	16.2	0.0	0.2			
Approach LOS	C					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			54.2%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	26	836	4	12	534
Future Vol, veh/h	8	26	836	4	12	534
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	27	862	4	12	551
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1439	864	0	0	866	0
Stage 1	864	-	-	-	-	-
Stage 2	575	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	148	357	-	-	786	-
Stage 1	416	-	-	-	-	-
Stage 2	567	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	146	357	-	-	786	-
Mov Cap-2 Maneuver	282	-	-	-	-	-
Stage 1	416	-	-	-	-	-
Stage 2	558	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	17	0		0.2		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	336	786	-	
HCM Lane V/C Ratio	-	-	0.104	0.016	-	
HCM Control Delay (s)	-	-	17	9.7	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	62	22	38	773	448	97
Future Volume (vph)	62	22	38	773	448	97
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1766	0	1805	1827	1696	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1766	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	67	24	41	840	487	105
Shared Lane Traffic (%)						
Lane Group Flow (vph)	91	0	41	840	487	105
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	52.1%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	62	22	38	773	448	97
Future Volume (Veh/h)	62	22	38	773	448	97
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	67	24	41	840	487	105
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage veh)				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1409	487	592			
vC1, stage 1 conf vol	487					
vC2, stage 2 conf vol	922					
vCu, unblocked vol	1409	487	592			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	80	96	96			
cM capacity (veh/h)	336	585	994			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	91	41	840	487	105	
Volume Left	67	41	0	0	0	
Volume Right	24	0	0	0	105	
cSH	378	994	1700	1700	1700	
Volume to Capacity	0.24	0.04	0.49	0.29	0.06	
Queue Length 95th (ft)	23	3	0	0	0	
Control Delay (s)	17.5	8.8	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	17.5	0.4	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			52.1%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	62	22	38	773	448	97
Future Vol, veh/h	62	22	38	773	448	97
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	67	24	41	840	487	105

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1409	487	592	0	-	0
Stage 1	487	-	-	-	-	-
Stage 2	922	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	154	585	994	-	-	-
Stage 1	622	-	-	-	-	-
Stage 2	391	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	148	585	994	-	-	-
Mov Cap-2 Maneuver	279	-	-	-	-	-
Stage 1	596	-	-	-	-	-
Stage 2	391	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	20.5	0.4	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	994	-	323	-	-
HCM Lane V/C Ratio	0.042	-	0.283	-	-
HCM Control Delay (s)	8.8	-	20.5	-	-
HCM Lane LOS	A	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	1.1	-	-

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Future Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.958			0.899						0.996	
Flt Protected		0.967			0.988		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1688	0	1805	1827	0	1805	1710	0
Flt Permitted		0.967			0.988		0.950			0.950		
Satd. Flow (perm)	0	1760	0	0	1688	0	1805	1827	0	1805	1710	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	9	0	4	5	0	15	4	854	0	2	490	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	20	0	4	854	0	2	504	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	51.4%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 PM Peak Hour
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	8	0	4	5	0	14	4	786	0	2	451	13
Future Volume (Veh/h)	8	0	4	5	0	14	4	786	0	2	451	13
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	0	4	5	0	15	4	854	0	2	490	14
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							TWLTL			TWLTL		
Median storage veh							2			2		
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1378	1363	497	1360	1370	854	504				854	
vC1, stage 1 conf vol	501	501		862	862							
vC2, stage 2 conf vol	877	862		498	508							
vCu, unblocked vol	1378	1363	497	1360	1370	854	504				854	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	97	100	99	98	100	96	100				100	
cM capacity (veh/h)	293	326	577	308	326	361	1071				794	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	13	20	4	854	2	504						
Volume Left	9	5	4	0	2	0						
Volume Right	4	15	0	0	0	14						
cSH	345	347	1071	1700	794	1700						
Volume to Capacity	0.04	0.06	0.00	0.50	0.00	0.30						
Queue Length 95th (ft)	3	5	0	0	0	0						
Control Delay (s)	15.8	16.0	8.4	0.0	9.5	0.0						
Lane LOS	C	C	A		A							
Approach Delay (s)	15.8	16.0	0.0		0.0							
Approach LOS	C	C										
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	51.4%			ICU Level of Service				A				
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔		↔	↔		↔	↔	
Traffic Vol, veh/h	8	0	4	5	0	14	4	786	0	2	451	13
Future Vol, veh/h	8	0	4	5	0	14	4	786	0	2	451	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	11	0
Mvmt Flow	9	0	4	5	0	15	4	854	0	2	490	14

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1371	1363	497	1365	1370	854	504	0	0	854	0	0
Stage 1	501	501	-	862	862	-	-	-	-	-	-	-
Stage 2	870	862	-	503	508	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	125	149	577	126	148	361	1071	-	-	794	-	-
Stage 1	556	546	-	353	375	-	-	-	-	-	-	-
Stage 2	349	375	-	555	542	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	119	148	577	124	147	361	1071	-	-	794	-	-
Mov Cap-2 Maneuver	119	148	-	124	147	-	-	-	-	-	-	-
Stage 1	554	544	-	352	374	-	-	-	-	-	-	-
Stage 2	333	374	-	549	540	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	29.2		21.4		0		0	
HCM LOS	D		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1071	-	-	162 240	794	-	-
HCM Lane V/C Ratio	0.004	-	-	0.081 0.086	0.003	-	-
HCM Control Delay (s)	8.4	-	-	29.2 21.4	9.5	-	-
HCM Lane LOS	A	-	-	D C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.3 0.3	0	-	-

Appendix D. Traffic Simulation Output

- Phase 1 Buildout Year (2025) Projected Conditions with Development, AM & PM
 - Mitigation:
 - With Traffic Signal at FM 973 at Driveway 2B
 - With Traffic Signal at FM 973 at Wesley Miller Lane
 - With Existing Timings at FM 973 at US 79

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850						0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				754						143		289
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	7	563	774	500	642	0	0	541	202	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.1	36.4	36.4	25.3	65.0			50.5	50.5		5.1
Actuated g/C Ratio		0.08	0.27	0.27	0.19	0.49			0.38	0.38		0.04
v/c Ratio		0.05	0.62	0.80	1.54	0.39			0.85	0.30		0.00
Control Delay		65.3	45.2	10.1	292.2	22.7			53.8	12.0		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		65.3	45.2	10.1	292.2	22.7			53.8	12.0		0.0
LOS		E	D	B	F	C			D	B		A
Approach Delay			25.1			140.7			42.4			
Approach LOS			C			F			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 133.2

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.54

Intersection Signal Delay: 69.9

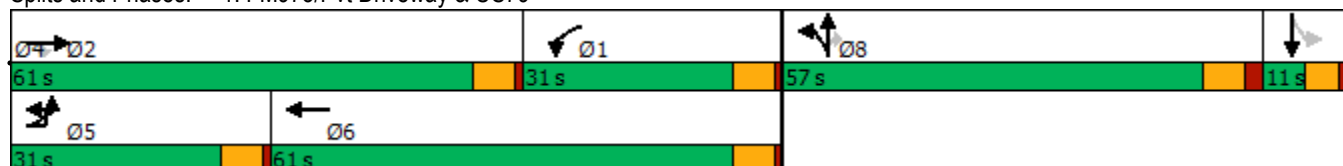
Intersection LOS: E

Intersection Capacity Utilization 91.4%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	40.8	40.8	25.0	71.8	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	35.2	35.2	25.0	66.2	50.0	50.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	29.8	29.8	25.0	60.8	50.0	50.0	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	24.4	24.4	25.0	55.4	50.0	50.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 133.2								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 160								
70th %ile Actuated Cycle: 134.8								
50th %ile Actuated Cycle: 129.2								
30th %ile Actuated Cycle: 123.8								
10th %ile Actuated Cycle: 118.4								

Queues
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	7	563	774	500	642	541	202	1
v/c Ratio	0.05	0.62	0.80	1.54	0.39	0.85	0.30	0.00
Control Delay	65.3	45.2	10.1	292.2	22.7	53.8	12.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.3	45.2	10.1	292.2	22.7	53.8	12.0	0.0
Queue Length 50th (ft)	6	218	12	~586	160	407	31	0
Queue Length 95th (ft)	25	305	150	#1034	290	#843	115	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	342	1381	1087	325	1648	633	683	340
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.41	0.71	1.54	0.39	0.85	0.30	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
RTOR Reduction (vph)	0	0	0	562	0	0	0	0	0	93	0	1
Lane Group Flow (vph)	0	7	563	212	500	642	0	0	541	109	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.7	36.4	36.4	30.4	65.1			50.5	50.5		0.8
Effective Green, g (s)		1.7	36.4	36.4	30.4	65.1			50.5	50.5		0.8
Actuated g/C Ratio		0.01	0.25	0.25	0.21	0.45			0.35	0.35		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		21	842	395	365	1534			589	553		9
v/s Ratio Prot		0.00	c0.17		c0.29	0.19			c0.32			c0.00
v/s Ratio Perm				0.14						0.07		
v/c Ratio		0.33	0.67	0.54	1.37	0.42			0.92	0.20		0.00
Uniform Delay, d1		70.1	47.9	46.1	56.3	26.3			44.3	32.2		70.8
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		1.00
Incremental Delay, d2		3.4	1.6	0.7	183.0	0.1			19.3	0.2		0.0
Delay (s)		73.5	49.5	46.8	238.4	25.7			63.6	32.4		70.8
Level of Service		E	D	D	F	C			E	C		E
Approach Delay (s)			48.1			118.8			55.1			70.8
Approach LOS			D			F			E			E
Intersection Summary												
HCM 2000 Control Delay			74.7		HCM 2000 Level of Service				E			
HCM 2000 Volume to Capacity ratio			0.95									
Actuated Cycle Length (s)			143.1		Sum of lost time (s)				25.0			
Intersection Capacity Utilization			91.4%		ICU Level of Service				F			
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑↗		↑↑		↑↗
Traffic Volume (vph)	501	280	0	1163	0	180
Future Volume (vph)	501	280	0	1163	0	180
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3282	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3282	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1072			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	0%	0%	7%	0%	0%
Adj. Flow (vph)	545	304	0	1264	0	196
Shared Lane Traffic (%)						
Lane Group Flow (vph)	545	304	0	1264	0	196
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	35.5%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	501	280	0	1163	0	180
Future Volume (Veh/h)	501	280	0	1163	0	180
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	545	304	0	1264	0	196
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1072					
pX, platoon unblocked			0.91		0.91	0.91
vC, conflicting volume			849		1177	272
vC1, stage 1 conf vol					545	
vC2, stage 2 conf vol					632	
vCu, unblocked vol			642		1002	10
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	80
cM capacity (veh/h)			869		433	981
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	272	272	304	632	632	196
Volume Left	0	0	0	0	0	0
Volume Right	0	0	304	0	0	196
cSH	1700	1700	1700	1700	1700	981
Volume to Capacity	0.16	0.16	0.18	0.37	0.37	0.20
Queue Length 95th (ft)	0	0	0	0	0	19
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.6
Lane LOS						A
Approach Delay (s)	0.0			0.0		
Approach LOS						A
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			35.5%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	501	280	0	1163	0	180
Future Vol, veh/h	501	280	0	1163	0	180
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	0	0	7	0	0
Mvmt Flow	545	304	0	1264	0	196
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	273
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	731
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	731
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.7	
HCM LOS					B	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	731	-	-	-		
HCM Lane V/C Ratio	0.268	-	-	-		
HCM Control Delay (s)	11.7	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	1.1	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	61	0	621	0	1128	0	0	0
Future Volume (vph)	61	0	621	0	1128	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	67	0	682	0	1240	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	67	682	0	1240	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	41.2%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	61	0	621	0	1128	0	0	0
Future Volume (Veh/h)	61	0	621	0	1128	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	682	0	1240	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1240		0			1581	620
vC1, stage 1 conf vol							1240	
vC2, stage 2 conf vol							341	
vCu, unblocked vol	0	1240		0			1581	620
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	569		0			229	436
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	341	341	620	620	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.20	0.20	0.36	0.36	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.2%		ICU Level of Service		A	
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.9							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	61	0	621	0	1128	0	0	0
Future Vol, veh/h	61	0	621	0	1128	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	67	0	682	0	1240	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	1240	1240	0	682	-	0	1715	620
Stage 1	-	-	-	-	-	-	1240	-
Stage 2	-	-	-	-	-	-	475	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	237	569	-	538	-	0	83	436
Stage 1	-	-	-	-	-	0	240	-
Stage 2	-	-	-	-	-	0	597	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	237	237	-	538	-	-	60	436
Mov Cap-2 Maneuver	-	-	-	-	-	-	60	-
Stage 1	-	-	-	-	-	-	172	-
Stage 2	-	-	-	-	-	-	597	-
Approach	EB		WB		SB			
HCM Control Delay, s	2.3		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	237	-	538	-	-			
HCM Lane V/C Ratio	0.283	-	-	-	-			
HCM Control Delay (s)	26.1	-	0	-	0			
HCM Lane LOS	D	-	A	-	A			
HCM 95th %tile Q(veh)	1.1	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.932	
Flt Protected	0.950			0.950				0.961			0.988	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Flt Permitted	0.950			0.950				0.961			0.988	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	8	731	11	8	1215	1	0	47	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	41.8%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (Veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1215			731			1372	1978	366	1616	1978	608
vC1, stage 1 conf vol							747	747		1231	1231	
vC2, stage 2 conf vol							625	1231		386	747	
vCu, unblocked vol	1215			731			1372	1978	366	1616	1978	608
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			87	96	100	98	99	99
cM capacity (veh/h)	581			883			289	210	637	180	214	444
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	8	366	366	11	8	608	608	1	47	12		
Volume Left	8	0	0	0	8	0	0	0	38	3		
Volume Right	0	0	0	11	0	0	0	1	1	6		
cSH	581	1700	1700	1700	883	1700	1700	1700	275	271		
Volume to Capacity	0.01	0.21	0.21	0.01	0.01	0.36	0.36	0.00	0.17	0.04		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	15	3		
Control Delay (s)	11.3	0.0	0.0	0.0	9.1	0.0	0.0	0.0	20.8	18.9		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				20.8	18.9		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.7											
Intersection Capacity Utilization	41.8%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Future Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	8	731	11	8	1215	1	38	8	1	3	3	6

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	1215	0	0	731	0	0	1372	1978	-	1617	1978	-
Stage 1	-	-	-	-	-	-	747	747	-	1231	1231	-
Stage 2	-	-	-	-	-	-	625	1231	-	386	747	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	581	-	-	883	-	-	107	63	0	70	63	0
Stage 1	-	-	-	-	-	-	376	423	0	191	252	0
Stage 2	-	-	-	-	-	-	444	252	0	614	423	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	581	-	-	883	-	-	101	62	-	62	62	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	101	62	-	62	62	-
Stage 1	-	-	-	-	-	-	371	417	-	188	250	-
Stage 2	-	-	-	-	-	-	434	250	-	594	417	-

Approach	EB			WB			NB		SB			
HCM Control Delay, s	0.1			0.1								
HCM LOS							-		-			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	581	-	-	883	-	-	-
HCM Lane V/C Ratio	-	0.014	-	-	0.009	-	-	-
HCM Control Delay (s)	-	11.3	-	-	9.1	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	67	632	103	0	1225
Future Volume (vph)	0	67	632	103	0	1225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1759	1615	0	1792
Flt Permitted						
Satd. Flow (perm)	0	1644	1759	1615	0	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	0	73	687	112	0	1332
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	73	687	112	0	1332
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.8%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 6: FM973 & Driveway 1B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	67	632	103	0	1225
Future Volume (Veh/h)	0	67	632	103	0	1225
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	73	687	112	0	1332
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)			545		636	
pX, platoon unblocked	0.64	0.64			0.64	
vC, conflicting volume	2019	687			799	
vC1, stage 1 conf vol	687					
vC2, stage 2 conf vol	1332					
vCu, unblocked vol	2308	235			409	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	86			100	
cM capacity (veh/h)	216	520			746	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	73	687	112	1332		
Volume Left	0	0	0	0		
Volume Right	73	0	112	0		
cSH	520	1700	1700	1700		
Volume to Capacity	0.14	0.40	0.07	0.78		
Queue Length 95th (ft)	12	0	0	0		
Control Delay (s)	13.0	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	13.0	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			67.8%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	67	632	103	0	1225
Future Vol, veh/h	0	67	632	103	0	1225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	0	73	687	112	0	1332
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	687	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	450	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	450	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	14.5	0		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	450		-		
HCM Lane V/C Ratio	-	0.162		-		
HCM Control Delay (s)	-	14.5		-		
HCM Lane LOS	-	B		-		
HCM 95th %tile Q(veh)	-	0.6		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	138	43	691	25	111	1113
Future Volume (vph)	138	43	691	25	111	1113
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		47		27		
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	150	47	751	27	121	1210
Shared Lane Traffic (%)						
Lane Group Flow (vph)	150	47	751	27	121	1210
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	3	1	1	3
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	50	50	250	50	50	250
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	50	50	6	50	50	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			122			122
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Detector 3 Position(ft)			244			244
Detector 3 Size(ft)			6			6

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex			Cl+Ex		
Detector 3 Channel						
Detector 3 Extend (s)	0.0			0.0		
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lead/Lag			Lead	Lead	Lag	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Act Effect Green (s)	12.0	12.0	41.0	41.0	10.0	57.0
Actuated g/C Ratio	0.15	0.15	0.51	0.51	0.12	0.71
v/c Ratio	0.55	0.17	0.83	0.03	0.54	0.95
Control Delay	38.8	10.3	19.9	0.5	46.4	28.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.8	10.3	19.9	0.5	46.4	28.9
LOS	D	B	B	A	D	C
Approach Delay	32.0		19.2			30.5
Approach LOS	C		B			C

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 22 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.95

Intersection Signal Delay: 26.8

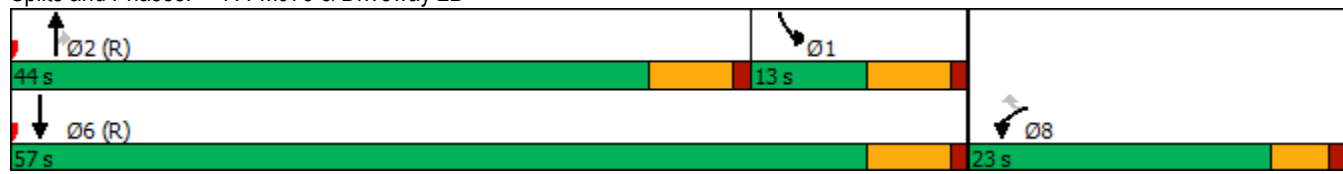
Intersection LOS: C

Intersection Capacity Utilization 75.4%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 7: FM973 & Driveway 2B



Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Phasings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag			Lead	Lead	Lag	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
90th %ile Green (s)	16.4	16.4	39.6	39.6	7.0	52.6
90th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
70th %ile Green (s)	13.7	13.7	42.3	42.3	7.0	55.3
70th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
50th %ile Green (s)	11.9	11.9	44.1	44.1	7.0	57.1
50th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
30th %ile Green (s)	10.1	10.1	40.8	40.8	12.1	58.9
30th %ile Term Code	Gap	Gap	Coord	Coord	Hold	Coord
10th %ile Green (s)	8.0	8.0	38.2	38.2	16.8	61.0
10th %ile Term Code	Min	Min	Coord	Coord	Hold	Coord
Intersection Summary						
Cycle Length: 80						
Actuated Cycle Length: 80						
Offset: 22 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Control Type: Actuated-Coordinated						

Queues
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	150	47	751	27	121	1210
v/c Ratio	0.55	0.17	0.83	0.03	0.54	0.95
Control Delay	38.8	10.3	19.9	0.5	46.4	28.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.8	10.3	19.9	0.5	46.4	28.9
Queue Length 50th (ft)	71	0	283	0	60	438
Queue Length 95th (ft)	120	27	#536	m0	#147	#893
Internal Link Dist (ft)	143		592			465
Turn Bay Length (ft)		100		250	150	
Base Capacity (vph)	406	399	901	840	225	1276
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.12	0.83	0.03	0.54	0.95
Intersection Summary						
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	138	43	691	25	111	1113
Future Volume (vph)	138	43	691	25	111	1113
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	150	47	751	27	121	1210
RTOR Reduction (vph)	0	40	0	13	0	0
Lane Group Flow (vph)	150	7	751	14	121	1210
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Actuated Green, G (s)	12.0	12.0	41.0	41.0	10.0	57.0
Effective Green, g (s)	12.0	12.0	41.0	41.0	10.0	57.0
Actuated g/C Ratio	0.15	0.15	0.51	0.51	0.12	0.71
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	270	242	901	827	225	1276
v/s Ratio Prot	c0.08		0.43		0.07	c0.68
v/s Ratio Perm		0.00		0.01		
v/c Ratio	0.56	0.03	0.83	0.02	0.54	0.95
Uniform Delay, d1	31.5	29.0	16.6	9.6	32.8	10.2
Progression Factor	1.00	1.00	0.61	0.11	1.00	1.00
Incremental Delay, d2	2.5	0.0	8.2	0.0	2.5	15.5
Delay (s)	34.0	29.1	18.3	1.0	35.3	25.7
Level of Service	C	C	B	A	D	C
Approach Delay (s)	32.8		17.7			26.6
Approach LOS	C		B			C
Intersection Summary						
HCM 2000 Control Delay			24.1		HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio			0.96			
Actuated Cycle Length (s)			80.0		Sum of lost time (s)	17.0
Intersection Capacity Utilization			75.4%		ICU Level of Service	D
Analysis Period (min)			15			
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis 7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	138	43	691	25	111	1113			
Future Volume (veh/h)	138	43	691	25	111	1113			
Number	3	18	2	12	1	6			
Initial Q, veh	0	0	0	0	0	0			
Ped-Bike Adj (A_pbT)	1.00	1.00		1.00	1.00				
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No		No			No			
Lanes Open During Work Zone									
Adj Sat Flow, veh/h/ln	1900	1900	1781	1900	1900	1811			
Adj Flow Rate, veh/h	150	47	751	27	121	1210			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Percent Heavy Veh, %	0	0	8	0	0	6			
Opposing Right Turn Influence	Yes				Yes				
Cap, veh/h	199	177	812	734	401	1363			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Prop Arrive On Green	0.11	0.11	0.46	0.46	0.22	0.75			
Unsig. Movement Delay									
Ln Grp Delay, s/veh	40.2	33.4	38.4	12.1	26.4	16.3			
Ln Grp LOS	D	C	D	B	C	B			
Approach Vol, veh/h	197		778			1331			
Approach Delay, s/veh	38.6		37.5			17.2			
Approach LOS	D		D			B			
Timer:		1	2	3	4	5	6	7	8
Assigned Phs		2	1	8			6		
Case No		7.0	2.0	9.0			4.0		
Phs Duration (G+Y+Rc), s		42.5	23.7	13.8			66.2		
Change Period (Y+Rc), s		6.0	6.0	5.0			6.0		
Max Green (Gmax), s		38.0	7.0	18.0			51.0		
Max Allow Headway (MAH), s		6.9	4.1	4.4			6.9		
Max Q Clear (g_c+l1), s		33.7	6.5	8.4			41.9		
Green Ext Time (g_e), s		2.7	0.0	0.5			7.9		
Prob of Phs Call (p_c)		1.00	0.93	0.99			1.00		
Prob of Max Out (p_x)		1.00	1.00	0.04			0.96		
Left-Turn Movement Data									
Assigned Mvmt		5	1	3					
Mvmt Sat Flow, veh/h		0	1810	1810					
Through Movement Data									
Assigned Mvmt		2		8			6		
Mvmt Sat Flow, veh/h		1781		0			1811		
Right-Turn Movement Data									
Assigned Mvmt		12		18			16		
Mvmt Sat Flow, veh/h		1610		1610			0		
Left Lane Group Data									
Assigned Mvmt		5	1	3	0	0	0	0	0
Lane Assignment		L (Prot)		L					

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

Lanes in Grp	0	1	1	0	0	0	0	0
Grp Vol (v), veh/h	0	121	150	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1810	1810	0	0	0	0	0
Q Serve Time (g_s), s	0.0	4.5	6.4	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	4.5	6.4	0.0	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	1810	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	36.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	401	199	0	0	0	0	0
V/C Ratio (X)	0.00	0.30	0.75	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	401	407	0	0	0	0	0
Upstream Filter (I)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	26.0	34.5	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.4	5.7	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	26.4	40.2	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.7	2.8	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	1.8	3.1	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.30	0.49	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Middle Lane Group Data								
Assigned Mvmt	2	0	8	0	0	6	0	0
Lane Assignment	T					T		
Lanes in Grp	1	0	0	0	0	1	0	0
Grp Vol (v), veh/h	751	0	0	0	0	1210	0	0
Grp Sat Flow (s), veh/h/ln	1781	0	0	0	0	1811	0	0
Q Serve Time (g_s), s	31.7	0.0	0.0	0.0	0.0	39.9	0.0	0.0
Cycle Q Clear Time (g_c), s	31.7	0.0	0.0	0.0	0.0	39.9	0.0	0.0
Lane Grp Cap (c), veh/h	812	0	0	0	0	1363	0	0
V/C Ratio (X)	0.93	0.00	0.00	0.00	0.00	0.89	0.00	0.00
Avail Cap (c_a), veh/h	846	0	0	0	0	1363	0	0
Upstream Filter (I)	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	20.5	0.0	0.0	0.0	0.0	7.4	0.0	0.0
Incr Delay (d2), s/veh	17.9	0.0	0.0	0.0	0.0	8.9	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	38.4	0.0	0.0	0.0	0.0	16.3	0.0	0.0
1st-Term Q (Q1), veh/ln	10.5	0.0	0.0	0.0	0.0	5.5	0.0	0.0
2nd-Term Q (Q2), veh/ln	4.0	0.0	0.0	0.0	0.0	3.4	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	14.5	0.0	0.0	0.0	0.0	8.9	0.0	0.0
%ile Storage Ratio (RQ%)	0.63	0.00	0.00	0.00	0.00	0.49	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Right Lane Group Data

Assigned Mvmt	12	0	18	0	0	16	0	0
Lane Assignment	R		R					
Lanes in Grp	1	0	1	0	0	0	0	0
Grp Vol (v), veh/h	27	0	47	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1610	0	1610	0	0	0	0	0
Q Serve Time (g_s), s	0.7	0.0	2.1	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.7	0.0	2.1	0.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	734	0	177	0	0	0	0	0
V/C Ratio (X)	0.04	0.00	0.27	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	765	0	362	0	0	0	0	0
Upstream Filter (I)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	12.1	0.0	32.6	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.1	0.0	0.8	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	12.1	0.0	33.4	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.2	0.0	0.8	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.2	0.0	0.9	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.02	0.00	0.21	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	25.9
HCM 6th LOS	C

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	90	643	12	207	1053
Future Volume (vph)	6	90	643	12	207	1053
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.998			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1654	0	1758	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1654	0	1758	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	106	756	14	244	1239
Shared Lane Traffic (%)						
Lane Group Flow (vph)	113	0	770	0	244	1239
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	68.0%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 8: FM973 & Taylor HS

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	90	643	12	207	1053
Future Volume (Veh/h)	6	90	643	12	207	1053
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	106	756	14	244	1239
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)			1184		672	
pX, platoon unblocked	0.43	0.78			0.78	
vC, conflicting volume	2490	763			770	
vC1, stage 1 conf vol	763					
vC2, stage 2 conf vol	1727					
vCu, unblocked vol	2622	550			559	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	65	75			69	
cM capacity (veh/h)	20	418			793	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	113	770	244	1239		
Volume Left	7	0	244	0		
Volume Right	106	14	0	0		
cSH	186	1700	793	1700		
Volume to Capacity	0.61	0.45	0.31	0.73		
Queue Length 95th (ft)	85	0	33	0		
Control Delay (s)	50.5	0.0	11.5	0.0		
Lane LOS	F		B			
Approach Delay (s)	50.5	0.0	1.9			
Approach LOS	F					
Intersection Summary						
Average Delay			3.6			
Intersection Capacity Utilization			68.0%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	90	643	12	207	1053
Future Vol, veh/h	6	90	643	12	207	1053
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	106	756	14	244	1239
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2490	763	0	0	770	0
Stage 1	763	-	-	-	-	-
Stage 2	1727	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	33	408	-	-	854	-
Stage 1	464	-	-	-	-	-
Stage 2	159	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	24	408	-	-	854	-
Mov Cap-2 Maneuver	90	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	114	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	21.2	0		1.8		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	-	334	854	-	
HCM Lane V/C Ratio	-	-	0.338	0.285	-	
HCM Control Delay (s)	-	-	21.2	10.9	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.5	1.2	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	93	33	12	561	1018	39
Future Volume (vph)	93	33	12	561	1018	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964					0.850
Flt Protected	0.965		0.950			
Satd. Flow (prot)	1767	0	1805	1759	1810	1615
Flt Permitted	0.965		0.950			
Satd. Flow (perm)	1767	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	103	37	13	623	1131	43
Shared Lane Traffic (%)						
Lane Group Flow (vph)	140	0	13	623	1131	43
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.4%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	93	33	12	561	1018	39
Future Volume (Veh/h)	93	33	12	561	1018	39
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	103	37	13	623	1131	43
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage (veh)				2	2	
Upstream signal (ft)				724	1132	
pX, platoon unblocked	0.50	0.39	0.39			
vC, conflicting volume	1780	1131	1174			
vC1, stage 1 conf vol	1131					
vC2, stage 2 conf vol	649					
vCu, unblocked vol	1007	543	655			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	52	82	96			
cM capacity (veh/h)	216	210	363			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	140	13	623	1131	43	
Volume Left	103	13	0	0	0	
Volume Right	37	0	0	0	43	
cSH	214	363	1700	1700	1700	
Volume to Capacity	0.65	0.04	0.37	0.67	0.03	
Queue Length 95th (ft)	99	3	0	0	0	
Control Delay (s)	48.7	15.3	0.0	0.0	0.0	
Lane LOS	E	C				
Approach Delay (s)	48.7	0.3		0.0		
Approach LOS	E					
Intersection Summary						
Average Delay			3.6			
Intersection Capacity Utilization			67.4%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	93	33	12	561	1018	39
Future Vol, veh/h	93	33	12	561	1018	39
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	103	37	13	623	1131	43
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1780	1131	1174	0	-	0
Stage 1	1131	-	-	-	-	-
Stage 2	649	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 91	250	602	-	-	-
Stage 1	311	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 89	250	602	-	-	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	304	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	45.4	0.2		0		
HCM LOS	E					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	602	-	222	-	-	
HCM Lane V/C Ratio	0.022	-	0.631	-	-	
HCM Control Delay (s)	11.1	-	45.4	-	-	
HCM Lane LOS	B	-	E	-	-	
HCM 95th %tile Q(veh)	0.1	-	3.8	-	-	
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Future Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.959			0.877			0.986			0.999	
Flt Protected		0.966			0.996		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1660	0	1805	1676	0	1805	1711	0
Flt Permitted		0.438			0.969		0.950			0.950		
Satd. Flow (perm)	0	798	0	0	1615	0	1805	1676	0	1805	1711	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		205			205			7			1	
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	14	0	6	19	0	201	2	435	44	400	790	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	220	0	2	479	0	400	797	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0		8.0	8.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	24.0	24.0		23.0	23.0		13.0	24.0		13.0	24.0	
Total Split (s)	24.0	24.0		24.0	24.0		13.0	32.0		24.0	43.0	
Total Split (%)	30.0%	30.0%		30.0%	30.0%		16.3%	40.0%		30.0%	53.8%	
Maximum Green (s)	19.0	19.0		19.0	19.0		7.0	26.0		18.0	37.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)		9.3			9.3		7.0	29.3		24.4	57.1	
Actuated g/C Ratio		0.12			0.12		0.09	0.37		0.30	0.71	
v/c Ratio		0.07			0.60		0.01	0.77		0.73	0.65	
Control Delay		0.5			13.4		33.5	33.8		28.4	8.4	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		0.5			13.4		33.5	33.8		28.4	8.4	
LOS		A			B		C	C		C	A	
Approach Delay		0.5			13.4			33.8			15.1	
Approach LOS		A			B			C			B	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 19.4

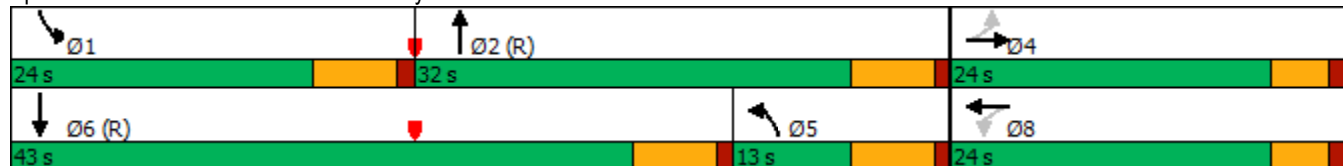
Intersection LOS: B

Intersection Capacity Utilization 68.6%

ICU Level of Service C

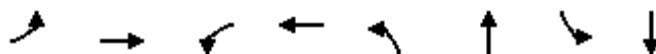
Analysis Period (min) 15

Splits and Phases: 10: FM973 & Wesley Miller



Phasings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Protected Phases		4		8	5	2	1	6
Permitted Phases	4		8					
Minimum Initial (s)	8.0	8.0	8.0	8.0	7.0	10.0	7.0	10.0
Minimum Split (s)	24.0	24.0	23.0	23.0	13.0	24.0	13.0	24.0
Total Split (s)	24.0	24.0	24.0	24.0	13.0	32.0	24.0	43.0
Total Split (%)	30.0%	30.0%	30.0%	30.0%	16.3%	40.0%	30.0%	53.8%
Maximum Green (s)	19.0	19.0	19.0	19.0	7.0	26.0	18.0	37.0
Yellow Time (s)	3.5	3.5	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag					Lag	Lag	Lead	Lead
Lead-Lag Optimize?					Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	None	None	None	C-Min	None	C-Min
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	13.3	13.3	13.3	13.3	7.0	26.0	23.7	42.7
90th %ile Term Code	Hold	Hold	Gap	Gap	Max	Coord	Max	Coord
70th %ile Green (s)	9.1	9.1	9.1	9.1	0.0	26.9	27.0	59.9
70th %ile Term Code	Hold	Hold	Gap	Gap	Skip	Coord	Gap	Coord
50th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	29.7	25.3	61.0
50th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
30th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	30.8	24.2	61.0
30th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
10th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	33.3	21.7	61.0
10th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
Intersection Summary								
Cycle Length: 80								
Actuated Cycle Length: 80								
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green								
Control Type: Actuated-Coordinated								

Queues
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	20	220	2	479	400	797
v/c Ratio	0.07	0.60	0.01	0.77	0.73	0.65
Control Delay	0.5	13.4	33.5	33.8	28.4	8.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.5	13.4	33.5	33.8	28.4	8.4
Queue Length 50th (ft)	0	7	1	203	156	88
Queue Length 95th (ft)	0	61	8	#379	m191	m312
Internal Link Dist (ft)	494	480		650		644
Turn Bay Length (ft)			80		400	
Base Capacity (vph)	345	539	157	619	550	1221
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.41	0.01	0.77	0.73	0.65

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Future Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.96			0.88		1.00	0.99		1.00	1.00	
Flt Protected		0.97			1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1761			1658		1805	1676		1805	1711	
Flt Permitted		0.44			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (perm)		798			1613		1805	1676		1805	1711	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	14	0	6	19	0	201	2	435	44	400	790	7
RTOR Reduction (vph)	0	18	0	0	181	0	0	4	0	0	0	0
Lane Group Flow (vph)	0	2	0	0	39	0	2	475	0	400	797	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Actuated Green, G (s)		9.3			9.3		1.4	29.3		24.4	52.3	
Effective Green, g (s)		9.3			9.3		1.4	29.3		24.4	52.3	
Actuated g/C Ratio		0.12			0.12		0.02	0.37		0.30	0.65	
Clearance Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		92			187		31	613		550	1118	
v/s Ratio Prot							0.00	c0.28		0.22	c0.47	
v/s Ratio Perm		0.00			c0.02							
v/c Ratio		0.03			0.21		0.06	0.77		0.73	0.71	
Uniform Delay, d1		31.3			32.0		38.7	22.4		24.8	9.0	
Progression Factor		1.00			1.00		1.00	1.00		0.90	0.64	
Incremental Delay, d2		0.1			0.6		0.9	9.2		3.0	2.4	
Delay (s)		31.4			32.6		39.5	31.7		25.3	8.2	
Level of Service		C			C		D	C		C	A	
Approach Delay (s)		31.4			32.6			31.7			13.9	
Approach LOS		C			C			C			B	
Intersection Summary												
HCM 2000 Control Delay		20.7			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.71										
Actuated Cycle Length (s)		80.0			Sum of lost time (s)			17.0				
Intersection Capacity Utilization		68.6%			ICU Level of Service			C				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3340	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				319						304		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	871	319	211	484	0	0	495	418	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 2

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.4	42.8	42.8	20.4	66.7			44.0	44.0		
Actuated g/C Ratio		0.08	0.34	0.34	0.16	0.53			0.35	0.35		
v/c Ratio		0.06	0.74	0.45	0.76	0.28			0.84	0.56		
Control Delay		64.5	42.4	5.5	71.3	17.8			53.7	13.1		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		64.5	42.4	5.5	71.3	17.8			53.7	13.1		
LOS		E	D	A	E	B			D	B		
Approach Delay			32.7			34.1			35.1			
Approach LOS			C			C			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 126.9

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 33.8

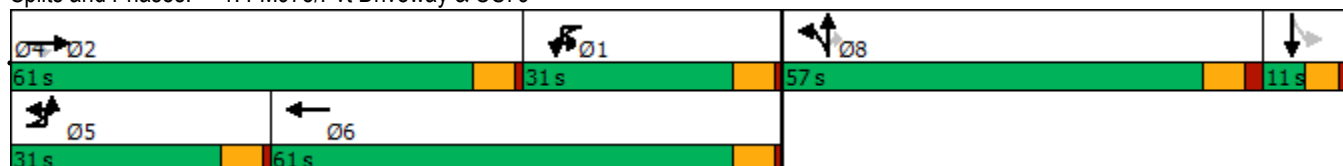
Intersection LOS: C

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
70th %ile Green (s)	0.0	50.5	50.5	25.0	81.5	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	44.6	44.6	22.1	72.7	50.0	50.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	38.0	38.0	18.2	62.2	41.0	41.0	0.0
30th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	27.8	27.8	12.7	46.5	29.6	29.6	0.0
10th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 126.9								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 149								
70th %ile Actuated Cycle: 144.5								
50th %ile Actuated Cycle: 135.7								
30th %ile Actuated Cycle: 116.2								
10th %ile Actuated Cycle: 89.1								

Queues
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	9	871	319	211	484	495	418
v/c Ratio	0.06	0.74	0.45	0.76	0.28	0.84	0.56
Control Delay	64.5	42.4	5.5	71.3	17.8	53.7	13.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.5	42.4	5.5	71.3	17.8	53.7	13.1
Queue Length 50th (ft)	8	361	0	179	114	387	69
Queue Length 95th (ft)	28	451	65	#304	186	#649	195
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	368	1558	829	350	1818	695	826
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.56	0.38	0.60	0.27	0.71	0.51

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3342			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3342			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
RTOR Reduction (vph)	0	0	0	215	0	0	0	0	0	202	0	0
Lane Group Flow (vph)	0	9	871	104	211	484	0	0	495	216	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.7	42.8	42.8	25.6	66.7			44.0	44.0		
Effective Green, g (s)		1.7	42.8	42.8	25.6	66.7			44.0	44.0		
Actuated g/C Ratio		0.01	0.33	0.33	0.19	0.51			0.33	0.33		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		23	1130	473	334	1696			570	530		
v/s Ratio Prot		0.00	c0.25		c0.12	0.14			c0.29			
v/s Ratio Perm				0.07						0.14		
v/c Ratio		0.39	0.77	0.22	0.63	0.29			0.87	0.41		
Uniform Delay, d1		64.3	39.9	32.2	48.6	18.6			41.0	33.7		
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		
Incremental Delay, d2		4.0	3.0	0.1	2.9	0.0			13.2	0.5		
Delay (s)		68.3	42.9	32.3	50.4	18.1			54.2	34.2		
Level of Service		E	D	C	D	B			D	C		
Approach Delay (s)			40.3			27.9			45.0			0.0
Approach LOS			D			C			D			A
Intersection Summary												
HCM 2000 Control Delay			38.8									HCM 2000 Level of Service D
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			131.4						25.0			Sum of lost time (s)
Intersection Capacity Utilization			73.3%									ICU Level of Service D
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	972	246	0	697	0	219
Future Volume (vph)	972	246	0	697	0	219
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3471	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3471	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1070			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	0%	7%	0%	0%
Adj. Flow (vph)	1057	267	0	758	0	238
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1057	267	0	758	0	238
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 47.1%						
ICU Level of Service A						
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	972	246	0	697	0	219
Future Volume (Veh/h)	972	246	0	697	0	219
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1057	267	0	758	0	238
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1070					
pX, platoon unblocked			0.80		0.80	0.80
vC, conflicting volume			1324		1436	528
vC1, stage 1 conf vol					1057	
vC2, stage 2 conf vol					379	
vCu, unblocked vol			892		1033	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	73
cM capacity (veh/h)			611		386	867
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	528	528	267	379	379	238
Volume Left	0	0	0	0	0	0
Volume Right	0	0	267	0	0	238
cSH	1700	1700	1700	1700	1700	867
Volume to Capacity	0.31	0.31	0.16	0.22	0.22	0.27
Queue Length 95th (ft)	0	0	0	0	0	28
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	10.7
Lane LOS						B
Approach Delay (s)	0.0			0.0	10.7	
Approach LOS						B
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			47.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	972	246	0	697	0	219
Future Vol, veh/h	972	246	0	697	0	219
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	0	7	0	0
Mvmt Flow	1057	267	0	758	0	238
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	529
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	499
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	499
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		18.6	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	499	-	-	-		
HCM Lane V/C Ratio	0.477	-	-	-		
HCM Control Delay (s)	18.6	-	-	-		
HCM Lane LOS	C	-	-	-		
HCM 95th %tile Q(veh)	2.5	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	47	0	1149	0	658	0	0	0
Future Volume (vph)	47	0	1149	0	658	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3471	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3471	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	51	0	1235	0	708	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	51	1235	0	708	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utilization 41.8%								
ICU Level of Service A								
Analysis Period (min) 15								

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	47	0	1149	0	658	0	0	0
Future Volume (Veh/h)	47	0	1149	0	658	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	1235	0	708	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	708		0			1326	354
vC1, stage 1 conf vol							708	
vC2, stage 2 conf vol							618	
vCu, unblocked vol	0	708		0			1326	354
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	900		0			360	648
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	618	618	354	354	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.36	0.36	0.21	0.21	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.8%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.3							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	47	0	1149	0	658	0	0	0
Future Vol, veh/h	47	0	1149	0	658	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	0	7	0	0	0
Mvmt Flow	51	0	1235	0	708	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	708	708	0	1235	-	0	1428	354
Stage 1	-	-	-	-	-	-	708	-
Stage 2	-	-	-	-	-	-	720	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	518	900	-	239	-	0	128	648
Stage 1	-	-	-	-	-	0	455	-
Stage 2	-	-	-	-	-	0	448	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	518	518	-	239	-	-	115	648
Mov Cap-2 Maneuver	-	-	-	-	-	-	115	-
Stage 1	-	-	-	-	-	-	410	-
Stage 2	-	-	-	-	-	-	448	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.5		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	518	-	239	-	-			
HCM Lane V/C Ratio	0.098	-	-	-	-			
HCM Control Delay (s)	12.7	-	0	-	0			
HCM Lane LOS	B	-	A	-	A			
HCM 95th %tile Q(veh)	0.3	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	5	1151	29	5	612	0	20	3	3	1	7	2
Future Volume (vph)	5	1151	29	5	612	0	20	3	3	1	7	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.986			0.975	
Flt Protected	0.950			0.950				0.962			0.995	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1843	0
Flt Permitted	0.950			0.950				0.962			0.995	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1843	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	6	1308	33	6	695	0	23	3	3	1	8	2
Shared Lane Traffic (%)												
Lane Group Flow (vph)	6	1308	33	6	695	0	0	29	0	0	11	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.4%						ICU Level of Service A					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 5: S Rio Grande & US79

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	1151	29	5	612	0	20	3	3	1	7	2
Future Volume (Veh/h)	5	1151	29	5	612	0	20	3	3	1	7	2
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	1308	33	6	695	0	23	3	3	1	8	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	695			1308			1684	2027	654	1374	2027	348
vC1, stage 1 conf vol							1320	1320		707	707	
vC2, stage 2 conf vol							364	707		668	1320	
vCu, unblocked vol	695			1308			1684	2027	654	1374	2027	348
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			86	99	99	100	96	100
cM capacity (veh/h)	910			536			160	201	414	292	197	654
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	6	654	654	33	6	348	348	0	29	11		
Volume Left	6	0	0	0	6	0	0	0	23	1		
Volume Right	0	0	0	33	0	0	0	0	3	2		
cSH	910	1700	1700	1700	536	1700	1700	1700	175	234		
Volume to Capacity	0.01	0.38	0.38	0.02	0.01	0.20	0.20	0.00	0.17	0.05		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	14	4		
Control Delay (s)	9.0	0.0	0.0	0.0	11.8	0.0	0.0	0.0	29.6	21.2		
Lane LOS	A				B				D	C		
Approach Delay (s)	0.0				0.1				29.6	21.2		
Approach LOS									D	C		
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	45.4%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	5	1151	29	5	612	0	20	3	3	1	7	2
Future Vol, veh/h	5	1151	29	5	612	0	20	3	3	1	7	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	6	1308	33	6	695	0	23	3	3	1	8	2

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	695	0	0	1308	0	0	1684	2027	-	1375	2027	-
Stage 1	-	-	-	-	-	-	1320	1320	-	707	707	-
Stage 2	-	-	-	-	-	-	364	707	-	668	1320	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	910	-	-	536	-	-	63	58	0	106	58	0
Stage 1	-	-	-	-	-	-	168	228	0	397	441	0
Stage 2	-	-	-	-	-	-	633	441	0	419	228	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	910	-	-	536	-	-	56	57	-	100	57	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	56	57	-	100	57	-
Stage 1	-	-	-	-	-	-	167	226	-	394	436	-
Stage 2	-	-	-	-	-	-	614	436	-	410	226	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	910	-	-	536	-	-	-
HCM Lane V/C Ratio	-	0.006	-	-	0.011	-	-	-
HCM Control Delay (s)	-	9	-	-	11.8	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	65	766	109	0	500
Future Volume (vph)	0	65	766	109	0	500
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1827	1615	0	1712
Flt Permitted						
Satd. Flow (perm)	0	1644	1827	1615	0	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	0	71	833	118	0	543
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	71	833	118	0	543
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	51.0%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

6: FM973 & Driveway 1B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	65	766	109	0	500
Future Volume (Veh/h)	0	65	766	109	0	500
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	71	833	118	0	543
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)			545		636	
pX, platoon unblocked	0.57	0.57			0.57	
vC, conflicting volume	1376	833			951	
vC1, stage 1 conf vol	833					
vC2, stage 2 conf vol	543					
vCu, unblocked vol	1283	334			540	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	83			100	
cM capacity (veh/h)	346	408			594	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	71	833	118	543		
Volume Left	0	0	0	0		
Volume Right	71	0	118	0		
cSH	408	1700	1700	1700		
Volume to Capacity	0.17	0.49	0.07	0.32		
Queue Length 95th (ft)	16	0	0	0		
Control Delay (s)	15.7	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	15.7	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			51.0%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	65	766	109	0	500
Future Vol, veh/h	0	65	766	109	0	500
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	0	71	833	118	0	543
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	833	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	372	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	372	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	16.9	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	372		-		
HCM Lane V/C Ratio	-	0.19		-		
HCM Control Delay (s)	-	16.9		-		
HCM Lane LOS	-	C		-		
HCM 95th %tile Q(veh)	-	0.7		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	98	35	840	17	72	428
Future Volume (vph)	98	35	840	17	72	428
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		38		18		
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	107	38	913	18	78	465
Shared Lane Traffic (%)						
Lane Group Flow (vph)	107	38	913	18	78	465
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	3	1	1	3
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	50	50	250	50	50	250
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	50	50	6	50	50	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			122			122
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Detector 3 Position(ft)			244			244
Detector 3 Size(ft)			6			6

Scenario 1 5:57 pm 08/19/2022 Baseline

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Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex			Cl+Ex		
Detector 3 Channel						
Detector 3 Extend (s)	0.0			0.0		
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Act Effect Green (s)	10.4	10.4	50.1	50.1	8.9	62.4
Actuated g/C Ratio	0.13	0.13	0.63	0.63	0.11	0.78
v/c Ratio	0.46	0.16	0.80	0.02	0.39	0.35
Control Delay	38.0	11.8	23.0	3.0	38.4	4.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	11.8	23.0	3.0	38.4	4.8
LOS	D	B	C	A	D	A
Approach Delay	31.1		22.6			9.6
Approach LOS	C		C			A

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 21 (26%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 19.0

Intersection LOS: B

Intersection Capacity Utilization 70.9%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: FM973 & Driveway 2B



Scenario 1 5:57 pm 08/19/2022 Baseline

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Phasings
7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
90th %ile Green (s)	13.9	13.9	38.0	38.0	11.1	55.1
90th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
70th %ile Green (s)	11.6	11.6	41.3	41.3	10.1	57.4
70th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
50th %ile Green (s)	10.1	10.1	44.1	44.1	8.8	58.9
50th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
30th %ile Green (s)	8.5	8.5	47.0	47.0	7.5	60.5
30th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
10th %ile Green (s)	0.0	0.0	74.0	74.0	0.0	74.0
10th %ile Term Code	Skip	Skip	Coord	Coord	Skip	Coord
Intersection Summary						
Cycle Length: 80						
Actuated Cycle Length: 80						
Offset: 21 (26%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Control Type: Actuated-Coordinated						

Queues
7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	107	38	913	18	78	465
v/c Ratio	0.46	0.16	0.80	0.02	0.39	0.35
Control Delay	38.0	11.8	23.0	3.0	38.4	4.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	11.8	23.0	3.0	38.4	4.8
Queue Length 50th (ft)	51	0	380	0	37	67
Queue Length 95th (ft)	93	25	#716	m1	76	132
Internal Link Dist (ft)	143		592			465
Turn Bay Length (ft)		100		250	150	
Base Capacity (vph)	406	392	1143	1017	200	1335
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.10	0.80	0.02	0.39	0.35
Intersection Summary						
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	98	35	840	17	72	428
Future Volume (vph)	98	35	840	17	72	428
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	107	38	913	18	78	465
RTOR Reduction (vph)	0	34	0	7	0	0
Lane Group Flow (vph)	107	4	913	11	78	465
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Actuated Green, G (s)	8.8	8.8	46.7	46.7	7.5	60.2
Effective Green, g (s)	8.8	8.8	46.7	46.7	7.5	60.2
Actuated g/C Ratio	0.11	0.11	0.58	0.58	0.09	0.75
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	198	177	1066	942	169	1288
v/s Ratio Prot	c0.06		c0.50		0.04	c0.27
v/s Ratio Perm		0.00		0.01		
v/c Ratio	0.54	0.02	0.86	0.01	0.46	0.36
Uniform Delay, d1	33.7	31.8	13.9	7.0	34.3	3.4
Progression Factor	1.00	1.00	0.95	0.59	1.00	1.00
Incremental Delay, d2	3.0	0.1	8.1	0.0	2.0	0.8
Delay (s)	36.7	31.8	21.3	4.1	36.3	4.2
Level of Service	D	C	C	A	D	A
Approach Delay (s)	35.4		20.9			8.8
Approach LOS	D		C			A
Intersection Summary						
HCM 2000 Control Delay			18.2		HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio			0.77			
Actuated Cycle Length (s)			80.0		Sum of lost time (s)	17.0
Intersection Capacity Utilization			70.9%		ICU Level of Service	C
Analysis Period (min)			15			
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	98	35	840	17	72	428			
Future Volume (veh/h)	98	35	840	17	72	428			
Number	3	18	2	12	1	6			
Initial Q, veh	0	0	0	0	0	0			
Ped-Bike Adj (A_pbT)	1.00	1.00		1.00	1.00				
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No		No			No			
Lanes Open During Work Zone									
Adj Sat Flow, veh/h/ln	1900	1900	1841	1900	1900	1737			
Adj Flow Rate, veh/h	107	38	913	18	78	465			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Percent Heavy Veh, %	0	0	4	0	0	11			
Opposing Right Turn Influence	Yes				Yes				
Cap, veh/h	174	155	1140	997	130	1331			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Prop Arrive On Green	0.10	0.10	0.62	0.62	0.07	0.77			
Unsig. Movement Delay									
Ln Grp Delay, s/veh	38.3	34.3	17.4	5.9	40.3	3.7			
Ln Grp LOS	D	C	B	A	D	A			
Approach Vol, veh/h	145		931			543			
Approach Delay, s/veh	37.2		17.2			9.0			
Approach LOS	D		B			A			
Timer:		1	2	3	4	5	6	7	8
Assigned Phs		1	2	8			6		
Case No		2.0	7.0	9.0			4.0		
Phs Duration (G+Y+Rc), s		11.8	55.6	12.7			67.3		
Change Period (Y+Rc), s		6.0	6.0	5.0			6.0		
Max Green (Gmax), s		7.0	38.0	18.0			51.0		
Max Allow Headway (MAH), s		4.1	6.9	4.4			7.0		
Max Q Clear (g_c+l1), s		5.3	32.0	6.5			8.8		
Green Ext Time (g_e), s		0.0	4.4	0.4			6.3		
Prob of Phs Call (p_c)		0.82	1.00	0.96			1.00		
Prob of Max Out (p_x)		1.00	1.00	0.01			0.00		
Left-Turn Movement Data									
Assigned Mvmt		1	5	3					
Mvmt Sat Flow, veh/h		1810	0	1810					
Through Movement Data									
Assigned Mvmt			2	8			6		
Mvmt Sat Flow, veh/h			1841	0			1737		
Right-Turn Movement Data									
Assigned Mvmt			12	18			16		
Mvmt Sat Flow, veh/h			1610	1610			0		
Left Lane Group Data									
Assigned Mvmt		1	5	3	0	0	0	0	0
Lane Assignment		L (Prot)		L					

Scenario 1 5:57 pm 08/19/2022 Baseline

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HCM 6th Signalized Intersection Capacity Analysis 7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

Lanes in Grp	1	0	1	0	0	0	0	0
Grp Vol (v), veh/h	78	0	107	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1810	0	1810	0	0	0	0	0
Q Serve Time (g_s), s	3.3	0.0	4.5	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	3.3	0.0	4.5	0.0	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	1810	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	49.6	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	130	0	174	0	0	0	0	0
V/C Ratio (X)	0.60	0.00	0.62	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	158	0	407	0	0	0	0	0
Upstream Filter (I)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	36.0	0.0	34.7	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	4.3	0.0	3.5	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	40.3	0.0	38.3	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	1.4	0.0	2.0	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.2	0.0	0.2	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	1.5	0.0	2.1	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.25	0.00	0.34	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Middle Lane Group Data								
Assigned Mvmt	0	2	8	0	0	6	0	0
Lane Assignment	T			T				
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	913	0	0	0	465	0	0
Grp Sat Flow (s), veh/h/ln	0	1841	0	0	0	1737	0	0
Q Serve Time (g_s), s	0.0	30.0	0.0	0.0	0.0	6.8	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	30.0	0.0	0.0	0.0	6.8	0.0	0.0
Lane Grp Cap (c), veh/h	0	1140	0	0	0	1331	0	0
V/C Ratio (X)	0.00	0.80	0.00	0.00	0.00	0.35	0.00	0.00
Avail Cap (c_a), veh/h	0	1140	0	0	0	1331	0	0
Upstream Filter (I)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	11.5	0.0	0.0	0.0	3.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	5.9	0.0	0.0	0.0	0.7	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	17.4	0.0	0.0	0.0	3.7	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	8.1	0.0	0.0	0.0	0.7	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	1.9	0.0	0.0	0.0	0.3	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	10.0	0.0	0.0	0.0	1.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.42	0.00	0.00	0.00	0.06	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Right Lane Group Data

Assigned Mvmt	0	12	18	0	0	16	0	0
Lane Assignment		R	R					
Lanes in Grp	0	1	1	0	0	0	0	0
Grp Vol (v), veh/h	0	18	38	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1610	1610	0	0	0	0	0
Q Serve Time (g_s), s	0.0	0.3	1.7	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.3	1.7	0.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	997	155	0	0	0	0	0
V/C Ratio (X)	0.00	0.02	0.25	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	997	362	0	0	0	0	0
Upstream Filter (I)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	5.9	33.5	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.8	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	5.9	34.3	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.1	0.7	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.1	0.7	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.01	0.18	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	16.2
HCM 6th LOS	B

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	26	836	4	12	534
Future Volume (vph)	8	26	836	4	12	534
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.896		0.999			
Flt Protected	0.989				0.950	
Satd. Flow (prot)	1684	0	1825	0	1805	1712
Flt Permitted	0.989				0.950	
Satd. Flow (perm)	1684	0	1825	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	27	862	4	12	551
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	866	0	12	551
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	54.2%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 8: FM973 & Taylor HS

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	26	836	4	12	534
Future Volume (Veh/h)	8	26	836	4	12	534
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	27	862	4	12	551
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)			1184		672	
pX, platoon unblocked	0.78	0.74			0.74	
vC, conflicting volume	1439	864			866	
vC1, stage 1 conf vol	864					
vC2, stage 2 conf vol	575					
vCu, unblocked vol	1177	641			644	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	92			98	
cM capacity (veh/h)	343	354			704	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	35	866	12	551		
Volume Left	8	0	12	0		
Volume Right	27	4	0	0		
cSH	352	1700	704	1700		
Volume to Capacity	0.10	0.51	0.02	0.32		
Queue Length 95th (ft)	8	0	1	0		
Control Delay (s)	16.4	0.0	10.2	0.0		
Lane LOS	C		B			
Approach Delay (s)	16.4	0.0	0.2			
Approach LOS	C					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			54.2%	ICU Level of Service	A	
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	26	836	4	12	534
Future Vol, veh/h	8	26	836	4	12	534
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	27	862	4	12	551
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1439	864	0	0	866	0
Stage 1	864	-	-	-	-	-
Stage 2	575	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	148	357	-	-	786	-
Stage 1	416	-	-	-	-	-
Stage 2	567	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	146	357	-	-	786	-
Mov Cap-2 Maneuver	282	-	-	-	-	-
Stage 1	416	-	-	-	-	-
Stage 2	558	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	17	0		0.2		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	336	786	-	
HCM Lane V/C Ratio	-	-	0.104	0.016	-	
HCM Control Delay (s)	-	-	17	9.7	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 PM Peak Hour (w/New Signalized)

Phase 1 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	62	22	38	773	448	97
Future Volume (vph)	62	22	38	773	448	97
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1766	0	1805	1827	1696	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1766	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	67	24	41	840	487	105
Shared Lane Traffic (%)						
Lane Group Flow (vph)	91	0	41	840	487	105
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	52.1%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	62	22	38	773	448	97
Future Volume (Veh/h)	62	22	38	773	448	97
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	67	24	41	840	487	105
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage (veh)				2	2	
Upstream signal (ft)				724	1132	
pX, platoon unblocked	0.76	0.96	0.96			
vC, conflicting volume	1409	487	592			
vC1, stage 1 conf vol	487					
vC2, stage 2 conf vol	922					
vCu, unblocked vol	1265	446	555			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	79	96	96			
cM capacity (veh/h)	314	593	985			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	91	41	840	487	105	
Volume Left	67	41	0	0	0	
Volume Right	24	0	0	0	105	
cSH	358	985	1700	1700	1700	
Volume to Capacity	0.25	0.04	0.49	0.29	0.06	
Queue Length 95th (ft)	25	3	0	0	0	
Control Delay (s)	18.4	8.8	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	18.4	0.4		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utilization			52.1%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	62	22	38	773	448	97
Future Vol, veh/h	62	22	38	773	448	97
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	67	24	41	840	487	105

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1409	487	592	0	-	0
Stage 1	487	-	-	-	-	-
Stage 2	922	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	154	585	994	-	-	-
Stage 1	622	-	-	-	-	-
Stage 2	391	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	148	585	994	-	-	-
Mov Cap-2 Maneuver	279	-	-	-	-	-
Stage 1	596	-	-	-	-	-
Stage 2	391	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	20.5	0.4	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	994	-	323	-	-
HCM Lane V/C Ratio	0.042	-	0.283	-	-
HCM Control Delay (s)	8.8	-	20.5	-	-
HCM Lane LOS	A	-	C	-	-
HCM 95th %tile Q(veh)	0.1	-	1.1	-	-

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Future Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.958			0.899						0.996	
Flt Protected		0.967			0.988		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1688	0	1805	1827	0	1805	1710	0
Flt Permitted					0.910		0.950			0.950		
Satd. Flow (perm)	0	1820	0	0	1554	0	1805	1827	0	1805	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		123			123						2	
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	9	0	4	5	0	15	4	854	0	2	490	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	20	0	4	854	0	2	504	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0		8.0	8.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		13.0	22.5		13.0	22.5	
Total Split (s)	22.5	22.5		22.5	22.5		13.0	44.5		13.0	44.5	
Total Split (%)	28.1%	28.1%		28.1%	28.1%		16.3%	55.6%		16.3%	55.6%	
Maximum Green (s)	17.5	17.5		17.5	17.5		7.0	38.5		7.0	38.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)		8.0			8.0		7.0	69.8		7.0	69.8	
Actuated g/C Ratio		0.10			0.10		0.09	0.87		0.09	0.87	
v/c Ratio		0.04			0.08		0.03	0.54		0.01	0.34	
Control Delay		0.2			0.6		33.8	6.6		32.5	4.3	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		0.2			0.6		33.8	6.6		32.5	4.3	
LOS		A			A		C	A		C	A	
Approach Delay		0.3			0.6			6.7			4.5	
Approach LOS		A			A			A			A	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.54

Intersection Signal Delay: 5.7

Intersection LOS: A

Intersection Capacity Utilization 57.2%

ICU Level of Service B

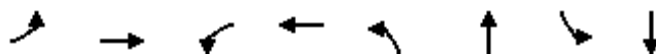
Analysis Period (min) 15

Splits and Phases: 10: FM973 & Wesley Miller



Phasings
10: FM973 & Wesley Miller

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Protected Phases		4		8	5	2	1	6
Permitted Phases	4		8					
Minimum Initial (s)	8.0	8.0	8.0	8.0	7.0	10.0	7.0	10.0
Minimum Split (s)	22.5	22.5	22.5	22.5	13.0	22.5	13.0	22.5
Total Split (s)	22.5	22.5	22.5	22.5	13.0	44.5	13.0	44.5
Total Split (%)	28.1%	28.1%	28.1%	28.1%	16.3%	55.6%	16.3%	55.6%
Maximum Green (s)	17.5	17.5	17.5	17.5	7.0	38.5	7.0	38.5
Yellow Time (s)	3.5	3.5	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag					Lead	Lag	Lead	Lag
Lead-Lag Optimize?					Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	None	None	None	C-Min	None	C-Min
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	8.0	8.0	8.0	8.0	7.0	48.0	7.0	48.0
90th %ile Term Code	Min	Min	Min	Min	Min	Coord	Min	Coord
70th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	61.0	0.0	61.0
70th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Skip	Coord
50th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
50th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
30th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
30th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
10th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
10th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
Intersection Summary								
Cycle Length: 80								
Actuated Cycle Length: 80								
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green								
Control Type: Actuated-Coordinated								

Queues
10: FM973 & Wesley Miller

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

	→	←	↖	↑	↘	↓
Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	13	20	4	854	2	504
v/c Ratio	0.04	0.08	0.03	0.54	0.01	0.34
Control Delay	0.2	0.6	33.8	6.6	32.5	4.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.2	0.6	33.8	6.6	32.5	4.3
Queue Length 50th (ft)	0	0	2	0	1	0
Queue Length 95th (ft)	0	0	11	454	m3	152
Internal Link Dist (ft)	494	480		650		644
Turn Bay Length (ft)			80		400	
Base Capacity (vph)	494	436	157	1594	157	1492
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.05	0.03	0.54	0.01	0.34
Intersection Summary						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2025 PM Peak Hour (w/New Signalized)
Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Future Volume (vph)	8	0	4	5	0	14	4	786	0	2	451	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.96			0.90		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1760			1687		1805	1827		1805	1709	
Flt Permitted		1.00			0.91		0.95	1.00		0.95	1.00	
Satd. Flow (perm)		1821			1554		1805	1827		1805	1709	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	0	4	5	0	15	4	854	0	2	490	14
RTOR Reduction (vph)	0	12	0	0	19	0	0	0	0	0	1	0
Lane Group Flow (vph)	0	1	0	0	1	0	4	854	0	2	503	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Actuated Green, G (s)		3.2			3.2		1.4	58.4		1.4	58.4	
Effective Green, g (s)		3.2			3.2		1.4	58.4		1.4	58.4	
Actuated g/C Ratio		0.04			0.04		0.02	0.73		0.02	0.73	
Clearance Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		72			62		31	1333		31	1247	
v/s Ratio Prot							c0.00	c0.47		0.00	0.29	
v/s Ratio Perm		0.00			c0.00							
v/c Ratio		0.01			0.01		0.13	0.64		0.06	0.40	
Uniform Delay, d1		36.9			36.9		38.7	5.5		38.7	4.1	
Progression Factor		1.00			1.00		1.00	1.00		0.97	1.00	
Incremental Delay, d2		0.0			0.1		1.9	2.4		0.9	0.9	
Delay (s)		36.9			37.0		40.6	7.9		38.3	5.1	
Level of Service		D			D		D	A		D	A	
Approach Delay (s)		36.9			37.0			8.0			5.2	
Approach LOS		D			D			A			A	
Intersection Summary												
HCM 2000 Control Delay		7.7			HCM 2000 Level of Service			A				
HCM 2000 Volume to Capacity ratio		0.60										
Actuated Cycle Length (s)		80.0			Sum of lost time (s)			17.0				
Intersection Capacity Utilization		57.2%			ICU Level of Service			B				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Appendix D. Traffic Simulation Output

- Phase 1 Buildout Year (2025) Projected Conditions with Development, AM Only
 - Mitigation:
 - With Optimized Timings at FM 973 at US 79
 - With TrafficSignal at FM 973 at Driveway 2B (half cycle to US 79 intersection)
 - With TrafficSignal at FM 973 at Wesley Miller Lane (half cycle to US 79 intersection)

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850						0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3375	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				651						164		204
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	7	563	774	500	642	0	0	541	202	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	16.0	16.0	35.0	35.0	44.0	63.0		50.0	50.0	50.0	11.0	11.0
Total Split (%)	11.4%	11.4%	25.0%	25.0%	31.4%	45.0%		35.7%	35.7%	35.7%	7.9%	7.9%
Maximum Green (s)	10.0	10.0	29.0	29.0	38.0	57.0		43.0	43.0	43.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.0	28.1	28.1	38.1	69.2			43.1	43.1		5.0
Actuated g/C Ratio		0.08	0.22	0.22	0.29	0.53			0.33	0.33		0.04
v/c Ratio		0.05	0.79	0.92	1.00	0.36			0.98	0.32		0.00
Control Delay		59.1	57.6	25.3	84.7	19.3			77.2	9.6		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		59.1	57.6	25.3	84.7	19.3			77.2	9.6		0.0
LOS		E	E	C	F	B			E	A		A
Approach Delay			39.0			47.9			58.8			
Approach LOS			D			D			E			

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 130.3

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 46.7

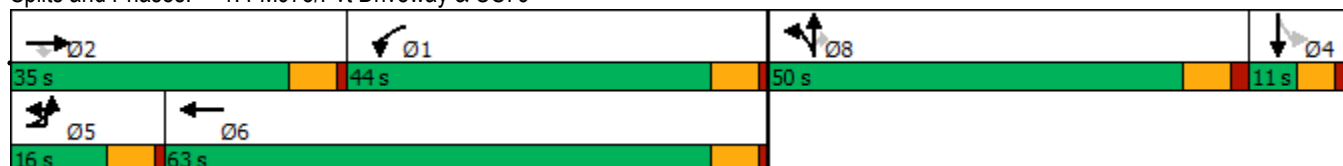
Intersection LOS: D

Intersection Capacity Utilization 91.4%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	16.0	35.0	35.0	44.0	63.0	50.0	50.0	11.0
Total Split (%)	11.4%	25.0%	25.0%	31.4%	45.0%	35.7%	35.7%	7.9%
Maximum Green (s)	10.0	29.0	29.0	38.0	57.0	43.0	43.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	29.0	29.0	38.0	57.0	43.0	43.0	5.0
90th %ile Term Code	Max	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	29.0	29.0	38.0	73.0	43.0	43.0	0.0
70th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	29.0	29.0	38.0	73.0	43.0	43.0	0.0
50th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	29.0	29.0	38.0	73.0	43.0	43.0	0.0
30th %ile Term Code	Skip	Max	Max	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	24.3	24.3	38.0	68.3	43.0	43.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 140								
Actuated Cycle Length: 130.3								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 140								
70th %ile Actuated Cycle: 129								
50th %ile Actuated Cycle: 129								
30th %ile Actuated Cycle: 129								
10th %ile Actuated Cycle: 124.3								

Queues

1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/New Signals, Optimized)

Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	7	563	774	500	642	541	202	1
v/c Ratio	0.05	0.79	0.92	1.00	0.36	0.98	0.32	0.00
Control Delay	59.1	57.6	25.3	84.7	19.3	77.2	9.6	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.1	57.6	25.3	84.7	19.3	77.2	9.6	0.0
Queue Length 50th (ft)	6	233	107	417	140	445	22	0
Queue Length 95th (ft)	23	331	#419	#732	271	#766	89	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	138	738	851	502	1791	552	628	259
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.76	0.91	1.00	0.36	0.98	0.32	0.00

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

1: FM973/Pvt Driveway & US79

Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
RTOR Reduction (vph)	0	0	0	520	0	0	0	0	0	114	0	1
Lane Group Flow (vph)	0	7	563	254	500	642	0	0	541	88	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.9	28.1	28.1	42.9	69.1			43.1	43.1		0.9
Effective Green, g (s)		1.9	28.1	28.1	42.9	69.1			43.1	43.1		0.9
Actuated g/C Ratio		0.01	0.20	0.20	0.31	0.49			0.31	0.31		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		24	664	311	526	1664			514	482		10
v/s Ratio Prot		0.00	c0.17		c0.29	0.19			c0.32			c0.00
v/s Ratio Perm				0.16						0.06		
v/c Ratio		0.29	0.85	0.82	0.95	0.39			1.05	0.18		0.00
Uniform Delay, d1		68.4	53.9	53.5	47.5	22.2			48.4	35.5		69.1
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		1.00
Incremental Delay, d2		2.5	9.5	14.4	27.0	0.1			54.2	0.2		0.0
Delay (s)		70.8	63.3	67.8	73.5	21.6			102.7	35.7		69.1
Level of Service		E	E	E	E	C			F	D		E
Approach Delay (s)			66.0			44.3			84.5			69.1
Approach LOS			E			D			F			E
Intersection Summary												
HCM 2000 Control Delay			62.6		HCM 2000 Level of Service				E			
HCM 2000 Volume to Capacity ratio			0.96									
Actuated Cycle Length (s)			140.0		Sum of lost time (s)				25.0			
Intersection Capacity Utilization			91.4%		ICU Level of Service				F			
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	501	280	0	1163	0	180
Future Volume (vph)	501	280	0	1163	0	180
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3282	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3282	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1072			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	0%	0%	7%	0%	0%
Adj. Flow (vph)	545	304	0	1264	0	196
Shared Lane Traffic (%)						
Lane Group Flow (vph)	545	304	0	1264	0	196
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	35.5%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

3: Driveway 2A & US79

Phase 1 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑		↑
Traffic Volume (veh/h)	501	280	0	1163	0	180
Future Volume (Veh/h)	501	280	0	1163	0	180
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	545	304	0	1264	0	196
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised			Raised		
Median storage veh	2			2		
Upstream signal (ft)	1072					
pX, platoon unblocked			0.90		0.90	0.90
vC, conflicting volume			849		1177	272
vC1, stage 1 conf vol					545	
vC2, stage 2 conf vol					632	
vCu, unblocked vol			617		980	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	80
cM capacity (veh/h)			878		436	985
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	272	272	304	632	632	196
Volume Left	0	0	0	0	0	0
Volume Right	0	0	304	0	0	196
cSH	1700	1700	1700	1700	1700	985
Volume to Capacity	0.16	0.16	0.18	0.37	0.37	0.20
Queue Length 95th (ft)	0	0	0	0	0	18
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.6
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.6
Approach LOS						A
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			35.5%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	501	280	0	1163	0	180
Future Vol, veh/h	501	280	0	1163	0	180
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	0	0	7	0	0
Mvmt Flow	545	304	0	1264	0	196
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	273
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	731
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	731
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.7	
HCM LOS					B	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	731	-	-	-		
HCM Lane V/C Ratio	0.268	-	-	-		
HCM Control Delay (s)	11.7	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	1.1	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	61	0	621	0	1128	0	0	0
Future Volume (vph)	61	0	621	0	1128	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	67	0	682	0	1240	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	67	682	0	1240	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utilization 41.2% ICU Level of Service A								
Analysis Period (min) 15								

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

4: US79 & Crossover

Phase 1 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	61	0	621	0	1128	0	0	0
Future Volume (Veh/h)	61	0	621	0	1128	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	682	0	1240	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1240		0			1581	620
vC1, stage 1 conf vol							1240	
vC2, stage 2 conf vol							341	
vCu, unblocked vol	0	1240		0			1581	620
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	569		0			229	436
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	341	341	620	620	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.20	0.20	0.36	0.36	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.2%		ICU Level of Service		A	
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.9							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	61	0	621	0	1128	0	0	0
Future Vol, veh/h	61	0	621	0	1128	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	67	0	682	0	1240	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	1240	1240	0	682	-	0	1715	620
Stage 1	-	-	-	-	-	-	1240	-
Stage 2	-	-	-	-	-	-	475	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	237	569	-	538	-	0	83	436
Stage 1	-	-	-	-	-	0	240	-
Stage 2	-	-	-	-	-	0	597	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	237	237	-	538	-	-	60	436
Mov Cap-2 Maneuver	-	-	-	-	-	-	60	-
Stage 1	-	-	-	-	-	-	172	-
Stage 2	-	-	-	-	-	-	597	-
Approach	EB		WB		SB			
HCM Control Delay, s	2.3		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	237	-	538	-	-			
HCM Lane V/C Ratio	0.283	-	-	-	-			
HCM Control Delay (s)	26.1	-	0	-	0			
HCM Lane LOS	D	-	A	-	A			
HCM 95th %tile Q(veh)	1.1	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (vph)	7	643	10	7	1069	1	33	7	1	3	3	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.932	
Flt Protected	0.950			0.950				0.961			0.988	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Flt Permitted	0.950			0.950				0.961			0.988	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	8	731	11	8	1215	1	0	47	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	41.8%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

5: S Rio Grande & US79

Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Future Volume (Veh/h)	7	643	10	7	1069	1	33	7	1	3	3	5
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	8	731	11	8	1215	1	38	8	1	3	3	6
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1215			731			1372	1978	366	1616	1978	608
vC1, stage 1 conf vol							747	747		1231	1231	
vC2, stage 2 conf vol							625	1231		386	747	
vCu, unblocked vol	1215			731			1372	1978	366	1616	1978	608
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			87	96	100	98	99	99
cM capacity (veh/h)	581			883			289	210	637	180	214	444
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	8	366	366	11	8	608	608	1	47	12		
Volume Left	8	0	0	0	8	0	0	0	38	3		
Volume Right	0	0	0	11	0	0	0	1	1	6		
cSH	581	1700	1700	1700	883	1700	1700	1700	275	271		
Volume to Capacity	0.01	0.21	0.21	0.01	0.01	0.36	0.36	0.00	0.17	0.04		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	15	3		
Control Delay (s)	11.3	0.0	0.0	0.0	9.1	0.0	0.0	0.0	20.8	18.9		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				20.8	18.9		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.7											
Intersection Capacity Utilization	41.8%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Future Vol, veh/h	7	643	10	7	1069	1	33	7	1	3	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	8	731	11	8	1215	1	38	8	1	3	3	6

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	1215	0	0	731	0	0	1372	1978	-	1617	1978	-
Stage 1	-	-	-	-	-	-	747	747	-	1231	1231	-
Stage 2	-	-	-	-	-	-	625	1231	-	386	747	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	581	-	-	883	-	-	107	63	0	70	63	0
Stage 1	-	-	-	-	-	-	376	423	0	191	252	0
Stage 2	-	-	-	-	-	-	444	252	0	614	423	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	581	-	-	883	-	-	101	62	-	62	62	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	101	62	-	62	62	-
Stage 1	-	-	-	-	-	-	371	417	-	188	250	-
Stage 2	-	-	-	-	-	-	434	250	-	594	417	-

Approach	EB			WB			NB		SB			
HCM Control Delay, s	0.1			0.1								
HCM LOS							-		-			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	581	-	-	883	-	-	-
HCM Lane V/C Ratio	-	0.014	-	-	0.009	-	-	-
HCM Control Delay (s)	-	11.3	-	-	9.1	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	67	632	103	0	1225
Future Volume (vph)	0	67	632	103	0	1225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1759	1615	0	1792
Flt Permitted						
Satd. Flow (perm)	0	1644	1759	1615	0	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	0	73	687	112	0	1332
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	73	687	112	0	1332
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 67.8%	ICU Level of Service C					
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized) 6: FM973 & Driveway 1B Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	67	632	103	0	1225
Future Volume (Veh/h)	0	67	632	103	0	1225
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	73	687	112	0	1332
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)			545		636	
pX, platoon unblocked	0.66	0.66			0.66	
vC, conflicting volume	2019	687			799	
vC1, stage 1 conf vol	687					
vC2, stage 2 conf vol	1332					
vCu, unblocked vol	2283	273			442	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	86			100	
cM capacity (veh/h)	216	511			748	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	73	687	112	1332		
Volume Left	0	0	0	0		
Volume Right	73	0	112	0		
cSH	511	1700	1700	1700		
Volume to Capacity	0.14	0.40	0.07	0.78		
Queue Length 95th (ft)	12	0	0	0		
Control Delay (s)	13.2	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	13.2	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			67.8%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	67	632	103	0	1225
Future Vol, veh/h	0	67	632	103	0	1225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	0	73	687	112	0	1332
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	687	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	450	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	450	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	14.5	0		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	450		-		
HCM Lane V/C Ratio	-	0.162		-		
HCM Control Delay (s)	-	14.5		-		
HCM Lane LOS	-	B		-		
HCM 95th %tile Q(veh)	-	0.6		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	138	43	691	25	111	1113
Future Volume (vph)	138	43	691	25	111	1113
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		47		27		
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	150	47	751	27	121	1210
Shared Lane Traffic (%)						
Lane Group Flow (vph)	150	47	751	27	121	1210
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	3	1	1	3
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	50	50	250	50	50	250
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	50	50	6	50	50	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			122			122
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Detector 3 Position(ft)			244			244
Detector 3 Size(ft)			6			6

Scenario 1 5:57 pm 08/19/2022 Baseline

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Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex			Cl+Ex		
Detector 3 Channel						
Detector 3 Extend (s)	0.0			0.0		
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	34.0	34.0	13.0	47.0
Total Split (%)	32.9%	32.9%	48.6%	48.6%	18.6%	67.1%
Maximum Green (s)	18.0	18.0	28.0	28.0	7.0	41.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lead/Lag			Lead	Lead	Lag	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Act Effect Green (s)	11.3	11.3	41.1	41.1	7.0	51.5
Actuated g/C Ratio	0.16	0.16	0.59	0.59	0.10	0.74
v/c Ratio	0.52	0.16	0.73	0.03	0.67	0.92
Control Delay	32.7	9.2	22.8	9.1	51.1	25.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.7	9.2	22.8	9.1	51.1	25.5
LOS	C	A	C	A	D	C
Approach Delay	27.1		22.3			27.8
Approach LOS	C		C			C

Intersection Summary

Area Type: Other

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 25.9

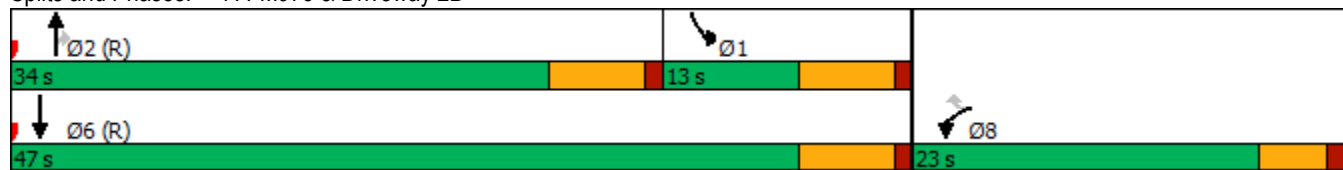
Intersection LOS: C

Intersection Capacity Utilization 75.4%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 7: FM973 & Driveway 2B



Scenario 1 5:57 pm 08/19/2022 Baseline

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Phasings
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	34.0	34.0	13.0	47.0
Total Split (%)	32.9%	32.9%	48.6%	48.6%	18.6%	67.1%
Maximum Green (s)	18.0	18.0	28.0	28.0	7.0	41.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag			Lead	Lead	Lag	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
90th %ile Green (s)	15.3	15.3	30.7	30.7	7.0	43.7
90th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
70th %ile Green (s)	12.8	12.8	33.2	33.2	7.0	46.2
70th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
50th %ile Green (s)	11.1	11.1	34.9	34.9	7.0	47.9
50th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
30th %ile Green (s)	9.3	9.3	36.7	36.7	7.0	49.7
30th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
10th %ile Green (s)	0.0	0.0	64.0	64.0	0.0	64.0
10th %ile Term Code	Skip	Skip	Coord	Coord	Skip	Coord
Intersection Summary						
Cycle Length: 70						
Actuated Cycle Length: 70						
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Control Type: Actuated-Coordinated						

Queues
7: FM973 & Driveway 2B

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	150	47	751	27	121	1210
v/c Ratio	0.52	0.16	0.73	0.03	0.67	0.92
Control Delay	32.7	9.2	22.8	9.1	51.1	25.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.7	9.2	22.8	9.1	51.1	25.5
Queue Length 50th (ft)	60	0	216	1	51	417
Queue Length 95th (ft)	105	24	m#435	m9	#125	#824
Internal Link Dist (ft)	143		592			465
Turn Bay Length (ft)		100		250	150	
Base Capacity (vph)	464	450	1032	959	180	1318
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.10	0.73	0.03	0.67	0.92
Intersection Summary						
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized) 7: FM973 & Driveway 2B

Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	138	43	691	25	111	1113
Future Volume (vph)	138	43	691	25	111	1113
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	150	47	751	27	121	1210
RTOR Reduction (vph)	0	40	0	12	0	0
Lane Group Flow (vph)	150	7	751	15	121	1210
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Actuated Green, G (s)	9.7	9.7	37.7	37.7	5.6	49.3
Effective Green, g (s)	9.7	9.7	37.7	37.7	5.6	49.3
Actuated g/C Ratio	0.14	0.14	0.54	0.54	0.08	0.70
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	250	223	947	869	144	1262
v/s Ratio Prot	c0.08		0.43		0.07	c0.68
v/s Ratio Perm		0.00		0.01		
v/c Ratio	0.60	0.03	0.79	0.02	0.84	0.96
Uniform Delay, d1	28.3	26.1	13.0	7.5	31.8	9.4
Progression Factor	1.00	1.00	1.12	1.90	1.00	1.00
Incremental Delay, d2	3.8	0.1	5.9	0.0	33.4	17.2
Delay (s)	32.2	26.1	20.4	14.4	65.2	26.6
Level of Service	C	C	C	B	E	C
Approach Delay (s)	30.7		20.2			30.1
Approach LOS	C		C			C
Intersection Summary						
HCM 2000 Control Delay			26.8		HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio			1.00			
Actuated Cycle Length (s)			70.0		Sum of lost time (s)	17.0
Intersection Capacity Utilization			75.4%		ICU Level of Service	D
Analysis Period (min)			15			
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)
 7: FM973 & Driveway 2B Phase 1 Build-Out Year w/Development

									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	138	43	691	25	111	1113			
Future Volume (veh/h)	138	43	691	25	111	1113			
Number	3	18	2	12	1	6			
Initial Q, veh	0	0	0	0	0	0			
Ped-Bike Adj (A_pbT)	1.00	1.00		1.00	1.00				
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No		No			No			
Lanes Open During Work Zone									
Adj Sat Flow, veh/h/ln	1900	1900	1781	1900	1900	1811			
Adj Flow Rate, veh/h	150	47	751	27	121	1210			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Percent Heavy Veh, %	0	0	8	0	0	6			
Opposing Right Turn Influence	Yes				Yes				
Cap, veh/h	205	183	713	644	441	1321			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Prop Arrive On Green	0.11	0.11	0.40	0.40	0.24	0.73			
Unsig. Movement Delay									
Ln Grp Delay, s/veh	34.9	29.1	69.8	12.9	21.8	19.1			
Ln Grp LOS	C	C	F	B	C	B			
Approach Vol, veh/h	197		778			1331			
Approach Delay, s/veh	33.5		67.8			19.4			
Approach LOS	C		E			B			
Timer:		1	2	3	4	5	6	7	8
Assigned Phs		2	1	8			6		
Case No		7.0	2.0	9.0			4.0		
Phs Duration (G+Y+Rc), s		34.0	23.1	12.9			57.1		
Change Period (Y+Rc), s		6.0	6.0	5.0			6.0		
Max Green (Gmax), s		28.0	7.0	18.0			41.0		
Max Allow Headway (MAH), s		6.9	4.1	4.4			6.9		
Max Q Clear (g_c+I1), s		30.0	5.8	7.6			40.1		
Green Ext Time (g_e), s		0.0	0.0	0.5			0.8		
Prob of Phs Call (p_c)		1.00	0.90	0.98			1.00		
Prob of Max Out (p_x)		1.00	1.00	0.02			1.00		
Left-Turn Movement Data									
Assigned Mvmt		5	1	3					
Mvmt Sat Flow, veh/h		0	1810	1810					
Through Movement Data									
Assigned Mvmt		2		8			6		
Mvmt Sat Flow, veh/h		1781		0			1811		
Right-Turn Movement Data									
Assigned Mvmt		12		18			16		
Mvmt Sat Flow, veh/h		1610		1610			0		
Left Lane Group Data									
Assigned Mvmt		5	1	3	0	0	0	0	0
Lane Assignment		L (Prot)		L					

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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HCM 6th Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)
 7: FM973 & Driveway 2B Phase 1 Build-Out Year w/Development

Lanes in Grp	0	1	1	0	0	0	0	0
Grp Vol (v), veh/h	0	121	150	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1810	1810	0	0	0	0	0
Q Serve Time (g_s), s	0.0	3.8	5.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	3.8	5.6	0.0	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	1810	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	28.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	441	205	0	0	0	0	0
V/C Ratio (X)	0.00	0.27	0.73	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	441	465	0	0	0	0	0
Upstream Filter (I)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	21.5	30.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.3	4.9	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	21.8	34.9	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.4	2.3	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	1.4	2.6	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.24	0.42	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Middle Lane Group Data								
Assigned Mvmt	2	0	8	0	0	6	0	0
Lane Assignment	T					T		
Lanes in Grp	1	0	0	0	0	1	0	0
Grp Vol (v), veh/h	751	0	0	0	0	1210	0	0
Grp Sat Flow (s), veh/h/ln	1781	0	0	0	0	1811	0	0
Q Serve Time (g_s), s	28.0	0.0	0.0	0.0	0.0	38.1	0.0	0.0
Cycle Q Clear Time (g_c), s	28.0	0.0	0.0	0.0	0.0	38.1	0.0	0.0
Lane Grp Cap (c), veh/h	713	0	0	0	0	1321	0	0
V/C Ratio (X)	1.05	0.00	0.00	0.00	0.00	0.92	0.00	0.00
Avail Cap (c_a), veh/h	713	0	0	0	0	1321	0	0
Upstream Filter (I)	1.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	21.0	0.0	0.0	0.0	0.0	7.7	0.0	0.0
Incr Delay (d2), s/veh	48.8	0.0	0.0	0.0	0.0	11.4	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	69.8	0.0	0.0	0.0	0.0	19.1	0.0	0.0
1st-Term Q (Q1), veh/ln	9.1	0.0	0.0	0.0	0.0	4.6	0.0	0.0
2nd-Term Q (Q2), veh/ln	9.7	0.0	0.0	0.0	0.0	4.2	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)
 7: FM973 & Driveway 2B Phase 1 Build-Out Year w/Development

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	18.7	0.0	0.0	0.0	0.0	8.8	0.0	0.0
%ile Storage Ratio (RQ%)	0.81	0.00	0.00	0.00	0.00	0.48	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	9.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Right Lane Group Data

Assigned Mvmt	12	0	18	0	0	16	0	0
Lane Assignment	R		R					
Lanes in Grp	1	0	1	0	0	0	0	0
Grp Vol (v), veh/h	27	0	47	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1610	0	1610	0	0	0	0	0
Q Serve Time (g_s), s	0.7	0.0	1.9	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.7	0.0	1.9	0.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	644	0	183	0	0	0	0	0
V/C Ratio (X)	0.04	0.00	0.26	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	644	0	414	0	0	0	0	0
Upstream Filter (I)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	12.8	0.0	28.3	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.1	0.0	0.7	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	12.9	0.0	29.1	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.2	0.0	0.7	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.2	0.0	0.7	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.02	0.00	0.18	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	36.9
HCM 6th LOS	D

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2025 AM Peak Hour (w/New Signals, Optimized)

Phase 1 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	90	643	12	207	1053
Future Volume (vph)	6	90	643	12	207	1053
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.998			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1654	0	1758	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1654	0	1758	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	106	756	14	244	1239
Shared Lane Traffic (%)						
Lane Group Flow (vph)	113	0	770	0	244	1239
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	68.0%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

8: FM973 & Taylor HS

Phase 1 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	90	643	12	207	1053
Future Volume (Veh/h)	6	90	643	12	207	1053
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	106	756	14	244	1239
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)			1184		672	
pX, platoon unblocked	0.45	0.76			0.76	
vC, conflicting volume	2490	763			770	
vC1, stage 1 conf vol	763					
vC2, stage 2 conf vol	1727					
vCu, unblocked vol	2485	528			538	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	67	75			69	
cM capacity (veh/h)	21	420			789	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	113	770	244	1239		
Volume Left	7	0	244	0		
Volume Right	106	14	0	0		
cSH	193	1700	789	1700		
Volume to Capacity	0.58	0.45	0.31	0.73		
Queue Length 95th (ft)	80	0	33	0		
Control Delay (s)	46.9	0.0	11.6	0.0		
Lane LOS	E		B			
Approach Delay (s)	46.9	0.0	1.9			
Approach LOS	E					
Intersection Summary						
Average Delay		3.4				
Intersection Capacity Utilization		68.0%		ICU Level of Service	C	
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	90	643	12	207	1053
Future Vol, veh/h	6	90	643	12	207	1053
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	106	756	14	244	1239
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2490	763	0	0	770	0
Stage 1	763	-	-	-	-	-
Stage 2	1727	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	33	408	-	-	854	-
Stage 1	464	-	-	-	-	-
Stage 2	159	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	24	408	-	-	854	-
Mov Cap-2 Maneuver	90	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	114	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	21.2	0		1.8		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	334	854	-	
HCM Lane V/C Ratio	-	-	0.338	0.285	-	
HCM Control Delay (s)	-	-	21.2	10.9	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.5	1.2	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2025 AM Peak Hour (w/New Signals, Optimized)

Phase 1 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	93	33	12	561	1018	39
Future Volume (vph)	93	33	12	561	1018	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964					0.850
Flt Protected	0.965		0.950			
Satd. Flow (prot)	1767	0	1805	1759	1810	1615
Flt Permitted	0.965		0.950			
Satd. Flow (perm)	1767	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	103	37	13	623	1131	43
Shared Lane Traffic (%)						
Lane Group Flow (vph)	140	0	13	623	1131	43
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.4%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized) 9: FM973 & Streamwood Phase 1 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	93	33	12	561	1018	39
Future Volume (Veh/h)	93	33	12	561	1018	39
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	103	37	13	623	1131	43
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh				2	2	
Upstream signal (ft)				724	1132	
pX, platoon unblocked	0.50	0.37	0.37			
vC, conflicting volume	1780	1131	1174			
vC1, stage 1 conf vol	1131					
vC2, stage 2 conf vol	649					
vCu, unblocked vol	898	499	616			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	53	83	96			
cM capacity (veh/h)	218	212	359			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	140	13	623	1131	43	
Volume Left	103	13	0	0	0	
Volume Right	37	0	0	0	43	
cSH	217	359	1700	1700	1700	
Volume to Capacity	0.65	0.04	0.37	0.67	0.03	
Queue Length 95th (ft)	97	3	0	0	0	
Control Delay (s)	47.7	15.4	0.0	0.0	0.0	
Lane LOS	E	C				
Approach Delay (s)	47.7	0.3		0.0		
Approach LOS	E					
Intersection Summary						
Average Delay			3.5			
Intersection Capacity Utilization			67.4%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	93	33	12	561	1018	39
Future Vol, veh/h	93	33	12	561	1018	39
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	103	37	13	623	1131	43
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1780	1131	1174	0	-	0
Stage 1	1131	-	-	-	-	-
Stage 2	649	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 91	250	602	-	-	-
Stage 1	311	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 89	250	602	-	-	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	304	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	45.4	0.2		0		
HCM LOS	E					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	602	-	222	-	-	
HCM Lane V/C Ratio	0.022	-	0.631	-	-	
HCM Control Delay (s)	11.1	-	45.4	-	-	
HCM Lane LOS	B	-	E	-	-	
HCM 95th %tile Q(veh)	0.1	-	3.8	-	-	
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Future Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.959			0.877			0.986			0.999	
Flt Protected		0.966			0.996		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1660	0	1805	1676	0	1805	1711	0
Flt Permitted		0.486			0.968		0.950			0.950		
Satd. Flow (perm)	0	886	0	0	1613	0	1805	1676	0	1805	1711	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		234			234			7			1	
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	14	0	6	19	0	201	2	435	44	400	790	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	220	0	2	479	0	400	797	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0		8.0	8.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	24.0	24.0		23.0	23.0		13.0	24.0		13.0	24.0	
Total Split (s)	24.0	24.0		24.0	24.0		13.0	25.0		21.0	33.0	
Total Split (%)	34.3%	34.3%		34.3%	34.3%		18.6%	35.7%		30.0%	47.1%	
Maximum Green (s)	19.0	19.0		19.0	19.0		7.0	19.0		15.0	27.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)		8.7			8.7		7.0	21.0		23.3	47.7	
Actuated g/C Ratio		0.12			0.12		0.10	0.30		0.33	0.68	
v/c Ratio		0.06			0.54		0.01	0.94		0.67	0.68	
Control Delay		0.4			9.2		28.5	55.3		20.9	14.5	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		0.4			9.2		28.5	55.3		20.9	14.5	
LOS		A			A		C	E		C	B	
Approach Delay		0.4			9.2			55.1			16.6	
Approach LOS		A			A			E			B	

Intersection Summary

Area Type: Other

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 25.3

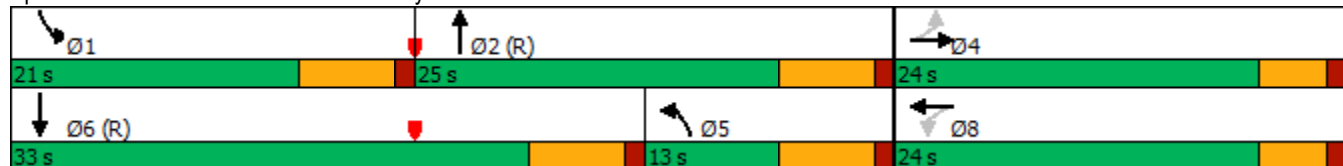
Intersection LOS: C

Intersection Capacity Utilization 68.6%

ICU Level of Service C

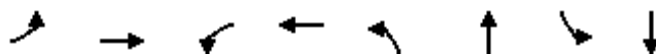
Analysis Period (min) 15

Splits and Phases: 10: FM973 & Wesley Miller



Phasings
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signals, Optimized)
Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Protected Phases		4		8	5	2	1	6
Permitted Phases	4		8					
Minimum Initial (s)	8.0	8.0	8.0	8.0	7.0	10.0	7.0	10.0
Minimum Split (s)	24.0	24.0	23.0	23.0	13.0	24.0	13.0	24.0
Total Split (s)	24.0	24.0	24.0	24.0	13.0	25.0	21.0	33.0
Total Split (%)	34.3%	34.3%	34.3%	34.3%	18.6%	35.7%	30.0%	47.1%
Maximum Green (s)	19.0	19.0	19.0	19.0	7.0	19.0	15.0	27.0
Yellow Time (s)	3.5	3.5	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag					Lag	Lag	Lead	Lead
Lead-Lag Optimize?					Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	None	None	None	C-Min	None	C-Min
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	11.4	11.4	11.4	11.4	7.0	19.0	22.6	34.6
90th %ile Term Code	Hold	Hold	Gap	Gap	Max	Coord	Max	Coord
70th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	20.3	24.7	51.0
70th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
50th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	21.5	23.5	51.0
50th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
30th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	22.0	23.0	51.0
30th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
10th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	22.4	22.6	51.0
10th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
Intersection Summary								
Cycle Length: 70								
Actuated Cycle Length: 70								
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green								
Control Type: Actuated-Coordinated								

Queues
10: FM973 & Wesley Miller

2025 AM Peak Hour (w/New Signals, Optimized)

Phase 1 Build-Out Year w/Development



Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	20	220	2	479	400	797
v/c Ratio	0.06	0.54	0.01	0.94	0.67	0.68
Control Delay	0.4	9.2	28.5	55.3	20.9	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.4	9.2	28.5	55.3	20.9	14.5
Queue Length 50th (ft)	0	0	1	195	133	225
Queue Length 95th (ft)	0	45	7	#387	m176	m#442
Internal Link Dist (ft)	494	480		650		644
Turn Bay Length (ft)			80		400	
Base Capacity (vph)	410	608	180	508	600	1166
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.36	0.01	0.94	0.67	0.68

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/New Signals, Optimized)

10: FM973 & Wesley Miller

Phase 1 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Future Volume (vph)	12	0	5	17	0	177	2	383	39	352	695	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.96			0.88		1.00	0.99		1.00	1.00	
Flt Protected		0.97			1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1761			1658		1805	1676		1805	1711	
Flt Permitted		0.49			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (perm)		886			1613		1805	1676		1805	1711	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	14	0	6	19	0	201	2	435	44	400	790	7
RTOR Reduction (vph)	0	18	0	0	193	0	0	5	0	0	0	0
Lane Group Flow (vph)	0	2	0	0	27	0	2	474	0	400	797	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Actuated Green, G (s)		8.7			8.7		1.4	21.0		23.3	42.9	
Effective Green, g (s)		8.7			8.7		1.4	21.0		23.3	42.9	
Actuated g/C Ratio		0.12			0.12		0.02	0.30		0.33	0.61	
Clearance Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		110			200		36	502		600	1048	
v/s Ratio Prot							0.00	c0.28		0.22	c0.47	
v/s Ratio Perm		0.00			c0.02							
v/c Ratio		0.02			0.14		0.06	0.94		0.67	0.76	
Uniform Delay, d1		26.9			27.3		33.7	23.9		20.0	9.8	
Progression Factor		1.00			1.00		1.00	1.00		0.82	1.27	
Incremental Delay, d2		0.1			0.3		0.6	28.5		1.9	3.5	
Delay (s)		27.0			27.6		34.3	52.4		18.3	16.0	
Level of Service		C			C		C	D		B	B	
Approach Delay (s)		27.0			27.6			52.3			16.8	
Approach LOS		C			C			D			B	
Intersection Summary												
HCM 2000 Control Delay		27.0			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.78										
Actuated Cycle Length (s)		70.0			Sum of lost time (s)			17.0				
Intersection Capacity Utilization		68.6%			ICU Level of Service			C				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Appendix D. Traffic Simulation Output

- Phase 1 Buildout Year (2025) Projected Conditions with Development, AM & PM
 - Mitigation:
 - With Dual Left Turn and Right Turn Only Lane on Northbound FM 973 Approach to US 79 and with Optimized Timings at FM 973 at US 79
 - With Traffic Signal at FM 973 at Driveway 2B (half cycle to US 79 intersection)
 - With Traffic Signal at FM 973 at Wesley Miller Lane (half cycle to US 79 intersection)

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/LTx2 on FM 973/79)

Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		450	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt				0.850						0.850		0.865
Flt Protected		0.950			0.950			0.950	0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3375	0	1588	1588	1568	0	1644
Flt Permitted		0.950			0.950			0.950	0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3375	0	1588	1588	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				754						202		289
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
Shared Lane Traffic (%)								50%				
Lane Group Flow (vph)	0	7	563	774	500	642	0	270	271	202	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			12			12
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/LTx2 on FM 973/79)
Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.5	32.6	32.6	26.3	63.0		24.6	24.6	24.6		5.3
Actuated g/C Ratio		0.10	0.31	0.31	0.25	0.60		0.23	0.23	0.23		0.05
v/c Ratio		0.04	0.55	0.77	1.16	0.32		0.73	0.73	0.39		0.00
Control Delay		56.1	32.6	8.8	133.1	13.4		50.8	51.0	7.4		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Delay		56.1	32.6	8.8	133.1	13.4		50.8	51.0	7.4		0.0
LOS		E	C	A	F	B		D	D	A		A
Approach Delay			19.0			65.8			39.1			
Approach LOS			B			E			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 105

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.16

Intersection Signal Delay: 40.2

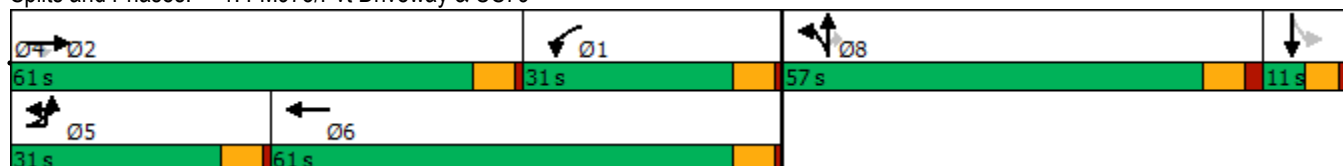
Intersection LOS: D

Intersection Capacity Utilization 90.3%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 AM Peak Hour (w/LTx2 on FM 973/79)

Phase 1 Build-Out Year w/Development

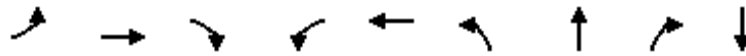
									
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBT
Protected Phases	5	2		1	6	8	8		4
Permitted Phases			2					8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None	None
Walk Time (s)									
Flash Dont Walk (s)									
Pedestrian Calls (#/hr)									
90th %ile Green (s)	10.0	54.2	54.2	25.0	69.2	41.7	41.7	41.7	5.0
90th %ile Term Code	Min	Gap	Gap	Max	Hold	Gap	Gap	Gap	Max
70th %ile Green (s)	0.0	37.1	37.1	25.0	68.1	28.4	28.4	28.4	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
50th %ile Green (s)	0.0	31.0	31.0	25.0	62.0	23.7	23.7	23.7	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
30th %ile Green (s)	0.0	25.2	25.2	25.0	56.2	18.7	18.7	18.7	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
10th %ile Green (s)	0.0	19.9	19.9	25.0	50.9	14.2	14.2	14.2	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
Intersection Summary									
Cycle Length: 160									
Actuated Cycle Length: 105									
Control Type: Actuated-Uncoordinated									
90th %ile Actuated Cycle: 150.9									
70th %ile Actuated Cycle: 109.5									
50th %ile Actuated Cycle: 98.7									
30th %ile Actuated Cycle: 87.9									
10th %ile Actuated Cycle: 78.1									

Queues

2025 AM Peak Hour (w/LTx2 on FM 973/79)

1: FM973/Pvt Driveway & US79

Phase 1 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBT
Lane Group Flow (vph)	7	563	774	500	642	270	271	202	1
v/c Ratio	0.04	0.55	0.77	1.16	0.32	0.73	0.73	0.39	0.00
Control Delay	56.1	32.6	8.8	133.1	13.4	50.8	51.0	7.4	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.1	32.6	8.8	133.1	13.4	50.8	51.0	7.4	0.0
Queue Length 50th (ft)	4	153	9	~374	86	164	165	0	0
Queue Length 95th (ft)	24	283	141	#968	264	353	354	62	0
Internal Link Dist (ft)		854			992		556		491
Turn Bay Length (ft)	100		615	880		250		450	
Base Capacity (vph)	452	1824	1194	430	2060	795	795	885	356
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.31	0.65	1.16	0.31	0.34	0.34	0.23	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 2025 AM Peak Hour (w/LTx2 on FM 973/79)

1: FM973/Pvt Driveway & US79

Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Future Volume (vph)	7	0	529	728	470	602	2	509	0	190	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373		1588	1588	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373		1588	1588	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	0	563	774	500	640	2	541	0	202	0	0
RTOR Reduction (vph)	0	0	0	540	0	0	0	0	0	159	0	1
Lane Group Flow (vph)	0	7	563	234	500	642	0	270	271	43	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	32.6	32.6	31.8	63.0		24.6	24.6	24.6		0.7
Effective Green, g (s)		1.4	32.6	32.6	31.8	63.0		24.6	24.6	24.6		0.7
Actuated g/C Ratio		0.01	0.28	0.28	0.28	0.55		0.21	0.21	0.21		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0		2.0
Lane Grp Cap (vph)		22	941	441	476	1852		340	340	336		10
v/s Ratio Prot		0.00	c0.17		c0.29	0.19		0.17	c0.17			c0.00
v/s Ratio Perm				0.15						0.03		
v/c Ratio		0.32	0.60	0.53	1.05	0.35		0.79	0.80	0.13		0.00
Uniform Delay, d1		56.2	35.4	34.6	41.5	14.4		42.7	42.7	36.4		56.7
Progression Factor		1.00	1.00	1.00	0.98	0.98		1.00	1.00	1.00		1.00
Incremental Delay, d2		3.0	0.7	0.6	55.1	0.0		12.0	12.2	0.2		0.0
Delay (s)		59.2	36.1	35.2	95.9	14.1		54.7	54.9	36.6		56.7
Level of Service		E	D	D	F	B		D	D	D		E
Approach Delay (s)			35.7			49.9			49.8			56.7
Approach LOS			D			D			D			E
Intersection Summary												
HCM 2000 Control Delay			44.0		HCM 2000 Level of Service				D			
HCM 2000 Volume to Capacity ratio			0.81									
Actuated Cycle Length (s)			114.7		Sum of lost time (s)				25.0			
Intersection Capacity Utilization			90.3%		ICU Level of Service				E			
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/LTx2 on FM 973/79))

Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		450	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950			0.950	0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3340	0	1618	1618	1583	0	1900
Flt Permitted		0.950			0.950			0.950	0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3340	0	1618	1618	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				319						418		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
Shared Lane Traffic (%)								50%				
Lane Group Flow (vph)	0	9	871	319	211	484	0	247	248	418	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			12			12
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/LTx2 on FM 973/79))
Phase 1 Build-Out Year w/Development

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/LTx2 on FM 973/79))

Phase 1 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	16.0	16.0	64.0	64.0	38.0	86.0		47.0	47.0	47.0	11.0	11.0
Total Split (%)	10.0%	10.0%	40.0%	40.0%	23.8%	53.8%		29.4%	29.4%	29.4%	6.9%	6.9%
Maximum Green (s)	10.0	10.0	58.0	58.0	32.0	80.0		40.0	40.0	40.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.8	37.8	37.8	18.6	60.6		23.9	23.9	23.9		
Actuated g/C Ratio		0.11	0.38	0.38	0.18	0.60		0.24	0.24	0.24		
v/c Ratio		0.05	0.67	0.43	0.67	0.24		0.64	0.65	0.60		
Control Delay		55.4	30.2	4.7	52.3	10.9		46.0	46.2	7.7		
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		
Total Delay		55.4	30.2	4.7	52.3	10.9		46.0	46.2	7.7		
LOS		E	C	A	D	B		D	D	A		
Approach Delay			23.6			23.5			28.5			
Approach LOS			C			C			C			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 100.8

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 25.2

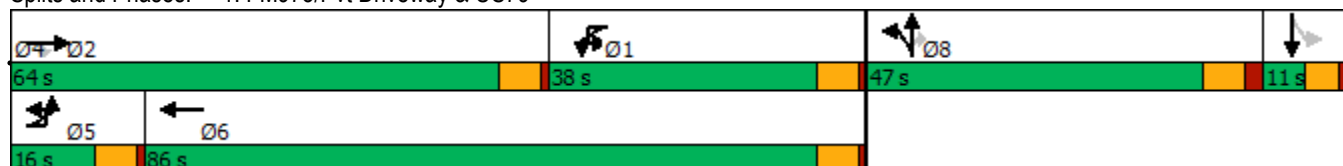
Intersection LOS: C

Intersection Capacity Utilization 71.9%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2025 PM Peak Hour (w/LTx2 on FM 973/79))

Phase 1 Build-Out Year w/Development

									
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8	8		4
Permitted Phases			2					8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	12.0	11.0
Total Split (s)	16.0	64.0	64.0	38.0	86.0	47.0	47.0	47.0	11.0
Total Split (%)	10.0%	40.0%	40.0%	23.8%	53.8%	29.4%	29.4%	29.4%	7%
Maximum Green (s)	10.0	58.0	58.0	32.0	80.0	40.0	40.0	40.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None	None
Walk Time (s)									
Flash Dont Walk (s)									
Pedestrian Calls (#/hr)									
90th %ile Green (s)	10.0	58.0	58.0	29.6	77.6	38.5	38.5	38.5	0.0
90th %ile Term Code	Max	Max	Max	Gap	Hold	Gap	Gap	Gap	Skip
70th %ile Green (s)	0.0	45.0	45.0	22.7	73.7	29.3	29.3	29.3	0.0
70th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
50th %ile Green (s)	0.0	36.8	36.8	18.1	60.9	23.0	23.0	23.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
30th %ile Green (s)	0.0	29.9	29.9	14.4	50.3	18.7	18.7	18.7	0.0
30th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
10th %ile Green (s)	0.0	21.9	21.9	10.2	38.1	12.8	12.8	12.8	0.0
10th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
Intersection Summary									
Cycle Length: 160									
Actuated Cycle Length: 100.8									
Control Type: Actuated-Uncoordinated									
90th %ile Actuated Cycle: 145.1									
70th %ile Actuated Cycle: 116									
50th %ile Actuated Cycle: 96.9									
30th %ile Actuated Cycle: 82									
10th %ile Actuated Cycle: 63.9									

Queues

2025 PM Peak Hour (w/LTx2 on FM 973/79))

1: FM973/Pvt Driveway & US79

Phase 1 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR
Lane Group Flow (vph)	9	871	319	211	484	247	248	418
v/c Ratio	0.05	0.67	0.43	0.67	0.24	0.64	0.65	0.60
Control Delay	55.4	30.2	4.7	52.3	10.9	46.0	46.2	7.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.4	30.2	4.7	52.3	10.9	46.0	46.2	7.7
Queue Length 50th (ft)	5	230	0	120	60	144	145	0
Queue Length 95th (ft)	28	417	61	273	157	313	315	88
Internal Link Dist (ft)		854			990		556	
Turn Bay Length (ft)	100		615	880		250		450
Base Capacity (vph)	193	2161	1026	590	2700	694	694	918
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.40	0.31	0.36	0.18	0.36	0.36	0.46
Intersection Summary								

HCM Signalized Intersection Capacity Analysis 2025 PM Peak Hour (w/LTx2 on FM 973/79))

1: FM973/Pvt Driveway & US79

Phase 1 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Future Volume (vph)	8	0	793	290	192	439	2	450	0	380	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		
Frt		1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3342		1618	1618	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3342		1618	1618	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	9	0	871	319	211	482	2	495	0	418	0	0
RTOR Reduction (vph)	0	0	0	204	0	0	0	0	0	323	0	0
Lane Group Flow (vph)	0	9	871	115	211	484	0	247	248	95	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	37.8	37.8	24.2	60.6		23.9	23.9	23.9		
Effective Green, g (s)		1.4	37.8	37.8	24.2	60.6		23.9	23.9	23.9		
Actuated g/C Ratio		0.01	0.36	0.36	0.23	0.58		0.23	0.23	0.23		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0		
Lane Grp Cap (vph)		24	1250	524	396	1930		368	368	360		
v/s Ratio Prot		0.00	c0.25		c0.12	0.14		0.15	c0.15			
v/s Ratio Perm				0.08						0.06		
v/c Ratio		0.38	0.70	0.22	0.53	0.25		0.67	0.67	0.26		
Uniform Delay, d1		51.3	28.7	23.3	35.4	10.9		36.9	36.9	33.3		
Progression Factor		1.00	1.00	1.00	0.98	0.98		1.00	1.00	1.00		
Incremental Delay, d2		3.6	1.4	0.1	0.7	0.0		4.8	4.8	0.4		
Delay (s)		54.9	30.0	23.4	35.5	10.7		41.7	41.8	33.7		
Level of Service		D	C	C	D	B		D	D	C		
Approach Delay (s)			28.5			18.3			38.0			0.0
Approach LOS			C			B			D			A
Intersection Summary												
HCM 2000 Control Delay			29.0			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.69									
Actuated Cycle Length (s)			104.9			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			71.9%			ICU Level of Service			C			
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Appendix D. Traffic Simulation Output

- Phase 2 Buildout Year (2027) Projected Conditions without Development, AM & PM

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	385	783	479	596	3	464	0	202	0	0
Future Volume (vph)	8	0	385	783	479	596	3	464	0	202	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				754						157		296
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	9	0	410	833	510	634	3	494	0	215	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	410	833	510	637	0	0	494	215	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.4	31.2	31.2	25.9	60.9			42.5	42.5		5.2
Actuated g/C Ratio		0.09	0.26	0.26	0.21	0.50			0.35	0.35		0.04
v/c Ratio		0.06	0.48	0.87	1.39	0.38			0.84	0.33		0.00
Control Delay		63.6	39.8	16.0	226.2	21.0			51.9	11.5		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		63.6	39.8	16.0	226.2	21.0			51.9	11.5		0.0
LOS		E	D	B	F	C			D	B		A
Approach Delay			24.1			112.2			39.6			
Approach LOS			C			F			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 121

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.39

Intersection Signal Delay: 60.2

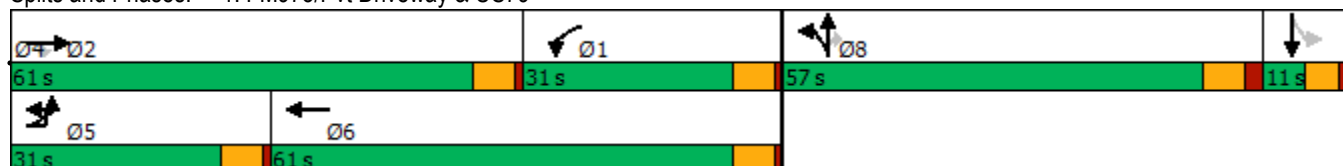
Intersection LOS: E

Intersection Capacity Utilization 94.2%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	38.9	38.9	25.0	69.9	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	27.1	27.1	25.0	58.1	44.0	44.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	23.2	23.2	25.0	54.2	37.7	37.7	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	17.9	17.9	25.0	48.9	30.0	30.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 121								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 160								
70th %ile Actuated Cycle: 132.9								
50th %ile Actuated Cycle: 115.1								
30th %ile Actuated Cycle: 104.9								
10th %ile Actuated Cycle: 91.9								

Queues

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour

Phase 2 Build-Out Year w/o Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	9	410	833	510	637	494	215	1
v/c Ratio	0.06	0.48	0.87	1.39	0.38	0.84	0.33	0.00
Control Delay	63.6	39.8	16.0	226.2	21.0	51.9	11.5	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.6	39.8	16.0	226.2	21.0	51.9	11.5	0.0
Queue Length 50th (ft)	6	140	47	~499	146	317	27	0
Queue Length 95th (ft)	30	218	271	#1046	296	#735	116	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	387	1562	1130	368	1746	716	761	353
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.26	0.74	1.39	0.36	0.69	0.28	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	385	783	479	596	3	464	0	202	0	0
Future Volume (vph)	8	0	385	783	479	596	3	464	0	202	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3372			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3372			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	9	0	410	833	510	634	3	494	0	215	0	0
RTOR Reduction (vph)	0	0	0	574	0	0	0	0	0	106	0	1
Lane Group Flow (vph)	0	9	410	259	510	637	0	0	494	109	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.5	31.2	31.2	31.2	60.9			42.5	42.5		0.8
Effective Green, g (s)		1.5	31.2	31.2	31.2	60.9			42.5	42.5		0.8
Actuated g/C Ratio		0.01	0.24	0.24	0.24	0.47			0.33	0.33		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		20	790	370	410	1571			543	509		10
v/s Ratio Prot		0.00	0.12		c0.30	0.19			c0.30			c0.00
v/s Ratio Perm				c0.17						0.07		
v/c Ratio		0.45	0.52	0.70	1.24	0.41			0.91	0.21		0.00
Uniform Delay, d1		64.2	43.2	45.5	49.7	23.0			42.3	32.0		64.6
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00
Incremental Delay, d2		5.8	0.2	4.6	128.8	0.1			19.1	0.2		0.0
Delay (s)		69.9	43.5	50.1	178.6	23.0			61.4	32.2		64.6
Level of Service		E	D	D	F	C			E	C		E
Approach Delay (s)			48.1			92.2			52.5			64.6
Approach LOS			D			F			D			E
Intersection Summary												
HCM 2000 Control Delay			65.4									HCM 2000 Level of Service E
HCM 2000 Volume to Capacity ratio			0.94									
Actuated Cycle Length (s)			130.7							25.0		
Intersection Capacity Utilization			94.2%							F		
ICU Level of Service												
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	3	0	607	0	1125	0	0	0
Future Volume (vph)	3	0	607	0	1125	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	3	0	667	0	1236	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	3	667	0	1236	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	34.4%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↕	↔	↕		↔	
Traffic Volume (veh/h)	3	0	607	0	1125	0	0	0
Future Volume (Veh/h)	3	0	607	0	1125	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	667	0	1236	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage veh			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1236		0			1570	618
vC1, stage 1 conf vol							1236	
vC2, stage 2 conf vol							334	
vCu, unblocked vol	0	1236		0			1570	618
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	571		0			230	437
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	334	334	618	618	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.20	0.20	0.36	0.36	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			34.4%		ICU Level of Service		A	
Analysis Period (min)			15					

HCM 6th TWSC
4: US79 & Crossover

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

Intersection								
Int Delay, s/veh	0							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	3	0	607	0	1125	0	0	0
Future Vol, veh/h	3	0	607	0	1125	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	3	0	667	0	1236	0	0	0
Major/Minor	Major1		Major2			Minor2		
Conflicting Flow All	1236	1236	0	667	-	0	1576	618
Stage 1	-	-	-	-	-	-	1236	-
Stage 2	-	-	-	-	-	-	340	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	238	571	-	550	-	0	102	437
Stage 1	-	-	-	-	-	0	241	-
Stage 2	-	-	-	-	-	0	698	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	238	238	-	550	-	-	101	437
Mov Cap-2 Maneuver	-	-	-	-	-	-	101	-
Stage 1	-	-	-	-	-	-	238	-
Stage 2	-	-	-	-	-	-	698	-
Approach	EB		WB			SB		
HCM Control Delay, s	0.1		0			0		
HCM LOS						A		
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	238	-	550	-	-			
HCM Lane V/C Ratio	0.014	-	-	-	-			
HCM Control Delay (s)	20.3	-	0	-	0			
HCM Lane LOS	C	-	A	-	A			
HCM 95th %tile Q(veh)	0	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	639	6	7	1073	1	29	7	1	3	3	1
Future Volume (vph)	4	639	6	7	1073	1	29	7	1	3	3	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.981	
Flt Protected	0.950			0.950				0.962			0.979	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1822	0	0	1825	0
Flt Permitted	0.950			0.950				0.962			0.979	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1822	0	0	1825	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	5	726	7	8	1219	1	33	8	1	3	3	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	5	726	7	8	1219	1	0	42	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	39.8%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	4	639	6	7	1073	1	29	7	1	3	3	1
Future Volume (Veh/h)	4	639	6	7	1073	1	29	7	1	3	3	1
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	5	726	7	8	1219	1	33	8	1	3	3	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1219			726			1363	1971	363	1612	1971	610
vC1, stage 1 conf vol							736	736		1235	1235	
vC2, stage 2 conf vol							627	1235		377	736	
vCu, unblocked vol	1219			726			1363	1971	363	1612	1971	610
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			89	96	100	98	99	100
cM capacity (veh/h)	579			886			295	212	640	179	215	443
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	5	363	363	7	8	610	610	1	42	7		
Volume Left	5	0	0	0	8	0	0	0	33	3		
Volume Right	0	0	0	7	0	0	0	1	1	1		
cSH	579	1700	1700	1700	886	1700	1700	1700	278	212		
Volume to Capacity	0.01	0.21	0.21	0.00	0.01	0.36	0.36	0.00	0.15	0.03		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	13	3		
Control Delay (s)	11.3	0.0	0.0	0.0	9.1	0.0	0.0	0.0	20.3	22.5		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				20.3	22.5		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	39.8%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	4	639	6	7	1073	1	29	7	1	3	3	1
Future Vol, veh/h	4	639	6	7	1073	1	29	7	1	3	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	5	726	7	8	1219	1	33	8	1	3	3	1

Major/Minor	Major1			Major2			Minor1		Minor2			
Conflicting Flow All	1219	0	0	726	0	0	1363	1971	-	1612	1971	-
Stage 1	-	-	-	-	-	-	736	736	-	1235	1235	-
Stage 2	-	-	-	-	-	-	627	1235	-	377	736	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	579	-	-	886	-	-	109	63	0	71	63	0
Stage 1	-	-	-	-	-	-	381	428	0	190	251	0
Stage 2	-	-	-	-	-	-	443	251	0	622	428	0
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	579	-	-	886	-	-	103	62	-	63	62	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	103	62	-	63	62	-
Stage 1	-	-	-	-	-	-	378	424	-	188	249	-
Stage 2	-	-	-	-	-	-	433	249	-	605	424	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	579	-	-	886	-	-	-
HCM Lane V/C Ratio	-	0.008	-	-	0.009	-	-	-
HCM Control Delay (s)	-	11.3	-	-	9.1	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	83	601	12	214	1059
Future Volume (vph)	6	83	601	12	214	1059
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.874		0.997			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1656	0	1757	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1656	0	1757	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	98	707	14	252	1246
Shared Lane Traffic (%)						
Lane Group Flow (vph)	105	0	721	0	252	1246
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	67.9%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	83	601	12	214	1059
Future Volume (Veh/h)	6	83	601	12	214	1059
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	98	707	14	252	1246
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2464	714			721	
vC1, stage 1 conf vol	714					
vC2, stage 2 conf vol	1750					
vCu, unblocked vol	2464	714			721	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	93	77			72	
cM capacity (veh/h)	106	435			890	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	105	721	252	1246		
Volume Left	7	0	252	0		
Volume Right	98	14	0	0		
cSH	360	1700	890	1700		
Volume to Capacity	0.29	0.42	0.28	0.73		
Queue Length 95th (ft)	30	0	29	0		
Control Delay (s)	19.1	0.0	10.6	0.0		
Lane LOS	C		B			
Approach Delay (s)	19.1	0.0	1.8			
Approach LOS	C					
Intersection Summary						
Average Delay		2.0				
Intersection Capacity Utilization		67.9%		ICU Level of Service		C
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	83	601	12	214	1059
Future Vol, veh/h	6	83	601	12	214	1059
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	98	707	14	252	1246
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	2464	714	0	0	721	0
Stage 1	714	-	-	-	-	-
Stage 2	1750	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	34	435	-	-	890	-
Stage 1	489	-	-	-	-	-
Stage 2	155	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	24	435	-	-	890	-
Mov Cap-2 Maneuver	89	-	-	-	-	-
Stage 1	489	-	-	-	-	-
Stage 2	111	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	19.9	0	1.8			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	345	890	-	
HCM Lane V/C Ratio	-	-	0.303	0.283	-	
HCM Control Delay (s)	-	-	19.9	10.6	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.3	1.2	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	79	33	12	533	1033	29
Future Volume (vph)	79	33	12	533	1033	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.960					0.850
Flt Protected	0.966		0.950			
Satd. Flow (prot)	1762	0	1805	1759	1810	1615
Flt Permitted	0.966		0.950			
Satd. Flow (perm)	1762	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	88	37	13	592	1148	32
Shared Lane Traffic (%)						
Lane Group Flow (vph)	125	0	13	592	1148	32
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization 67.4%				ICU Level of Service C		
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	79	33	12	533	1033	29
Future Volume (Veh/h)	79	33	12	533	1033	29
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	88	37	13	592	1148	32
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh)				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1766	1148	1180			
vC1, stage 1 conf vol	1148					
vC2, stage 2 conf vol	618					
vCu, unblocked vol	1766	1148	1180			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	67	85	98			
cM capacity (veh/h)	269	244	599			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	125	13	592	1148	32	
Volume Left	88	13	0	0	0	
Volume Right	37	0	0	0	32	
cSH	261	599	1700	1700	1700	
Volume to Capacity	0.48	0.02	0.35	0.68	0.02	
Queue Length 95th (ft)	60	2	0	0	0	
Control Delay (s)	30.8	11.1	0.0	0.0	0.0	
Lane LOS	D	B				
Approach Delay (s)	30.8	0.2		0.0		
Approach LOS	D					
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utilization			67.4%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	2.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	79	33	12	533	1033	29
Future Vol, veh/h	79	33	12	533	1033	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	88	37	13	592	1148	32
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1766	1148	1180	0	-	0
Stage 1	1148	-	-	-	-	-
Stage 2	618	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	93	244	599	-	-	-
Stage 1	305	-	-	-	-	-
Stage 2	542	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	91	244	599	-	-	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	298	-	-	-	-	-
Stage 2	542	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	40.4	0.2		0		
HCM LOS	E					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	599	-	221	-	-	
HCM Lane V/C Ratio	0.022	-	0.563	-	-	
HCM Control Delay (s)	11.1	-	40.4	-	-	
HCM Lane LOS	B	-	E	-	-	
HCM 95th %tile Q(veh)	0.1	-	3.1	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	9	0	5	19	0	193	3	344	43	383	682	3
Future Volume (vph)	9	0	5	19	0	193	3	344	43	383	682	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.949			0.877			0.983			0.999	
Flt Protected		0.970			0.995		0.950			0.950		
Satd. Flow (prot)	0	1749	0	0	1658	0	1805	1674	0	1805	1711	0
Flt Permitted		0.970			0.995		0.950			0.950		
Satd. Flow (perm)	0	1749	0	0	1658	0	1805	1674	0	1805	1711	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	10	0	6	22	0	219	3	391	49	435	775	3
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	241	0	3	440	0	435	778	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	64.7%											
Analysis Period (min)	15											
	ICU Level of Service C											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 AM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	9	0	5	19	0	193	3	344	43	383	682	3
Future Volume (Veh/h)	9	0	5	19	0	193	3	344	43	383	682	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	10	0	6	22	0	219	3	391	49	435	775	3
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							TWLTL			TWLTL		
Median storage (veh)							2			2		
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2262	2092	776	2072	2070	416	778				440	
vC1, stage 1 conf vol	1646	1646		422	422							
vC2, stage 2 conf vol	616	446		1651	1648							
vCu, unblocked vol	2262	2092	776	2072	2070	416	778				440	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	26	100	99	69	100	66	100				62	
cM capacity (veh/h)	14	83	400	71	91	641	848				1131	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	16	241	3	440	435	778						
Volume Left	10	22	3	0	435	0						
Volume Right	6	219	0	49	0	3						
cSH	21	371	848	1700	1131	1700						
Volume to Capacity	0.76	0.65	0.00	0.26	0.38	0.46						
Queue Length 95th (ft)	54	110	0	0	46	0						
Control Delay (s)	366.5	31.0	9.3	0.0	10.2	0.0						
Lane LOS	F	D	A		B							
Approach Delay (s)	366.5	31.0	0.1		3.6							
Approach LOS	F	D										
Intersection Summary												
Average Delay			9.3									
Intersection Capacity Utilization			64.7%	ICU Level of Service				C				
Analysis Period (min)			15									

Intersection												
Int Delay, s/veh	22.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	9	0	5	19	0	193	3	344	43	383	682	3
Future Vol, veh/h	9	0	5	19	0	193	3	344	43	383	682	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	11	0
Mvmt Flow	10	0	6	22	0	219	3	391	49	435	775	3
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2178	2093	777	2072	2070	416	778	0	0	440	0	0
Stage 1	1647	1647	-	422	422	-	-	-	-	-	-	-
Stage 2	531	446	-	1650	1648	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	34	53	400	40	55	641	848	-	-	1131	-	-
Stage 1	127	158	-	613	592	-	-	-	-	-	-	-
Stage 2	536	577	-	126	158	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	16	32	400	28	34	641	848	-	-	1131	-	-
Mov Cap-2 Maneuver	16	32	-	28	34	-	-	-	-	-	-	-
Stage 1	126	97	-	611	590	-	-	-	-	-	-	-
Stage 2	351	575	-	76	97	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s\$	303.9		142.7		0.1		3.6					
HCM LOS	F		F									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	848	-	-	24	216	1131	-	-				
HCM Lane V/C Ratio	0.004	-	-	0.663	1.115	0.385	-	-				
HCM Control Delay (s)	9.3	-	-	\$ 303.9	142.7	10.2	-	-				
HCM Lane LOS	A	-	-	F	F	B	-	-				
HCM 95th %tile Q(veh)	0	-	-	2	11.2	1.8	-	-				
Notes												
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined				*: All major volume in platoon				

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	734	378	332	431	3	363	0	326	0	0
Future Volume (vph)	9	0	734	378	332	431	3	363	0	326	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				415						323		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	10	0	807	415	365	474	3	399	0	358	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	807	415	365	477	0	0	399	358	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.3	39.6	39.6	25.8	69.0			34.3	34.3		
Actuated g/C Ratio		0.09	0.33	0.33	0.22	0.58			0.29	0.29		
v/c Ratio		0.06	0.70	0.55	0.98	0.25			0.81	0.52		
Control Delay		61.2	38.6	5.7	91.7	14.9			54.4	8.4		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		61.2	38.6	5.7	91.7	14.9			54.4	8.4		
LOS		E	D	A	F	B			D	A		
Approach Delay			27.7			48.2			32.6			
Approach LOS			C			D			C			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 119.3

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 35.1

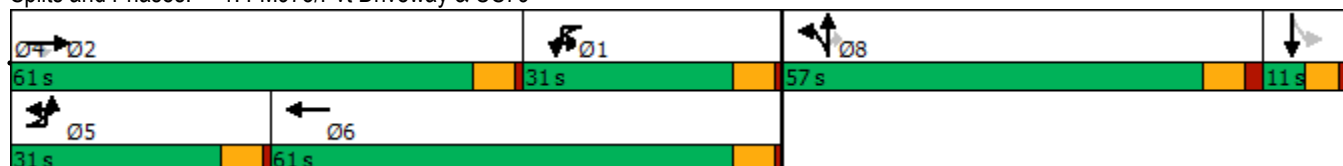
Intersection LOS: D

Intersection Capacity Utilization 74.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
70th %ile Green (s)	0.0	47.0	47.0	25.0	78.0	41.6	41.6	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
50th %ile Green (s)	0.0	39.9	39.9	25.0	70.9	34.8	34.8	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	33.2	33.2	25.0	64.2	27.8	27.8	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	26.0	26.0	25.0	57.0	21.1	21.1	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 119.3								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 149								
70th %ile Actuated Cycle: 132.6								
50th %ile Actuated Cycle: 118.7								
30th %ile Actuated Cycle: 105								
10th %ile Actuated Cycle: 91.1								

Queues
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	10	807	415	365	477	399	358
v/c Ratio	0.06	0.70	0.55	0.98	0.25	0.81	0.52
Control Delay	61.2	38.6	5.7	91.7	14.9	54.4	8.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.2	38.6	5.7	91.7	14.9	54.4	8.4
Queue Length 50th (ft)	7	280	0	~285	85	282	19
Queue Length 95th (ft)	30	410	75	#650	191	464	106
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	389	1648	908	371	1932	735	867
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.49	0.46	0.98	0.25	0.54	0.41
Intersection Summary							
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.							
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	734	378	332	431	3	363	0	326	0	0
Future Volume (vph)	9	0	734	378	332	431	3	363	0	326	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	10	0	807	415	365	474	3	399	0	358	0	0
RTOR Reduction (vph)	0	0	0	282	0	0	0	0	0	234	0	0
Lane Group Flow (vph)	0	10	807	133	365	477	0	0	399	124	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.6	39.6	39.6	31.0	69.0			34.3	34.3		
Effective Green, g (s)		1.6	39.6	39.6	31.0	69.0			34.3	34.3		
Actuated g/C Ratio		0.01	0.32	0.32	0.25	0.56			0.28	0.28		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		23	1109	465	430	1860			471	438		
v/s Ratio Prot		0.01	c0.23		c0.21	0.14			c0.23			
v/s Ratio Perm				0.09						0.08		
v/c Ratio		0.43	0.73	0.29	0.85	0.26			0.85	0.28		
Uniform Delay, d1		60.7	37.4	31.6	44.2	14.2			42.3	35.2		
Progression Factor		1.00	1.00	1.00	1.00	1.00			1.00	1.00		
Incremental Delay, d2		4.7	2.0	0.1	13.9	0.0			13.2	0.4		
Delay (s)		65.4	39.4	31.7	58.1	14.2			55.5	35.5		
Level of Service		E	D	C	E	B			E	D		
Approach Delay (s)			37.0			33.2			46.1			0.0
Approach LOS			D			C			D			A
Intersection Summary												
HCM 2000 Control Delay			38.3									HCM 2000 Level of Service D
HCM 2000 Volume to Capacity ratio			0.85									
Actuated Cycle Length (s)			123.9						25.0			Sum of lost time (s)
Intersection Capacity Utilization			74.7%									ICU Level of Service D
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
4: US79 & Crossover

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

							
Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	0	1083	0	793	0	0	0
Future Volume (vph)	0	1083	0	793	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		100		0	0	0
Storage Lanes	1		1		0	1	0
Taper Length (ft)	100		100			100	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt							
Flt Protected							
Satd. Flow (prot)	1900	3471	1900	3374	0	1900	0
Flt Permitted							
Satd. Flow (perm)	1900	3471	1900	3374	0	1900	0
Link Speed (mph)		65		65		30	
Link Distance (ft)		739		504		317	
Travel Time (s)		7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	0	1165	0	853	0	0	0
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	1165	0	853	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)		60		60		12	
Link Offset(ft)		0		0		0	
Crosswalk Width(ft)		16		16		16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9		9	15	9
Sign Control		Free		Free		Stop	
Intersection Summary							
Area Type:	Other						
Control Type:	Unsignalized						
Intersection Capacity Utilization	33.3%			ICU Level of Service A			
Analysis Period (min)	15						

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

							
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (veh/h)	0	1083	0	793	0	0	0
Future Volume (Veh/h)	0	1083	0	793	0	0	0
Sign Control		Free		Free		Stop	
Grade		0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	1165	0	853	0	0	0
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		Raised		Raised			
Median storage (veh)		2		2			
Upstream signal (ft)							
pX, platoon unblocked			0.00				
vC, conflicting volume	853		0			1436	426
vC1, stage 1 conf vol						853	
vC2, stage 2 conf vol						582	
vCu, unblocked vol	853		0			1436	426
tC, single (s)	4.1		0.0			6.8	6.9
tC, 2 stage (s)						5.8	
tF (s)	2.2		0.0			3.5	3.3
p0 queue free %	100		0			100	100
cM capacity (veh/h)	795		0			324	582
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1
Volume Total	0	582	582	426	426	0	0
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	0
cSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.00	0.34	0.34	0.25	0.25	0.00	0.00
Queue Length 95th (ft)	0	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							A
Approach Delay (s)	0.0			0.0			0.0
Approach LOS							A
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			33.3%		ICU Level of Service		A
Analysis Period (min)			15				

Intersection							
Int Delay, s/veh	0						
Movement	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Vol, veh/h	0	1083	0	793	0	0	0
Future Vol, veh/h	0	1083	0	793	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	-	None	-	None
Storage Length	100	-	100	-	-	0	-
Veh in Median Storage, #	-	0	-	0	-	0	-
Grade, %	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93
Heavy Vehicles, %	0	4	0	7	0	0	0
Mvmt Flow	0	1165	0	853	0	0	0
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	853	0	1165	-	0	1436	427
Stage 1	-	-	-	-	-	853	-
Stage 2	-	-	-	-	-	583	-
Critical Hdwy	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	795	-	265	-	0	127	582
Stage 1	-	-	-	-	0	383	-
Stage 2	-	-	-	-	0	527	-
Platoon blocked, %		-		-			
Mov Cap-1 Maneuver	795	-	265	-	-	127	582
Mov Cap-2 Maneuver	-	-	-	-	-	127	-
Stage 1	-	-	-	-	-	383	-
Stage 2	-	-	-	-	-	527	-
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		0		
HCM LOS					A		
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1		
Capacity (veh/h)	795	-	265	-	-		
HCM Lane V/C Ratio	-	-	-	-	-		
HCM Control Delay (s)	0	-	0	-	0		
HCM Lane LOS	A	-	A	-	A		
HCM 95th %tile Q(veh)	0	-	0	-	-		

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	3	1092	27	6	751	0	17	3	3	1	7	0
Future Volume (vph)	3	1092	27	6	751	0	17	3	3	1	7	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.984				
Flt Protected	0.950			0.950				0.963			0.994	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1800	0	0	1889	0
Flt Permitted	0.950			0.950				0.963			0.994	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1800	0	0	1889	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	3	1241	31	7	853	0	19	3	3	1	8	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	1241	31	7	853	0	0	25	0	0	9	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	42.0%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	3	1092	27	6	751	0	17	3	3	1	7	0
Future Volume (Veh/h)	3	1092	27	6	751	0	17	3	3	1	7	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	3	1241	31	7	853	0	19	3	3	1	8	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh)	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	853			1241			1692	2114	620	1495	2114	426
vC1, stage 1 conf vol							1247	1247		867	867	
vC2, stage 2 conf vol							444	867		628	1247	
vCu, unblocked vol	853			1241			1692	2114	620	1495	2114	426
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			89	99	99	100	96	100
cM capacity (veh/h)	795			568			175	204	435	259	200	582
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	3	620	620	31	7	426	426	0	25	9		
Volume Left	3	0	0	0	7	0	0	0	19	1		
Volume Right	0	0	0	31	0	0	0	0	3	0		
cSH	795	1700	1700	1700	568	1700	1700	1700	192	205		
Volume to Capacity	0.00	0.36	0.36	0.02	0.01	0.25	0.25	0.00	0.13	0.04		
Queue Length 95th (ft)	0	0	0	0	1	0	0	0	11	3		
Control Delay (s)	9.5	0.0	0.0	0.0	11.4	0.0	0.0	0.0	26.5	23.3		
Lane LOS	A				B				D	C		
Approach Delay (s)	0.0				0.1				26.5	23.3		
Approach LOS									D	C		
Intersection Summary												
Average Delay	0.5											
Intersection Capacity Utilization	42.0%			ICU Level of Service			A					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	3	1092	27	6	751	0	17	3	3	1	7	0
Future Vol, veh/h	3	1092	27	6	751	0	17	3	3	1	7	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	3	1241	31	7	853	0	19	3	3	1	8	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	853	0	0	1241	0	0	1692	2114	-	1495	2114	-
Stage 1	-	-	-	-	-	-	1247	1247	-	867	867	-
Stage 2	-	-	-	-	-	-	445	867	-	628	1247	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	795	-	-	568	-	-	62	51	0	87	51	0
Stage 1	-	-	-	-	-	-	187	247	0	318	373	0
Stage 2	-	-	-	-	-	-	567	373	0	442	247	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	795	-	-	568	-	-	54	50	-	81	50	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	54	50	-	81	50	-
Stage 1	-	-	-	-	-	-	186	246	-	317	369	-
Stage 2	-	-	-	-	-	-	548	369	-	434	246	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	795	-	-	568	-	-	-
HCM Lane V/C Ratio	-	0.004	-	-	0.012	-	-	-
HCM Control Delay (s)	-	9.5	-	-	11.4	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	18	676	4	4	727
Future Volume (vph)	8	18	676	4	4	727
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.905		0.999			
Flt Protected	0.985				0.950	
Satd. Flow (prot)	1694	0	1825	0	1805	1712
Flt Permitted	0.985				0.950	
Satd. Flow (perm)	1694	0	1825	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	19	697	4	4	749
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	0	701	0	4	749
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	48.3%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	18	676	4	4	727
Future Volume (Veh/h)	8	18	676	4	4	727
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	19	697	4	4	749
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1456	699			701	
vC1, stage 1 conf vol	699					
vC2, stage 2 conf vol	757					
vCu, unblocked vol	1456	699			701	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	96			100	
cM capacity (veh/h)	359	443			905	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	27	701	4	749		
Volume Left	8	0	4	0		
Volume Right	19	4	0	0		
cSH	414	1700	905	1700		
Volume to Capacity	0.07	0.41	0.00	0.44		
Queue Length 95th (ft)	5	0	0	0		
Control Delay (s)	14.3	0.0	9.0	0.0		
Lane LOS	B		A			
Approach Delay (s)	14.3	0.0	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			48.3%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	18	676	4	4	727
Future Vol, veh/h	8	18	676	4	4	727
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	19	697	4	4	749
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1456	699	0	0	701	0
Stage 1	699	-	-	-	-	-
Stage 2	757	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	144	443	-	-	905	-
Stage 1	497	-	-	-	-	-
Stage 2	467	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	143	443	-	-	905	-
Mov Cap-2 Maneuver	284	-	-	-	-	-
Stage 1	497	-	-	-	-	-
Stage 2	465	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15.2	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	378	905	-	
HCM Lane V/C Ratio	-	-	0.071	0.005	-	
HCM Control Delay (s)	-	-	15.2	9	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	0.2	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	52	22	38	622	650	89
Future Volume (vph)	52	22	38	622	650	89
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.960					0.850
Flt Protected	0.966		0.950			
Satd. Flow (prot)	1762	0	1805	1827	1696	1615
Flt Permitted	0.966		0.950			
Satd. Flow (perm)	1762	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	57	24	41	676	707	97
Shared Lane Traffic (%)						
Lane Group Flow (vph)	81	0	41	676	707	97
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	45.1%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	52	22	38	622	650	89
Future Volume (Veh/h)	52	22	38	622	650	89
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	57	24	41	676	707	97
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1465	707	804			
vC1, stage 1 conf vol	707					
vC2, stage 2 conf vol	758					
vCu, unblocked vol	1465	707	804			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	84	95	95			
cM capacity (veh/h)	347	439	829			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	81	41	676	707	97	
Volume Left	57	41	0	0	0	
Volume Right	24	0	0	0	97	
cSH	370	829	1700	1700	1700	
Volume to Capacity	0.22	0.05	0.40	0.42	0.06	
Queue Length 95th (ft)	21	4	0	0	0	
Control Delay (s)	17.4	9.6	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	17.4	0.5		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			45.1%	ICU Level of Service		A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	52	22	38	622	650	89
Future Vol, veh/h	52	22	38	622	650	89
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	57	24	41	676	707	97
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1465	707	804	0	-	0
Stage 1	707	-	-	-	-	-
Stage 2	758	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	143	439	829	-	-	-
Stage 1	493	-	-	-	-	-
Stage 2	466	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	136	439	829	-	-	-
Mov Cap-2 Maneuver	275	-	-	-	-	-
Stage 1	469	-	-	-	-	-
Stage 2	466	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	20.7	0.6		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	829	-	309	-	-	
HCM Lane V/C Ratio	0.05	-	0.26	-	-	
HCM Control Delay (s)	9.6	-	20.7	-	-	
HCM Lane LOS	A	-	C	-	-	
HCM 95th %tile Q(veh)	0.2	-	1	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	6	0	4	5	0	15	4	635	0	3	655	11
Future Volume (vph)	6	0	4	5	0	15	4	635	0	3	655	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.951			0.897						0.998	
Flt Protected		0.969			0.988		0.950			0.950		
Satd. Flow (prot)	0	1751	0	0	1684	0	1805	1827	0	1805	1711	0
Flt Permitted		0.969			0.988		0.950			0.950		
Satd. Flow (perm)	0	1751	0	0	1684	0	1805	1827	0	1805	1711	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	7	0	4	5	0	16	4	690	0	3	712	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	11	0	0	21	0	4	690	0	3	724	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.1%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 PM Peak Hour
Phase 2 Build-Out Year w/o Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	6	0	4	5	0	15	4	635	0	3	655	11
Future Volume (Veh/h)	6	0	4	5	0	15	4	635	0	3	655	11
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	0	4	5	0	16	4	690	0	3	712	12
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLTL						TWLTL					
Median storage veh	2						2					
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1438	1422	718	1420	1428	690	724				690	
vC1, stage 1 conf vol	724	724		698	698							
vC2, stage 2 conf vol	714	698		722	730							
vCu, unblocked vol	1438	1422	718	1420	1428	690	724				690	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	98	100	99	98	100	96	100				100	
cM capacity (veh/h)	303	328	432	311	326	449	888				914	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	11	21	4	690	3	724						
Volume Left	7	5	4	0	3	0						
Volume Right	4	16	0	0	0	12						
cSH	340	406	888	1700	914	1700						
Volume to Capacity	0.03	0.05	0.00	0.41	0.00	0.43						
Queue Length 95th (ft)	3	4	0	0	0	0						
Control Delay (s)	15.9	14.4	9.1	0.0	9.0	0.0						
Lane LOS	C	B	A		A							
Approach Delay (s)	15.9	14.4	0.1		0.0							
Approach LOS	C	B										
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	45.1%			ICU Level of Service					A			
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	6	0	4	5	0	15	4	635	0	3	655	11
Future Vol, veh/h	6	0	4	5	0	15	4	635	0	3	655	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	11	0
Mvmt Flow	7	0	4	5	0	16	4	690	0	3	712	12

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1430	1422	718	1424	1428	690	724	0	0	690	0	0
Stage 1	724	724	-	698	698	-	-	-	-	-	-	-
Stage 2	706	698	-	726	730	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	113	137	432	115	136	449	888	-	-	914	-	-
Stage 1	420	433	-	434	445	-	-	-	-	-	-	-
Stage 2	430	445	-	419	431	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	108	136	432	113	135	449	888	-	-	914	-	-
Mov Cap-2 Maneuver	108	136	-	113	135	-	-	-	-	-	-	-
Stage 1	418	432	-	432	443	-	-	-	-	-	-	-
Stage 2	413	443	-	413	430	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	30.1		20.2		0.1		0	
HCM LOS	D		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	888	-	-	154 258	914	-	-
HCM Lane V/C Ratio	0.005	-	-	0.071 0.084	0.004	-	-
HCM Control Delay (s)	9.1	-	-	30.1 20.2	9	-	-
HCM Lane LOS	A	-	-	D C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.2 0.3	0	-	-

Appendix D. Traffic Simulation Output

- Phase 2 Buildout Year (2027) Projected Conditions with Development, AM & PM
 - No Mitigation (Existing Timings at FM 973 at US 79)

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				753						143		267
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	600	833	528	709	0	0	595	218	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.1	39.3	39.3	25.2	67.8			50.4	50.4		5.0
Actuated g/C Ratio		0.07	0.29	0.29	0.19	0.50			0.37	0.37		0.04
v/c Ratio		0.07	0.63	0.85	1.66	0.42			0.96	0.32		0.00
Control Delay		66.2	44.8	14.3	343.7	22.8			70.4	13.8		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		66.2	44.8	14.3	343.7	22.8			70.4	13.8		0.0
LOS		E	D	B	F	C			E	B		A
Approach Delay			27.3			159.8			55.2			
Approach LOS			C			F			E			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 136

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.66

Intersection Signal Delay: 80.7

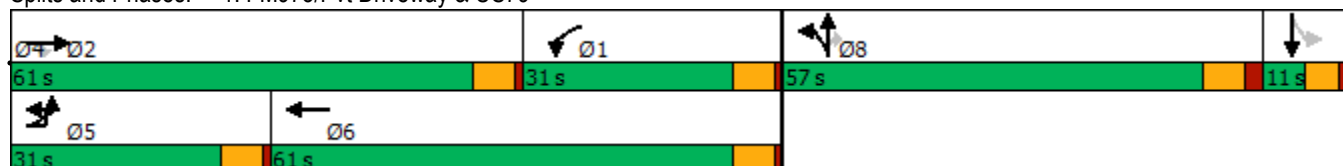
Intersection LOS: F

Intersection Capacity Utilization 96.5%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	45.5	45.5	25.0	76.5	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	38.8	38.8	25.0	69.8	50.0	50.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	33.2	33.2	25.0	64.2	50.0	50.0	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	26.7	26.7	25.0	57.7	50.0	50.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 136								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 160								
70th %ile Actuated Cycle: 139.5								
50th %ile Actuated Cycle: 132.8								
30th %ile Actuated Cycle: 127.2								
10th %ile Actuated Cycle: 120.7								

Queues
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	9	600	833	528	709	595	218	1
v/c Ratio	0.07	0.63	0.85	1.66	0.42	0.96	0.32	0.00
Control Delay	66.2	44.8	14.3	343.7	22.8	70.4	13.8	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.2	44.8	14.3	343.7	22.8	70.4	13.8	0.0
Queue Length 50th (ft)	7	235	52	~660	181	493	42	0
Queue Length 95th (ft)	30	328	273	#1097	324	#966	133	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	334	1351	1079	318	1681	619	671	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.44	0.77	1.66	0.42	0.96	0.32	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
RTOR Reduction (vph)	0	0	0	550	0	0	0	0	0	94	0	1
Lane Group Flow (vph)	0	9	600	283	528	709	0	0	595	124	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.7	39.3	39.3	30.3	67.9			50.5	50.5		0.9
Effective Green, g (s)		1.7	39.3	39.3	30.3	67.9			50.5	50.5		0.9
Actuated g/C Ratio		0.01	0.27	0.27	0.21	0.47			0.35	0.35		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		21	891	418	356	1568			577	542		10
v/s Ratio Prot		0.00	0.18		c0.31	0.21			c0.36			c0.00
v/s Ratio Perm				c0.18						0.08		
v/c Ratio		0.43	0.67	0.68	1.48	0.45			1.03	0.23		0.00
Uniform Delay, d1		71.7	47.6	47.7	57.9	26.5			47.8	33.9		72.1
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		1.00
Incremental Delay, d2		5.0	1.6	3.4	232.0	0.1			45.7	0.2		0.0
Delay (s)		76.7	49.2	51.1	288.7	25.8			93.5	34.1		72.1
Level of Service		E	D	D	F	C			F	C		E
Approach Delay (s)			50.5			138.0			77.6			72.1
Approach LOS			D			F			E			E
Intersection Summary												
HCM 2000 Control Delay			87.8			HCM 2000 Level of Service			F			
HCM 2000 Volume to Capacity ratio			1.02									
Actuated Cycle Length (s)			146.0			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			96.5%			ICU Level of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	547	284	0	1253	0	207
Future Volume (vph)	547	284	0	1253	0	207
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3282	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3282	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1072			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	0%	0%	7%	0%	0%
Adj. Flow (vph)	595	309	0	1362	0	225
Shared Lane Traffic (%)						
Lane Group Flow (vph)	595	309	0	1362	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	38.0%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	547	284	0	1253	0	207
Future Volume (Veh/h)	547	284	0	1253	0	207
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	595	309	0	1362	0	225
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1072					
pX, platoon unblocked			0.89		0.89	0.89
vC, conflicting volume			904		1276	298
vC1, stage 1 conf vol					595	
vC2, stage 2 conf vol					681	
vCu, unblocked vol			654		1071	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	77
cM capacity (veh/h)			842		409	975
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	298	298	309	681	681	225
Volume Left	0	0	0	0	0	0
Volume Right	0	0	309	0	0	225
cSH	1700	1700	1700	1700	1700	975
Volume to Capacity	0.17	0.17	0.18	0.40	0.40	0.23
Queue Length 95th (ft)	0	0	0	0	0	22
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.8
Lane LOS						A
Approach Delay (s)	0.0				0.0	9.8
Approach LOS						A
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			38.0%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	547	284	0	1253	0	207
Future Vol, veh/h	547	284	0	1253	0	207
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	0	0	7	0	0
Mvmt Flow	595	309	0	1362	0	225
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	298
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	704
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	704
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.5	
HCM LOS					B	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	704	-	-	-		
HCM Lane V/C Ratio	0.32	-	-	-		
HCM Control Delay (s)	12.5	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	1.4	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	75	0	682	0	1207	0	0	0
Future Volume (vph)	75	0	682	0	1207	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	82	0	749	0	1326	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	82	749	0	1326	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	44.2%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	75	0	682	0	1207	0	0	0
Future Volume (Veh/h)	75	0	682	0	1207	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	749	0	1326	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage (veh)			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1326		0			1700	663
vC1, stage 1 conf vol							1326	
vC2, stage 2 conf vol							374	
vCu, unblocked vol	0	1326		0			1700	663
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	527		0			206	409
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	374	374	663	663	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.22	0.22	0.39	0.39	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			44.2%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	1.3							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	75	0	682	0	1207	0	0	0
Future Vol, veh/h	75	0	682	0	1207	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	82	0	749	0	1326	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	1326	1326	0	749	-	0	1865	663
Stage 1	-	-	-	-	-	-	1326	-
Stage 2	-	-	-	-	-	-	539	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	209	527	-	488	-	0	66	409
Stage 1	-	-	-	-	-	0	216	-
Stage 2	-	-	-	-	-	0	555	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	209	209	-	488	-	-	40	409
Mov Cap-2 Maneuver	-	-	-	-	-	-	40	-
Stage 1	-	-	-	-	-	-	131	-
Stage 2	-	-	-	-	-	-	555	-
Approach	EB		WB		SB			
HCM Control Delay, s	3.3		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	209	-	488	-	-			
HCM Lane V/C Ratio	0.394	-	-	-	-			
HCM Control Delay (s)	33	-	0	-	0			
HCM Lane LOS	D	-	A	-	A			
HCM 95th %tile Q(veh)	1.8	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	8	704	12	7	1144	1	36	7	1	3	3	5
Future Volume (vph)	8	704	12	7	1144	1	36	7	1	3	3	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.932	
Flt Protected	0.950			0.950				0.961			0.988	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Flt Permitted	0.950			0.950				0.961			0.988	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	9	800	14	8	1300	1	41	8	1	3	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	9	800	14	8	1300	1	0	50	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	44.4%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	8	704	12	7	1144	1	36	7	1	3	3	5
Future Volume (Veh/h)	8	704	12	7	1144	1	36	7	1	3	3	5
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	9	800	14	8	1300	1	41	8	1	3	3	6
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1300			800			1486	2134	400	1738	2134	650
vC1, stage 1 conf vol							818	818		1316	1316	
vC2, stage 2 conf vol							668	1316		422	818	
vCu, unblocked vol	1300			800			1486	2134	400	1738	2134	650
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			99			84	96	100	98	98	99
cM capacity (veh/h)	540			832			262	189	605	159	194	417
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	9	400	400	14	8	650	650	1	50	12		
Volume Left	9	0	0	0	8	0	0	0	41	3		
Volume Right	0	0	0	14	0	0	0	1	1	6		
cSH	540	1700	1700	1700	832	1700	1700	1700	249	246		
Volume to Capacity	0.02	0.24	0.24	0.01	0.01	0.38	0.38	0.00	0.20	0.05		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	18	4		
Control Delay (s)	11.8	0.0	0.0	0.0	9.4	0.0	0.0	0.0	23.0	20.4		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				23.0	20.4		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.7											
Intersection Capacity Utilization	44.4%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	8	704	12	7	1144	1	36	7	1	3	3	5
Future Vol, veh/h	8	704	12	7	1144	1	36	7	1	3	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	9	800	14	8	1300	1	41	8	1	3	3	6

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1300	0	0	800	0	0	1486	2134	-	1738	2134	-
Stage 1	-	-	-	-	-	-	818	818	-	1316	1316	-
Stage 2	-	-	-	-	-	-	668	1316	-	422	818	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	540	-	-	832	-	-	88	50	0	57	50	0
Stage 1	-	-	-	-	-	-	340	393	0	169	229	0
Stage 2	-	-	-	-	-	-	419	229	0	585	393	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	540	-	-	832	-	-	82	49	-	49	49	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	82	49	-	49	49	-
Stage 1	-	-	-	-	-	-	334	386	-	166	227	-
Stage 2	-	-	-	-	-	-	409	227	-	563	386	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	540	-	-	832	-	-	-
HCM Lane V/C Ratio	-	0.017	-	-	0.01	-	-	-
HCM Control Delay (s)	-	11.8	-	-	9.4	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	78	686	105	0	1306
Future Volume (vph)	0	78	686	105	0	1306
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1759	1615	0	1792
Flt Permitted						
Satd. Flow (perm)	0	1644	1759	1615	0	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	0	85	746	114	0	1420
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	85	746	114	0	1420
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	72.1%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

6: FM973 & Driveway 1B

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	78	686	105	0	1306
Future Volume (Veh/h)	0	78	686	105	0	1306
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	85	746	114	0	1420
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)					636	
pX, platoon unblocked						
vC, conflicting volume	2166	746			860	
vC1, stage 1 conf vol	746					
vC2, stage 2 conf vol	1420					
vCu, unblocked vol	2166	746			860	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	80			100	
cM capacity (veh/h)	202	417			790	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	85	746	114	1420		
Volume Left	0	0	0	0		
Volume Right	85	0	114	0		
cSH	417	1700	1700	1700		
Volume to Capacity	0.20	0.44	0.07	0.84		
Queue Length 95th (ft)	19	0	0	0		
Control Delay (s)	15.8	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	15.8	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			72.1%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	78	686	105	0	1306
Future Vol, veh/h	0	78	686	105	0	1306
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	0	85	746	114	0	1420
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	746	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	417	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	417	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15.8	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	417		-		
HCM Lane V/C Ratio	-	0.203		-		
HCM Control Delay (s)	-	15.8		-		
HCM Lane LOS	-	C		-		
HCM 95th %tile Q(veh)	-	0.8		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	155	53	737	26	112	1195
Future Volume (vph)	155	53	737	26	112	1195
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	168	58	801	28	122	1299
Shared Lane Traffic (%)						
Lane Group Flow (vph)	168	58	801	28	122	1299
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	78.1%			ICU Level of Service D		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	155	53	737	26	112	1195
Future Volume (Veh/h)	155	53	737	26	112	1195
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	168	58	801	28	122	1299
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		4				
Median type			TWLT			TWLT
Median storage veh)			2			2
Upstream signal (ft)						1181
pX, platoon unblocked						
vC, conflicting volume	2344	801			829	
vC1, stage 1 conf vol	801					
vC2, stage 2 conf vol	1543					
vCu, unblocked vol	2344	801			829	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	0	85			85	
cM capacity (veh/h)	153	388			811	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	226	801	28	122	1299	
Volume Left	168	0	0	122	0	
Volume Right	58	0	28	0	0	
cSH	194	1700	1700	811	1700	
Volume to Capacity	1.17	0.47	0.02	0.15	0.76	
Queue Length 95th (ft)	286	0	0	13	0	
Control Delay (s)	166.2	0.0	0.0	10.2	0.0	
Lane LOS	F			B		
Approach Delay (s)	166.2	0.0		0.9		
Approach LOS	F					
Intersection Summary						
Average Delay			15.7			
Intersection Capacity Utilization			78.1%		ICU Level of Service	D
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	19.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	155	53	737	26	112	1195
Future Vol, veh/h	155	53	737	26	112	1195
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	250	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	168	58	801	28	122	1299
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2344	801	0	0	829	0
Stage 1	801	-	-	-	-	-
Stage 2	1543	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	~ 40	388	-	-	811	-
Stage 1	445	-	-	-	-	-
Stage 2	196	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	~ 34	388	-	-	811	-
Mov Cap-2 Maneuver	~ 123	-	-	-	-	-
Stage 1	445	-	-	-	-	-
Stage 2	~ 167	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	209.4	0		0.9		
HCM LOS	F					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	123	388	811	-
HCM Lane V/C Ratio	-	-	1.37	0.148	0.15	-
HCM Control Delay (s)	-	-	275.5	15.9	10.2	-
HCM Lane LOS	-	-	F	C	B	-
HCM 95th %tile Q(veh)	-	-	11.3	0.5	0.5	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	97	684	12	227	1133
Future Volume (vph)	6	97	684	12	227	1133
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.998			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1654	0	1758	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1654	0	1758	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	114	805	14	267	1333
Shared Lane Traffic (%)						
Lane Group Flow (vph)	121	0	819	0	267	1333
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	72.6%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	97	684	12	227	1133
Future Volume (Veh/h)	6	97	684	12	227	1133
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	114	805	14	267	1333
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2679	812			819	
vC1, stage 1 conf vol	812					
vC2, stage 2 conf vol	1867					
vCu, unblocked vol	2679	812			819	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	70			67	
cM capacity (veh/h)	87	382			818	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	121	819	267	1333		
Volume Left	7	0	267	0		
Volume Right	114	14	0	0		
cSH	320	1700	818	1700		
Volume to Capacity	0.38	0.48	0.33	0.78		
Queue Length 95th (ft)	43	0	36	0		
Control Delay (s)	23.0	0.0	11.5	0.0		
Lane LOS	C		B			
Approach Delay (s)	23.0	0.0	1.9			
Approach LOS	C					
Intersection Summary						
Average Delay		2.3				
Intersection Capacity Utilization		72.6%		ICU Level of Service		C
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	2.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	97	684	12	227	1133
Future Vol, veh/h	6	97	684	12	227	1133
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	114	805	14	267	1333
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2679	812	0	0	819	0
Stage 1	812	-	-	-	-	-
Stage 2	1867	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	25	382	-	-	818	-
Stage 1	440	-	-	-	-	-
Stage 2	136	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	17	382	-	-	818	-
Mov Cap-2 Maneuver	74	-	-	-	-	-
Stage 1	440	-	-	-	-	-
Stage 2	92	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	24.2	0		1.9		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	307	818	-	
HCM Lane V/C Ratio	-	-	0.395	0.326	-	
HCM Control Delay (s)	-	-	24.2	11.5	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.8	1.4	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	94	33	12	601	1095	42
Future Volume (vph)	94	33	12	601	1095	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.965					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1767	0	1805	1759	1810	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1767	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	104	37	13	668	1217	47
Shared Lane Traffic (%)						
Lane Group Flow (vph)	141	0	13	668	1217	47
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 71.5%				ICU Level of Service C		
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	94	33	12	601	1095	42
Future Volume (Veh/h)	94	33	12	601	1095	42
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	104	37	13	668	1217	47
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage (veh)				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1911	1217	1264			
vC1, stage 1 conf vol	1217					
vC2, stage 2 conf vol	694					
vCu, unblocked vol	1911	1217	1264			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	58	83	98			
cM capacity (veh/h)	247	223	557			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	141	13	668	1217	47	
Volume Left	104	13	0	0	0	
Volume Right	37	0	0	0	47	
cSH	240	557	1700	1700	1700	
Volume to Capacity	0.59	0.02	0.39	0.72	0.03	
Queue Length 95th (ft)	84	2	0	0	0	
Control Delay (s)	39.4	11.6	0.0	0.0	0.0	
Lane LOS	E	B				
Approach Delay (s)	39.4	0.2		0.0		
Approach LOS	E					
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utilization			71.5%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	94	33	12	601	1095	42
Future Vol, veh/h	94	33	12	601	1095	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	104	37	13	668	1217	47
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1911	1217	1264	0	-	0
Stage 1	1217	-	-	-	-	-
Stage 2	694	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 76	223	557	-	-	-
Stage 1	283	-	-	-	-	-
Stage 2	499	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 74	223	557	-	-	-
Mov Cap-2 Maneuver	193	-	-	-	-	-
Stage 1	276	-	-	-	-	-
Stage 2	499	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	57.3		0.2		0	
HCM LOS	F					
Minor Lane/Major Mvmt	NBL		NBT	EBLn1	SBT	SBR
Capacity (veh/h)	557		-	200	-	-
HCM Lane V/C Ratio	0.024		-	0.706	-	-
HCM Control Delay (s)	11.6		-	57.3	-	-
HCM Lane LOS	B		-	F	-	-
HCM 95th %tile Q(veh)	0.1		-	4.5	-	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Future Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.959			0.877			0.986			0.999	
Flt Protected		0.966			0.995		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1658	0	1805	1676	0	1805	1711	0
Flt Permitted		0.966			0.995		0.950			0.950		
Satd. Flow (perm)	0	1760	0	0	1658	0	1805	1676	0	1805	1711	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	14	0	6	22	0	219	3	464	49	435	842	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	241	0	3	513	0	435	849	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	68.0%						ICU Level of Service C					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 AM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	12	0	5	19	0	193	3	408	43	383	741	6
Future Volume (Veh/h)	12	0	5	19	0	193	3	408	43	383	741	6
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	14	0	6	22	0	219	3	464	49	435	842	7
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							TWLTL			TWLTL		
Median storage (veh)							2			2		
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2404	2234	846	2212	2214	488	849			513		
vC1, stage 1 conf vol	1716	1716		494	494							
vC2, stage 2 conf vol	689	519		1718	1719							
vCu, unblocked vol	2404	2234	846	2212	2214	488	849			513		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	100	98	65	100	62	100			59		
cM capacity (veh/h)	10	68	365	62	81	583	798			1063		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	20	241	3	513	435	849						
Volume Left	14	22	3	0	435	0						
Volume Right	6	219	0	49	0	7						
cSH	14	330	798	1700	1063	1700						
Volume to Capacity	1.44	0.73	0.00	0.30	0.41	0.50						
Queue Length 95th (ft)	79	136	0	0	51	0						
Control Delay (s)	784.2	40.3	9.5	0.0	10.7	0.0						
Lane LOS	F	E	A		B							
Approach Delay (s)	784.2	40.3	0.1		3.6							
Approach LOS	F	E										
Intersection Summary												
Average Delay			14.6									
Intersection Capacity Utilization			68.0%		ICU Level of Service		C					
Analysis Period (min)			15									

Intersection												
Int Delay, s/veh	39.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	12	0	5	19	0	193	3	408	43	383	741	6
Future Vol, veh/h	12	0	5	19	0	193	3	408	43	383	741	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	11	0
Mvmt Flow	14	0	6	22	0	219	3	464	49	435	842	7

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2320	2235	846	2214	2214	489	849	0	0	513	0	0
Stage 1	1716	1716	-	495	495	-	-	-	-	-	-	-
Stage 2	604	519	-	1719	1719	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	27	43	365	32	44	583	798	-	-	1063	-	-
Stage 1	115	146	-	560	549	-	-	-	-	-	-	-
Stage 2	489	536	-	115	146	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	~ 11	25	365	~ 21	26	583	798	-	-	1063	-	-
Mov Cap-2 Maneuver	~ 11	25	-	~ 21	26	-	-	-	-	-	-	-
Stage 1	115	86	-	558	547	-	-	-	-	-	-	-
Stage 2	304	534	-	67	86	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	688.3	262.1	0.1	3.6
HCM LOS	F	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	798	-	-	15 172	1063	-	-
HCM Lane V/C Ratio	0.004	-	-	1.288 1.401	0.409	-	-
HCM Control Delay (s)	9.5	-	-	\$ 688.3 262.1	10.7	-	-
HCM Lane LOS	A	-	-	F F	B	-	-
HCM 95th %tile Q(veh)	0	-	-	3 14.7	2	-	-

Notes			
~: Volume exceeds capacity	\$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				415						254		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	954	415	380	527	0	0	470	332	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

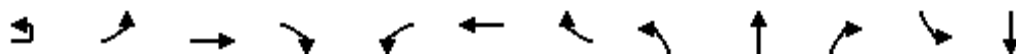
Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases					2					8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.2	47.9	47.9	25.4	73.6			43.1	43.1		
Actuated g/C Ratio		0.08	0.35	0.35	0.19	0.54			0.32	0.32		
v/c Ratio		0.07	0.78	0.53	1.18	0.29			0.87	0.49		
Control Delay		66.2	44.6	5.5	156.3	18.9			61.8	12.0		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		66.2	44.6	5.5	156.3	18.9			61.8	12.0		
LOS		E	D	A	F	B			E	B		
Approach Delay			33.0			76.5			41.2			
Approach LOS			C			E			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 135.7

Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 47.9

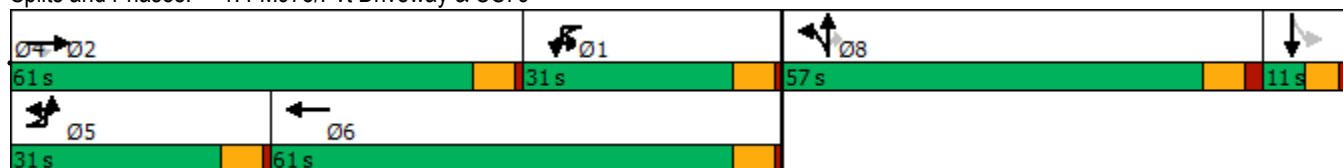
Intersection LOS: D

Intersection Capacity Utilization 82.7%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79





Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
70th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
70th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	53.0	53.0	25.0	84.0	47.8	47.8	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	44.5	44.5	25.0	75.5	40.1	40.1	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	33.7	33.7	25.0	64.7	29.5	29.5	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 135.7								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 149								
70th %ile Actuated Cycle: 149								
50th %ile Actuated Cycle: 144.8								
30th %ile Actuated Cycle: 128.6								
10th %ile Actuated Cycle: 107.2								

Queues
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	10	954	415	380	527	470	332
v/c Ratio	0.07	0.78	0.53	1.18	0.29	0.87	0.49
Control Delay	66.2	44.6	5.5	156.3	18.9	61.8	12.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.2	44.6	5.5	156.3	18.9	61.8	12.0
Queue Length 50th (ft)	9	412	0	~454	121	410	52
Queue Length 95th (ft)	30	506	75	#689	205	#595	143
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	337	1429	843	321	1811	637	751
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.67	0.49	1.18	0.29	0.74	0.44
Intersection Summary							
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.							
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
RTOR Reduction (vph)	0	0	0	272	0	0	0	0	0	175	0	0
Lane Group Flow (vph)	0	10	954	143	380	527	0	0	470	157	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		3.6	47.9	47.9	29.3	73.6			43.1	43.1		
Effective Green, g (s)		3.6	47.9	47.9	29.3	73.6			43.1	43.1		
Actuated g/C Ratio		0.03	0.34	0.34	0.21	0.53			0.31	0.31		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		46	1193	500	361	1765			526	489		
v/s Ratio Prot		0.01	c0.27		c0.22	0.16			c0.28			
v/s Ratio Perm				0.10						0.10		
v/c Ratio		0.22	0.80	0.29	1.05	0.30			0.89	0.32		
Uniform Delay, d1		66.5	41.4	33.2	55.0	18.4			45.9	36.9		
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		
Incremental Delay, d2		0.9	3.6	0.1	61.9	0.0			17.4	0.4		
Delay (s)		67.3	44.9	33.4	115.7	17.9			63.3	37.3		
Level of Service		E	D	C	F	B			E	D		
Approach Delay (s)			41.6			58.9			52.5			0.0
Approach LOS			D			E			D			A
Intersection Summary												
HCM 2000 Control Delay			49.5			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.94									
Actuated Cycle Length (s)			139.3			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			82.7%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	952	267	0	894	0	225
Future Volume (vph)	952	267	0	894	0	225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3471	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3471	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1070			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	0%	7%	0%	0%
Adj. Flow (vph)	1035	290	0	972	0	245
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1035	290	0	972	0	245
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 46.9%						
ICU Level of Service A						
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis

3: Driveway 2A & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	952	267	0	894	0	225
Future Volume (Veh/h)	952	267	0	894	0	225
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1035	290	0	972	0	245
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1070					
pX, platoon unblocked			0.77		0.77	0.77
vC, conflicting volume			1325		1521	518
vC1, stage 1 conf vol					1035	
vC2, stage 2 conf vol					486	
vCu, unblocked vol			833		1086	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	71
cM capacity (veh/h)			625		391	843
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	518	518	290	486	486	245
Volume Left	0	0	0	0	0	0
Volume Right	0	0	290	0	0	245
cSH	1700	1700	1700	1700	1700	843
Volume to Capacity	0.30	0.30	0.17	0.29	0.29	0.29
Queue Length 95th (ft)	0	0	0	0	0	30
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	11.0
Lane LOS						B
Approach Delay (s)	0.0				0.0	11.0
Approach LOS						B
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			46.9%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	952	267	0	894	0	225
Future Vol, veh/h	952	267	0	894	0	225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	0	7	0	0
Mvmt Flow	1035	290	0	972	0	245
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	518
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	508
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	508
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		18.5	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	508	-	-	-		
HCM Lane V/C Ratio	0.481	-	-	-		
HCM Control Delay (s)	18.5	-	-	-		
HCM Lane LOS	C	-	-	-		
HCM 95th %tile Q(veh)	2.6	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	50	0	1135	0	855	0	0	0
Future Volume (vph)	50	0	1135	0	855	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3471	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3471	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	54	0	1220	0	919	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	54	1220	0	919	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	41.4%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↕↕	↔	↕↕		↕↕	
Traffic Volume (veh/h)	50	0	1135	0	855	0	0	0
Future Volume (Veh/h)	50	0	1135	0	855	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	1220	0	919	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage (veh)			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	919		0			1529	460
vC1, stage 1 conf vol							919	
vC2, stage 2 conf vol							610	
vCu, unblocked vol	0	919		0			1529	460
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	751		0			301	554
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	610	610	460	460	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.36	0.36	0.27	0.27	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.4%		ICU Level of Service		A	
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.4							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	50	0	1135	0	855	0	0	0
Future Vol, veh/h	50	0	1135	0	855	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	0	7	0	0	0
Mvmt Flow	54	0	1220	0	919	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	919	919	0	1220	-	0	1637	460
Stage 1	-	-	-	-	-	-	919	-
Stage 2	-	-	-	-	-	-	718	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	380	751	-	244	-	0	93	554
Stage 1	-	-	-	-	-	0	354	-
Stage 2	-	-	-	-	-	0	449	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	380	380	-	244	-	-	80	554
Mov Cap-2 Maneuver	-	-	-	-	-	-	80	-
Stage 1	-	-	-	-	-	-	304	-
Stage 2	-	-	-	-	-	-	449	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.7		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	380	-	244	-	-			
HCM Lane V/C Ratio	0.141	-	-	-	-			
HCM Control Delay (s)	16	-	0	-	0			
HCM Lane LOS	C	-	A	-	A			
HCM 95th %tile Q(veh)	0.5	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	5	1137	31	6	804	0	22	3	3	1	7	3
Future Volume (vph)	5	1137	31	6	804	0	22	3	3	1	7	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.987			0.966	
Flt Protected	0.950			0.950				0.961			0.996	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1828	0
Flt Permitted	0.950			0.950				0.961			0.996	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1828	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	6	1292	35	7	914	0	25	3	3	1	8	3
Shared Lane Traffic (%)												
Lane Group Flow (vph)	6	1292	35	7	914	0	0	31	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	46.1%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	1137	31	6	804	0	22	3	3	1	7	3
Future Volume (Veh/h)	5	1137	31	6	804	0	22	3	3	1	7	3
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	1292	35	7	914	0	25	3	3	1	8	3
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	914			1292			1779	2232	646	1588	2232	457
vC1, stage 1 conf vol							1304	1304		928	928	
vC2, stage 2 conf vol							475	928		660	1304	
vCu, unblocked vol	914			1292			1779	2232	646	1588	2232	457
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			84	98	99	100	96	99
cM capacity (veh/h)	754			543			161	189	419	238	186	556
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	6	646	646	35	7	457	457	0	31	12		
Volume Left	6	0	0	0	7	0	0	0	25	1		
Volume Right	0	0	0	35	0	0	0	0	3	3		
cSH	754	1700	1700	1700	543	1700	1700	1700	174	228		
Volume to Capacity	0.01	0.38	0.38	0.02	0.01	0.27	0.27	0.00	0.18	0.05		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	16	4		
Control Delay (s)	9.8	0.0	0.0	0.0	11.7	0.0	0.0	0.0	30.2	21.7		
Lane LOS	A				B				D	C		
Approach Delay (s)	0.0				0.1				30.2	21.7		
Approach LOS									D	C		
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	46.1%			ICU Level of Service			A					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	5	1137	31	6	804	0	22	3	3	1	7	3
Future Vol, veh/h	5	1137	31	6	804	0	22	3	3	1	7	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	6	1292	35	7	914	0	25	3	3	1	8	3

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	914	0	0	1292	0	0	1779	2232	-	1588	2232	-
Stage 1	-	-	-	-	-	-	1304	1304	-	928	928	-
Stage 2	-	-	-	-	-	-	475	928	-	660	1304	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	754	-	-	543	-	-	53	43	0	74	43	0
Stage 1	-	-	-	-	-	-	172	232	0	292	349	0
Stage 2	-	-	-	-	-	-	545	349	0	423	232	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	754	-	-	543	-	-	45	42	-	68	42	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	45	42	-	68	42	-
Stage 1	-	-	-	-	-	-	171	230	-	290	344	-
Stage 2	-	-	-	-	-	-	526	344	-	413	230	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	754	-	-	543	-	-	-
HCM Lane V/C Ratio	-	0.008	-	-	0.013	-	-	-
HCM Control Delay (s)	-	9.8	-	-	11.7	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	67	663	116	0	743
Future Volume (vph)	0	67	663	116	0	743
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1827	1615	0	1712
Flt Permitted						
Satd. Flow (perm)	0	1644	1827	1615	0	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	0	73	721	126	0	808
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	73	721	126	0	808
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	45.7%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

6: FM973 & Driveway 1B

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	67	663	116	0	743
Future Volume (Veh/h)	0	67	663	116	0	743
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	73	721	126	0	808
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)					636	
pX, platoon unblocked						
vC, conflicting volume	1529	721			847	
vC1, stage 1 conf vol	721					
vC2, stage 2 conf vol	808					
vCu, unblocked vol	1529	721			847	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	83			100	
cM capacity (veh/h)	343	431			799	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	73	721	126	808		
Volume Left	0	0	0	0		
Volume Right	73	0	126	0		
cSH	431	1700	1700	1700		
Volume to Capacity	0.17	0.42	0.07	0.48		
Queue Length 95th (ft)	15	0	0	0		
Control Delay (s)	15.1	0.0	0.0	0.0		
Lane LOS	C					
Approach Delay (s)	15.1	0.0		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay		0.6				
Intersection Capacity Utilization		45.7%		ICU Level of Service		A
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	67	663	116	0	743
Future Vol, veh/h	0	67	663	116	0	743
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	0	73	721	126	0	808
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	721	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	431	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	431	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	- 431		-		
HCM Lane V/C Ratio	-	- 0.169		-		
HCM Control Delay (s)	-	- 15		-		
HCM Lane LOS	-	- C		-		
HCM 95th %tile Q(veh)	-	- 0.6		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	102	37	742	19	75	668
Future Volume (vph)	102	37	742	19	75	668
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	111	40	807	21	82	726
Shared Lane Traffic (%)						
Lane Group Flow (vph)	111	40	807	21	82	726
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	58.9%			ICU Level of Service B		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	102	37	742	19	75	668
Future Volume (Veh/h)	102	37	742	19	75	668
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	111	40	807	21	82	726
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		4				
Median type			TWLT			TWLT
Median storage veh			2			2
Upstream signal (ft)						1181
pX, platoon unblocked						
vC, conflicting volume	1697	807			828	
vC1, stage 1 conf vol	807					
vC2, stage 2 conf vol	890					
vCu, unblocked vol	1697	807			828	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	61	90			90	
cM capacity (veh/h)	288	385			812	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	151	807	21	82	726	
Volume Left	111	0	0	82	0	
Volume Right	40	0	21	0	0	
cSH	392	1700	1700	812	1700	
Volume to Capacity	0.39	0.47	0.01	0.10	0.43	
Queue Length 95th (ft)	44	0	0	8	0	
Control Delay (s)	22.6	0.0	0.0	9.9	0.0	
Lane LOS	C			A		
Approach Delay (s)	22.6	0.0		1.0		
Approach LOS	C					
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization			58.9%		ICU Level of Service	B
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	102	37	742	19	75	668
Future Vol, veh/h	102	37	742	19	75	668
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	250	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	111	40	807	21	82	726
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1697	807	0	0	828	0
Stage 1	807	-	-	-	-	-
Stage 2	890	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	~ 103	385	-	-	812	-
Stage 1	442	-	-	-	-	-
Stage 2	404	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	~ 93	385	-	-	812	-
Mov Cap-2 Maneuver	224	-	-	-	-	-
Stage 1	442	-	-	-	-	-
Stage 2	363	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	30.4	0		1		
HCM LOS	D					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	224	385	812	-
HCM Lane V/C Ratio	-	-	0.495	0.104	0.1	-
HCM Control Delay (s)	-	-	35.9	15.4	9.9	-
HCM Lane LOS	-	-	E	C	A	-
HCM 95th %tile Q(veh)	-	-	2.5	0.3	0.3	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	28	738	4	13	778
Future Volume (vph)	8	28	738	4	13	778
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.894		0.999			
Flt Protected	0.989				0.950	
Satd. Flow (prot)	1680	0	1825	0	1805	1712
Flt Permitted	0.989				0.950	
Satd. Flow (perm)	1680	0	1825	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	29	761	4	13	802
Shared Lane Traffic (%)						
Lane Group Flow (vph)	37	0	765	0	13	802
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	50.9%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

8: FM973 & Taylor HS

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	28	738	4	13	778
Future Volume (Veh/h)	8	28	738	4	13	778
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	29	761	4	13	802
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1591	763			765	
vC1, stage 1 conf vol	763					
vC2, stage 2 conf vol	828					
vCu, unblocked vol	1591	763			765	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	93			98	
cM capacity (veh/h)	327	408			857	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	37	765	13	802		
Volume Left	8	0	13	0		
Volume Right	29	4	0	0		
cSH	387	1700	857	1700		
Volume to Capacity	0.10	0.45	0.02	0.47		
Queue Length 95th (ft)	8	0	1	0		
Control Delay (s)	15.3	0.0	9.3	0.0		
Lane LOS	C		A			
Approach Delay (s)	15.3	0.0	0.1			
Approach LOS	C					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			50.9%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	28	738	4	13	778
Future Vol, veh/h	8	28	738	4	13	778
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	29	761	4	13	802
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1591	763	0	0	765	0
Stage 1	763	-	-	-	-	-
Stage 2	828	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	119	408	-	-	857	-
Stage 1	464	-	-	-	-	-
Stage 2	432	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	117	408	-	-	857	-
Mov Cap-2 Maneuver	256	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	426	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	16.1	0		0.2		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	360	857	-	
HCM Lane V/C Ratio	-	-	0.103	0.016	-	
HCM Control Delay (s)	-	-	16.1	9.3	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	63	22	38	673	692	98
Future Volume (vph)	63	22	38	673	692	98
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.965					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1767	0	1805	1827	1696	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1767	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	68	24	41	732	752	107
Shared Lane Traffic (%)						
Lane Group Flow (vph)	92	0	41	732	752	107
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	47.9%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	63	22	38	673	692	98
Future Volume (Veh/h)	63	22	38	673	692	98
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	68	24	41	732	752	107
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage veh				2	2	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1566	752	859			
vC1, stage 1 conf vol	752					
vC2, stage 2 conf vol	814					
vCu, unblocked vol	1566	752	859			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	79	94	95			
cM capacity (veh/h)	325	413	791			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	92	41	732	752	107	
Volume Left	68	41	0	0	0	
Volume Right	24	0	0	0	107	
cSH	344	791	1700	1700	1700	
Volume to Capacity	0.27	0.05	0.43	0.44	0.06	
Queue Length 95th (ft)	27	4	0	0	0	
Control Delay (s)	19.3	9.8	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	19.3	0.5		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utilization			47.9%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	63	22	38	673	692	98
Future Vol, veh/h	63	22	38	673	692	98
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	68	24	41	732	752	107
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1566	752	859	0	-	0
Stage 1	752	-	-	-	-	-
Stage 2	814	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	124	413	791	-	-	-
Stage 1	469	-	-	-	-	-
Stage 2	439	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	118	413	791	-	-	-
Mov Cap-2 Maneuver	256	-	-	-	-	-
Stage 1	445	-	-	-	-	-
Stage 2	439	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	23.7	0.5		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	791	-	284	-	-	
HCM Lane V/C Ratio	0.052	-	0.325	-	-	
HCM Control Delay (s)	9.8	-	23.7	-	-	
HCM Lane LOS	A	-	C	-	-	
HCM 95th %tile Q(veh)	0.2	-	1.4	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Future Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.958			0.897						0.997	
Flt Protected		0.967			0.988		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1684	0	1805	1827	0	1805	1710	0
Flt Permitted		0.967			0.988		0.950			0.950		
Satd. Flow (perm)	0	1760	0	0	1684	0	1805	1827	0	1805	1710	0
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	9	0	4	5	0	16	4	743	0	3	755	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	21	0	4	743	0	3	769	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	47.4%											
Analysis Period (min)	15											
	ICU Level of Service A											

HCM Unsignalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 PM Peak Hour
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	8	0	4	5	0	15	4	684	0	3	695	13
Future Volume (Veh/h)	8	0	4	5	0	15	4	684	0	3	695	13
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	0	4	5	0	16	4	743	0	3	755	14
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							TWLTL			TWLTL		
Median storage veh							2			2		
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1535	1519	762	1516	1526	743	769				743	
vC1, stage 1 conf vol	768	768		751	751							
vC2, stage 2 conf vol	767	751		765	775							
vCu, unblocked vol	1535	1519	762	1516	1526	743	769				743	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)	6.1	5.5		6.1	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	97	100	99	98	100	96	100				100	
cM capacity (veh/h)	281	308	408	290	307	418	854				873	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	13	21	4	743	3	769						
Volume Left	9	5	4	0	3	0						
Volume Right	4	16	0	0	0	14						
cSH	311	378	854	1700	873	1700						
Volume to Capacity	0.04	0.06	0.00	0.44	0.00	0.45						
Queue Length 95th (ft)	3	4	0	0	0	0						
Control Delay (s)	17.1	15.1	9.2	0.0	9.1	0.0						
Lane LOS	C	C	A		A							
Approach Delay (s)	17.1	15.1	0.0		0.0							
Approach LOS	C	C										
Intersection Summary												
Average Delay				0.4								
Intersection Capacity Utilization				47.4%	ICU Level of Service				A			
Analysis Period (min)				15								

Intersection												
Int Delay, s/veh	0.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	8	0	4	5	0	15	4	684	0	3	695	13
Future Vol, veh/h	8	0	4	5	0	15	4	684	0	3	695	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	80	-	-	400	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	11	0
Mvmt Flow	9	0	4	5	0	16	4	743	0	3	755	14

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1527	1519	762	1521	1526	743	769	0	0	743	0	0
Stage 1	768	768	-	751	751	-	-	-	-	-	-	-
Stage 2	759	751	-	770	775	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	97	120	408	98	119	418	854	-	-	873	-	-
Stage 1	397	414	-	406	421	-	-	-	-	-	-	-
Stage 2	402	421	-	396	411	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	93	119	408	96	118	418	854	-	-	873	-	-
Mov Cap-2 Maneuver	93	119	-	96	118	-	-	-	-	-	-	-
Stage 1	395	413	-	404	419	-	-	-	-	-	-	-
Stage 2	385	419	-	390	410	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	37.1		22.5		0.1		0	
HCM LOS	E		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	854	-	-	125 227	873	-	-
HCM Lane V/C Ratio	0.005	-	-	0.104 0.096	0.004	-	-
HCM Control Delay (s)	9.2	-	-	37.1 22.5	9.1	-	-
HCM Lane LOS	A	-	-	E C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.3 0.3	0	-	-

Appendix D. Traffic Simulation Output

- Phase 2 Buildout Year (2027) Projected Conditions with Development, AM & PM
 - Mitigation:
 - With Traffic Signal at FM 973 at Driveway 2B
 - With Traffic Signal at FM 973 at Wesley Miller Lane
 - With Existing Timings at FM 973 at US 79

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	0	1671	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				753						143		267
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	9	600	833	528	709	0	0	595	218	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 2

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.1	39.3	39.3	25.2	67.8			50.4	50.4		5.0
Actuated g/C Ratio		0.07	0.29	0.29	0.19	0.50			0.37	0.37		0.04
v/c Ratio		0.07	0.63	0.85	1.66	0.42			0.96	0.32		0.00
Control Delay		66.2	44.8	14.3	343.7	22.8			70.4	13.8		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Delay		66.2	44.8	14.3	343.7	22.8			70.4	13.8		0.0
LOS		E	D	B	F	C			E	B		A
Approach Delay			27.3			159.8			55.2			
Approach LOS			C			F			E			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 136

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.66

Intersection Signal Delay: 80.7

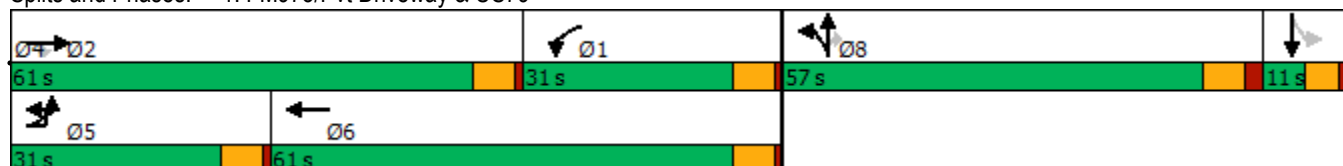
Intersection LOS: F

Intersection Capacity Utilization 96.5%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Max
70th %ile Green (s)	0.0	45.5	45.5	25.0	76.5	50.0	50.0	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	38.8	38.8	25.0	69.8	50.0	50.0	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
30th %ile Green (s)	0.0	33.2	33.2	25.0	64.2	50.0	50.0	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
10th %ile Green (s)	0.0	26.7	26.7	25.0	57.7	50.0	50.0	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Max	Max	Skip
Intersection Summary								
Cycle Length: 160								
Actuated Cycle Length: 136								
Control Type: Actuated-Uncoordinated								
90th %ile Actuated Cycle: 160								
70th %ile Actuated Cycle: 139.5								
50th %ile Actuated Cycle: 132.8								
30th %ile Actuated Cycle: 127.2								
10th %ile Actuated Cycle: 120.7								

Queues

2027 AM Peak Hour (w/New Signalized)

1: FM973/Pvt Driveway & US79

Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	9	600	833	528	709	595	218	1
v/c Ratio	0.07	0.63	0.85	1.66	0.42	0.96	0.32	0.00
Control Delay	66.2	44.8	14.3	343.7	22.8	70.4	13.8	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.2	44.8	14.3	343.7	22.8	70.4	13.8	0.0
Queue Length 50th (ft)	7	235	52	~660	181	493	42	0
Queue Length 95th (ft)	30	328	273	#1097	324	#966	133	0
Internal Link Dist (ft)		854			992	556		491
Turn Bay Length (ft)	100		615	880				
Base Capacity (vph)	334	1351	1079	318	1681	619	671	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.44	0.77	1.66	0.42	0.96	0.32	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373			1671	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373			1671	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
RTOR Reduction (vph)	0	0	0	550	0	0	0	0	0	94	0	1
Lane Group Flow (vph)	0	9	600	283	528	709	0	0	595	124	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.7	39.3	39.3	30.3	67.9			50.5	50.5		0.9
Effective Green, g (s)		1.7	39.3	39.3	30.3	67.9			50.5	50.5		0.9
Actuated g/C Ratio		0.01	0.27	0.27	0.21	0.47			0.35	0.35		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		2.0
Lane Grp Cap (vph)		21	891	418	356	1568			577	542		10
v/s Ratio Prot		0.00	0.18		c0.31	0.21			c0.36			c0.00
v/s Ratio Perm				c0.18						0.08		
v/c Ratio		0.43	0.67	0.68	1.48	0.45			1.03	0.23		0.00
Uniform Delay, d1		71.7	47.6	47.7	57.9	26.5			47.8	33.9		72.1
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		1.00
Incremental Delay, d2		5.0	1.6	3.4	232.0	0.1			45.7	0.2		0.0
Delay (s)		76.7	49.2	51.1	288.7	25.8			93.5	34.1		72.1
Level of Service		E	D	D	F	C			F	C		E
Approach Delay (s)			50.5			138.0			77.6			72.1
Approach LOS			D			F			E			E
Intersection Summary												
HCM 2000 Control Delay			87.8									HCM 2000 Level of Service F
HCM 2000 Volume to Capacity ratio			1.02									
Actuated Cycle Length (s)			146.0						25.0			
Intersection Capacity Utilization			96.5%									ICU Level of Service F
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	547	284	0	1253	0	207
Future Volume (vph)	547	284	0	1253	0	207
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3282	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3282	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1072			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	0%	0%	7%	0%	0%
Adj. Flow (vph)	595	309	0	1362	0	225
Shared Lane Traffic (%)						
Lane Group Flow (vph)	595	309	0	1362	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 38.0%						
ICU Level of Service A						
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis 3: Driveway 2A & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	547	284	0	1253	0	207
Future Volume (Veh/h)	547	284	0	1253	0	207
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	595	309	0	1362	0	225
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1072					
pX, platoon unblocked			0.89		0.89	0.89
vC, conflicting volume			904		1276	298
vC1, stage 1 conf vol					595	
vC2, stage 2 conf vol					681	
vCu, unblocked vol			654		1071	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	77
cM capacity (veh/h)			842		409	975
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	298	298	309	681	681	225
Volume Left	0	0	0	0	0	0
Volume Right	0	0	309	0	0	225
cSH	1700	1700	1700	1700	1700	975
Volume to Capacity	0.17	0.17	0.18	0.40	0.40	0.23
Queue Length 95th (ft)	0	0	0	0	0	22
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.8
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.8
Approach LOS						A
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			38.0%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	547	284	0	1253	0	207
Future Vol, veh/h	547	284	0	1253	0	207
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	0	0	7	0	0
Mvmt Flow	595	309	0	1362	0	225
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	298
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	704
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	704
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.5	
HCM LOS					B	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	704	-	-	-		
HCM Lane V/C Ratio	0.32	-	-	-		
HCM Control Delay (s)	12.5	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	1.4	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	75	0	682	0	1207	0	0	0
Future Volume (vph)	75	0	682	0	1207	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3282	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3282	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	10%	0%	7%	0%	0%	0%
Adj. Flow (vph)	82	0	749	0	1326	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	82	749	0	1326	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utilization 44.2%				ICU Level of Service A				
Analysis Period (min) 15								

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	75	0	682	0	1207	0	0	0
Future Volume (Veh/h)	75	0	682	0	1207	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	0	0	749	0	1326	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage (veh)			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	1326		0			1700	663
vC1, stage 1 conf vol							1326	
vC2, stage 2 conf vol							374	
vCu, unblocked vol	0	1326		0			1700	663
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	527		0			206	409
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	374	374	663	663	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.22	0.22	0.39	0.39	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			44.2%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	1.3							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	75	0	682	0	1207	0	0	0
Future Vol, veh/h	75	0	682	0	1207	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	10	0	7	0	0	0
Mvmt Flow	82	0	749	0	1326	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	1326	1326	0	749	-	0	1865	663
Stage 1	-	-	-	-	-	-	1326	-
Stage 2	-	-	-	-	-	-	539	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	209	527	-	488	-	0	66	409
Stage 1	-	-	-	-	-	0	216	-
Stage 2	-	-	-	-	-	0	555	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	209	209	-	488	-	-	40	409
Mov Cap-2 Maneuver	-	-	-	-	-	-	40	-
Stage 1	-	-	-	-	-	-	131	-
Stage 2	-	-	-	-	-	-	555	-
Approach	EB		WB		SB			
HCM Control Delay, s	3.3		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	209	-	488	-	-			
HCM Lane V/C Ratio	0.394	-	-	-	-			
HCM Control Delay (s)	33	-	0	-	0			
HCM Lane LOS	D	-	A	-	A			
HCM 95th %tile Q(veh)	1.8	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			 			 	
Traffic Volume (vph)	8	704	12	7	1144	1	36	7	1	3	3	5
Future Volume (vph)	8	704	12	7	1144	1	36	7	1	3	3	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.997			0.932	
Flt Protected	0.950			0.950				0.961			0.988	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Flt Permitted	0.950			0.950				0.961			0.988	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1615	0	1820	0	0	1750	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	9	800	14	8	1300	1	41	8	1	3	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	9	800	14	8	1300	1	0	50	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	44.4%						ICU Level of Service A					
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis

5: S Rio Grande & US79

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	8	704	12	7	1144	1	36	7	1	3	3	5
Future Volume (Veh/h)	8	704	12	7	1144	1	36	7	1	3	3	5
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	9	800	14	8	1300	1	41	8	1	3	3	6
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1300			800			1486	2134	400	1738	2134	650
vC1, stage 1 conf vol							818	818		1316	1316	
vC2, stage 2 conf vol							668	1316		422	818	
vCu, unblocked vol	1300			800			1486	2134	400	1738	2134	650
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			99			84	96	100	98	98	99
cM capacity (veh/h)	540			832			262	189	605	159	194	417
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	9	400	400	14	8	650	650	1	50	12		
Volume Left	9	0	0	0	8	0	0	0	41	3		
Volume Right	0	0	0	14	0	0	0	1	1	6		
cSH	540	1700	1700	1700	832	1700	1700	1700	249	246		
Volume to Capacity	0.02	0.24	0.24	0.01	0.01	0.38	0.38	0.00	0.20	0.05		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	18	4		
Control Delay (s)	11.8	0.0	0.0	0.0	9.4	0.0	0.0	0.0	23.0	20.4		
Lane LOS	B				A				C	C		
Approach Delay (s)	0.1				0.1				23.0	20.4		
Approach LOS									C	C		
Intersection Summary												
Average Delay	0.7											
Intersection Capacity Utilization	44.4%			ICU Level of Service			A					
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	8	704	12	7	1144	1	36	7	1	3	3	5
Future Vol, veh/h	8	704	12	7	1144	1	36	7	1	3	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	9	800	14	8	1300	1	41	8	1	3	3	6

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1300	0	0	800	0	0	1486	2134	-	1738	2134	-
Stage 1	-	-	-	-	-	-	818	818	-	1316	1316	-
Stage 2	-	-	-	-	-	-	668	1316	-	422	818	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	540	-	-	832	-	-	88	50	0	57	50	0
Stage 1	-	-	-	-	-	-	340	393	0	169	229	0
Stage 2	-	-	-	-	-	-	419	229	0	585	393	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	540	-	-	832	-	-	82	49	-	49	49	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	82	49	-	49	49	-
Stage 1	-	-	-	-	-	-	334	386	-	166	227	-
Stage 2	-	-	-	-	-	-	409	227	-	563	386	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	540	-	-	832	-	-	-
HCM Lane V/C Ratio	-	0.017	-	-	0.01	-	-	-
HCM Control Delay (s)	-	11.8	-	-	9.4	-	-	-
HCM Lane LOS	-	B	-	-	A	-	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	78	686	105	0	1306
Future Volume (vph)	0	78	686	105	0	1306
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1759	1615	0	1792
Flt Permitted						
Satd. Flow (perm)	0	1644	1759	1615	0	1792
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	0	85	746	114	0	1420
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	85	746	114	0	1420
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	72.1%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 6: FM973 & Driveway 1B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	78	686	105	0	1306
Future Volume (Veh/h)	0	78	686	105	0	1306
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	85	746	114	0	1420
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh)			2		2	
Upstream signal (ft)			545		636	
pX, platoon unblocked	0.59	0.59			0.59	
vC, conflicting volume	2166	746			860	
vC1, stage 1 conf vol	746					
vC2, stage 2 conf vol	1420					
vCu, unblocked vol	2623	229			421	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	82			100	
cM capacity (veh/h)	195	484			682	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	85	746	114	1420		
Volume Left	0	0	0	0		
Volume Right	85	0	114	0		
cSH	484	1700	1700	1700		
Volume to Capacity	0.18	0.44	0.07	0.84		
Queue Length 95th (ft)	16	0	0	0		
Control Delay (s)	14.0	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	14.0	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			72.1%		ICU Level of Service	C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	78	686	105	0	1306
Future Vol, veh/h	0	78	686	105	0	1306
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	0	85	746	114	0	1420
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	746	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	417	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	417	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15.8	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	417		-		
HCM Lane V/C Ratio	-	0.203		-		
HCM Control Delay (s)	-	15.8		-		
HCM Lane LOS	-	C		-		
HCM 95th %tile Q(veh)	-	0.8		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	155	53	737	26	112	1195
Future Volume (vph)	155	53	737	26	112	1195
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		58		28		
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	168	58	801	28	122	1299
Shared Lane Traffic (%)						
Lane Group Flow (vph)	168	58	801	28	122	1299
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	3	1	1	3
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	50	50	250	50	50	250
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	50	50	6	50	50	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			122			122
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Detector 3 Position(ft)			244			244
Detector 3 Size(ft)			6			6

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Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex			Cl+Ex		
Detector 3 Channel						
Detector 3 Extend (s)	0.0			0.0		
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Act Effect Green (s)	12.7	12.7	40.9	40.9	9.4	56.3
Actuated g/C Ratio	0.16	0.16	0.51	0.51	0.12	0.70
v/c Ratio	0.59	0.19	0.89	0.03	0.58	1.03
Control Delay	39.1	9.4	23.6	2.1	46.6	49.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	39.1	9.4	23.6	2.1	46.6	49.0
LOS	D	A	C	A	D	D
Approach Delay	31.5		22.8			48.8
Approach LOS	C		C			D

Intersection Summary

Area Type: Other
Cycle Length: 80
Actuated Cycle Length: 80
Offset: 7 (9%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 100
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.03
Intersection Signal Delay: 38.5
Intersection Capacity Utilization 80.6%
Analysis Period (min) 15

Intersection LOS: D

ICU Level of Service D

Splits and Phases: 7: FM973 & Driveway 2B



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Phasings
7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	44.0	44.0	13.0	57.0
Total Split (%)	28.8%	28.8%	55.0%	55.0%	16.3%	71.3%
Maximum Green (s)	18.0	18.0	38.0	38.0	7.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
90th %ile Green (s)	17.4	17.4	38.0	38.0	7.6	51.6
90th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
70th %ile Green (s)	14.7	14.7	38.0	38.0	10.3	54.3
70th %ile Term Code	Gap	Gap	Coord	Coord	Max	Coord
50th %ile Green (s)	12.7	12.7	38.7	38.7	11.6	56.3
50th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
30th %ile Green (s)	10.7	10.7	42.4	42.4	9.9	58.3
30th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
10th %ile Green (s)	8.0	8.0	47.5	47.5	7.5	61.0
10th %ile Term Code	Min	Min	Coord	Coord	Gap	Coord
Intersection Summary						
Cycle Length: 80						
Actuated Cycle Length: 80						
Offset: 7 (9%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Control Type: Actuated-Coordinated						

Queues
7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	168	58	801	28	122	1299
v/c Ratio	0.59	0.19	0.89	0.03	0.58	1.03
Control Delay	39.1	9.4	23.6	2.1	46.6	49.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	39.1	9.4	23.6	2.1	46.6	49.0
Queue Length 50th (ft)	79	0	144	0	57	~706
Queue Length 95th (ft)	130	29	m#565	m2	#142	#1003
Internal Link Dist (ft)	143		592			465
Turn Bay Length (ft)		100		250	150	
Base Capacity (vph)	406	408	899	839	211	1261
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.41	0.14	0.89	0.03	0.58	1.03
Intersection Summary						
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.						
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	155	53	737	26	112	1195
Future Volume (vph)	155	53	737	26	112	1195
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1805	1615	1759	1615	1805	1792
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1805	1615	1759	1615	1805	1792
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	168	58	801	28	122	1299
RTOR Reduction (vph)	0	49	0	14	0	0
Lane Group Flow (vph)	168	9	801	14	122	1299
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Actuated Green, G (s)	12.7	12.7	40.9	40.9	9.4	56.3
Effective Green, g (s)	12.7	12.7	40.9	40.9	9.4	56.3
Actuated g/C Ratio	0.16	0.16	0.51	0.51	0.12	0.70
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	286	256	899	825	212	1261
v/s Ratio Prot	c0.09		0.46		0.07	c0.72
v/s Ratio Perm		0.01		0.01		
v/c Ratio	0.59	0.04	0.89	0.02	0.58	1.03
Uniform Delay, d1	31.2	28.5	17.5	9.6	33.4	11.9
Progression Factor	1.00	1.00	0.47	0.44	1.00	1.00
Incremental Delay, d2	3.1	0.1	11.6	0.0	3.7	33.4
Delay (s)	34.3	28.5	19.9	4.3	37.2	45.2
Level of Service	C	C	B	A	D	D
Approach Delay (s)	32.8		19.4			44.5
Approach LOS	C		B			D
Intersection Summary						
HCM 2000 Control Delay			35.0		HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			1.04			
Actuated Cycle Length (s)			80.0		Sum of lost time (s)	17.0
Intersection Capacity Utilization			80.6%		ICU Level of Service	D
Analysis Period (min)			15			
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis 7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	155	53	737	26	112	1195			
Future Volume (veh/h)	155	53	737	26	112	1195			
Number	3	18	2	12	1	6			
Initial Q, veh	0	0	0	0	0	0			
Ped-Bike Adj (A_pbT)	1.00	1.00		1.00	1.00				
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No		No			No			
Lanes Open During Work Zone									
Adj Sat Flow, veh/h/ln	1900	1900	1781	1900	1900	1811			
Adj Flow Rate, veh/h	168	58	801	28	122	1299			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Percent Heavy Veh, %	0	0	8	0	0	6			
Opposing Right Turn Influence	Yes				Yes				
Cap, veh/h	219	195	1036	936	154	1343			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Prop Arrive On Green	0.12	0.12	0.58	0.58	0.09	0.74			
Unsig. Movement Delay									
Ln Grp Delay, s/veh	39.6	32.9	18.4	7.2	58.8	27.4			
Ln Grp LOS	D	C	B	A	E	C			
Approach Vol, veh/h	226		829			1421			
Approach Delay, s/veh	37.9		18.0			30.1			
Approach LOS	D		B			C			
Timer:		1	2	3	4	5	6	7	8
Assigned Phs		1	2	8			6		
Case No		2.0	7.0	9.0			4.0		
Phs Duration (G+Y+Rc), s		12.8	52.5	14.7			65.3		
Change Period (Y+Rc), s		6.0	6.0	5.0			6.0		
Max Green (Gmax), s		7.0	38.0	18.0			51.0		
Max Allow Headway (MAH), s		4.1	6.9	4.4			6.9		
Max Q Clear (g_c+I1), s		7.3	29.4	9.2			54.5		
Green Ext Time (g_e), s		0.0	5.3	0.5			0.0		
Prob of Phs Call (p_c)		0.93	1.00	0.99			1.00		
Prob of Max Out (p_x)		1.00	0.89	0.07			1.00		
Left-Turn Movement Data									
Assigned Mvmt		1	5	3					
Mvmt Sat Flow, veh/h		1810	0	1810					
Through Movement Data									
Assigned Mvmt			2	8			6		
Mvmt Sat Flow, veh/h			1781	0			1811		
Right-Turn Movement Data									
Assigned Mvmt			12	18			16		
Mvmt Sat Flow, veh/h			1610	1610			0		
Left Lane Group Data									
Assigned Mvmt		1	5	3	0	0	0	0	0
Lane Assignment		L (Prot)		L					

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HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

Lanes in Grp	1	0	1	0	0	0	0	0
Grp Vol (v), veh/h	122	0	168	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1810	0	1810	0	0	0	0	0
Q Serve Time (g_s), s	5.3	0.0	7.2	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	5.3	0.0	7.2	0.0	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	1810	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	46.5	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	154	0	219	0	0	0	0	0
V/C Ratio (X)	0.79	0.00	0.77	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	158	0	407	0	0	0	0	0
Upstream Filter (I)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	35.9	0.0	34.1	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	22.9	0.0	5.6	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	58.8	0.0	39.6	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	2.1	0.0	3.1	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	1.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	3.1	0.0	3.4	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.52	0.00	0.55	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Middle Lane Group Data								
Assigned Mvmt	0	2	8	0	0	6	0	0
Lane Assignment	T			T				
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	801	0	0	0	1299	0	0
Grp Sat Flow (s), veh/h/ln	0	1781	0	0	0	1811	0	0
Q Serve Time (g_s), s	0.0	27.4	0.0	0.0	0.0	52.5	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	27.4	0.0	0.0	0.0	52.5	0.0	0.0
Lane Grp Cap (c), veh/h	0	1036	0	0	0	1343	0	0
V/C Ratio (X)	0.00	0.77	0.00	0.00	0.00	0.97	0.00	0.00
Avail Cap (c_a), veh/h	0	1036	0	0	0	1343	0	0
Upstream Filter (I)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	12.7	0.0	0.0	0.0	9.5	0.0	0.0
Incr Delay (d2), s/veh	0.0	5.6	0.0	0.0	0.0	17.9	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	18.4	0.0	0.0	0.0	27.4	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	7.7	0.0	0.0	0.0	8.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	1.6	0.0	0.0	0.0	6.7	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	9.3	0.0	0.0	0.0	14.7	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.40	0.00	0.00	0.00	0.80	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Right Lane Group Data

Assigned Mvmt	0	12	18	0	0	16	0	0
Lane Assignment		R	R					
Lanes in Grp	0	1	1	0	0	0	0	0
Grp Vol (v), veh/h	0	28	58	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1610	1610	0	0	0	0	0
Q Serve Time (g_s), s	0.0	0.6	2.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.6	2.6	0.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	936	195	0	0	0	0	0
V/C Ratio (X)	0.00	0.03	0.30	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	936	362	0	0	0	0	0
Upstream Filter (I)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	7.1	32.1	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.1	0.8	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	7.2	32.9	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.2	1.0	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.2	1.0	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.02	0.26	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	26.7
HCM 6th LOS	C

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 AM Peak Hour (w/New Signalized)

Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	6	97	684	12	227	1133
Future Volume (vph)	6	97	684	12	227	1133
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.873		0.998			
Flt Protected	0.997				0.950	
Satd. Flow (prot)	1654	0	1758	0	1805	1792
Flt Permitted	0.997				0.950	
Satd. Flow (perm)	1654	0	1758	0	1805	1792
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	0%	0%	8%	0%	0%	6%
Adj. Flow (vph)	7	114	805	14	267	1333
Shared Lane Traffic (%)						
Lane Group Flow (vph)	121	0	819	0	267	1333
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	72.6%			ICU Level of Service C		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 8: FM973 & Taylor HS

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	6	97	684	12	227	1133
Future Volume (Veh/h)	6	97	684	12	227	1133
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	7	114	805	14	267	1333
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)			1184		672	
pX, platoon unblocked	0.44	0.75			0.75	
vC, conflicting volume	2679	812			819	
vC1, stage 1 conf vol	812					
vC2, stage 2 conf vol	1867					
vCu, unblocked vol	2870	577			586	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	33	71			64	
cM capacity (veh/h)	10	388			744	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	121	819	267	1333		
Volume Left	7	0	267	0		
Volume Right	114	14	0	0		
cSH	126	1700	744	1700		
Volume to Capacity	0.96	0.48	0.36	0.78		
Queue Length 95th (ft)	161	0	41	0		
Control Delay (s)	137.2	0.0	12.5	0.0		
Lane LOS	F		B			
Approach Delay (s)	137.2	0.0	2.1			
Approach LOS	F					
Intersection Summary						
Average Delay		7.9				
Intersection Capacity Utilization		72.6%		ICU Level of Service	C	
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	2.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	97	684	12	227	1133
Future Vol, veh/h	6	97	684	12	227	1133
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	0	0	8	0	0	6
Mvmt Flow	7	114	805	14	267	1333
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	2679	812	0	0	819	0
Stage 1	812	-	-	-	-	-
Stage 2	1867	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	25	382	-	-	818	-
Stage 1	440	-	-	-	-	-
Stage 2	136	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	17	382	-	-	818	-
Mov Cap-2 Maneuver	74	-	-	-	-	-
Stage 1	440	-	-	-	-	-
Stage 2	92	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	24.2	0		1.9		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	307	818	-	
HCM Lane V/C Ratio	-	-	0.395	0.326	-	
HCM Control Delay (s)	-	-	24.2	11.5	-	
HCM Lane LOS	-	-	C	B	-	
HCM 95th %tile Q(veh)	-	-	1.8	1.4	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	94	33	12	601	1095	42
Future Volume (vph)	94	33	12	601	1095	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.965					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1767	0	1805	1759	1810	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1767	0	1805	1759	1810	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	8%	5%	0%
Adj. Flow (vph)	104	37	13	668	1217	47
Shared Lane Traffic (%)						
Lane Group Flow (vph)	141	0	13	668	1217	47
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 71.5%				ICU Level of Service C		
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis 9: FM973 & Streamwood

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	94	33	12	601	1095	42
Future Volume (Veh/h)	94	33	12	601	1095	42
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	104	37	13	668	1217	47
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT	TWLT	
Median storage (veh)				2	2	
Upstream signal (ft)				724	1132	
pX, platoon unblocked	0.47	0.34	0.34			
vC, conflicting volume	1911	1217	1264			
vC1, stage 1 conf vol	1217					
vC2, stage 2 conf vol	694					
vCu, unblocked vol	1110	669	807			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	38	76	95			
cM capacity (veh/h)	169	157	282			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	141	13	668	1217	47	
Volume Left	104	13	0	0	0	
Volume Right	37	0	0	0	47	
cSH	165	282	1700	1700	1700	
Volume to Capacity	0.85	0.05	0.39	0.72	0.03	
Queue Length 95th (ft)	148	4	0	0	0	
Control Delay (s)	90.8	18.4	0.0	0.0	0.0	
Lane LOS	F	C				
Approach Delay (s)	90.8	0.4		0.0		
Approach LOS	F					
Intersection Summary						
Average Delay			6.3			
Intersection Capacity Utilization			71.5%	ICU Level of Service		C
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	94	33	12	601	1095	42
Future Vol, veh/h	94	33	12	601	1095	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	8	5	0
Mvmt Flow	104	37	13	668	1217	47
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1911	1217	1264	0	-	0
Stage 1	1217	-	-	-	-	-
Stage 2	694	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 76	223	557	-	-	-
Stage 1	283	-	-	-	-	-
Stage 2	499	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 74	223	557	-	-	-
Mov Cap-2 Maneuver	193	-	-	-	-	-
Stage 1	276	-	-	-	-	-
Stage 2	499	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	57.3		0.2		0	
HCM LOS	F					
Minor Lane/Major Mvmt	NBL		NBT	EBLn1	SBT	SBR
Capacity (veh/h)	557		-	200	-	-
HCM Lane V/C Ratio	0.024		-	0.706	-	-
HCM Control Delay (s)	11.6		-	57.3	-	-
HCM Lane LOS	B		-	F	-	-
HCM 95th %tile Q(veh)	0.1		-	4.5	-	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Future Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.959			0.877			0.986			0.999	
Flt Protected		0.966			0.995		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1658	0	1805	1676	0	1805	1711	0
Flt Permitted		0.448			0.966		0.950			0.950		
Satd. Flow (perm)	0	816	0	0	1610	0	1805	1676	0	1805	1711	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		205			219			7			1	
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Adj. Flow (vph)	14	0	6	22	0	219	3	464	49	435	842	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	241	0	3	513	0	435	849	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0		8.0	8.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		13.0	24.0		14.5	24.0	
Total Split (s)	23.0	23.0		23.0	23.0		13.0	30.6		26.4	44.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%		16.3%	38.3%		33.0%	55.0%	
Maximum Green (s)	18.0	18.0		18.0	18.0		7.0	24.6		20.4	38.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)		9.6			9.6		7.0	28.6		24.8	56.8	
Actuated g/C Ratio		0.12			0.12		0.09	0.36		0.31	0.71	
v/c Ratio		0.07			0.63		0.02	0.85		0.78	0.70	
Control Delay		0.5			14.1		33.7	41.2		30.1	14.7	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		0.5			14.1		33.7	41.2		30.1	14.7	
LOS		A			B		C	D		C	B	
Approach Delay		0.5			14.1			41.2			19.9	
Approach LOS		A			B			D			B	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 24.4

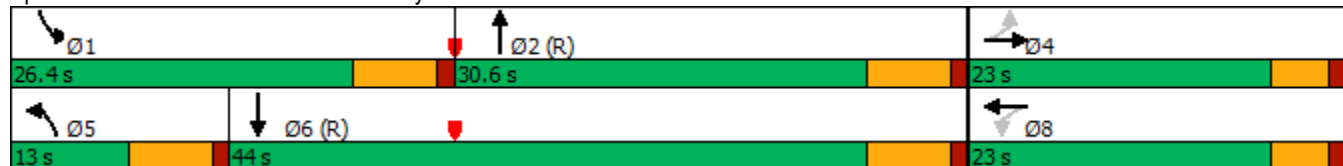
Intersection LOS: C

Intersection Capacity Utilization 72.2%

ICU Level of Service C

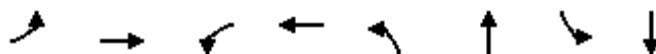
Analysis Period (min) 15

Splits and Phases: 10: FM973 & Wesley Miller



Phasings
10: FM973 & Wesley Miller

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Protected Phases		4		8	5	2	1	6
Permitted Phases	4		8					
Minimum Initial (s)	8.0	8.0	8.0	8.0	7.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	23.0	23.0	13.0	24.0	14.5	24.0
Total Split (s)	23.0	23.0	23.0	23.0	13.0	30.6	26.4	44.0
Total Split (%)	28.8%	28.8%	28.8%	28.8%	16.3%	38.3%	33.0%	55.0%
Maximum Green (s)	18.0	18.0	18.0	18.0	7.0	24.6	20.4	38.0
Yellow Time (s)	3.5	3.5	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag					Lead	Lag	Lead	Lag
Lead-Lag Optimize?					Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	None	None	None	C-Min	None	C-Min
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	14.1	14.1	14.1	14.1	7.0	24.6	24.3	41.9
90th %ile Term Code	Hold	Hold	Gap	Gap	Min	Coord	Max	Coord
70th %ile Green (s)	9.7	9.7	9.7	9.7	0.0	24.6	28.7	59.3
70th %ile Term Code	Hold	Hold	Gap	Gap	Skip	Coord	Max	Coord
50th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	28.7	26.3	61.0
50th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
30th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	30.4	24.6	61.0
30th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
10th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	34.7	20.3	61.0
10th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Gap	Coord
Intersection Summary								
Cycle Length: 80								
Actuated Cycle Length: 80								
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green								
Control Type: Actuated-Coordinated								

Queues
10: FM973 & Wesley Miller

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development



Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	20	241	3	513	435	849
v/c Ratio	0.07	0.63	0.02	0.85	0.78	0.70
Control Delay	0.5	14.1	33.7	41.2	30.1	14.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.5	14.1	33.7	41.2	30.1	14.7
Queue Length 50th (ft)	0	10	1	229	200	238
Queue Length 95th (ft)	0	66	9	#435	m236	m388
Internal Link Dist (ft)	494	480		650		644
Turn Bay Length (ft)			80		400	
Base Capacity (vph)	342	531	157	603	561	1215
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.45	0.02	0.85	0.78	0.70

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 AM Peak Hour (w/New Signalized)
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Future Volume (vph)	12	0	5	19	0	193	3	408	43	383	741	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.96			0.88		1.00	0.99		1.00	1.00	
Flt Protected		0.97			1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1761			1659		1805	1676		1805	1711	
Flt Permitted		0.45			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (perm)		816			1611		1805	1676		1805	1711	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	14	0	6	22	0	219	3	464	49	435	842	7
RTOR Reduction (vph)	0	18	0	0	193	0	0	4	0	0	0	0
Lane Group Flow (vph)	0	2	0	0	48	0	3	509	0	435	849	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	11%	0%
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Actuated Green, G (s)		9.6			9.6		1.4	28.6		24.8	52.0	
Effective Green, g (s)		9.6			9.6		1.4	28.6		24.8	52.0	
Actuated g/C Ratio		0.12			0.12		0.02	0.36		0.31	0.65	
Clearance Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		97			193		31	599		559	1112	
v/s Ratio Prot							0.00	c0.30		c0.24	c0.50	
v/s Ratio Perm		0.00			c0.03							
v/c Ratio		0.02			0.25		0.10	0.85		0.78	0.76	
Uniform Delay, d1		31.1			31.9		38.7	23.7		25.1	9.7	
Progression Factor		1.00			1.00		1.00	1.00		0.94	1.35	
Incremental Delay, d2		0.1			0.7		1.4	14.0		3.7	2.7	
Delay (s)		31.2			32.6		40.0	37.7		27.3	15.8	
Level of Service		C			C		D	D		C	B	
Approach Delay (s)		31.2			32.6			37.7			19.7	
Approach LOS		C			C			D			B	
Intersection Summary												
HCM 2000 Control Delay		25.8			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio		0.74										
Actuated Cycle Length (s)		80.0			Sum of lost time (s)			17.0				
Intersection Capacity Utilization		72.2%			ICU Level of Service			C				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		0	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950				0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Flt Permitted		0.950			0.950				0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3341	0	0	1703	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				415						254		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	954	415	380	527	0	0	470	332	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			0			0
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.2	47.9	47.9	25.4	73.6			43.1	43.1		
Actuated g/C Ratio		0.08	0.35	0.35	0.19	0.54			0.32	0.32		
v/c Ratio		0.07	0.78	0.53	1.18	0.29			0.87	0.49		
Control Delay		66.2	44.6	5.5	156.3	18.9			61.8	12.0		
Queue Delay		0.0	0.0	0.0	0.0	0.0			0.0	0.0		
Total Delay		66.2	44.6	5.5	156.3	18.9			61.8	12.0		
LOS		E	D	A	F	B			E	B		
Approach Delay			33.0			76.5			41.2			
Approach LOS			C			E			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 135.7

Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 47.9

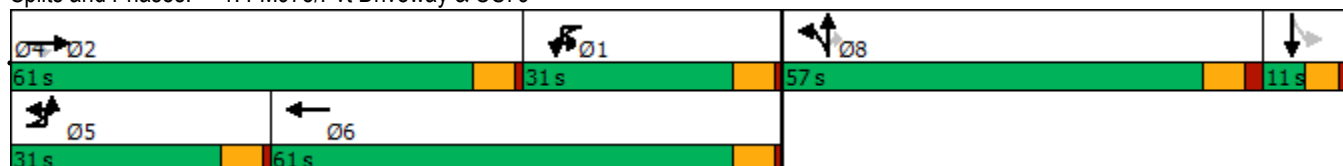
Intersection LOS: D

Intersection Capacity Utilization 82.7%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8		4
Permitted Phases			2				8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	7%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
70th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	50.0	50.0	0.0
70th %ile Term Code	Min	Max	Max	Max	Hold	Max	Max	Skip
50th %ile Green (s)	0.0	53.0	53.0	25.0	84.0	47.8	47.8	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
30th %ile Green (s)	0.0	44.5	44.5	25.0	75.5	40.1	40.1	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip
10th %ile Green (s)	0.0	33.7	33.7	25.0	64.7	29.5	29.5	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Skip

Intersection Summary

Cycle Length: 160

Actuated Cycle Length: 135.7

Control Type: Actuated-Uncoordinated

90th %ile Actuated Cycle: 149

70th %ile Actuated Cycle: 149

50th %ile Actuated Cycle: 144.8

30th %ile Actuated Cycle: 128.6

10th %ile Actuated Cycle: 107.2

Queues
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

							
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR
Lane Group Flow (vph)	10	954	415	380	527	470	332
v/c Ratio	0.07	0.78	0.53	1.18	0.29	0.87	0.49
Control Delay	66.2	44.6	5.5	156.3	18.9	61.8	12.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.2	44.6	5.5	156.3	18.9	61.8	12.0
Queue Length 50th (ft)	9	412	0	~454	121	410	52
Queue Length 95th (ft)	30	506	75	#689	205	#595	143
Internal Link Dist (ft)		854			990	556	
Turn Bay Length (ft)	100		615	880			
Base Capacity (vph)	337	1429	843	321	1811	637	751
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.67	0.49	1.18	0.29	0.74	0.44
Intersection Summary							
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.							
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.							

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95			1.00	1.00		
Frt		1.00	1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341			1703	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00			0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341			1703	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
RTOR Reduction (vph)	0	0	0	272	0	0	0	0	0	175	0	0
Lane Group Flow (vph)	0	10	954	143	380	527	0	0	470	157	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		3.6	47.9	47.9	29.3	73.6			43.1	43.1		
Effective Green, g (s)		3.6	47.9	47.9	29.3	73.6			43.1	43.1		
Actuated g/C Ratio		0.03	0.34	0.34	0.21	0.53			0.31	0.31		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0			7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0			3.0	3.0		
Lane Grp Cap (vph)		46	1193	500	361	1765			526	489		
v/s Ratio Prot		0.01	c0.27		c0.22	0.16			c0.28			
v/s Ratio Perm				0.10						0.10		
v/c Ratio		0.22	0.80	0.29	1.05	0.30			0.89	0.32		
Uniform Delay, d1		66.5	41.4	33.2	55.0	18.4			45.9	36.9		
Progression Factor		1.00	1.00	1.00	0.98	0.97			1.00	1.00		
Incremental Delay, d2		0.9	3.6	0.1	61.9	0.0			17.4	0.4		
Delay (s)		67.3	44.9	33.4	115.7	17.9			63.3	37.3		
Level of Service		E	D	C	F	B			E	D		
Approach Delay (s)			41.6			58.9			52.5			0.0
Approach LOS			D			E			D			A
Intersection Summary												
HCM 2000 Control Delay			49.5			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.94									
Actuated Cycle Length (s)			139.3			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			82.7%			ICU Level of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
3: Driveway 2A & US79

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (vph)	952	267	0	894	0	225
Future Volume (vph)	952	267	0	894	0	225
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		250	0		0	0
Storage Lanes		1	0		0	1
Taper Length (ft)			100		100	
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frt		0.850				0.865
Flt Protected						
Satd. Flow (prot)	3471	1615	0	3374	0	1644
Flt Permitted						
Satd. Flow (perm)	3471	1615	0	3374	0	1644
Link Speed (mph)	65			65	30	
Link Distance (ft)	1070			739	365	
Travel Time (s)	11.2			7.8	8.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	0%	7%	0%	0%
Adj. Flow (vph)	1035	290	0	972	0	245
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1035	290	0	972	0	245
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	60			60	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 46.9%						
ICU Level of Service A						
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis 3: Driveway 2A & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Volume (veh/h)	952	267	0	894	0	225
Future Volume (Veh/h)	952	267	0	894	0	225
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1035	290	0	972	0	245
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised		Raised			
Median storage veh)	2		2			
Upstream signal (ft)	1070					
pX, platoon unblocked			0.77		0.77	0.77
vC, conflicting volume			1325		1521	518
vC1, stage 1 conf vol					1035	
vC2, stage 2 conf vol					486	
vCu, unblocked vol			833		1086	0
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	71
cM capacity (veh/h)			625		391	843
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	518	518	290	486	486	245
Volume Left	0	0	0	0	0	0
Volume Right	0	0	290	0	0	245
cSH	1700	1700	1700	1700	1700	843
Volume to Capacity	0.30	0.30	0.17	0.29	0.29	0.29
Queue Length 95th (ft)	0	0	0	0	0	30
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	11.0
Lane LOS						B
Approach Delay (s)	0.0				0.0	11.0
Approach LOS						B
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			46.9%	ICU Level of Service		A
Analysis Period (min)			15			

HCM 6th TWSC
3: Driveway 2A & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

Intersection						
Int Delay, s/veh	1.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Traffic Vol, veh/h	952	267	0	894	0	225
Future Vol, veh/h	952	267	0	894	0	225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	250	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	0	7	0	0
Mvmt Flow	1035	290	0	972	0	245
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	-	-	-	518
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.3
Pot Cap-1 Maneuver	-	-	0	-	0	508
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	-	-	-	508
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		18.5	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	508	-	-	-		
HCM Lane V/C Ratio	0.481	-	-	-		
HCM Control Delay (s)	18.5	-	-	-		
HCM Lane LOS	C	-	-	-		
HCM 95th %tile Q(veh)	2.6	-	-	-		

Lanes, Volumes, Timings
4: US79 & Crossover

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

								
Lane Group	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Volume (vph)	50	0	1135	0	855	0	0	0
Future Volume (vph)	50	0	1135	0	855	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		100		0	0	0
Storage Lanes		1		1		0	1	0
Taper Length (ft)		100		100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Frt								
Flt Protected		0.950						
Satd. Flow (prot)	0	1805	3471	1900	3374	0	1900	0
Flt Permitted		0.950						
Satd. Flow (perm)	0	1805	3471	1900	3374	0	1900	0
Link Speed (mph)			65		65		30	
Link Distance (ft)			739		504		317	
Travel Time (s)			7.8		5.3		7.2	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	0%	7%	0%	0%	0%
Adj. Flow (vph)	54	0	1220	0	919	0	0	0
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	54	1220	0	919	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	R NA	Left	Right	Left	Right
Median Width(ft)			60		60		12	
Link Offset(ft)			0		0		0	
Crosswalk Width(ft)			16		16		16	
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9		9	15	9
Sign Control			Free		Free		Stop	
Intersection Summary								
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	41.4%				ICU Level of Service A			
Analysis Period (min)	15							

HCM Unsignalized Intersection Capacity Analysis

4: US79 & Crossover

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development



Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔↔	↔	↔↔		↔	
Traffic Volume (veh/h)	50	0	1135	0	855	0	0	0
Future Volume (Veh/h)	50	0	1135	0	855	0	0	0
Sign Control			Free		Free		Stop	
Grade			0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	1220	0	919	0	0	0
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			Raised		Raised			
Median storage (veh)			2		2			
Upstream signal (ft)								
pX, platoon unblocked	0.00			0.00				
vC, conflicting volume	0	919		0			1529	460
vC1, stage 1 conf vol							919	
vC2, stage 2 conf vol							610	
vCu, unblocked vol	0	919		0			1529	460
tC, single (s)	0.0	4.1		0.0			6.8	6.9
tC, 2 stage (s)							5.8	
tF (s)	0.0	2.2		0.0			3.5	3.3
p0 queue free %	0	100		0			100	100
cM capacity (veh/h)	0	751		0			301	554
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	SB 1	
Volume Total	0	610	610	460	460	0	0	
Volume Left	0	0	0	0	0	0	0	
Volume Right	0	0	0	0	0	0	0	
cSH	1700	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.36	0.36	0.27	0.27	0.00	0.00	
Queue Length 95th (ft)	0	0	0	0	0	0	0	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS							A	
Approach Delay (s)	0.0			0.0			0.0	
Approach LOS							A	
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilization			41.4%		ICU Level of Service			A
Analysis Period (min)			15					

Intersection								
Int Delay, s/veh	0.4							
Movement	EBU	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations								
Traffic Vol, veh/h	50	0	1135	0	855	0	0	0
Future Vol, veh/h	50	0	1135	0	855	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	None
Storage Length	-	100	-	100	-	-	0	-
Veh in Median Storage, #	-	-	0	-	0	-	0	-
Grade, %	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	0	7	0	0	0
Mvmt Flow	54	0	1220	0	919	0	0	0
Major/Minor	Major1		Major2		Minor2			
Conflicting Flow All	919	919	0	1220	-	0	1637	460
Stage 1	-	-	-	-	-	-	919	-
Stage 2	-	-	-	-	-	-	718	-
Critical Hdwy	6.4	4.1	-	6.4	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.8	-
Follow-up Hdwy	2.5	2.2	-	2.5	-	-	3.5	3.3
Pot Cap-1 Maneuver	380	751	-	244	-	0	93	554
Stage 1	-	-	-	-	-	0	354	-
Stage 2	-	-	-	-	-	0	449	-
Platoon blocked, %			-		-			
Mov Cap-1 Maneuver	380	380	-	244	-	-	80	554
Mov Cap-2 Maneuver	-	-	-	-	-	-	80	-
Stage 1	-	-	-	-	-	-	304	-
Stage 2	-	-	-	-	-	-	449	-
Approach	EB		WB		SB			
HCM Control Delay, s	0.7		0		0			
HCM LOS					A			
Minor Lane/Major Mvmt	EBL	EBT	WBU	WBT	SBLn1			
Capacity (veh/h)	380	-	244	-	-			
HCM Lane V/C Ratio	0.141	-	-	-	-			
HCM Control Delay (s)	16	-	0	-	0			
HCM Lane LOS	C	-	A	-	A			
HCM 95th %tile Q(veh)	0.5	-	0	-	-			

Lanes, Volumes, Timings
5: S Rio Grande & US79

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	5	1137	31	6	804	0	22	3	3	1	7	3
Future Volume (vph)	5	1137	31	6	804	0	22	3	3	1	7	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		350	175		330	0		0	0		0
Storage Lanes	1		1	1		1	0		0	0		0
Taper Length (ft)	100			100			0			0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.987			0.966	
Flt Protected	0.950			0.950				0.961			0.996	
Satd. Flow (prot)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1828	0
Flt Permitted	0.950			0.950				0.961			0.996	
Satd. Flow (perm)	1805	3539	1615	1805	3374	1900	0	1802	0	0	1828	0
Link Speed (mph)		65			65			30			30	
Link Distance (ft)		572			566			552			557	
Travel Time (s)		6.0			5.9			12.5			12.7	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	2%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	6	1292	35	7	914	0	25	3	3	1	8	3
Shared Lane Traffic (%)												
Lane Group Flow (vph)	6	1292	35	7	914	0	0	31	0	0	12	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		60			60			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	46.1%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis 5: S Rio Grande & US79

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	1137	31	6	804	0	22	3	3	1	7	3
Future Volume (Veh/h)	5	1137	31	6	804	0	22	3	3	1	7	3
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	1292	35	7	914	0	25	3	3	1	8	3
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	Raised			Raised								
Median storage veh	2			2								
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	914			1292			1779	2232	646	1588	2232	457
vC1, stage 1 conf vol							1304	1304		928	928	
vC2, stage 2 conf vol							475	928		660	1304	
vCu, unblocked vol	914			1292			1779	2232	646	1588	2232	457
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
tC, 2 stage (s)							6.5	5.5		6.5	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			84	98	99	100	96	99
cM capacity (veh/h)	754			543			161	189	419	238	186	556
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	6	646	646	35	7	457	457	0	31	12		
Volume Left	6	0	0	0	7	0	0	0	25	1		
Volume Right	0	0	0	35	0	0	0	0	3	3		
cSH	754	1700	1700	1700	543	1700	1700	1700	174	228		
Volume to Capacity	0.01	0.38	0.38	0.02	0.01	0.27	0.27	0.00	0.18	0.05		
Queue Length 95th (ft)	1	0	0	0	1	0	0	0	16	4		
Control Delay (s)	9.8	0.0	0.0	0.0	11.7	0.0	0.0	0.0	30.2	21.7		
Lane LOS	A				B				D	C		
Approach Delay (s)	0.0				0.1				30.2	21.7		
Approach LOS									D	C		
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	46.1%			ICU Level of Service						A		
Analysis Period (min)	15											

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	5	1137	31	6	804	0	22	3	3	1	7	3
Future Vol, veh/h	5	1137	31	6	804	0	22	3	3	1	7	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Stop	-	-	Stop	-	-	Free	-	-	Free
Storage Length	200	-	350	175	-	330	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	2	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	6	1292	35	7	914	0	25	3	3	1	8	3

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	914	0	0	1292	0	0	1779	2232	-	1588	2232	-
Stage 1	-	-	-	-	-	-	1304	1304	-	928	928	-
Stage 2	-	-	-	-	-	-	475	928	-	660	1304	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	-	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	-	3.5	4	-
Pot Cap-1 Maneuver	754	-	-	543	-	-	53	43	0	74	43	0
Stage 1	-	-	-	-	-	-	172	232	0	292	349	0
Stage 2	-	-	-	-	-	-	545	349	0	423	232	0
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	754	-	-	543	-	-	45	42	-	68	42	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	45	42	-	68	42	-
Stage 1	-	-	-	-	-	-	171	230	-	290	344	-
Stage 2	-	-	-	-	-	-	526	344	-	413	230	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1		
HCM LOS			-	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	754	-	-	543	-	-	-
HCM Lane V/C Ratio	-	0.008	-	-	0.013	-	-	-
HCM Control Delay (s)	-	9.8	-	-	11.7	-	-	-
HCM Lane LOS	-	A	-	-	B	-	-	-
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

Lanes, Volumes, Timings
6: FM973 & Driveway 1B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	0	67	663	116	0	743
Future Volume (vph)	0	67	663	116	0	743
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	150		250	100	
Storage Lanes	0	0		1	0	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865		0.850		
Flt Protected						
Satd. Flow (prot)	0	1644	1827	1615	0	1712
Flt Permitted						
Satd. Flow (perm)	0	1644	1827	1615	0	1712
Link Speed (mph)	30		55			55
Link Distance (ft)	429		545			636
Travel Time (s)	9.8		6.8			7.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	0	73	721	126	0	808
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	73	721	126	0	808
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	Yes		Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	45.7%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 6: FM973 & Driveway 1B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	67	663	116	0	743
Future Volume (Veh/h)	0	67	663	116	0	743
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	73	721	126	0	808
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLT		TWLT	
Median storage veh			2		2	
Upstream signal (ft)			545		636	
pX, platoon unblocked	0.67	0.67			0.67	
vC, conflicting volume	1529	721			847	
vC1, stage 1 conf vol	721					
vC2, stage 2 conf vol	808					
vCu, unblocked vol	1543	332			521	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	85			100	
cM capacity (veh/h)	329	476			704	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	73	721	126	808		
Volume Left	0	0	0	0		
Volume Right	73	0	126	0		
cSH	476	1700	1700	1700		
Volume to Capacity	0.15	0.42	0.07	0.48		
Queue Length 95th (ft)	13	0	0	0		
Control Delay (s)	13.9	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	13.9	0.0		0.0		
Approach LOS	B					
Intersection Summary						
Average Delay		0.6				
Intersection Capacity Utilization		45.7%		ICU Level of Service	A	
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↗	↗		↗
Traffic Vol, veh/h	0	67	663	116	0	743
Future Vol, veh/h	0	67	663	116	0	743
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	250	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	0	73	721	126	0	808
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	-	721	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	431	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	431	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15	0		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBT		
Capacity (veh/h)	-	431		-		
HCM Lane V/C Ratio	-	0.169		-		
HCM Control Delay (s)	-	15		-		
HCM Lane LOS	-	C		-		
HCM 95th %tile Q(veh)	-	0.6		-		

Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	102	37	742	19	75	668
Future Volume (vph)	102	37	742	19	75	668
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	100		250	150	
Storage Lanes	1	1		1	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		40		21		
Link Speed (mph)	30		55			55
Link Distance (ft)	223		672			545
Travel Time (s)	5.1		8.3			6.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	111	40	807	21	82	726
Shared Lane Traffic (%)						
Lane Group Flow (vph)	111	40	807	21	82	726
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	3	1	1	3
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	50	50	250	50	50	250
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	50	50	6	50	50	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			122			122
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Detector 3 Position(ft)			244			244
Detector 3 Size(ft)			6			6

Scenario 1 5:57 pm 08/19/2022 Baseline

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Lanes, Volumes, Timings
7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex			Cl+Ex		
Detector 3 Channel						
Detector 3 Extend (s)	0.0			0.0		
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	17.0	24.0
Total Split (s)	23.0	23.0	40.0	40.0	17.0	57.0
Total Split (%)	28.8%	28.8%	50.0%	50.0%	21.3%	71.3%
Maximum Green (s)	18.0	18.0	34.0	34.0	11.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Act Effect Green (s)	10.5	10.5	49.6	49.6	9.2	62.3
Actuated g/C Ratio	0.13	0.13	0.62	0.62	0.12	0.78
v/c Ratio	0.47	0.16	0.71	0.02	0.39	0.55
Control Delay	38.1	11.5	18.3	2.8	37.8	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.1	11.5	18.3	2.8	37.8	7.0
LOS	D	B	B	A	D	A
Approach Delay	31.1		17.9			10.1
Approach LOS	C		B			B

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 13 (16%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 15.5

Intersection LOS: B

Intersection Capacity Utilization 65.7%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 7: FM973 & Driveway 2B



Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Phasings
7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Minimum Initial (s)	8.0	8.0	10.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	24.0	24.0	17.0	24.0
Total Split (s)	23.0	23.0	40.0	40.0	17.0	57.0
Total Split (%)	28.8%	28.8%	50.0%	50.0%	21.3%	71.3%
Maximum Green (s)	18.0	18.0	34.0	34.0	11.0	51.0
Yellow Time (s)	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	C-Min	C-Min	None	C-Min
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
90th %ile Green (s)	14.1	14.1	36.6	36.6	12.3	54.9
90th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
70th %ile Green (s)	11.8	11.8	40.9	40.9	10.3	57.2
70th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
50th %ile Green (s)	10.2	10.2	43.8	43.8	9.0	58.8
50th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
30th %ile Green (s)	8.6	8.6	46.8	46.8	7.6	60.4
30th %ile Term Code	Gap	Gap	Coord	Coord	Gap	Coord
10th %ile Green (s)	0.0	0.0	74.0	74.0	0.0	74.0
10th %ile Term Code	Skip	Skip	Coord	Coord	Skip	Coord
Intersection Summary						
Cycle Length: 80						
Actuated Cycle Length: 80						
Offset: 13 (16%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Control Type: Actuated-Coordinated						

Queues
7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	111	40	807	21	82	726
v/c Ratio	0.47	0.16	0.71	0.02	0.39	0.55
Control Delay	38.1	11.5	18.3	2.8	37.8	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.1	11.5	18.3	2.8	37.8	7.0
Queue Length 50th (ft)	52	0	303	0	39	133
Queue Length 95th (ft)	96	26	#611	m1	77	265
Internal Link Dist (ft)	143		592			465
Turn Bay Length (ft)		100		250	150	
Base Capacity (vph)	406	394	1133	1009	254	1332
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.10	0.71	0.02	0.32	0.55
Intersection Summary						
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	102	37	742	19	75	668
Future Volume (vph)	102	37	742	19	75	668
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1805	1615	1827	1615	1805	1712
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1805	1615	1827	1615	1805	1712
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	111	40	807	21	82	726
RTOR Reduction (vph)	0	36	0	9	0	0
Lane Group Flow (vph)	111	4	807	12	82	726
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Actuated Green, G (s)	8.9	8.9	46.3	46.3	7.8	60.1
Effective Green, g (s)	8.9	8.9	46.3	46.3	7.8	60.1
Actuated g/C Ratio	0.11	0.11	0.58	0.58	0.10	0.75
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	200	179	1057	934	175	1286
v/s Ratio Prot	c0.06		c0.44		0.05	c0.42
v/s Ratio Perm		0.00		0.01		
v/c Ratio	0.56	0.02	0.76	0.01	0.47	0.56
Uniform Delay, d1	33.7	31.7	12.7	7.2	34.1	4.3
Progression Factor	1.00	1.00	0.84	0.56	1.00	1.00
Incremental Delay, d2	3.3	0.1	5.0	0.0	2.0	1.8
Delay (s)	37.0	31.7	15.6	4.0	36.1	6.1
Level of Service	D	C	B	A	D	A
Approach Delay (s)	35.6		15.4			9.1
Approach LOS	D		B			A
Intersection Summary						
HCM 2000 Control Delay			14.3		HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio			0.74			
Actuated Cycle Length (s)			80.0		Sum of lost time (s)	17.0
Intersection Capacity Utilization			65.7%		ICU Level of Service	C
Analysis Period (min)			15			
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis
7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

									
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations									
Traffic Volume (veh/h)	102	37	742	19	75	668			
Future Volume (veh/h)	102	37	742	19	75	668			
Number	3	18	2	12	1	6			
Initial Q, veh	0	0	0	0	0	0			
Ped-Bike Adj (A_pbT)	1.00	1.00		1.00	1.00				
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach	No		No			No			
Lanes Open During Work Zone									
Adj Sat Flow, veh/h/ln	1900	1900	1841	1900	1900	1737			
Adj Flow Rate, veh/h	111	40	807	21	82	726			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Percent Heavy Veh, %	0	0	4	0	0	11			
Opposing Right Turn Influence	Yes				Yes				
Cap, veh/h	175	155	1137	994	133	1331			
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00			
Prop Arrive On Green	0.10	0.10	0.62	0.62	0.07	0.77			
Unsig. Movement Delay									
Ln Grp Delay, s/veh	38.6	34.3	14.2	6.0	40.6	5.4			
Ln Grp LOS	D	C	B	A	D	A			
Approach Vol, veh/h	151		828			808			
Approach Delay, s/veh	37.5		14.0			8.9			
Approach LOS	D		B			A			
Timer:		1	2	3	4	5	6	7	8
Assigned Phs		1	2	8			6		
Case No		2.0	7.0	9.0			4.0		
Phs Duration (G+Y+Rc), s		11.9	55.4	12.7			67.3		
Change Period (Y+Rc), s		6.0	6.0	5.0			6.0		
Max Green (Gmax), s		11.0	34.0	18.0			51.0		
Max Allow Headway (MAH), s		4.1	6.9	4.4			7.0		
Max Q Clear (g_c+l1), s		5.5	25.9	6.7			15.4		
Green Ext Time (g_e), s		0.1	5.1	0.4			11.2		
Prob of Phs Call (p_c)		0.84	1.00	0.97			1.00		
Prob of Max Out (p_x)		0.23	0.92	0.01			0.10		
Left-Turn Movement Data									
Assigned Mvmt		1	5	3					
Mvmt Sat Flow, veh/h		1810	0	1810					
Through Movement Data									
Assigned Mvmt			2	8			6		
Mvmt Sat Flow, veh/h			1841	0			1737		
Right-Turn Movement Data									
Assigned Mvmt			12	18			16		
Mvmt Sat Flow, veh/h			1610	1610			0		
Left Lane Group Data									
Assigned Mvmt		1	5	3	0	0	0	0	0
Lane Assignment		L (Prot)		L					

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

Lanes in Grp	1	0	1	0	0	0	0	0
Grp Vol (v), veh/h	82	0	111	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1810	0	1810	0	0	0	0	0
Q Serve Time (g_s), s	3.5	0.0	4.7	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	3.5	0.0	4.7	0.0	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	1810	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	49.4	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	133	0	175	0	0	0	0	0
V/C Ratio (X)	0.62	0.00	0.64	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	249	0	407	0	0	0	0	0
Upstream Filter (I)	1.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	36.0	0.0	34.8	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	4.6	0.0	3.8	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	40.6	0.0	38.6	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	1.4	0.0	2.0	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.2	0.0	0.2	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	1.6	0.0	2.2	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.27	0.00	0.36	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Middle Lane Group Data								
Assigned Mvmt	0	2	8	0	0	6	0	0
Lane Assignment	T			T				
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	807	0	0	0	726	0	0
Grp Sat Flow (s), veh/h/ln	0	1841	0	0	0	1737	0	0
Q Serve Time (g_s), s	0.0	23.9	0.0	0.0	0.0	13.4	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	23.9	0.0	0.0	0.0	13.4	0.0	0.0
Lane Grp Cap (c), veh/h	0	1137	0	0	0	1331	0	0
V/C Ratio (X)	0.00	0.71	0.00	0.00	0.00	0.55	0.00	0.00
Avail Cap (c_a), veh/h	0	1137	0	0	0	1331	0	0
Upstream Filter (I)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	10.4	0.0	0.0	0.0	3.8	0.0	0.0
Incr Delay (d2), s/veh	0.0	3.8	0.0	0.0	0.0	1.6	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	14.2	0.0	0.0	0.0	5.4	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	6.5	0.0	0.0	0.0	1.5	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	1.2	0.0	0.0	0.0	0.6	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis

7: FM973 & Driveway 2B

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	7.6	0.0	0.0	0.0	2.1	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.32	0.00	0.00	0.00	0.12	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Right Lane Group Data

Assigned Mvmt	0	12	18	0	0	16	0	0
Lane Assignment		R	R					
Lanes in Grp	0	1	1	0	0	0	0	0
Grp Vol (v), veh/h	0	21	40	0	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1610	1610	0	0	0	0	0
Q Serve Time (g_s), s	0.0	0.4	1.8	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.4	1.8	0.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	994	155	0	0	0	0	0
V/C Ratio (X)	0.00	0.02	0.26	0.00	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	994	362	0	0	0	0	0
Upstream Filter (I)	0.00	1.00	1.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	5.9	33.5	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.9	0.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	6.0	34.3	0.0	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.1	0.7	0.0	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.1	0.7	0.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.01	0.19	0.00	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	13.7
HCM 6th LOS	B

Lanes, Volumes, Timings
8: FM973 & Taylor HS

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	28	738	4	13	778
Future Volume (vph)	8	28	738	4	13	778
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	100				100	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.894		0.999			
Flt Protected	0.989				0.950	
Satd. Flow (prot)	1680	0	1825	0	1805	1712
Flt Permitted	0.989				0.950	
Satd. Flow (perm)	1680	0	1825	0	1805	1712
Link Speed (mph)	30		65			65
Link Distance (ft)	376		460			672
Travel Time (s)	8.5		4.8			7.0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	4%	0%	0%	11%
Adj. Flow (vph)	8	29	761	4	13	802
Shared Lane Traffic (%)						
Lane Group Flow (vph)	37	0	765	0	13	802
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization 50.9%			ICU Level of Service A			
Analysis Period (min) 15						

HCM Unsignalized Intersection Capacity Analysis 8: FM973 & Taylor HS

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	28	738	4	13	778
Future Volume (Veh/h)	8	28	738	4	13	778
Sign Control	Stop		Free		Free	Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	8	29	761	4	13	802
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL		TWLTL	
Median storage veh			2		2	
Upstream signal (ft)			1184		672	
pX, platoon unblocked	0.90	0.81			0.81	
vC, conflicting volume	1591	763			765	
vC1, stage 1 conf vol	763					
vC2, stage 2 conf vol	828					
vCu, unblocked vol	1130	588			591	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	93			98	
cM capacity (veh/h)	344	414			804	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	37	765	13	802		
Volume Left	8	0	13	0		
Volume Right	29	4	0	0		
cSH	397	1700	804	1700		
Volume to Capacity	0.09	0.45	0.02	0.47		
Queue Length 95th (ft)	8	0	1	0		
Control Delay (s)	15.0	0.0	9.6	0.0		
Lane LOS	C		A			
Approach Delay (s)	15.0	0.0	0.2			
Approach LOS	C					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			50.9%	ICU Level of Service	A	
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	28	738	4	13	778
Future Vol, veh/h	8	28	738	4	13	778
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	4	0	0	11
Mvmt Flow	8	29	761	4	13	802
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1591	763	0	0	765	0
Stage 1	763	-	-	-	-	-
Stage 2	828	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	119	408	-	-	857	-
Stage 1	464	-	-	-	-	-
Stage 2	432	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	117	408	-	-	857	-
Mov Cap-2 Maneuver	256	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	426	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	16.1	0		0.2		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	360	857	-	
HCM Lane V/C Ratio	-	-	0.103	0.016	-	
HCM Control Delay (s)	-	-	16.1	9.3	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

Lanes, Volumes, Timings
9: FM973 & Streamwood

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	63	22	38	673	692	98
Future Volume (vph)	63	22	38	673	692	98
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			250
Storage Lanes	1	0	1			1
Taper Length (ft)	100		100			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.965					0.850
Flt Protected	0.964		0.950			
Satd. Flow (prot)	1767	0	1805	1827	1696	1615
Flt Permitted	0.964		0.950			
Satd. Flow (perm)	1767	0	1805	1827	1696	1615
Link Speed (mph)	30			65	65	
Link Distance (ft)	373			724	460	
Travel Time (s)	8.5			7.6	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	12%	0%
Adj. Flow (vph)	68	24	41	732	752	107
Shared Lane Traffic (%)						
Lane Group Flow (vph)	92	0	41	732	752	107
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				Yes	Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	47.9%			ICU Level of Service A		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis

9: FM973 & Streamwood

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	63	22	38	673	692	98
Future Volume (Veh/h)	63	22	38	673	692	98
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	68	24	41	732	752	107
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh				2	2	
Upstream signal (ft)				724	1132	
pX, platoon unblocked	0.88	0.84	0.84			
vC, conflicting volume	1566	752	859			
vC1, stage 1 conf vol	752					
vC2, stage 2 conf vol	814					
vCu, unblocked vol	1167	610	737			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3	2.2			
p0 queue free %	80	94	94			
cM capacity (veh/h)	335	419	738			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	92	41	732	752	107	
Volume Left	68	41	0	0	0	
Volume Right	24	0	0	0	107	
cSH	353	738	1700	1700	1700	
Volume to Capacity	0.26	0.06	0.43	0.44	0.06	
Queue Length 95th (ft)	26	4	0	0	0	
Control Delay (s)	18.8	10.2	0.0	0.0	0.0	
Lane LOS	C	B				
Approach Delay (s)	18.8	0.5		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay		1.2				
Intersection Capacity Utilization		47.9%		ICU Level of Service		A
Analysis Period (min)		15				

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	63	22	38	673	692	98
Future Vol, veh/h	63	22	38	673	692	98
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	250
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	12	0
Mvmt Flow	68	24	41	732	752	107
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	1566	752	859	0	-	0
Stage 1	752	-	-	-	-	-
Stage 2	814	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	124	413	791	-	-	-
Stage 1	469	-	-	-	-	-
Stage 2	439	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	118	413	791	-	-	-
Mov Cap-2 Maneuver	256	-	-	-	-	-
Stage 1	445	-	-	-	-	-
Stage 2	439	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	23.7	0.5		0		
HCM LOS	C					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	791	-	284	-	-	
HCM Lane V/C Ratio	0.052	-	0.325	-	-	
HCM Control Delay (s)	9.8	-	23.7	-	-	
HCM Lane LOS	A	-	C	-	-	
HCM 95th %tile Q(veh)	0.2	-	1.4	-	-	

Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Future Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	80		0	400		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.958			0.897						0.997	
Flt Protected		0.967			0.988		0.950			0.950		
Satd. Flow (prot)	0	1760	0	0	1684	0	1805	1827	0	1805	1710	0
Flt Permitted					0.914		0.950			0.950		
Satd. Flow (perm)	0	1820	0	0	1558	0	1805	1827	0	1805	1710	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		123			123						2	
Link Speed (mph)		30			30			65			65	
Link Distance (ft)		574			560			730			724	
Travel Time (s)		13.0			12.7			7.7			7.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Adj. Flow (vph)	9	0	4	5	0	16	4	743	0	3	755	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	21	0	4	743	0	3	769	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
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Lanes, Volumes, Timings
10: FM973 & Wesley Miller

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8								
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0		8.0	8.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		13.0	24.0		13.0	24.0	
Total Split (s)	23.0	23.0		23.0	23.0		13.0	44.0		13.0	44.0	
Total Split (%)	28.8%	28.8%		28.8%	28.8%		16.3%	55.0%		16.3%	55.0%	
Maximum Green (s)	18.0	18.0		18.0	18.0		7.0	38.0		7.0	38.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effect Green (s)		8.0			8.0		7.0	69.8		7.0	69.8	
Actuated g/C Ratio		0.10			0.10		0.09	0.87		0.09	0.87	
v/c Ratio		0.04			0.08		0.03	0.47		0.02	0.52	
Control Delay		0.2			0.6		33.8	5.5		30.7	5.8	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		0.2			0.6		33.8	5.5		30.7	5.8	
LOS		A			A		C	A		C	A	
Approach Delay		0.3			0.6			5.7			5.9	
Approach LOS		A			A			A			A	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 5.7

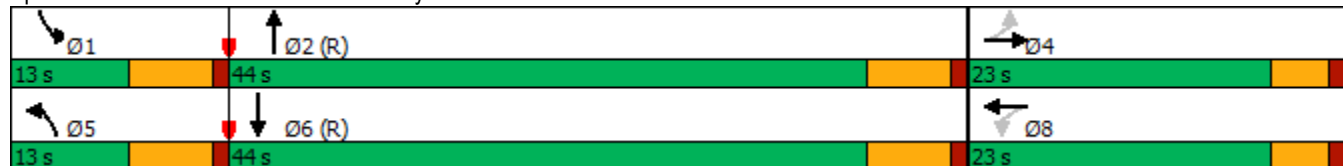
Intersection LOS: A

Intersection Capacity Utilization 53.2%

ICU Level of Service A

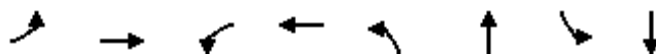
Analysis Period (min) 15

Splits and Phases: 10: FM973 & Wesley Miller



Phasings
10: FM973 & Wesley Miller

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Protected Phases		4		8	5	2	1	6
Permitted Phases	4		8					
Minimum Initial (s)	8.0	8.0	8.0	8.0	7.0	10.0	7.0	10.0
Minimum Split (s)	23.0	23.0	23.0	23.0	13.0	24.0	13.0	24.0
Total Split (s)	23.0	23.0	23.0	23.0	13.0	44.0	13.0	44.0
Total Split (%)	28.8%	28.8%	28.8%	28.8%	16.3%	55.0%	16.3%	55.0%
Maximum Green (s)	18.0	18.0	18.0	18.0	7.0	38.0	7.0	38.0
Yellow Time (s)	3.5	3.5	3.5	3.5	5.0	5.0	5.0	5.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.0	1.0	1.0	1.0
Lead/Lag					Lead	Lag	Lead	Lag
Lead-Lag Optimize?					Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	None	None	None	None	C-Min	None	C-Min
Walk Time (s)								
Flash Dont Walk (s)								
Pedestrian Calls (#/hr)								
90th %ile Green (s)	8.0	8.0	8.0	8.0	7.0	48.0	7.0	48.0
90th %ile Term Code	Min	Min	Min	Min	Min	Coord	Min	Coord
70th %ile Green (s)	8.0	8.0	8.0	8.0	0.0	61.0	0.0	61.0
70th %ile Term Code	Hold	Hold	Min	Min	Skip	Coord	Skip	Coord
50th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
50th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
30th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
30th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
10th %ile Green (s)	0.0	0.0	0.0	0.0	0.0	74.0	0.0	74.0
10th %ile Term Code	Skip	Skip	Skip	Skip	Skip	Coord	Skip	Coord
Intersection Summary								
Cycle Length: 80								
Actuated Cycle Length: 80								
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green								
Control Type: Actuated-Coordinated								

Queues
10: FM973 & Wesley Miller

2027 PM Peak Hour (w/New Signals)
Phase 2 Build-Out Year w/Development

	→	←	↖	↑	↗	↓
Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	13	21	4	743	3	769
v/c Ratio	0.04	0.08	0.03	0.47	0.02	0.52
Control Delay	0.2	0.6	33.8	5.5	30.7	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.2	0.6	33.8	5.5	30.7	5.8
Queue Length 50th (ft)	0	0	2	0	1	0
Queue Length 95th (ft)	0	0	11	350	m4	267
Internal Link Dist (ft)	494	480		650		644
Turn Bay Length (ft)			80		400	
Base Capacity (vph)	504	445	157	1594	157	1492
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.05	0.03	0.47	0.02	0.52
Intersection Summary						
m Volume for 95th percentile queue is metered by upstream signal.						

HCM Signalized Intersection Capacity Analysis

10: FM973 & Wesley Miller

2027 PM Peak Hour (w/New Signals)

Phase 2 Build-Out Year w/Development

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Future Volume (vph)	8	0	4	5	0	15	4	684	0	3	695	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.96			0.90		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1760			1685		1805	1827		1805	1710	
Flt Permitted		1.00			0.91		0.95	1.00		0.95	1.00	
Satd. Flow (perm)		1821			1558		1805	1827		1805	1710	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	0	4	5	0	16	4	743	0	3	755	14
RTOR Reduction (vph)	0	12	0	0	20	0	0	0	0	0	1	0
Lane Group Flow (vph)	0	1	0	0	1	0	4	743	0	3	768	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	4%	0%	0%	11%	0%
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								
Actuated Green, G (s)		3.2			3.2		1.4	58.4		1.4	58.4	
Effective Green, g (s)		3.2			3.2		1.4	58.4		1.4	58.4	
Actuated g/C Ratio		0.04			0.04		0.02	0.73		0.02	0.73	
Clearance Time (s)		5.0			5.0		6.0	6.0		6.0	6.0	
Vehicle Extension (s)		3.0			3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)		72			62		31	1333		31	1248	
v/s Ratio Prot							c0.00	0.41		0.00	c0.45	
v/s Ratio Perm		0.00			c0.00							
v/c Ratio		0.01			0.01		0.13	0.56		0.10	0.62	
Uniform Delay, d1		36.9			36.9		38.7	4.9		38.7	5.3	
Progression Factor		1.00			1.00		1.00	1.00		0.91	0.91	
Incremental Delay, d2		0.0			0.1		1.9	1.7		1.2	2.1	
Delay (s)		36.9			37.0		40.6	6.6		36.4	6.9	
Level of Service		D			D		D	A		D	A	
Approach Delay (s)		36.9			37.0			6.8			7.0	
Approach LOS		D			D			A			A	
Intersection Summary												
HCM 2000 Control Delay		7.6			HCM 2000 Level of Service			A				
HCM 2000 Volume to Capacity ratio		0.57										
Actuated Cycle Length (s)		80.0			Sum of lost time (s)			17.0				
Intersection Capacity Utilization		53.2%			ICU Level of Service			A				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Appendix D. Traffic Simulation Output

- Phase 2 Buildout Year (2027) Projected Conditions with Development, AM & PM
 - Mitigation:
 - With Dual Left Turn and Right Turn Only Lane on Northbound FM 973 Approach to US 79 and with Optimized Timings at FM 973 at US 79
 - With Traffic Signal at FM 973 at Driveway 2B (half cycle to US 79 intersection)
 - With Traffic Signal at FM 973 at Wesley Miller Lane (half cycle to US 79 intersection)

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/LTx2 on FM973/79)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		450	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt				0.850		0.999				0.850		0.865
Flt Protected		0.950			0.950			0.950	0.950			
Satd. Flow (prot)	0	1805	3312	1553	1719	3371	0	1588	1588	1568	0	1644
Flt Permitted		0.950			0.950			0.950	0.950			
Satd. Flow (perm)	0	1805	3312	1553	1719	3371	0	1588	1588	1568	0	1644
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				753						218		267
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1072			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
Shared Lane Traffic (%)								50%				
Lane Group Flow (vph)	0	9	600	833	528	709	0	297	298	218	0	1
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			12			12
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.94
Heavy Vehicles (%)	0%
Adj. Flow (vph)	1
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/LTx2 on FM973/79)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	31.0	31.0	61.0	61.0	31.0	61.0		57.0	57.0	57.0	11.0	11.0
Total Split (%)	19.4%	19.4%	38.1%	38.1%	19.4%	38.1%		35.6%	35.6%	35.6%	6.9%	6.9%
Maximum Green (s)	25.0	25.0	55.0	55.0	25.0	55.0		50.0	50.0	50.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.5	36.0	36.0	26.2	66.2		27.7	27.7	27.7		5.2
Actuated g/C Ratio		0.09	0.32	0.32	0.24	0.59		0.25	0.25	0.25		0.05
v/c Ratio		0.05	0.56	0.82	1.31	0.35		0.75	0.75	0.39		0.00
Control Delay		59.1	34.0	12.4	190.3	14.8		52.8	53.0	7.0		0.0
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Delay		59.1	34.0	12.4	190.3	14.8		52.8	53.0	7.0		0.0
LOS		E	C	B	F	B		D	D	A		A
Approach Delay			21.7			89.7			40.6			
Approach LOS			C			F			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 111.4

Natural Cycle: 130

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.31

Intersection Signal Delay: 50.2

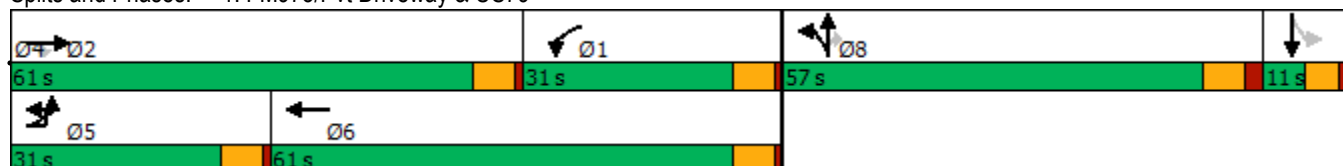
Intersection LOS: D

Intersection Capacity Utilization 95.1%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79



Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/LTx2 on FM973/79)
Phase 2 Build-Out Year w/Development

									
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBT
Protected Phases	5	2		1	6	8	8		4
Permitted Phases			2					8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	12.0	11.0
Total Split (s)	31.0	61.0	61.0	31.0	61.0	57.0	57.0	57.0	11.0
Total Split (%)	19.4%	38.1%	38.1%	19.4%	38.1%	35.6%	35.6%	35.6%	6.9%
Maximum Green (s)	25.0	55.0	55.0	25.0	55.0	50.0	50.0	50.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None	None
Walk Time (s)									
Flash Dont Walk (s)									
Pedestrian Calls (#/hr)									
90th %ile Green (s)	10.0	55.0	55.0	25.0	70.0	45.7	45.7	45.7	5.0
90th %ile Term Code	Min	Max	Max	Max	Hold	Gap	Gap	Gap	Max
70th %ile Green (s)	0.0	42.3	42.3	25.0	73.3	32.4	32.4	32.4	0.0
70th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
50th %ile Green (s)	0.0	34.9	34.9	25.0	65.9	26.7	26.7	26.7	0.0
50th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
30th %ile Green (s)	0.0	28.9	28.9	25.0	59.9	22.1	22.1	22.1	0.0
30th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
10th %ile Green (s)	0.0	22.1	22.1	25.0	53.1	15.8	15.8	15.8	0.0
10th %ile Term Code	Skip	Gap	Gap	Max	Hold	Gap	Gap	Gap	Skip
Intersection Summary									
Cycle Length: 160									
Actuated Cycle Length: 111.4									
Control Type: Actuated-Uncoordinated									
90th %ile Actuated Cycle: 155.7									
70th %ile Actuated Cycle: 118.7									
50th %ile Actuated Cycle: 105.6									
30th %ile Actuated Cycle: 95									
10th %ile Actuated Cycle: 81.9									

Queues

2027 AM Peak Hour (w/LTx2 on FM973/79)

1: FM973/Pvt Driveway & US79

Phase 2 Build-Out Year w/Development

									
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBT
Lane Group Flow (vph)	9	600	833	528	709	297	298	218	1
v/c Ratio	0.05	0.56	0.82	1.31	0.35	0.75	0.75	0.39	0.00
Control Delay	59.1	34.0	12.4	190.3	14.8	52.8	53.0	7.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.1	34.0	12.4	190.3	14.8	52.8	53.0	7.0	0.0
Queue Length 50th (ft)	6	173	38	~461	107	194	195	0	0
Queue Length 95th (ft)	30	315	264	#1065	308	392	395	64	0
Internal Link Dist (ft)		854			992		556		491
Turn Bay Length (ft)	100		615	880		250		450	
Base Capacity (vph)	424	1714	1167	404	2017	746	746	853	332
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.35	0.71	1.31	0.35	0.40	0.40	0.26	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: FM973/Pvt Driveway & US79

2027 AM Peak Hour (w/LTx2 on FM973/79)

Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Future Volume (vph)	8	0	564	783	496	664	3	559	0	205	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		1.00
Frt		1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85		0.86
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		1.00
Satd. Flow (prot)		1805	3312	1553	1719	3373		1588	1588	1568		1644
Flt Permitted		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		1.00
Satd. Flow (perm)		1805	3312	1553	1719	3373		1588	1588	1568		1644
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	9	0	600	833	528	706	3	595	0	218	0	0
RTOR Reduction (vph)	0	0	0	529	0	0	0	0	0	168	0	1
Lane Group Flow (vph)	0	9	600	304	528	709	0	297	298	50	0	0
Heavy Vehicles (%)	0%	0%	9%	4%	5%	7%	0%	8%	0%	3%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		NA
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		1.4	36.0	36.0	31.6	66.2		27.7	27.7	27.7		0.7
Effective Green, g (s)		1.4	36.0	36.0	31.6	66.2		27.7	27.7	27.7		0.7
Actuated g/C Ratio		0.01	0.30	0.30	0.26	0.55		0.23	0.23	0.23		0.01
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0		2.0
Lane Grp Cap (vph)		20	985	462	448	1845		363	363	358		9
v/s Ratio Prot		0.00	0.18		c0.31	0.21		0.19	c0.19			c0.00
v/s Ratio Perm				c0.20						0.03		
v/c Ratio		0.45	0.61	0.66	1.18	0.38		0.82	0.82	0.14		0.00
Uniform Delay, d1		59.4	36.5	37.1	44.7	15.7		44.3	44.3	37.2		59.8
Progression Factor		1.00	1.00	1.00	0.98	0.97		1.00	1.00	1.00		1.00
Incremental Delay, d2		5.8	0.7	2.6	101.4	0.0		13.4	13.8	0.2		0.0
Delay (s)		65.2	37.2	39.7	145.3	15.3		57.6	58.1	37.3		59.8
Level of Service		E	D	D	F	B		E	E	D		E
Approach Delay (s)			38.8			70.8			52.4			59.8
Approach LOS			D			E			D			E
Intersection Summary												
HCM 2000 Control Delay			53.3									HCM 2000 Level of Service D
HCM 2000 Volume to Capacity ratio			0.87									
Actuated Cycle Length (s)			121.0							25.0		
Intersection Capacity Utilization			95.1%									ICU Level of Service F
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	1
Future Volume (vph)	1
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	1
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/LTx2 on FM 973/79)

Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		100		615	880		0	250		450	0	
Storage Lanes		1		1	1		0	1		1	0	
Taper Length (ft)		100			100			100			100	
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt				0.850		0.999				0.850		
Flt Protected		0.950			0.950			0.950	0.950			
Satd. Flow (prot)	0	1805	3471	1455	1719	3341	0	1618	1618	1583	0	1900
Flt Permitted		0.950			0.950			0.950	0.950			
Satd. Flow (perm)	0	1805	3471	1455	1719	3341	0	1618	1618	1583	0	1900
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)				415		1				332		
Link Speed (mph)			65			65			55			55
Link Distance (ft)			934			1070			636			571
Travel Time (s)			9.8			11.2			7.9			7.1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
Shared Lane Traffic (%)								50%				
Lane Group Flow (vph)	0	10	954	415	380	527	0	235	235	332	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	R NA	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)			60			60			12			12
Link Offset(ft)			0			0			0			0
Crosswalk Width(ft)			16			16			16			16
Two way Left Turn Lane									Yes			Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	9	15		9	15		9	15		9	15	
Number of Detectors	1	1	4	1	1	4		1	1	1	1	1
Detector Template	Left	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	50	350	50	50	350		20	50	50	20	50
Trailing Detector (ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	50	6	50	50	6		20	50	50	20	50
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			114			114						
Detector 2 Size(ft)			6			6						
Detector 2 Type			Cl+Ex			Cl+Ex						
Detector 2 Channel												
Detector 2 Extend (s)			0.0			0.0						
Detector 3 Position(ft)			229			229						
Detector 3 Size(ft)			6			6						

Scenario 1 5:57 pm 08/19/2022 Baseline

Synchro 11 Report
Page 1

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Storage Length (ft)	0
Storage Lanes	0
Taper Length (ft)	
Lane Util. Factor	1.00
Frt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	0.91
Heavy Vehicles (%)	0%
Adj. Flow (vph)	0
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Detector 3 Position(ft)	
Detector 3 Size(ft)	

Lanes, Volumes, Timings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/LTx2 on FM 973/79)
Phase 2 Build-Out Year w/Development

												
Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Detector 3 Type	Cl+Ex				Cl+Ex							
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0							
Detector 4 Position(ft)	344				344							
Detector 4 Size(ft)	6				6							
Detector 4 Type	Cl+Ex				Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)	0.0				0.0							
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Detector Phase	5	5	2	2	1	6		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0	15.0	15.0	5.0	15.0		5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	16.0	21.0	21.0	11.0	21.0		12.0	12.0	12.0	11.0	11.0
Total Split (s)	16.0	16.0	60.0	60.0	51.0	95.0		38.0	38.0	38.0	11.0	11.0
Total Split (%)	10.0%	10.0%	37.5%	37.5%	31.9%	59.4%		23.8%	23.8%	23.8%	6.9%	6.9%
Maximum Green (s)	10.0	10.0	54.0	54.0	45.0	89.0		31.0	31.0	31.0	5.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0		2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		6.0
Lead/Lag	Lead	Lead	Lead	Lead	Lag	Lag		Lead	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0	2.0	2.0
Recall Mode	None	None	Min	Min	None	Min		None	None	None	None	None
Act Effct Green (s)		10.5	45.4	45.4	32.9	79.2		24.3	24.3	24.3		
Actuated g/C Ratio		0.09	0.37	0.37	0.27	0.65		0.20	0.20	0.20		
v/c Ratio		0.06	0.74	0.52	0.82	0.24		0.73	0.73	0.57		
Control Delay		64.1	39.0	5.4	58.7	10.4		63.4	63.4	9.1		
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0		
Total Delay		64.1	39.0	5.4	58.7	10.4		63.4	63.4	9.1		
LOS		E	D	A	E	B		E	E	A		
Approach Delay			29.1			30.6			40.9			
Approach LOS			C			C			D			

Intersection Summary

Area Type: Other

Cycle Length: 160

Actuated Cycle Length: 122.5

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 32.6

Intersection LOS: C

Intersection Capacity Utilization 77.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: FM973/Pvt Driveway & US79

 02	 01	 08	
60 s	51 s	38 s	11 s
 05	 06		
16 s	95 s		

Lane Group	SBR
Detector 3 Type	
Detector 3 Channel	
Detector 3 Extend (s)	
Detector 4 Position(ft)	
Detector 4 Size(ft)	
Detector 4 Type	
Detector 4 Channel	
Detector 4 Extend (s)	
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Phasings
1: FM973/Pvt Driveway & US79

2027 PM Peak Hour (w/LTx2 on FM 973/79)
Phase 2 Build-Out Year w/Development



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	Ø4
Protected Phases	5	2		1	6	8	8		4
Permitted Phases			2					8	
Minimum Initial (s)	10.0	15.0	15.0	5.0	15.0	5.0	5.0	5.0	5.0
Minimum Split (s)	16.0	21.0	21.0	11.0	21.0	12.0	12.0	12.0	11.0
Total Split (s)	16.0	60.0	60.0	51.0	95.0	38.0	38.0	38.0	11.0
Total Split (%)	10.0%	37.5%	37.5%	31.9%	59.4%	23.8%	23.8%	23.8%	7%
Maximum Green (s)	10.0	54.0	54.0	45.0	89.0	31.0	31.0	31.0	5.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead	Lead	Lag	Lag	Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Minimum Gap (s)	2.0	2.0	2.0	2.0	2.0	3.0	3.0	3.0	2.0
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	Min	None	Min	None	None	None	None
Walk Time (s)									
Flash Dont Walk (s)									
Pedestrian Calls (#/hr)									
90th %ile Green (s)	10.0	54.0	54.0	45.0	89.0	31.0	31.0	31.0	0.0
90th %ile Term Code	Max	Max	Max	Max	Hold	Max	Max	Max	Skip
70th %ile Green (s)	10.0	54.0	54.0	40.4	84.4	31.0	31.0	31.0	0.0
70th %ile Term Code	Max	Max	Max	Gap	Hold	Max	Max	Max	Skip
50th %ile Green (s)	0.0	50.2	50.2	34.6	90.8	26.2	26.2	26.2	0.0
50th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
30th %ile Green (s)	0.0	40.0	40.0	27.7	73.7	20.4	20.4	20.4	0.0
30th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip
10th %ile Green (s)	0.0	29.1	29.1	19.4	54.5	14.4	14.4	14.4	0.0
10th %ile Term Code	Skip	Gap	Gap	Gap	Hold	Gap	Gap	Gap	Skip

Intersection Summary

Cycle Length: 160

Actuated Cycle Length: 122.5

Control Type: Actuated-Uncoordinated

90th %ile Actuated Cycle: 149

70th %ile Actuated Cycle: 144.4

50th %ile Actuated Cycle: 130

30th %ile Actuated Cycle: 107.1

10th %ile Actuated Cycle: 81.9

Queues

2027 PM Peak Hour (w/LTx2 on FM 973/79)

1: FM973/Pvt Driveway & US79

Phase 2 Build-Out Year w/Development

								
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR
Lane Group Flow (vph)	10	954	415	380	527	235	235	332
v/c Ratio	0.06	0.74	0.52	0.82	0.24	0.73	0.73	0.57
Control Delay	64.1	39.0	5.4	58.7	10.4	63.4	63.4	9.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.1	39.0	5.4	58.7	10.4	63.4	63.4	9.1
Queue Length 50th (ft)	8	355	0	303	72	196	196	0
Queue Length 95th (ft)	30	513	76	461	151	330	330	87
Internal Link Dist (ft)		854			990		556	
Turn Bay Length (ft)	100		615	880		250		450
Base Capacity (vph)	154	1607	896	663	2501	430	430	664
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.59	0.46	0.57	0.21	0.55	0.55	0.50
Intersection Summary								

HCM Signalized Intersection Capacity Analysis 2027 PM Peak Hour (w/LTx2 on FM 973/79)
 1: FM973/Pvt Driveway & US79 Phase 2 Build-Out Year w/Development

												
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Future Volume (vph)	9	0	868	378	346	477	3	428	0	302	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		
Frt		1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		
Satd. Flow (prot)		1805	3471	1455	1719	3341		1618	1618	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00		0.95	0.95	1.00		
Satd. Flow (perm)		1805	3471	1455	1719	3341		1618	1618	1583		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	10	0	954	415	380	524	3	470	0	332	0	0
RTOR Reduction (vph)	0	0	0	265	0	0	0	0	0	268	0	0
Lane Group Flow (vph)	0	10	954	150	380	527	0	235	235	64	0	0
Heavy Vehicles (%)	0%	0%	4%	11%	5%	8%	0%	6%	0%	2%	0%	0%
Turn Type	Prot	Prot	NA	Perm	Prot	NA		Split	NA	Perm		
Protected Phases	5	5	2		1	6		8	8			4
Permitted Phases				2						8	4	
Actuated Green, G (s)		3.3	45.4	45.4	37.1	79.2		24.3	24.3	24.3		
Effective Green, g (s)		3.3	45.4	45.4	37.1	79.2		24.3	24.3	24.3		
Actuated g/C Ratio		0.03	0.36	0.36	0.29	0.63		0.19	0.19	0.19		
Clearance Time (s)		6.0	6.0	6.0	6.0	6.0		7.0	7.0	7.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0		3.0	3.0	3.0		
Lane Grp Cap (vph)		47	1252	525	506	2103		312	312	305		
v/s Ratio Prot		0.01	c0.27		c0.22	0.16		c0.15	0.15			
v/s Ratio Perm				0.10						0.04		
v/c Ratio		0.21	0.76	0.29	0.75	0.25		0.75	0.75	0.21		
Uniform Delay, d1		60.0	35.4	28.6	40.2	10.2		47.9	47.9	42.7		
Progression Factor		1.00	1.00	1.00	0.98	0.97		1.00	1.00	1.00		
Incremental Delay, d2		0.8	2.5	0.1	5.5	0.0		9.9	9.9	0.3		
Delay (s)		60.8	38.0	28.8	44.9	9.9		57.8	57.8	43.0		
Level of Service		E	D	C	D	A		E	E	D		
Approach Delay (s)			35.4			24.6			51.7			0.0
Approach LOS			D			C			D			A
Intersection Summary												
HCM 2000 Control Delay			36.4			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.80									
Actuated Cycle Length (s)			125.8			Sum of lost time (s)			25.0			
Intersection Capacity Utilization			77.7%			ICU Level of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.91
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 6th Edition methodology supports speed limit in the range of 25 to 55 mph.

Appendix E – Traffic Signal Warrant Analysis

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	221			
Land Use	Multifamily Housing Mid Rise			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	6			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	1.2%	0.4%	1	0
1-2 AM	0.6%	0.3%	0	0
2-3 AM	0.3%	0.1%	0	0
3-4 AM	0.2%	0.2%	0	0
4-5 AM	0.1%	0.5%	0	0
5-6 AM	0.4%	2.0%	0	2
6-7 AM	1.0%	7.8%	1	7
7-8 AM	2.5%	14.7%	2	12
8-9 AM	3.0%	12.5%	2	11
9-10 AM	2.2%	6.9%	2	6
10-11 AM	2.7%	4.6%	2	4
11-12 PM	3.4%	4.0%	3	3
12-1 PM	4.3%	4.8%	4	4
1-2 PM	4.4%	4.4%	4	4
2-3 PM	4.1%	3.7%	3	3
3-4 PM	5.9%	3.8%	5	3
4-5 PM	9.2%	5.1%	8	4
5-6 PM	13.1%	5.8%	11	5
6-7 PM	12.1%	6.0%	10	5
7-8 PM	9.4%	5.4%	8	5
8-9 PM	7.7%	3.1%	6	3
9-10 PM	6.5%	1.5%	5	1
10-11 PM	3.7%	1.6%	3	1
11-12 AM	2.1%	0.8%	2	1

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	821 (as 820)			
Land Use	Shopping Plaza			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	6			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	0.1%	0.1%	0	0
1-2 AM	0.1%	0.1%	0	0
2-3 AM	0.1%	0.1%	0	0
3-4 AM	0.1%	0.1%	0	0
4-5 AM	0.1%	0.1%	0	0
5-6 AM	0.4%	0.4%	1	1
6-7 AM	0.6%	0.8%	2	3
7-8 AM	1.5%	1.9%	5	6
8-9 AM	2.3%	2.8%	7	9
9-10 AM	3.9%	4.8%	12	15
10-11 AM	9.8%	6.8%	18	21
11-12 PM	7.5%	8.2%	23	25
12-1 PM	8.9%	9.3%	27	29
1-2 PM	8.5%	8.4%	26	26
2-3 PM	7.9%	7.5%	24	23
3-4 PM	7.9%	7.8%	25	24
4-5 PM	8.3%	8.3%	26	26
5-6 PM	8.5%	8.4%	26	26
6-7 PM	7.4%	7.4%	23	23
7-8 PM	6.5%	6.3%	20	19
8-9 PM	5.1%	4.1%	16	13
9-10 PM	3.1%	1.8%	10	6
10-11 PM	3.6%	3.2%	11	10
11-12 AM	1.8%	1.6%	6	5

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	934			
Land Use	Fast-Food w/Drive Through Window			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	53			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	0.8%	0.8%	2	2
1-2 AM	0.4%	0.5%	1	1
2-3 AM	0.3%	0.3%	1	1
3-4 AM	0.2%	0.3%	1	1
4-5 AM	0.3%	0.3%	1	1
5-6 AM	0.8%	0.7%	2	2
6-7 AM	2.3%	1.9%	7	6
7-8 AM	3.4%	3.1%	10	10
8-9 AM	3.5%	3.4%	11	11
9-10 AM	3.4%	3.3%	11	10
10-11 AM	4.0%	3.7%	12	11
11-12 PM	9.1%	7.7%	28	24
12-1 PM	11.9%	12.0%	37	37
1-2 PM	7.9%	8.7%	24	27
2-3 PM	5.9%	6.5%	18	20
3-4 PM	5.7%	5.7%	18	18
4-5 PM	5.9%	5.6%	18	17
5-6 PM	6.9%	6.5%	21	20
6-7 PM	7.4%	7.4%	23	23
7-8 PM	6.3%	6.6%	20	21
8-9 PM	5.6%	5.8%	17	18
9-10 PM	4.1%	4.6%	13	14
10-11 PM	2.5%	2.9%	8	9
11-12 AM	1.4%	1.8%	4	6

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	310			
Land Use	Hotel			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	1			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM			0	0
1-2 AM			0	0
2-3 AM			0	0
3-4 AM			0	0
4-5 AM			0	0
5-6 AM			0	0
6-7 AM	0.8%	2.2%	10	26
7-8 AM	2.4%	8.2%	29	99
8-9 AM	3.3%	6.3%	40	75
9-10 AM	4.8%	6.7%	57	81
10-11 AM	5.9%	7.8%	71	94
11-12 PM	7.4%	8.1%	89	98
12-1 PM	10.3%	6.2%	124	75
1-2 PM	10.7%	7.2%	129	87
2-3 PM	7.8%	8.0%	94	96
3-4 PM	8.2%	7.0%	99	84
4-5 PM	7.1%	7.6%	85	91
5-6 PM	8.8%	6.5%	106	79
6-7 PM	5.4%	4.8%	65	58
7-8 PM	5.8%	3.2%	70	38
8-9 PM	3.1%	3.2%	37	39
9-10 PM	2.1%	2.8%	26	33
10-11 PM	3.1%	1.8%	37	21
11-12 AM	1.5%	0.9%	18	11

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	932			
Land Use	High Turnover Sit Down Restaurant			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	37			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	0.3%	0.8%	4	11
1-2 AM	0.1%	0.3%	2	4
2-3 AM	0.3%	0.2%	4	3
3-4 AM	0.2%	0.4%	3	5
4-5 AM	0.2%	0.3%	3	4
5-6 AM	0.7%	0.2%	10	3
6-7 AM	1.7%	0.9%	25	13
7-8 AM	2.7%	1.8%	39	26
8-9 AM	3.4%	3.1%	48	44
9-10 AM	4.0%	3.4%	57	48
10-11 AM	5.5%	4.4%	77	62
11-12 PM	12.1%	6.9%	171	98
12-1 PM	12.4%	12.3%	175	174
1-2 PM	6.5%	11.2%	91	158
2-3 PM	4.0%	5.1%	56	72
3-4 PM	3.3%	4.0%	47	56
4-5 PM	6.2%	3.9%	88	55
5-6 PM	10.2%	6.8%	144	97
6-7 PM	10.1%	8.7%	143	122
7-8 PM	7.2%	9.2%	101	130
8-9 PM	4.3%	7.1%	60	101
9-10 PM	2.6%	4.8%	37	68
10-11 PM	1.4%	2.9%	20	41
11-12 AM	0.5%	1.4%	7	19

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	710			
Land Use	General Office			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	11			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	0.0	0.0	0	0
1-2 AM	0.0	0.0	0	0
2-3 AM	0.0	0.0	0	0
3-4 AM	0.0	0.0	0	0
4-5 AM	0.0	0.0	0	0
5-6 AM	0.0	0.0	1	0
6-7 AM	4.8%	0.5%	9	1
7-8 AM	13.6%	2.0%	27	4
8-9 AM	14.3%	3.4%	28	7
9-10 AM	4.8%	4.4%	12	81
10-11 AM	5.5%	6.0%	11	12
11-12 PM	6.9%	10.3%	12	20
12-1 PM	10.2%	10.1%	20	20
1-2 PM	9.0%	6.6%	18	13
2-3 PM	8.3%	6.5%	16	13
3-4 PM	7.3%	8.4%	14	17
4-5 PM	5.4%	15.2%	11	30
5-6 PM	4.0%	15.8%	8	31
6-7 PM	1.7%	2.6%	3	5
7-8 PM	0.9%	2.3%	2	5
8-9 PM	0.7%	1.3%	1	3
9-10 PM	0.5%	1.6%	1	3
10-11 PM	0.3%	2.1%	1	4
11-12 AM	0.4%	0.2%	1	0

Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use				
Source: ITE <i>Trip Generation Manual</i> , 11th Edition				
Land Use Code	945			
Land Use	C-Store/Gas Station			
Setting	General Urban/Suburban			
Time Period	Weekday			
Trip Type	Vehicle			
# Data Sites	38			
Time	% of 24-Hour Traffic		Est Daily Traffic:	
	Entering	Exiting	Entering Trips	Exiting Trips
12-1 AM	0.9%	0.9%	16	17
1-2 AM	0.5%	0.5%	9	10
2-3 AM	0.4%	0.5%	8	9
3-4 AM	0.5%	0.6%	9	11
4-5 AM	1.0%	1.1%	18	20
5-6 AM	2.3%	2.2%	43	41
6-7 AM	4.7%	4.5%	87	84
7-8 AM	6.2%	6.1%	114	113
8-9 AM	5.8%	5.9%	107	109
9-10 AM	5.0%	5.1%	93	94
10-11 AM	5.2%	5.2%	95	95
11-12 PM	5.3%	5.2%	98	96
12-1 PM	5.8%	5.8%	106	108
1-2 PM	5.4%	5.3%	100	98
2-3 PM	6.1%	5.8%	112	108
3-4 PM	6.5%	6.4%	120	119
4-5 PM	7.2%	7.1%	133	130
5-6 PM	7.0%	6.9%	128	128
6-7 PM	6.5%	6.6%	119	121
7-8 PM	5.3%	5.3%	97	98
8-9 PM	4.3%	4.5%	79	82
9-10 PM	3.7%	3.7%	68	69
10-11 PM	2.7%	2.8%	50	52
11-12 AM	1.8%	1.9%	34	35

Trip Distribution, Overall Site, At Intersection Considered for Traffic Signal Warrants								
at FM 973/Driveway 2B								
Time	Total Site (excluding Fine Dining)		Distribution					
	Entering Trips	Exiting Trips	NBR	SBL	x	WBL	WBR	x
12-1 AM	24	31	1	5	0	10	3	0
1-2 AM	13	16	1	3	0	5	2	0
2-3 AM	14	13	1	3	0	4	1	0
3-4 AM	13	17	1	3	0	5	2	0
4-5 AM	23	26	1	5	0	8	3	0
5-6 AM	58	50	3	12	0	16	5	0
6-7 AM	140	139	7	30	0	44	15	0
7-8 AM	226	270	11	48	0	85	29	0
8-9 AM	243	265	12	52	0	83	28	0
9-10 AM	243	262	12	52	0	82	28	0
10-11 AM	287	299	14	61	0	94	32	0
11-12 PM	425	365	21	80	0	115	39	0
12-1 PM	494	444	24	105	0	140	48	0
1-2 PM	391	413	19	83	0	130	44	0
2-3 PM	324	335	16	69	0	105	36	0
3-4 PM	327	320	16	69	0	101	34	0
4-5 PM	369	353	18	78	0	111	38	0
5-6 PM	444	385	22	94	0	121	41	0
6-7 PM	386	357	19	82	0	112	38	0
7-8 PM	318	316	16	68	0	99	34	0
8-9 PM	218	258	11	46	0	81	28	0
9-10 PM	160	194	8	34	0	61	21	0
10-11 PM	129	138	6	27	0	43	15	0
11-12 AM	71	77	3	15	0	24	8	0

TRAFFIC SIGNAL WARRANT STUDY ANALYSIS INFORMATION

Municipality: Taylor
 County: Williamson
 TxDOT District: Austin

Analysis Date: 02/22/2024
 Conducted By: APV
 Agency/Company Name: Voigt Assoc., Inc.

Analysis Information

Analysis Condition (Existing/Future): Projected Conditions w/Development, Phase 2 Buildout (2027)
 Data Collection Date: 10/25/2023
 Day of the Week: Wednesday

Is the intersection in a built-up area of an isolated community of <10,000 population? No

Major Street Information

Major Street Name and Route Number: FM 973

Major Street Approach #1 Direction: N-Bound

Major Street Approach #2 Direction: S-Bound

Number of Lanes for Moving Traffic on Each Major Street Approach: 1 LANE(S)

Speed Limit or 85th Percentile Speed on the Major Street: 55 MPH

Minor Street Information

Minor Street Name and Route Number: Driveway 2B

Minor Street Approach #1 Direction: N/A

Minor Street Approach #2 Direction: W-Bound

Number of Lanes for Moving Traffic on Each Minor Street Approach: 2 LANE(S)

TRAFFIC SIGNAL WARRANT ANALYSIS FINDINGS/SUMMARY

	Applicable?	Warrant Met?
Warrant 1, Eight-Hour Vehicular Volume	Yes	Yes
Warrant 2, Four-Hour Vehicular Volume	Yes	Yes
Warrant 3, Peak Hour	Yes	Yes
Warrant 4, Pedestrian Volume	No	N/A
Warrant 5, School Crossing	No	N/A
Warrant 6, Coordinated Signal System	No	N/A
Warrant 7, Crash Experience	No	N/A
Warrant 8, Roadway Network	No	N/A
Warrant 9, Intersection Near a Grade Crossing	No	N/A

Comments:

Warrant 1 (Eight Hour) met for 13 hours (8 required)
 Warrant 2 (Four Hour) met for 12 hours (4 required)
 Warrant 3 (Peak Hour) met for 9 hours (1 required, if applicable)

ENTER VOLUME DATA PER HOUR INTERVAL, PER APPROACH

Time Interval	Major Street Approach #1 (N-Bound)	Major Street Approach #2 (S-Bound)	Major Street Combined	Minor Street Approach #1 (N/A)	Minor Street Approach #2 (W-Bound)
Begin At	Volume	Volume	Total Volume	Volume	Volume
12:00 AM	26	5	32	0	13
1:00 AM	47	25	72	0	7
2:00 AM	67	24	91	0	5
3:00 AM	46	72	118	0	7
4:00 AM	144	300	444	0	11
5:00 AM	235	1607	1842	0	21
6:00 AM	297	1526	1823	0	58
7:00 AM	406	1050	1456	0	114
8:00 AM	493	914	1407	0	112
9:00 AM	255	410	665	0	111
10:00 AM	317	327	644	0	126
11:00 AM	693	521	1214	0	154
12:00 PM	692	934	1626	0	188
1:00 PM	443	550	992	0	174
2:00 PM	390	396	786	0	141
3:00 PM	659	506	1166	0	135
4:00 PM	1081	616	1697	0	149
5:00 PM	1131	518	1648	0	163
6:00 PM	954	394	1348	0	151
7:00 PM	425	293	718	0	133
8:00 PM	166	160	325	0	109
9:00 PM	133	154	287	0	82
10:00 PM	99	123	222	0	58
11:00 PM	50	16	66	0	33
Approach Totals:	9744	13452		0	2255

TxMUTCD WARRANT 1, EIGHT-HOUR VEHICULAR VOLUME, W/O A+B COMBINATION

Number of Lanes for Moving Traffic on Each Approach	
Major Street:	1 Lane
Minor Street:	2 or More Lanes

Built-up Isolated Community With Less Than 10,000 Population or Above 40 MPH on Major Street?	Yes
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Combination of Conditions A and B Necessary?*: No

**Only applicable for Warrant 1 if after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems. See Section 4C.02 of the 2011 Texas MUTCD for application.*

Condition A - Minimum Vehicular Volume									
Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor street approach (one direction only)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	500	400	350	280	150	120	105	84
2 or More	1	600	480	420	336	150	120	105	84
2 or More	2 or More	600	480	420	336	200	160	140	112
1	2 or More	500	400	350	280	200	160	140	112

Condition B - Interruption of Continuous Traffic									
Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor street approach (one direction only)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	750	600	525	420	75	60	53	42
2 or More	1	900	720	630	504	75	60	53	42
2 or More	2 or More	900	720	630	504	100	80	70	56
1	2 or More	750	600	525	420	100	80	70	56

Condition A Evaluation

Number of Unique Hours Met: 7

Condition A Satisfied? No

Condition B Evaluation

Number of Unique Hours Met: 13

Condition B Satisfied? Yes

TxMUTCD WARRANT 1, EIGHT-HOUR VEHICULAR VOLUME, W/COMBINATION A+B

Number of Lanes for Moving Traffic on Each Approach	
Major Street:	1 Lane
Minor Street:	2 or More Lanes

Built-up Isolated Community With Less Than 10,000 Population or Above 40 MPH on Major Street?	Yes
---	-----

Combination of Conditions A and B Necessary?*: Yes

**Only applicable for Warrant 1 if after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems. See Section 4C.02 of the 2011 Texas MUTCD for application.*

Condition A - Minimum Vehicular Volume									
Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor street approach (one direction only)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	500	400	350	280	150	120	105	84
2 or More	1	600	480	420	336	150	120	105	84
2 or More	2 or More	600	480	420	336	200	160	140	112
1	2 or More	500	400	350	280	200	160	140	112

Condition B - Interruption of Continuous Traffic									
Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor street approach (one direction only)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	750	600	525	420	75	60	53	42
2 or More	1	900	720	630	504	75	60	53	42
2 or More	2 or More	900	720	630	504	100	80	70	56
1	2 or More	750	600	525	420	100	80	70	56

Combination of Condition A and Condition B Evaluation

Number of Unique Hours Met for Condition A: 7

Number of Unique Hours Met for Condition B: 13

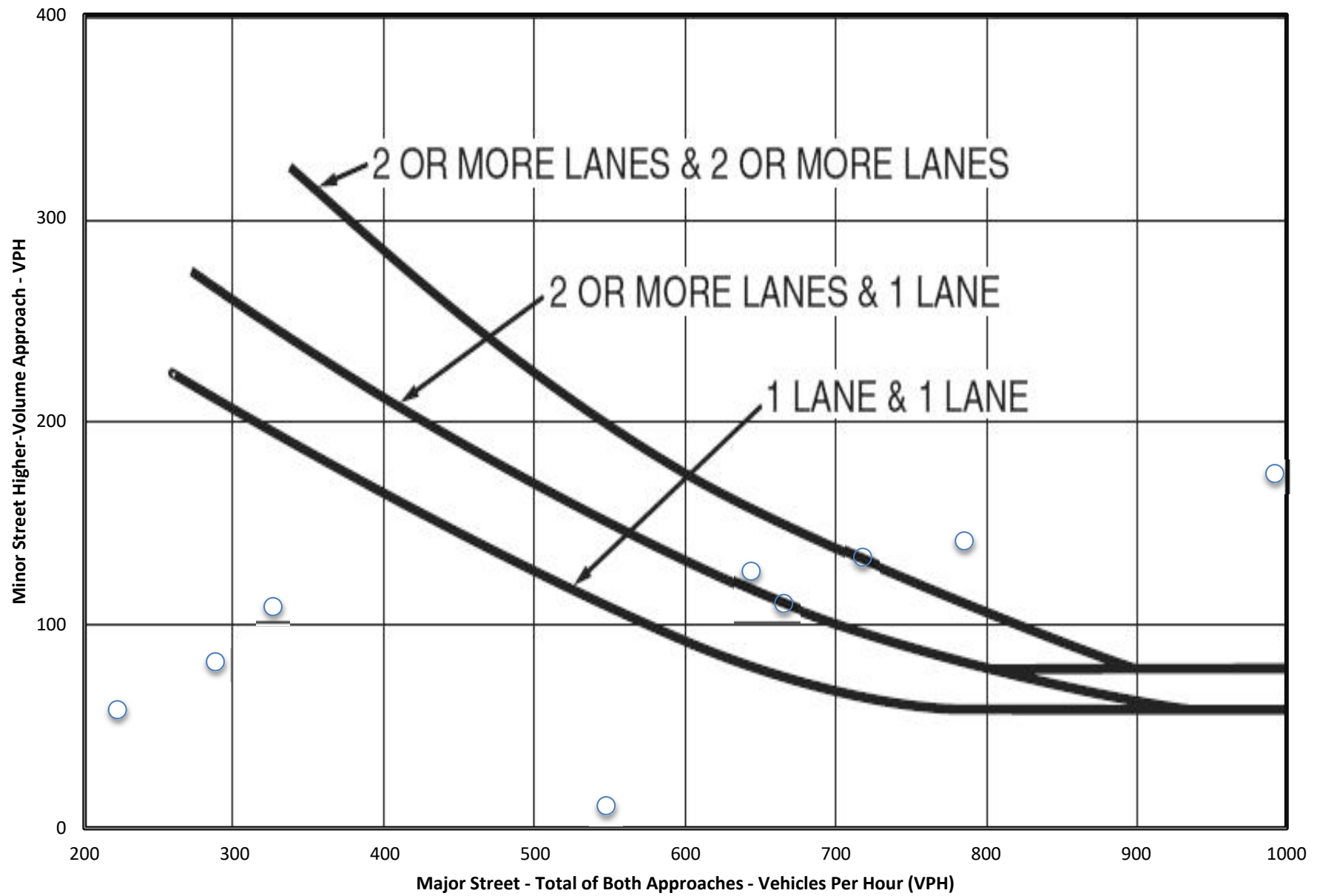
Combination of Condition A and Condition B Satisfied? No

TxMUTCD WARRANT 2, FOUR-HOUR VEHICULAR VOLUME

Number of Lanes for Moving Traffic on Each Approach		Total Number of Unique Hours Met On Figure 4C-2
Major Street:	1 Lane	12
Minor Street:	2 or More Lanes	

Built-up Isolated Community With Less Than 10,000 Population or Above 40 MPH on Major Street?	Yes
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Hourly Vehicular Volume			
Hour Interval	Major Street Combined	Highest Minor Street Approach	Hour Met?
Beginning At	Vehicles Per Hour (VPH)	Vehicles Per Hour (VPH)	
12:00 AM	32	13	
1:00 AM	72	7	
2:00 AM	91	5	
3:00 AM	118	7	
4:00 AM	547	11	
5:00 AM	4246	21	
6:00 AM	1823	58	
7:00 AM	1456	114	Met
8:00 AM	1407	112	Met
9:00 AM	665	111	
10:00 AM	644	126	Met
11:00 AM	1214	154	Met
12:00 PM	1626	188	Met
1:00 PM	992	174	Met
2:00 PM	786	141	Met
3:00 PM	1166	135	Met
4:00 PM	1697	149	Met
5:00 PM	1648	163	Met
6:00 PM	1348	151	Met
7:00 PM	718	133	Met
8:00 PM	325	109	
9:00 PM	287	82	
10:00 PM	222	58	
11:00 PM	66	33	

MUTCD Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)

TxMUTCD WARRANT 3, PEAK HOUR

Number of Lanes for Moving Traffic on Each Approach	
Major Street:	1 Lane
Minor Street:	2 or More Lanes

Built-up Isolated Community With Less Than 10,000 Population or Above 40 MPH on Major Street?	Yes
---	-----

Is this signal warrant being applied for an unusual case, such as office complexes, manufacturing plants, industrial complexes, or high-occupancy vehicle facilities that attract or discharge large numbers of vehicles over a short time?

Yes

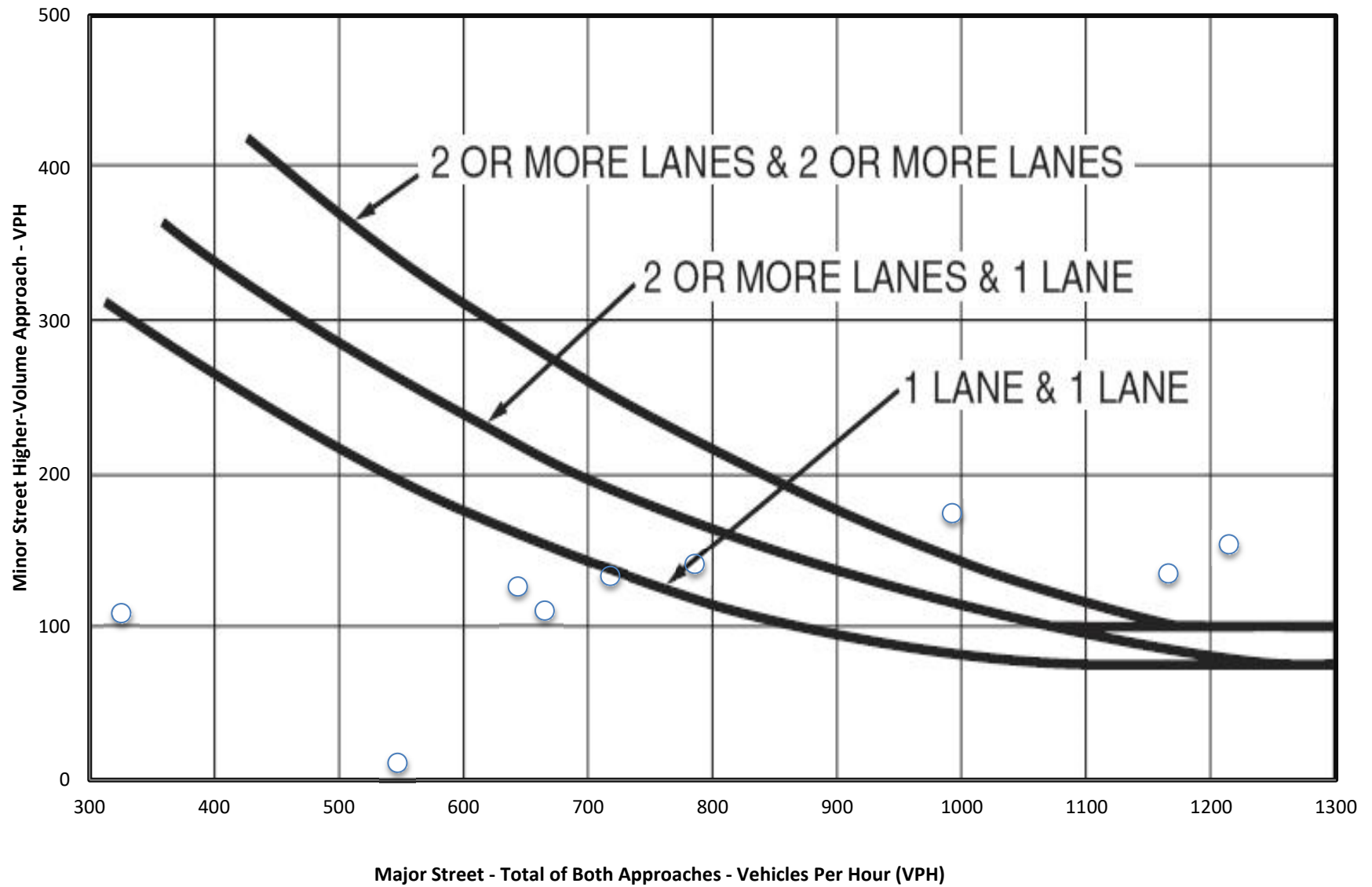
Indicate whether all three of the following conditions for the same 1 hour (any four consecutive 15-minute periods) of an average day are present*

Does the total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a STOP sign equal or exceed 4 vehicle-hours for a one-lane approach or 5 vehicle-hours for a two-lane approach?	Yes
Does the volume on the same minor-street approach (one direction only) equal or exceed 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes?	Yes
Does the total entering volume serviced during the hour equal or exceed 650 vehicles per hour for intersection with three approaches or 800 vehicles per hour for intersections with four or more approaches?	Yes
<i>*If applicable, attach all supporting calculations and documentation.</i>	

**Total Number of Unique Hours Met
On Figure 4C-4**

9

Hourly Vehicular Volume			
Hour Interval	Major Street Combined	Highest Minor Street Approach	Hour Met?
Beginning At	Vehicles Per Hour (VPH)	Vehicles Per Hour (VPH)	
12:00 AM	32	13	
1:00 AM	72	7	
2:00 AM	91	5	
3:00 AM	118	7	
4:00 AM	547	11	
5:00 AM	4246	21	
6:00 AM	1823	58	
7:00 AM	1456	114	Met
8:00 AM	1407	112	Met
9:00 AM	665	111	
10:00 AM	644	126	
11:00 AM	1214	154	Met
12:00 PM	1626	188	Met
1:00 PM	992	174	Met
2:00 PM	786	141	
3:00 PM	1166	135	Met
4:00 PM	1697	149	Met
5:00 PM	1648	163	Met
6:00 PM	1348	151	Met
7:00 PM	718	133	
8:00 PM	325	109	
9:00 PM	287	82	
10:00 PM	222	58	
11:00 PM	66	33	

MUTCD Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

MUTCD WARRANT 4, PEDESTRIAN VOLUME

Built-up Isolated Community With Less Than 10,000 Population or Above 35 MPH on Major Street?

Yes

15th Percentile Pedestrian Crossing Speed Less than 3.5 f/s?*

No

**If applicable, attach all supporting calculations, documentation, and findings.*

Is the distance to the nearest traffic control signal or STOP sign controlling the major street that pedestrians desire to cross less than 300 feet?

No

If the distance to the nearest traffic control signal or STOP sign controlling the major street that pedestrians desire to cross is less than 300 feet, will the proposed traffic control signal restrict the progressive movement of traffic?*

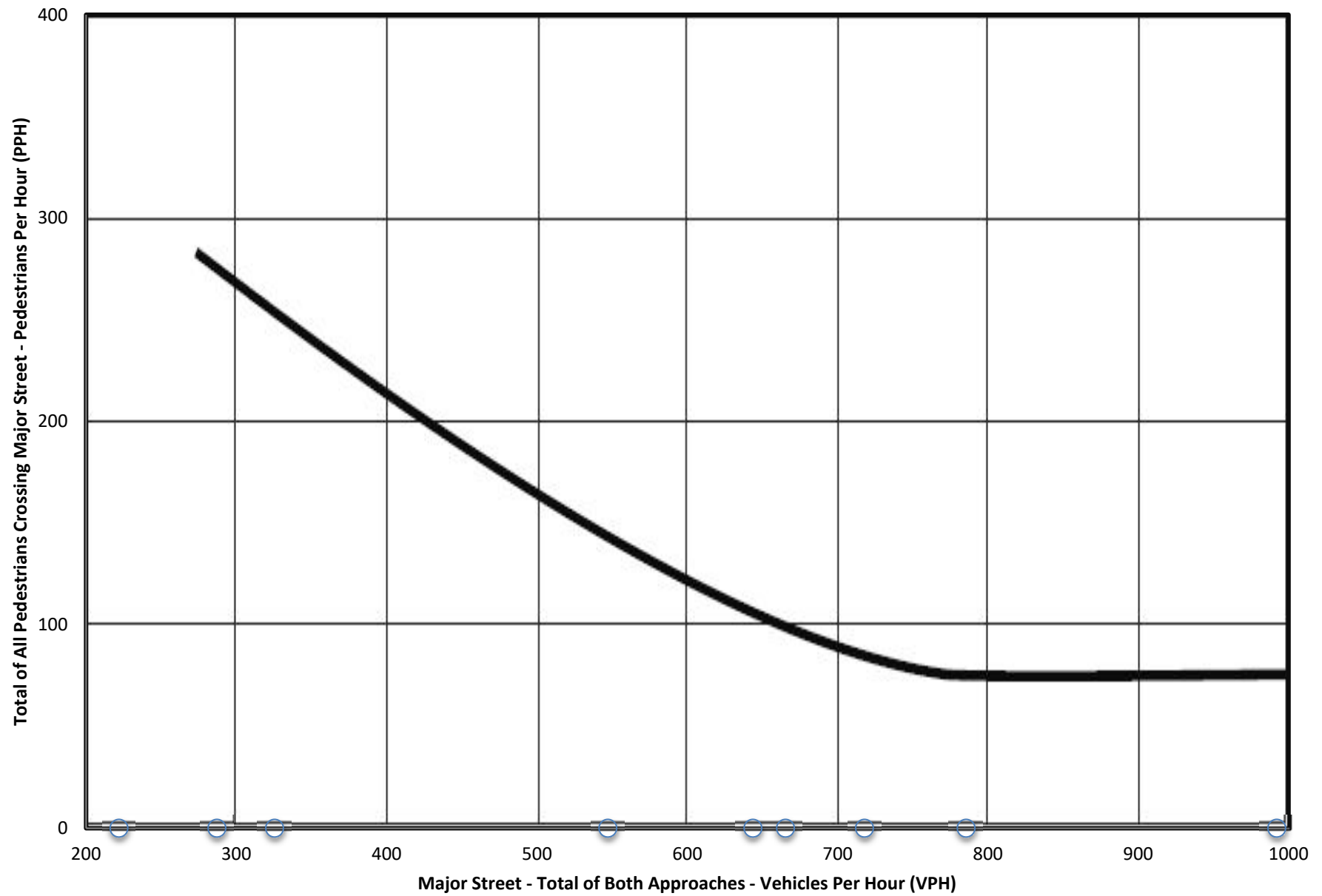
No

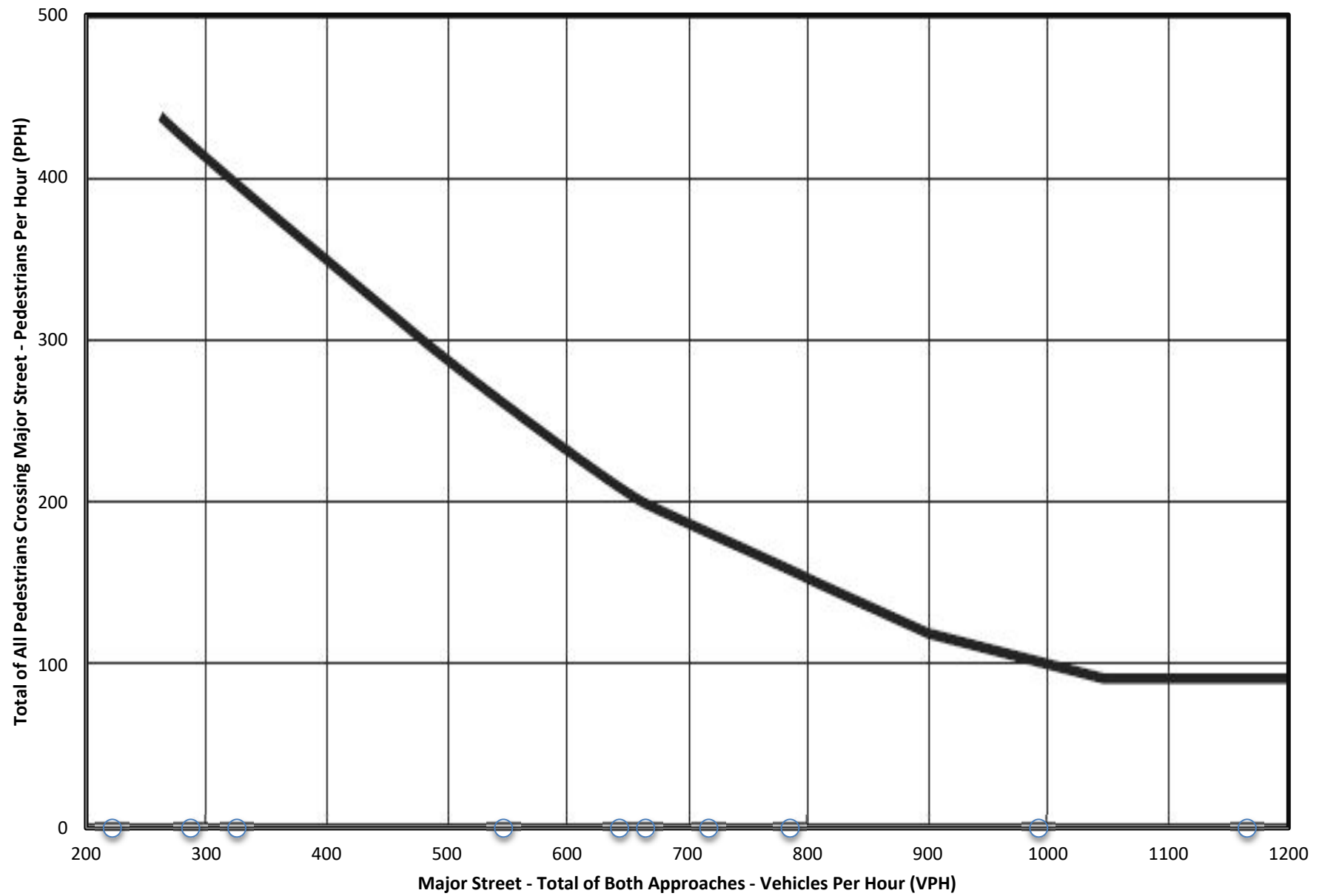
**If applicable, attach supporting justification.*

Total Number of Unique Hours Met for Criterion A: 0

Total Number of Unique Hours Met for Criterion B: 0

Hourly Vehicular & Pedestrian Volume				
Hour Interval	Major Street Combined	Total of All Pedestrians Crossing Major Street	Criterion A: 4-Hour	Criterion B: 1-Hour
Beginning At	Vehicles Per Hour (VPH)	Pedestrians Per Hour (PPH)	Hour Met on Figure 4C-6?	Hour Met on Figure 4C-8?
12:00 AM	32	0		
1:00 AM	72	0		
2:00 AM	91	0		
3:00 AM	118	0		
4:00 AM	547	0		
5:00 AM	4246	0		
6:00 AM	1823	0		
7:00 AM	1456	0		
8:00 AM	1407	0		
9:00 AM	665	0		
10:00 AM	644	0		
11:00 AM	1214	0		
12:00 PM	1626	0		
1:00 PM	992	0		
2:00 PM	786	0		
3:00 PM	1166	0		
4:00 PM	1697	0		
5:00 PM	1648	0		
6:00 PM	1348	0		
7:00 PM	718	0		
8:00 PM	325	0		
9:00 PM	287	0		
10:00 PM	222	0		
11:00 PM	66	0		

MUTCD Figure 4C-6. Warrant 4, Pedestrian Four-Hour Volume (70% Factor)

MUTCD Figure 4C-8. Warrant 4, Pedestrian Peak Hour (70% Factor)

TxMUTCD WARRANT 5, SCHOOL CROSSING

Do schoolchildren (elementary through high school students) cross the major street? No

Has consideration been given to implement other remedial measures, such as warning signs and flashers, school speed zones, school crossing guards, or a grade-separated crossing? No

Is the distance to the nearest traffic control signal along the major street less than 300 feet? No

If the distance to the nearest traffic control signal along the major street is less than 300 feet, will the proposed traffic control signal restrict the progressive movement of traffic? No

Minimum of 20 schoolchildren during the highest crossing hour? No

Has a traffic engineering study been conducted to determine the adequacy and frequency of gaps in the vehicular traffic stream as related to the number and size of groups of schoolchildren at an established school crossing across the major street? No

Pedestrian Gap Acceptance Engineering and Traffic Study Evaluation*

Data Collection Date: 10/25/2023
Day of the Week: Wednesday

Sufficient median for major street Crossing 1? Yes
Sufficient median for major street Crossing 2? Yes

Study Period	Study Duration (mins)	Crossing 1 (Stage 1)		Crossing 1 (Stage 2)		Crossing 2 (Stage 1)		Crossing 2 (Stage 2)	
		Total Adequate Gaps	Met?	Total Adequate Gaps	Met?	Total Adequate Gaps	Met?	Total Adequate Gaps	Met?
1 Morning			N/A		N/A		N/A		N/A
2 Afternoon			N/A		N/A		N/A		N/A
3			N/A		N/A		N/A		N/A
4			N/A		N/A		N/A		N/A
5			N/A		N/A		N/A		N/A
Summary:		Not Met		Not Met		Not Met		Not Met	

*Refer to ITE's Manual of Transportation Engineering Studies for specific details related to conducting a pedestrian gap acceptance engineering and traffic study. Attach all supplementary documentation and calculations.

TxMUTCD WARRANT 6, COORDINATED SIGNAL SYSTEM*

On a one-way street or a street that has traffic predominantly in one direction, the adjacent traffic control signals are so far apart that they do not provide the necessary degree of vehicular platooning. N/A

On a two-way street, adjacent traffic control signals do not provide the necessary degree of platooning and the proposed and adjacent traffic control signals will collectively provide a progressive operation. No

*Warrant 6 should not be applied where the resultant spacing of traffic control signals would be less than 1,000 feet.

TxMUTCD WARRANT 7, CRASH EXPERIENCE

Built-up Isolated Community With Less Than 10,000 Population or Above 40 MPH on Major Street?	Yes
---	-----

Number of Lanes for Moving Traffic on Each Approach	
Major Street:	1 Lane
Minor Street:	2 or More Lanes

Has adequate trial of alternatives with satisfactory observance and enforcement failed to reduce the crash frequency? No

Five or more reportable and/or non-reportable crashes, of types susceptible to correction by a traffic control signal, have occurred within a 12-month period during the most recent 3 years of available crash data.* N/A
**If applicable, attach a summary of the crash data analysis used for this criterion.*

For each of any 8 hours of an average day, the vehicles per hour given in both the 80% columns of Condition A in Table 4C-1 exists on the major-street and the higher-volume minor-street approach, respectively, to the intersection. Yes

For each of any 8 hours of an average day, the vehicles per hour given in both the 80% columns of Condition B in Table 4C-1 exists on the major-street and the higher-volume minor-street approach, respectively, to the intersection. Yes

The volume of pedestrian traffic is not less than 80% of the requirements specified in Warrant 4, the Pedestrian Volume warrant.* No
**If applicable, attach all supporting calculations and documentation.*

TxMUTCD WARRANT 8, ROADWAY NETWORK*

Is the major street classified as an Urban Extension, Principal Arterial, or Minor Arterial that is a reasonable connection between two Principal Arterials and/or Urban Extensions as shown on the official Functional Classification Map? Yes

Does the intersection have a total existing, or immediately projected, entering volume of at least 1,000 vehicles per hour during the peak hour of a typical weekday and has 5-year projected traffic volumes, based on an engineering study, that meet one or more of Warrants 1,2, and 3 during an average weekday? Yes

Does the intersection have a total existing or immediately projected entering volume of at least 1,000 vehicles per hour for each of any 5 hours of a non-normal business day (Saturday or Sunday)? No

Is the major street part of the street or highway system that serves as the principal roadway network for through traffic flow? Yes

Does the major street include rural or suburban highways outside, entering, or traversing a city? Yes

Does the major street appear as a major route on an official plan, such as a major street plan in an urban area traffic and transportation study? Yes

TxMUTCD WARRANT 9, INTERSECTION NEAR A GRADE CROSSING

Does the grade crossing exist on an approach controlled by a STOP or Yield sign and the center of the track nearest to the intersection is within 140 feet of the stop line or yield line on the approach? N/A

During the highest traffic volume hour during which rail traffic uses the crossing, the plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the minor-street approach that crosses the track (one direction only, approaching the intersection) falls above the applicable curve in Figure 4C-9 or 4C-10 for the existing combination of approach lanes over the track and the distance D. N/A

Number of approach lanes on the minor street approach that crosses the track: 1

Clear Storage Distance (D): 0 feet

Highest Traffic Volume Hour During Which Rail Traffic Uses the Crossing*

**If the rail traffic arrival times are unknown, the highest traffic volume hour of the day should be used.*

Major Street Volume (Total of Both Approaches): 0 vph
Actual Minor-Street Volume (One Direction Only, Approaching the Intersection): 0 vph

Apply Adjustment Factors to the Minor-Street Volume?: No

Minor-Street Approach Volume Adjustments*

**Refer to Section 4C.10 of the Texas MUTCD for details on the application of these adjustment factors.*

	Inputs	Adjustment Factor
Occurrences of Rail Traffic per Day:	1	0.67
% of High-Occupancy Buses (buses with at least 20 people) on Minor-Street Approach:	0%	1.00
% of Tractor-Trailer Trucks on Minor-Street Approach:	0% to 2.5%	0.50

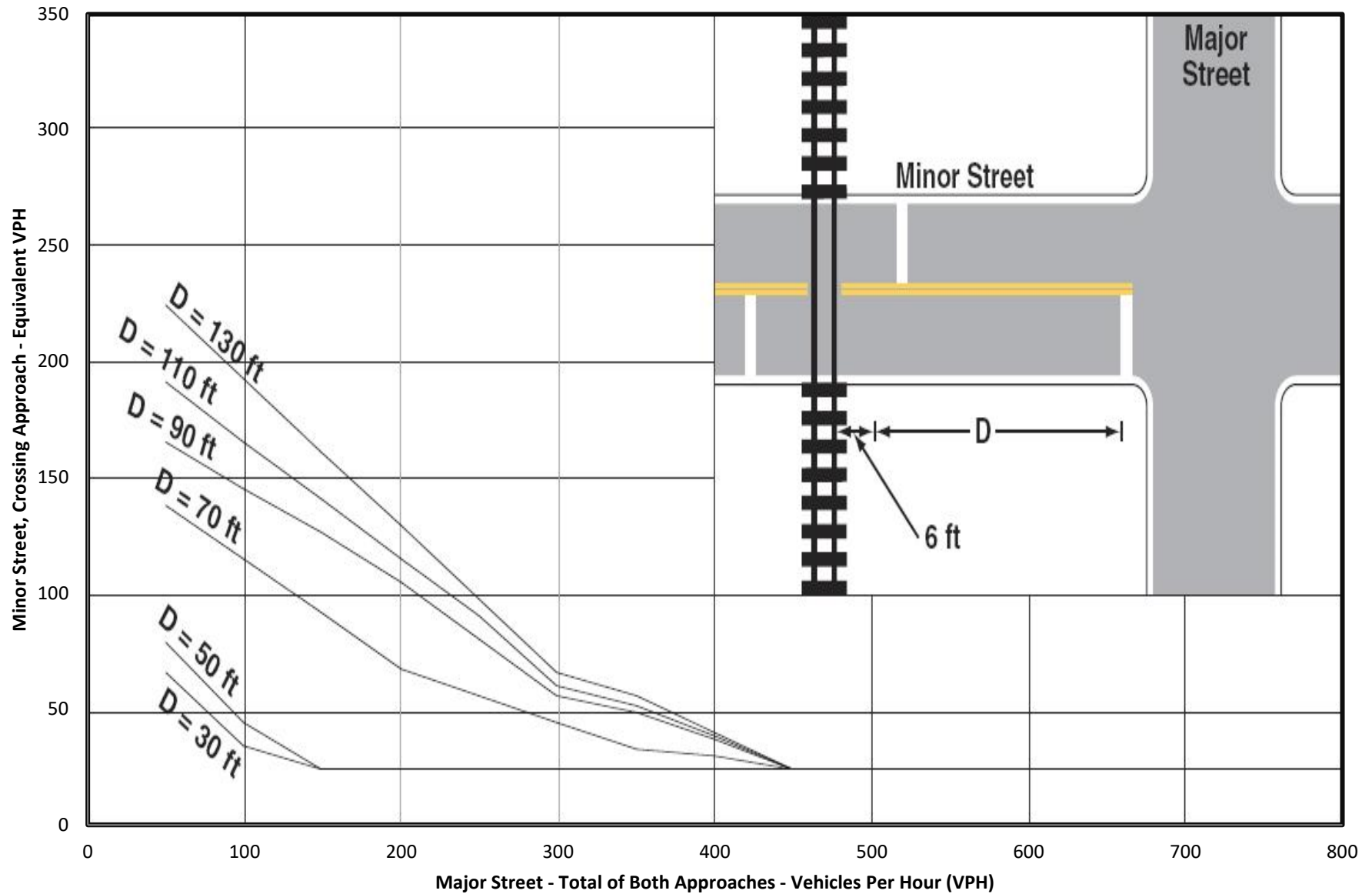
Adjusted Minor-Street Volume (One Direction Only, Approaching the Intersection): N/A vph

Traffic Volumes for Figure Comparison

Major Street Volume (Total of Both Approaches): 0 vph
Minor-Street Volume (One Direction Only, Approaching the Intersection): 0 vph

Applicable Figure for Comparison: Figure 4C-9

**MUTCD Figure 4C-9. Warrant 9, Intersection Near a Grade Crossing
(One Approach Lane at the Track Crossing)**



Appendix F – Selected Intersection Approach Photos



Photograph 1. US 79, Looking East, West of FM 973

Photograph Date: 7/9/2022



Photograph 2. FM 973, Looking South, South of US 79

Photograph Date: 7/9/2022



Photograph 3. FM 973, Looking South, at Taylor High School North Driveway

Photograph Date: 7/9/2022



**Photograph 4. FM 973, Looking North, North of Taylor High School North Driveway
Proposed site driveways to right.**

Photograph Date: 7/9/2022



Photograph 5. US 79, Looking East, East of FM 973
Proposed site driveways to right.

Photograph Date: 7/9/2022



**Photograph 5. US 79, Looking East, East of FM 973
Proposed site driveway (2A) to right.**

Photograph Date: 7/9/2022

Appendix G – Traffic Signal Timing Data



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Configuration Controller Sequence**Phase Ring Sequence and Assignment (MM) 1-1-1**

Hardware Alternate Sequence Enable: Yes

Phase Ring Sequence.....(Note: Sequences identical to the prior one are not printed)

	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16
Sequence 1																
Ring 1	2	1	3	4	9	10	13	14	.	.	.	.	.	.	.	.
Ring 2	5	6	7	8	11	12	15	16	.	.	.	.	.	.	.	.
Sequence 2																
Ring 1	2	1	3	4	10	9	13	14	.	.	.	.	.	.	.	.
Ring 2	5	6	7	8	11	12	15	16	.	.	.	.	.	.	.	.
Sequence 3																
Ring 1	1	2	4	3	9	10	14	13	.	.	.	.	.	.	.	.
Ring 2	5	6	7	8	11	12	15	16	.	.	.	.	.	.	.	.
Sequence 4																
Ring 1	2	1	4	3	10	9	14	13	.	.	.	.	.	.	.	.
Ring 2	5	6	7	8	11	12	15	16	.	.	.	.	.	.	.	.
Sequence 5																
Ring 1	1	2	3	4	9	10	13	14	.	.	.	.	.	.	.	.
Ring 2	6	5	7	8	12	11	15	16	.	.	.	.	.	.	.	.
Sequence 6																
Ring 1	2	1	3	4	10	9	13	14	.	.	.	.	.	.	.	.
Ring 2	6	5	7	8	12	11	15	16	.	.	.	.	.	.	.	.
Sequence 7																
Ring 1	1	2	4	3	9	10	14	13	.	.	.	.	.	.	.	.
Ring 2	6	5	7	8	12	11	15	16	.	.	.	.	.	.	.	.
Sequence 8																
Ring 1	2	1	4	3	10	9	14	13	.	.	.	.	.	.	.	.
Ring 2	6	5	7	8	12	11	15	16	.	.	.	.	.	.	.	.
Sequence 9																
Ring 1	1	2	3	4	9	10	13	14	.	.	.	.	.	.	.	.
Ring 2	5	6	8	7	11	12	16	15	.	.	.	.	.	.	.	.
Sequence 10																
Ring 1	2	1	3	4	10	9	13	14	.	.	.	.	.	.	.	.
Ring 2	5	6	8	7	11	12	16	15	.	.	.	.	.	.	.	.
Sequence 11																
Ring 1	1	2	4	3	9	10	14	13	.	.	.	.	.	.	.	.
Ring 2	5	6	8	7	11	12	16	15	.	.	.	.	.	.	.	.
Sequence 12																
Ring 1	2	1	4	3	10	9	14	13	.	.	.	.	.	.	.	.
Ring 2	5	6	8	7	11	12	16	15	.	.	.	.	.	.	.	.
Sequence 13																

Ring 1	1	2	3	4	9	10	13	14	.	.	.	.	.	.	.	.	.
Ring 2	6	5	8	7	12	11	16	15	.	.	.	.	.	.	.	.	.
Sequence 14																	
Ring 1	2	1	3	4	10	9	13	14	.	.	.	.	.	.	.	.	.
Ring 2	6	5	8	7	12	11	16	15	.	.	.	.	.	.	.	.	.
Sequence 15																	
Ring 1	1	2	4	3	9	10	14	13	.	.	.	.	.	.	.	.	.
Ring 2	6	5	8	7	12	11	16	15	.	.	.	.	.	.	.	.	.
Sequence 16																	
Ring 1	2	1	4	3	10	9	14	13	.	.	.	.	.	.	.	.	.
Ring 2	6	5	8	7	12	11	16	15	.	.	.	.	.	.	.	.	.

Phases In Use/Exclusive Ped (MM) 1-2

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Phases In Use	X	X	X	X	X	X										
Exclusive Ped																

Phase Compatibility (MM) 1-1-2

Phase	
1	6
2	5
2	6
3	7
3	8
4	7
4	8
9	11
9	12
10	11
10	12
13	15
13	16
14	15
14	16

Phase and Overlap Descriptions

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Overlap	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P
Description																

Administration (MM) 1-7-1

Enable Controller/Cabinet No
 Interlock CRC
 CRC (16 bit) E6AA
 No

Enable Automatic Backup
to Datakey

Backup Prevent (MM) 1-1-3

Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Timing	1	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Phases	2	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	3	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	4	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	5	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	6	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	7	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	8	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	9	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	10	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	11	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	12	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	13	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	14	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	16	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Simultaneous Gap (MM) 1-1-4

Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
	1	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	2	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	3	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	4	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	5	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Phase	6	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Must	7	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Gap	8	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
With	9	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Phase	10	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	11	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	12	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	13	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	14	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
	16	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Disable	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Load Switch Assignments (MM) 1-3

	Phase / Overlap	Type	Dimming				Power Up	Auto		Flash Together
			Red	Yellow	Green	Dark		Red	Yellow	
1	1	V				-	Auto	X		
2	2	V				-	Auto		X	X
3	3	V				-	Auto	X		
4	4	V				-	Auto	X		X
5	5	V				+	Auto	X		
6	6	V				+	Auto		X	X
7	7	V				+	Auto	X		
8	8	V				+	Auto	X		X
9	2	P				-	Auto			

10	4	P				-	Auto			
11	6	P				+	Auto			
12	8	P				+	Auto			
13	1	O				-	Auto		X	
14	2	O				+	Auto	X		X
15	3	O				-	Auto		X	
16	4	O				+	Auto	X		X



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Configuration Port 1 (SDLC)**Port 1 SDLC (MM) 1-4-1**

BIU	1	2	3	4	5	6	7	8
Term & Facility	X	X						
Detector Rack	X							

Enable TS2/MMU Type Cabinet: No

Enable MMU Extended Status: No

Enable SDLC Stop Time: No

Enable 3 Critical RFE's Lockup: No

MMU Program (MM) 1-4-2

Channel Can Serve With Channel	
Channel 1	Channel 2
1	5
1	6
1	11
1	15
2	5
2	6
2	9
2	11
2	13
2	15
3	7
3	8
3	12
3	16
4	7
4	8
4	10
4	12
4	14
4	16
5	9
5	13
6	9
6	11
6	13
6	15

7	10
7	14
8	10
8	12
8	14
8	16
9	11
9	13
9	15
10	12
10	14
10	16
11	13
11	15
12	14
12	16
13	15
14	16

Color Check Enable (MM) 1-4-3

Enable Color Check: No

MMU/LS	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Green	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X
Yellow	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X
Red	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X

Secondary Stations/Tests (MM) 1-4-4

ID	1	2	3	4	5	6	7	8	MMU
Term & Facility									

ID	1	2	3	4	5	6	7	8	Diag
Detector Rack									

Enable SDLC Diagnostic Test: No



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Configuration Communications 1 (SDLC)**Ethernet Port Configuration
(MM) 1-5-1**

Controller IP: 10.2.1.101
 Subnet Mask: 255.255.255.0
 Default Gateway IP: 10.2.1.1
 Server IP: 10.148.3.231

NTCIP (MM) 1-5-5

NTCIP Backup Time (Sec): 0
 NTCIP UDP Port: 501
 Ethernet Priority: 1
 Port 2 Priority (Port C50S for 2070): 4
 Port 3A Priority (Port C21S for 2070): 2
 Port 3B Priority (Port C22S for 2070): 3

Port Configuration (MM) 1-5-2 to 1-5-4

Port	2 (C50S)	3A (C21S)	3B (C22S)
Protocol	TERMINAL	NTCIP	ECPIP
Enable	No	Yes	No
Data Rate (BPS)	9600	9600	1200
Data, Parity, Stop	8 N 1	8 N 1	8 N 1
Address	0	1	0
Telemetry Response Delay	0.0	0.0	0.9
Duplex - Half or Full	Half	Full	Full
Flow Control	Yes	No	Yes
Group Address	0	0	0
Single Flag Enable	Yes	Yes	Yes
RTS to CTS Delay	n/a	n/a	14.0
RTS Turn Off Delay	n/a	n/a	2.0
Dropout Time	10	10	10
Early RTS	n/a	n/a	No
Telemetry Mode	n/a	n/a	FSK
ATCS Railroad	0	n/a	n/a
ATCS Railroad Line	0	n/a	n/a
ATCS Group	0	n/a	n/a
Wayside Device	0	n/a	n/a
ATC Device	0	n/a	n/a
Wayside Subnode	0	n/a	n/a
ATC Subnode	0	n/a	n/a

ECPIP (MM) 1-5-6

Controller Address: 0
 Expanded System Detector Address: 0

**System Detector
Assignment**

System Detector	Local Detector
--------------------	-------------------



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Configuration Logging / Display**Event Logging (MM) 1-6-1**

Critical RFE's (MMU/TF)	Yes	3 Critical Errors Within 24 Hours	Yes
MMU Flash Faults	Yes	Local Flash Fault	Yes
Non-Critical RFE's (Det/Test)	Yes	Detector Errors	Yes
Coordination Errors	Yes	Controller Download	Yes
Preemption Events	Yes	TSP Events	Yes
Power On/Off	Yes	Low Battery	Yes
Access	Yes	Data Change	Yes
Online / Offline	Yes		

Alarm Event	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Enable Logging	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X

Display Options (MM) 1-7-2

Key Click Enable:	No
Backlight Enable:	Yes
LED Mode:	Auto
Display Mode:	Advanced
Screen Format:	Advanced
Trans Mode Pop-Up Disable:	No

Sign On (MM) 8-5

Sign On Message Line 1:
Sign On Message Line 2:

Software Modules (MM) 8-7

Application Version: 02.47.10
OS (Boot) Version: 01.12.05



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Logic Processor Page 1

Logic Statement Control (MM) 1-8-1

Logic #	Statement Control
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MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Logic Processor Page 2

Logic Statements (MM) 1-8-2



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Controller Timing Plan (MM) 2-1**Plan 1**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction																
Min Green	5	15	5	5	10	15	0	0	0	0	0	0	0	0	0	0
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	7	0	7	0	0	0	0	0	0	0	0	0	0	0	0
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	15	0	12	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	2.0	2.0	2.0	3.0	2.0	2.0	2.0	2.0	5.0	5.0	5.0	5.0	5.0	5.0	0.0	0.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	25	55	5	50	25	55	0	0	0	0	0	0	0	0	0	0
Max2	0	0	0	30	0	0	0	0	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	4.0	5.0	5.0	5.0	5.0	5.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Clear	1.0	1.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Plan 2

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction																
Min Green	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	10	0	10	0	10	0	10	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	16	0	16	0	16	0	16	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35
Max2	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Plan 3

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction																
Min Green	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	10	0	10	0	10	0	10	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	16	0	16	0	16	0	16	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35
Max2	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Plan 4

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction																
Min Green	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	10	0	10	0	10	0	10	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	16	0	16	0	16	0	16	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35	35
Max2	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Controller Overlaps**Vehicle Overlaps (MM) 2-2**

Overlap	Type	Lag Green	Yellow	Red	Adv. Green
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Phases

Overlap	Phase	Included	Protect	Ped Protect	Not Overlap	Modifier	Lag X Phases	Lag 2 Phases	Flash Green
A	2	Yes	No	No	No		No	No	.
C	6	Yes	No	No	No		No	No	.

PPLT FYA

Overlap	Protected Phase (Left Turn)	Permissive Phase (Opposing Thru)	Flashing Arrow Output	Flashing Arrow Output CH	Delay Start of FYA	Delay Start of Clearance	Action Plan SF Bit Disable	Ped Protected Enable
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Guaranteed Minimum Time Data (MM) 2-4

Phase	Min Green	Walk	Ped Clear	Yellow	Red Clear	Overlap Green
A01	0	0	0	3.0	0.0	0
B02	0	0	0	3.0	0.0	0
C03	0	0	0	3.0	0.0	0
D04	0	0	0	3.0	0.0	0
E05	0	0	0	3.0	0.0	0
F06	0	0	0	3.0	0.0	0
G07	0	0	0	3.0	0.0	0
H08	0	0	0	3.0	0.0	0
I09	0	0	0	3.0	0.0	0
J10	0	7	10	3.0	0.0	0
K11	0	0	0	3.0	0.0	0
L12	0	7	12	3.0	0.0	0
M13	0	0	0	3.0	0.0	0
N14	0	0	0	3.0	0.0	0
O15	0	0	0	3.0	0.0	0
P16	0	0	0	3.0	0.0	0



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Controller Pedestrian Overlaps

Vehicle / Pedestrian Overlaps (MM) 2-3

Included	Pedestrian Overlaps
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MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Controller Start / Flash Data (MM) 2-5**Start Up**

Phase	Phase Setting
1	.
2	G
3	.
4	.
5	.
6	G
7	.
8	.
9	.
10	.
11	.
12	.
13	.
14	.
15	.
16	.

Overlap
A
B
C
D

Flash Thru Mon: No
Flash Time: 0
All Red: 6
Power Start Seq: 2
MUTCD Enabled: No
Y->G: n/a

Automatic Flash

Entry
4

Exit
2
6

--

Overlap Exit

A
B
C
D

Flash Thru Mon: No

Exit Flash: W

Minimum Flash: 8

Minimum Recall: No

Cycle Through Phase: No



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Controller Options**Controller Options (MM) 2-6-1**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Flashing Grn Ph	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
Guar Passage																
Non-Act I	X				X											
Non-Act II			X				X									
Dual Entry	X	X	X	X	X											
Cond Service																
Cond Reservice																
Ped Re-Service																
Rest In Walk	X															
Flashing Walk																
Ped Clr-Yel																
Ped Clr-Red																
IGRN + Veh Ext																

Ped Clear Protect: Off Unit Red Revert: 2.0 MUTCD 3 Seconds Don't Walk: No

Pre-Timed Mode (MM) 2-7

Enable Pre-Timed Mode: No Free Input Disables Pre-Timed: No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Pre-Timed																

Phase Recall Options (MM) 2-8**Plan # 1**

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Lock Detector	X				X											
Vehicle Recall		X				X										
Ped Recall																
Max Recall																
Soft Recall																
No Rest																
AI Calc																



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Coordination Options**Options (MM) 3-1**

Manual Pattern	Auto	ECPI Coord	Yes
System Source	TBC	System Format	PTN
Splits In	Seconds	Offsets In	Seconds
Transition	Smooth	Max Select	MAXINH
Dwell / Add Time	0		
Delay Coord Wk-LZ	No	Force Off	Float
Offset Reference	Lead	Use Ped Time	No
Ped Recall	No	Ped Reservice	No
Local Zero	No	FO Added Ini	No
Override		Green	
Re-sync Count	0	Multisync	No

Auto Perm Minimum Green (Seconds) (MM) 3-4

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Minimum Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Split Demand (MM) 3-5

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Demand 1																
Demand 2																

Demand	1	2
Detector	0	0
Call Time (Sec)	0	0
Cycle Count	0	0



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Coordination Pattern Data
Coordinator Pattern Data (MM) 3-2



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Coordination Split Pattern
Split Pattern Data (MM) 3-3



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Preempt Plan

Preempt Plan (MM) 4-1

No Enabled Preempts



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

**Preempt Preempt Filtering
Enable Preempt Filtering &
TSP/SCP (MM) 4-2**

Input	Solid	Pulsing
1	...BYPASSED...	...BYPASSED...
2	...BYPASSED...	...BYPASSED...
3	PREEMPTION 3	PREEMPTION 7
4	PREEMPTION 4	PREEMPTION 8
5	PREEMPTION 5	PREEMPTION 9
6	PREEMPTION 6	PREEMPTION 10
7	...BYPASSED...	...BYPASSED...
8	...BYPASSED...	...BYPASSED...
9	...BYPASSED...	...BYPASSED...
10	...BYPASSED...	...BYPASSED...



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Preempt TSP/SCP Plan and Split**TSP / SCP Plan (MM) 4-3**

TSP/SCP Plan	Enable Option	Signal Type	Det Lock	Delay Time	Max Presence	PMT Enables Reservice	No Delay in TSP	Action SF Inhibit	Reservice Cycles	Bus Heading
1	No	Solid	No	0	0	No	False	0	0	NB
2	No	Solid	No	0	0	No	False	0	0	SB
3	No	Solid	No	0	0	No	False	0	0	EB
4	No	Solid	No	0	0	No	False	0	0	WB
5	No	Solid	No	0	0	No	False	0	0	.
6	No	Solid	No	0	0	No	False	0	0	.

Mode: TSP

Free Default Pattern: 120

Headway Allowance: 0

TSP/SCP Plan	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
2	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
3	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
4	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
5	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
6	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

TSP / SCP Split Pattern (MM) 4-4

TSP/SCP Split Pattern	Max Type	Phase															
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
4	Max Reduction	255	255	255	255	255	255	255	255	255	255	255	255	255	255	255	255



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Time Base Clock/Calendar
Clock/Calendar Data (MM) 5-1

Manual Action Plan: 0
SYNC Reference Time: 00:00
SYNC Reference: Reference Time
Day Light Savings: USDLS
Time Reset Input Set Time: 3:30:00
Standard Time From GMT: 0



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Time Base Action Plan Action Plan (MM) 5-2

Action Plan - 1

Pattern	Free	Override Sys	No
Timing Plan	0	Sequence	2
Veh Detector Plan	0	Det Log	None
Flash	No	Red Rest	No
Veh Det Diag Plan	0	Ped Det Diag Plan	0
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Action Plan - 2

Pattern 2 Override Sys No
 Timing Plan 0 Sequence 0
 Veh Detector Plan 0 Det Log None
 Flash No Red Rest No
 Veh Det Diag 0 Ped Det Diag 0
 Plan Plan
 Dimming Enable No Pmt Veh Priority No
 Ret
 Pmt Ped Priority No Pmt Queue Delay No
 Ret
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Action Plan - 3

Pattern 3 Override Sys No
 Timing Plan 0 Sequence 0
 Veh Detector Plan 0 Det Log None
 Flash No Red Rest No
 Veh Det Diag 0 Ped Det Diag 0
 Plan Plan
 Dimming Enable No Pmt Veh Priority No
 Ret
 Pmt Ped Priority No Pmt Queue Delay No
 Ret
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Action Plan - 4

Pattern 4 Override Sys No
 Timing Plan 0 Sequence 0
 Veh Detector Plan 0 Det Log None
 Flash No Red Rest No
 Veh Det Diag 0 Ped Det Diag 0
 Plan Plan
 Dimming Enable No Pmt Veh Priority No
 Ret Ret
 Pmt Ped Priority No Pmt Queue Delay No
 Ret Ret
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Action Plan - 99

Pattern Flash Override Sys No
 Timing Plan 0 Sequence 0
 Veh Detector Plan 0 Det Log None
 Flash Yes Red Rest No
 Veh Det Diag 0 Ped Det Diag 0
 Plan Plan
 Dimming Enable No Pmt Veh Priority No
 Ret Ret
 Pmt Ped Priority No Pmt Queue Delay No
 Ret Ret
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

Action Plan - 100

Pattern	Free	Override Sys	No
Timing Plan	0	Sequence	2
Veh Detector Plan	0	Det Log	None
Flash	No	Red Rest	No
Veh Det Diag Plan	0	Ped Det Diag Plan	0
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Time Base Day Plan/Schedule
Day Plan (MM) 5-3**Day Plan #1**

Event	Action Plan	Start Time
1	99	00:00
2	100	05:15
3	99	23:00

Day Plan #2

Event	Action Plan	Start Time
1	99	00:00
2	100	06:00
3	99	23:00

Schedule (MM) 5-4



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Time Base Exceptions

Exception Day Program (MM) 5-5

Excep Day	Float/Fixed	Mon/Mon	DOW/DOM	WOM/Year	Day Plan
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MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Detectors**Detectors - Pg 1****Veh Det Phase Assignment (MM) 6-1****Vehicle Detector Plan Number - 1**

Veh Detector	Assigned Phase	Called Phase	Type
1	6		P
2	1		N
3	4		N
5	3		N
6	2		P
7	5		N
11	2		N
12	6		N

Vehicle Detector Plan Number - 2

Veh Detector	Assigned Phase	Called Phase	Type
1	1		P
2	2		N
3	3		N
4	4		N
5	5		N
6	6		P
7	7		N
8	8		N
9	9		N
10	10		N
11	11		N
12	12		N
13	13		N
14	14		N
15	15		N
16	16		N

Vehicle Detector Plan Number - 3

Veh Detector	Assigned Phase	Called Phase	Type
1	1		P
2	2		N
3	3		N
4	4		N
5	5		N
6	6		P
7	7		N

8	8		N
9	9		N
10	10		N
11	11		N
12	12		N
13	13		N
14	14		N
15	15		N
16	16		N

Vehicle Detector Plan Number - 4

Veh Detector	Assigned Phase	Called Phase	Type
1	1		P
2	2		N
3	3		N
4	4		N
5	5		N
6	6		P
7	7		N
8	8		N
9	9		N
10	10		N
11	11		N
12	12		N
13	13		N
14	14		N
15	15		N
16	16		N

Vehicle Detector Setup (MM) 6-2

Veh Detector	Type	TS2 Detector	Description
1	P-PASSAGE	Yes	
2	N-NTCIP	Yes	
3	N-NTCIP	Yes	
4	N-NTCIP	Yes	
5	N-NTCIP	Yes	
6	P-PASSAGE	Yes	
7	N-NTCIP	Yes	
8	N-NTCIP	Yes	
9	N-NTCIP	Yes	
10	N-NTCIP	Yes	
11	N-NTCIP	Yes	
12	N-NTCIP	Yes	
13	N-NTCIP	Yes	
14	N-NTCIP	Yes	
15	N-NTCIP	Yes	
16	N-NTCIP	Yes	
17	N-NTCIP	Yes	
18	N-NTCIP	Yes	
19	N-NTCIP	Yes	

20	N-NTCIP	Yes	
21	N-NTCIP	Yes	
22	N-NTCIP	Yes	
23	N-NTCIP	Yes	
24	N-NTCIP	Yes	
25	G-GREEN EXT	Yes	

Vehicle Detector Plan Number - 1

Veh Detector	Phase	ECPI Log	Call Option	Delay Time	Ext Option	Extend Time / Passage Time	Queue Lim. / Discon. Time	Use Added Initial	Cross Switch Ph	Lock In	NTCIP Vol.	NTCIP Occ.	Pmt Queue Delay
1	6	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
2	1	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
3	4	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
4	0	No	Yes	8.0	Passage	0.0	0	No	0	None	No	No	No
5	3	No	Yes	10.0	Passage	0.0	0	No	0	None	No	No	No
6	2	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
7	5	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
8	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
9	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
10	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
11	2	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
12	6	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
13	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
14	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
15	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
16	0	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No

Vehicle Detector Plan Number - 2

Veh Detector	Phase	ECPI Log	Call Option	Delay Time	Ext Option	Extend Time / Passage Time	Queue Lim. / Discon. Time	Use Added Initial	Cross Switch Ph	Lock In	NTCIP Vol.	NTCIP Occ.	Pmt Queue Delay
1	1	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
2	2	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
3	3	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
4	4	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
5	5	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
6	6	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
7	7	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
8	8	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
9	9	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
10	10	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
11	11	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
12	12	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
13	13	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
14	14	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
15	15	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
16	16	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No

Vehicle Detector Plan Number - 3

Veh Detector	Phase	ECPI Log	Call Option	Delay Time	Ext Option	Extend Time / Passage Time	Queue Lim. / Discon. Time	Use Added Initial	Cross Switch Ph	Lock In	NTCIP Vol.	NTCIP Occ.	Pmt Queue Delay
1	1	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
2	2	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
3	3	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
4	4	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
5	5	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
6	6	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
7	7	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
8	8	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
9	9	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
10	10	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
11	11	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
12	12	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
13	13	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
14	14	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
15	15	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
16	16	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No

Vehicle Detector Plan Number - 4

Veh Detector	Phase	ECPI Log	Call Option	Delay Time	Ext Option	Extend Time / Passage Time	Queue Lim. / Discon. Time	Use Added Initial	Cross Switch Ph	Lock In	NTCIP Vol.	NTCIP Occ.	Pmt Queue Delay
1	1	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
2	2	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
3	3	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
4	4	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
5	5	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
6	6	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
7	7	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
8	8	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
9	9	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
10	10	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
11	11	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
12	12	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
13	13	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
14	14	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
15	15	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No
16	16	No	Yes	0.0	Passage	0.0	0	No	0	None	No	No	No

Ped Detector Phase Assignment (MM) 6-3**Mode: NTCIP**

Called Phase	Detector
1	1

Called Phase	Detector
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16



MOVING TRAFFIC FORWARD

US 79 @ FM 973 - US 79 @ FM 973 - Econolite Type - ASC/3

Detectors**Detectors - Pg 2****Log - Speed Detector Setup (MM) 6-4**

NTCIP Log ECPI Log Length Unit:
 Period: 60 Period: 0 Inches

Speed Detector	Local Detector	One/Two Detector	Vehicle Length	Trap length	Enable Log
1	0	1	0	0	No
2	0	1	0	0	No
3	0	1	0	0	No
4	0	1	0	0	No
5	0	1	0	0	No
6	0	1	0	0	No
7	0	1	0	0	No
8	0	1	0	0	No
9	0	1	0	0	No
10	0	1	0	0	No
11	0	1	0	0	No
12	0	1	0	0	No
13	0	1	0	0	No
14	0	1	0	0	No
15	0	1	0	0	No
16	0	1	0	0	No

Vehicle Detector Diagnostics (MM) 6-5**Veh Diagnostic Plan Number - 1**

Det	Counts	Act	Pres	Multiplier	Failed Time	Failed Call Delay
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Veh Diagnostic Plan Number - 2

Det	Counts	Act	Pres	Multiplier	Failed Time	Failed Call Delay
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Veh Diagnostic Plan Number - 3

Det	Counts	Act	Pres	Multiplier	Failed Time	Failed Call Delay
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Veh Diagnostic Plan Number - 4

Det	Counts	Act	Pres	Multiplier	Failed Time	Failed Call Delay
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Pedestrian Detector Diagnostics (MM) 6-6**Ped Diagnostic Plan Number - 1**

Det	Counts	Act	Pres	Multiplier
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Ped Diagnostic Plan Number - 2

Det	Counts	Act	Pres	Multiplier
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Ped Diagnostic Plan Number - 3

Det	Counts	Act	Pres	Multiplier
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Ped Diagnostic Plan Number - 4

Det	Counts	Act	Pres	Multiplier
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Appendix H – Pass-By Trip Support

Vehicle Pass-By Rates by Land Use									
Source: ITE Trip Generation Manual , 11th Edition									
Land Use Code	821								
Land Use	Shopping Plaza (40 - 150k)								
Setting	General Urban/Suburban								
Time Period	Weekday PM Peak Period								
# Data Sites	15								
Average Pass-By Rate	40%								
	Pass-By Characteristics for Individual Sites								
GLA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
					Primary (%)	Diverted (%)	Total (%)		
45	Florida	1992	844	56	24	20	44	—	30
50	Florida	1992	555	41	41	18	59	—	30
52	Florida	1995	665	42	33	25	58	—	30
53	Florida	1993	162	59	—	—	41	—	30
57.23	Kentucky	1993	247	31	53	16	69	2659	34
60	Florida	1995	1583	40	38	22	60	—	30
69.4	Kentucky	1993	109	25	42	33	75	1559	34
77	Florida	1992	365	46	—	—	54	—	30
78	Florida	1991	702	55	23	22	45	—	30
82	Florida	1992	336	34	—	—	66	—	30
92.857	Kentucky	1993	133	22	50	28	78	3555	34
100.888	Kentucky	1993	281	28	50	22	72	2111	34
121.54	Kentucky	1993	210	53	30	17	47	2636	34
144	New Jersey	1990	176	32	44	24	68	—	24
146.8	Kentucky	1993	—	36	39	25	64	—	34

Vehicle Pass-By Rates by Land Use									
Source: ITE Trip Generation Manual , 11th Edition									
Land Use Code	932								
Land Use	High-Turnover (Sit-Down) Restaurant								
Setting	General Urban/Suburban								
Time Period	Weekday PM Peak Period								
# Data Sites	12								
Average Pass-By Rate	43%								
	Pass-By Characteristics for Individual Sites								
GFA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
					Primary (%)	Diverted (%)	Total (%)		
2.9	Kentucky	1993	41	37	27	36	63	3935	2
3.1	Kentucky	1993	21	38	29	33	62	2580	2
4.6	Florida	1992	276	63	—	—	37	—	30
5	Florida	1992	65	58	—	—	42	—	30
5.3	Kentucky	1993	24	50	37	13	50	1615	2
5.7	Florida	1994	308	57	—	—	43	—	30
5.8	Florida	1992	150	32	—	—	68	—	30
6.2	Florida	1995	521	46	43	11	54	—	30
7.1	Indiana	1993	—	23	23	54	77	1565	2
8	Florida	1995	664	40	39	21	60	—	30
11	Florida	1996	267	38	43	19	62	—	30
12	Florida	1996	317	29	51	20	71	—	30

Vehicle Pass-By Rates by Land Use									
Source: ITE Trip Generation Manual , 11th Edition									
Land Use Code	934								
Land Use	Fast-Food Restaurant with Drive-Through Window								
Setting	General Urban/Suburban								
Time Period	Weekday AM Peak Period								
# Data Sites	5								
Average Pass-By Rate	50%								
	Pass-By Characteristics for Individual Sites								
					Non-Pass-By Trips			Adj Street Peak	
GFA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Primary (%)	Diverted (%)	Total (%)	Hour Volume	Source
1.4	Kentucky	1993	—	62	22	16	38	1407	2
3	Kentucky	1993	—	43	14	43	57	2903	2
3.3	--	1996	—	68	—	—	32	—	21
3.6	Kentucky	1993	—	32	47	21	68	437	2
4.2	Indiana	1993	—	46	23	31	54	1049	2

Vehicle Pass-By Rates by Land Use									
Source: ITE Trip Generation Manual , 11th Edition									
Land Use Code	934								
Land Use	Fast-Food Restaurant with Drive-Through Window								
Setting	General Urban/Suburban								
Time Period	Weekday PM Peak Period								
# Data Sites	11								
Average Pass-By Rate	55%								
	Pass-By Characteristics for Individual Sites								
GFA (000)	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
					Primary (%)	Diverted (%)	Total (%)		
1.3	Kentucky	1993	—	68	22	10	32	2055	2
1.9	Kentucky	1993	33	67	24	9	33	2447	2
2.8	Florida	1995	47	66	—	—	34	—	30
2.9	Florida	1996	271	41	41	18	59	—	30
3	Kentucky	1993	—	31	31	38	69	4250	2
3.1	Florida	1995	28	71	—	—	29	—	30
3.1	Florida	1996	29	38	—	—	62	—	30
3.2	Florida	1996	202	40	39	21	60	—	30
3.3	—	1996	—	62	—	—	38	—	21
4.2	Indiana	1993	—	56	25	19	44	1632	2
4.3	Florida	1994	304	62	—	—	38	—	30

Vehicle Pass-By Rates by Land Use										
Source: ITE Trip Generation Manual , 11th Edition										
Land Use Code	945									
Land Use	Convenience Store/Gas Station									
Setting	General Urban/Suburban									
Time Period	Weekday AM Peak Period									
# Data Sites	16 Sites with between 2 and 8 VFP					28 Sites with between 9 and 20 VFP				
Average Pass-By Rate	60% for Sites with between 2 and 8 VFP					76% for Sites with between 9 and 20 VFP				
Pass-By Characteristics for Individual Sites										

Vehicle Pass-By Rates by Land Use										
Source: ITE Trip Generation Manual , 11th Edition										
Land Use Code	945									
Land Use	Convenience Store/Gas Station									
Setting	General Urban/Suburban									
Time Period	Weekday PM Peak Period									
# Data Sites	12 Sites with between 2 and 8 VFP					28 Sites with between 9 and 20 VFP				
Average Pass-By Rate	56% for Sites with between 2 and 8 VFP					75% for Sites with between 9 and 20 VFP				
Pass-By Characteristics for Individual Sites										
GFA (000)	VFP	State or Province	Survey Year	# Interviews	Pass-By Trip (%)	Non-Pass-By Trips			Adj Street Peak Hour Volume	Source
						Primary (%)	Diverted (%)	Total (%)		
2.1	8	Maryland	1992	31	52	13	35	48	1785	25
2.1	6	Maryland	1992	30	53	20	27	47	1060	25
2.2	< 8	Indiana	1993	115	48	16	36	52	820	2
2.3	< 8	Kentucky	1993	67	57	16	27	43	1954	2
2.3	6	Maryland	1992	55	40	11	49	60	2760	25
2.4	< 8	Kentucky	1993	—	58	13	29	42	2655	2
2.6	< 8	Kentucky	1993	68	67	15	18	33	950	2
2.8	< 8	Kentucky	1993	—	62	11	27	38	2875	2
3	< 8	Indiana	1993	80	65	15	20	35	1165	2
3.6	< 8	Kentucky	1993	60	56	17	27	44	2505	2
3.7	< 8	Kentucky	1993	70	61	16	23	39	2175	2
4.2	< 8	Kentucky	1993	61	58	26	16	42	2300	2
4.694	12	Maryland	2000	—	78	—	—	22	3549	30
4.694	12	Maryland	2000	—	67	—	—	33	2272	30
4.694	12	Maryland	2000	—	66	—	—	34	3514	30
4.848	12	Virginia	2000	—	71	—	—	29	2350	30
5.06	12	Pennsylvania	2000	—	91	—	—	9	4181	30
5.242	12	Virginia	2000	—	70	—	—	30	2445	30
5.242	12	Virginia	2000	—	56	—	—	44	950	30
5.488	12	Delaware	2000	—	73	—	—	27	—	30
5.5	12	Pennsylvania	2000	—	84	—	—	16	4025	30
4.694	16	Maryland	2000	—	89	—	—	11	2755	30
4.694	16	Delaware	2000	—	73	—	—	27	1858	30
4.694	16	Delaware	2000	—	59	—	—	41	1344	30
4.694	16	Delaware	2000	—	72	—	—	28	3434	30
4.694	16	New Jersey	2000	—	81	—	—	19	1734	30
4.694	20	Delaware	2000	—	76	—	—	24	1616	30
4.848	16	Virginia	2000	—	67	—	—	33	2.954	30
4.848	16	Virginia	2000	—	78	—	—	22	3086	30
4.848	16	Virginia	2000	—	83	—	—	17	4143	30
4.848	16	Virginia	2000	—	73	—	—	27	2534	30
4.993	16	Pennsylvania	2000	—	72	—	—	28	2917	30
5.094	16	New Jersey	2000	—	86	—	—	14	1730	30
5.5	16	Pennsylvania	2000	—	90	—	—	10	2616	30
5.543	16	Pennsylvania	2000	—	87	—	—	13	2363	30
5.565	16	Pennsylvania	2000	—	81	—	—	19	2770	30
5.565	16	Pennsylvania	2000	—	76	—	—	24	3362	30
5.565	16	New Jersey	2000	—	61	—	—	39	1713	30
5.565	16	New Jersey	2000	—	86	—	—	14	1721	30
5.565	16	New Jersey	2000	---	81	---	---	19	2227	30

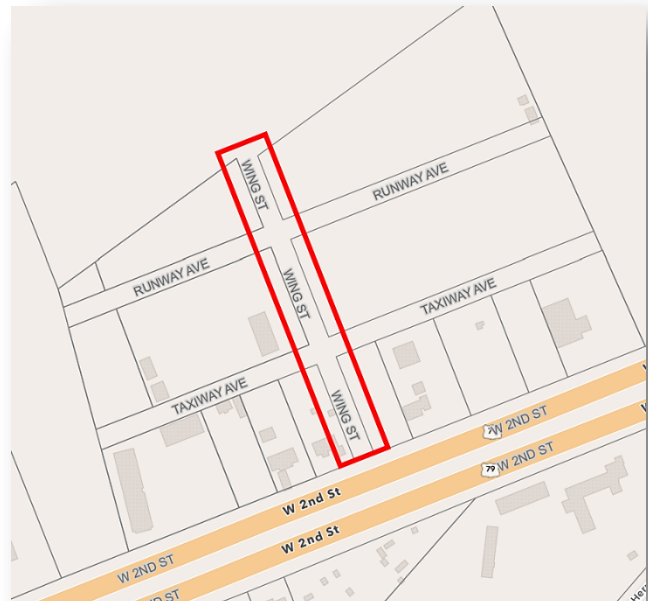
City of Taylor
PZ-2024-2238
Subdivision Variance
Staff Report

Item Details

Subject Property:	Generally located at 14501 US 79
Total Acreage:	Approximately 69.015 acres
Legal Description:	Approximately 69.015-acre tract of land part of and out of the H.G. Johnson Survey, Section 6, Abstract No. 348, more particularly described by the Williamson Central Appraisal District Parcels R019338 and R019992, Taylor, Williamson County, Texas
Applicant:	Katherine Nicely, MWSW Texas
Property Owner(s):	Douglas Wiley
Request:	A variance from the Engineering Manual Section 2.10.10 related to Sidewalks, Shared-Use Paths, and Trails.
Case History:	This is the first hearing of this request.

Overview of Applicant's Request & Background

The applicant has requested a variance to the Engineering Manual Section 2.10.10 related to Sidewalks, Shared-Use Paths, and Trails. The request is to vary from the Engineering Manual and not install sidewalks as part of the public improvements along the southern section of the Wing Street roadway extension (general area outlined in red in adjacent image). Typically, the construction of sidewalks is required when public roadways and utilities are installed. However, the extension of Wing Street remains off-site and acts as a second means of access to the development further north. While the installation of the roadway and extension of utilities is required the construction of sidewalks at this phase can be postponed until adjacent properties abutting the roadway submit a development application.



Location:

The subject request is generally located along the proposed Wing Street roadway nearest 3212 W. 2nd Street.

Physical and Natural Features:

The subject property is vacant land that is relatively flat and devoid of foliage.

Public Notification

As required by the Local Government Code and the City of Taylor Code of Ordinances, all property owners within a 200-foot radius of the subject property were notified of the subdivision variance request. Twelve (12) notices were mailed to the property owners and a legal notice advertising the public hearing was placed in the Taylor Press on Sunday, September 29, 2024.

Staff Analysis

The request for the subject property is a variance from the Engineering Manual Sections 2.10.10 relate to Sidewalks, Shared-Use Paths, and Trails. Below identifies the pertinent sections of the Engineering Manual requirements requesting to be waived by the applicant.

Section 2.10 Pedestrian Facilities**2.10.10 – Sidewalks, Shared-Use Paths, and Trails****2.10.10 A Requirements**

- Sidewalks are to be constructed with the paving of streets or building construction unless deferred by the city.

City of Taylor Engineering Response:

The City of Taylor Engineering Department has provided the following analysis and determination:

Sidewalk Installation Variance – Approved

- Engineering concurs that the sidewalk along the off-site portion of Wing Street being constructed be deferred to when the currently undeveloped properties along this position of the Wing Street are planned and developed.

Staff Recommendation

For the reasons stated in the staff analysis, staff proposed approval of the sidewalk variance.

Planning and Zoning Commission Recommendation

The Planning and Zoning Commission is charged with reviewing certain subdivision standards of the Land Development Code (LDC). In determining a recommendation on a variance request, the Planning and Zoning Commission members consider the following factors:

- 1. That the granting of the variance will not be detrimental to the public health, safety or welfare or injurious to other property in the area or to the City in administering the Land Development Code (LDC) or Engineering Manual;**
 - The request would likely not be detrimental to the public health, safety or welfare or injurious to other property in the area or to the City.
- 2. That the granting of the variance would not substantially conflict with the Comprehensive Plan and the purposes of this LDC and Engineering Manual.**
 - The granting of the sidewalk variance does not conflict with the purposes and requirements of the Engineering Manual.
- 3. That the conditions that create the need for the variance do not generally apply to other property in the vicinity.**
 - The request for a sidewalk variance can be deferred until construction by others on adjacent property.
- 4. That application of a provision of the LDC/Engineering Manual will render subdivision of the land impossible.**
 - The requests will not render the subdivision of the land impossible.
- 5. Where the literal enforcement of these regulations would result in an unnecessary hardship.**
 - Literal enforcement of the regulations within the Engineering Manual pertaining to sidewalks does not result in an unnecessary hardship.

Attachments:

1. Letter of Intent from Applicant
2. General Site Plan from Applicant
3. Location Map
4. Notification Map



METCALFE WOLFF
STUART & WILLIAMS, LLP

TALLEY WILLIAMS
Partner

twilliams@mwswtexas.com
512-404-2234

August 30, 2024

Mr. Tom Yantis
Assistant City Manager
City of Taylor
400 Porter Street
Taylor, Texas 76574

Via Online Submittal

Re: Formal Request for a Variance for sidewalk along the southern portion of the Proposed Wing Street, located between the southeast portion of the Property and State Highway 79, as depicted on the attached exhibits ("*South Wing Street*")
Owner: Taylor 79, LLC
Project Address: 14501 Hwy 79, Taylor, TX 76574

Dear Mr. Yantis:

Please accept the following as a formal variance request letter for sidewalk along South Wing Street. The applicable ordinance requirements that we will be seeking variances from are listed below:

1. SIDEWALK VARIANCE

ENGINEER MANUAL, Section 2.10.10 Sidewalks, Shared-Use Paths, and Trails

2.10.10.A Requirements

- Sidewalks are to be constructed with the paving of streets or building construction unless deferred by the City.
- Sidewalks in public ROW and constructed as part of a new neighborhood or re-development are considered public facilities and shall be constructed according to the requirements outlined in this section.
- When a lot is developed, pedestrian facilities along frontage of property shall be required at time of lot development. Sidewalks, ramps, and landing areas shall be installed along all thoroughfares and common areas prior to final acceptance.

2.10.10.B Geometry

- Width – Pedestrian facilities shall have a minimum width of five (5) feet when not adjacent to the back of curb. Sidewalks adjacent to curb shall be a minimum of six (6) feet in width.
- Paths shall have a minimum ROW width of ten (10) feet and minimum pavement width of six (6) feet.



Mr. Yantis
August 30, 2024
Page 2

- Sidewalk geometry, locations, pavement widths are to be confirmed with the City and shall be established during the planning process when the development is covered by a Neighborhood Plan or Employment Center Plan.

To provide two entry/exits from the Property and comply with code, the current development has proposed to build the extension of Wing Street. It is challenging to fit all of the improvements that are required by code between the road edge and the right-of-way. If sidewalks along South Wing Road are constructed, these improvements will likely have to be torn up and rebuilt to connect when the surrounding properties are developed.

Installation of sidewalks along South Wing Street will not provide a destination to anything at this time. The proposed industrial development being constructed on the Property will almost exclusively be accessed by vehicular means.

In summary, the Owner is requesting the foregoing variance due to the challenges related to lack of destination of the sidewalks, and lack of walkability since the property is being used as an industrial site that will be accessed primarily by car.

If you have any questions concerning this matter, please do not hesitate to contact me.

Sincerely,

Talley Williams
Partner, Metcalfe Wolff
Stuart & Williams, LLP
On behalf of Owner

Cc: Collin Harrison, Director (*via electronic delivery*)
Courtney Peres, Planning Manager (*via electronic delivery*)

Attachments:

Exhibit 1 – Existing Conditions & Site Boundary

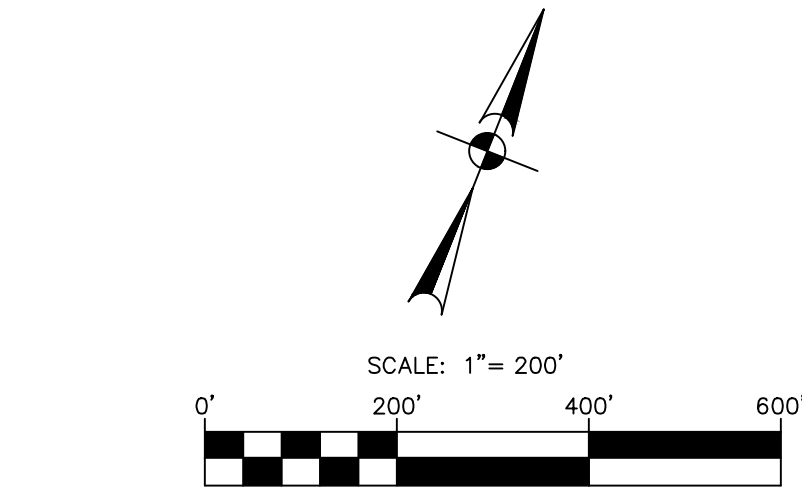
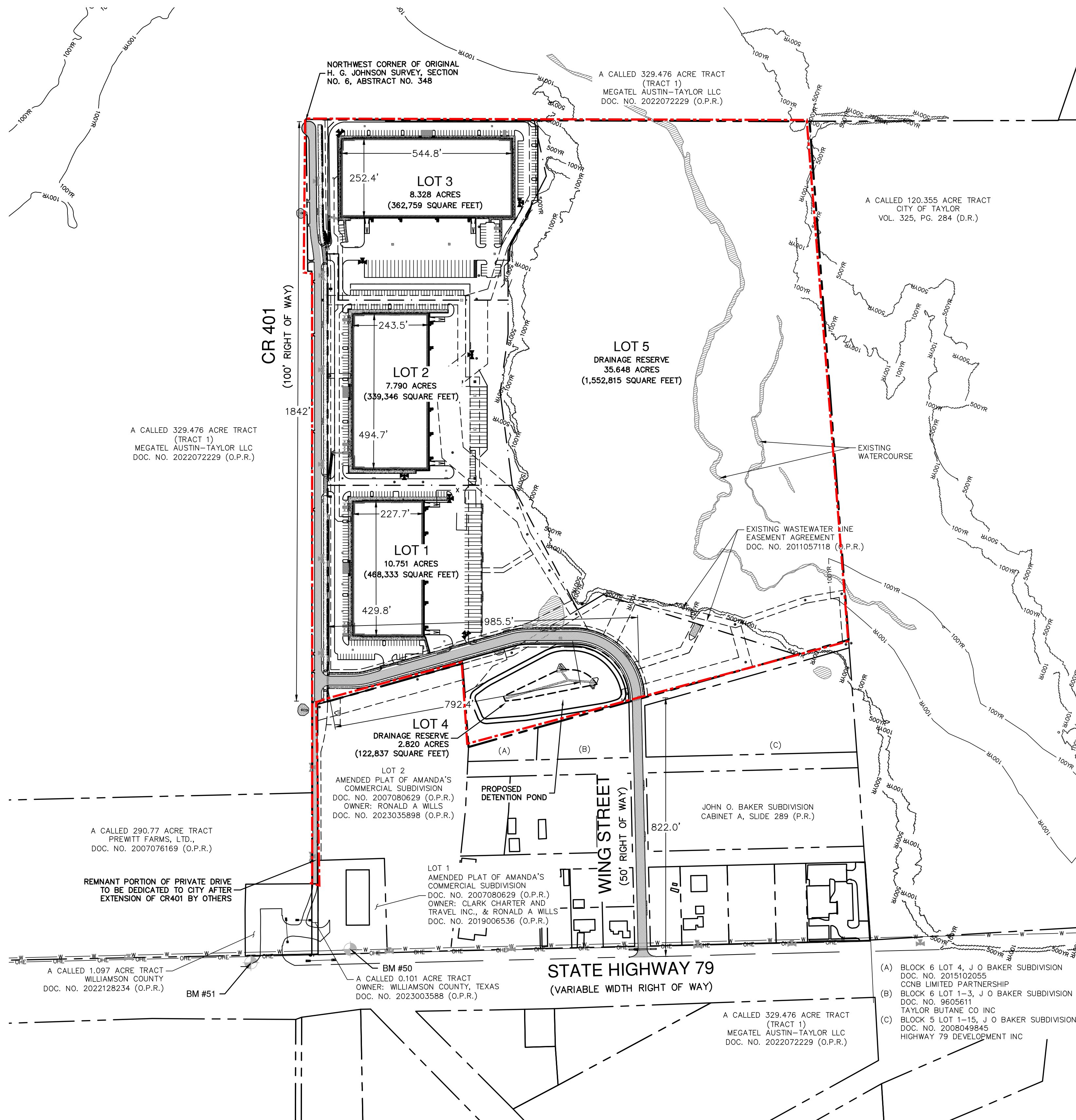


Mr. Yantis
August 30, 2024
Page 3

Exhibit 2 – Zoning Map
Exhibit 3 – General Site Plan
Exhibit 4 – Variance Exhibit
Exhibit 5 – Agent Authorization Form
Exhibit 6 – Deed
Exhibit 7 – Applicant Agent Affidavit

Date: Jun 27, 2024, 2:48pm User ID: AFerozKerber
File: H:\Projects\51429\03\001 Construction Documents\Exhibits\Exh 51429-00 Overall Site Plan.dwg

THIS DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL.



LEGEND	
	PROPERTY BOUNDARY
	PROPOSED ROW DEDICATION
	ADJACENT PROPERTY BOUNDARY
	PROPOSED LOT LINE
	PROPOSED EASEMENT LINE
	EXISTING EASEMENT LINE
	500-YEAR FEMA FLOODPLAIN
	100-YEAR FEMA FLOODPLAIN
	EXISTING EDGE OF PAVEMENT
	EXISTING FIRE HYDRANT
	PROPOSED FIRE HYDRANT
	EXISTING WASTEWATER MANHOLE
	EXISTING UTILITY POLE
	STREET BUILT WITH PUBLIC INFRASTRUCTURE
	SIDEWALK BUILT WITH PUBLIC INFRASTRUCTURE

PARKING COUNT:	
STANDARD SPACE -	342
HANDICAP SPACE -	14
TRUCK PARKING -	70
TOTAL -	426

IMPERVIOUS COVER TABLE				
LOT #	LOT ACREAGE	IC (SF)	IC (AC)	PERCENTAGE OF SITE %
LOT 1	10.751 AC	242,104.40	5.56 AC	51.72%
LOT 2	7.790 AC	258,658.83	5.94 AC	76.25%
LOT 3	8.328 AC	300,409.14	6.90 AC	82.85%
TOTAL IMPERVIOUS COVERAGE		801,372.37 SF	18.4	68.48%
TOTAL SITE AREA		1,170,438.07	26.869	100.00

NOTES:
DECEL LANE ALONG STATE HIGHWAY 79 TO BE PROVIDED TO TxDOT SEPARATE DRAWING SET.

ZONING :
THE PROPERTY IS IDENTIFIED AS EC (EMPLOYMENT CENTER) BY THE CITY OF TAYLOR.
THE PROPERTY WAS PREVIOUSLY ZONED AS M-1 (LIGHT INDUSTRIAL) PRIOR TO THE UPDATED ZONING MAP.

BENCHMARKS:
BENCHMARKS ARE BASED ON NAD83
COORDINATES ARE STATE PLANE CENTRAL ZONE WITH A COMBINED SCALE FACTOR OF 0.99988

BM #50: SET MAGNETIC NAIL IN CONCRETE
GRID NORTH: 10,181,619.81
GRID EAST: 3,204,414.68
ELEV: 609.87
BM #51: SET MAGNETIC NAIL IN BASE OF TRAFFIC POLE
GRID NORTH: 10,181,463.29
GRID EAST: 3,204,136.64
ELEV: 607.83

REVISIONS					
NO.	DESCRIPTION	REVISE (R) ADD (A) VOID (V) SHEET NO'S	DESIGN ENGINEER SIGNATURE	CITY OF TAYLOR APPROVAL	APPROVAL DATE

NO.	REVISION	DATE

**PAPE-DAWSON
ENGINEERS**
AUSTIN | SAN ANTONIO | HOUSTON | FORT WORTH | DALLAS
1801 N. MOPEC EXPY., SUITE 3, STE 200 | AUSTIN, TX 78759 | 512.664.6711
TYPE FIRM REGISTRATION #470 | TYPE FIRM REGISTRATION #10028601

PARK 79 COMMERCE CENTER
TAYLOR, TEXAS 76574

OVERALL SITE PLAN

JOB NO. 51429-00
DATE MAY 2024
DESIGNER AG
CHECKED DRAWN JR
SHEET EXH

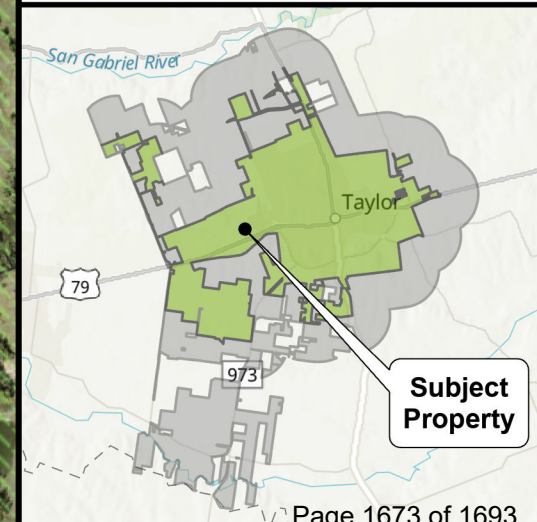
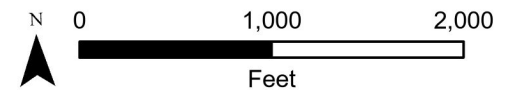
PZ-2024-2151



PZ-2024-2238

14501 Hwy 79
Subdivision Variance
Location Map
Approximately 70 acres

-  City Limits
-  ETJ Boundary
-  Subject Property
-  Parcels

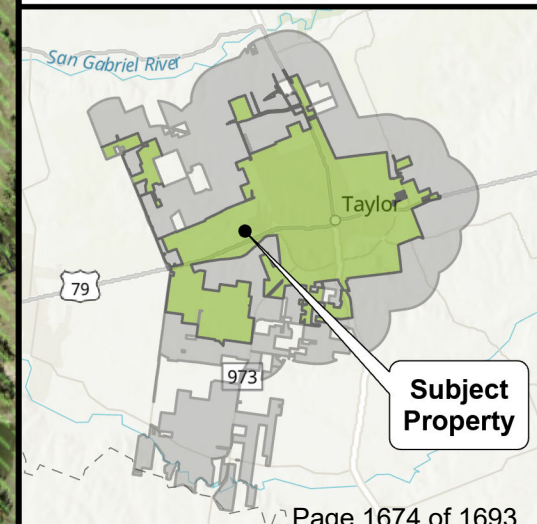
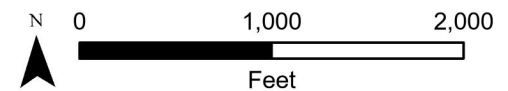




PZ-2024-2238

14501 Hwy 79
Subdivision Variance
Notification Map
Approximately 70 acres

- City Limits
- ETJ Boundary
- Subject Property
- 200-ft. Buffer
- Notified Properties
- Parcels



City of Taylor
PZ-2024-2212
Infill Neighborhood Plan
Staff Report

Item Details

Subject Property:	1120 W. Lake Drive
Total Acreage:	Approximately 1.240 acres
Legal Description:	Approximately 1.240 acres of land generally located at 1120 W. Lake Drive, more particularly described by the Williamson Central Appraisal District Parcel R017151, Murphy Addition, Hague Heights, Block 1, Lot 8-13, Taylor, Williamson County, Texas
Property Owner:	Jonathan Downs, Smart Nest LLC
Request:	A request for an Infill Neighborhood Plan for property generally located at 1120 W. Lake Drive.
Case History:	This is the first hearing of the Infill Neighborhood Plan. A zoning amendment request from Place Type P2.5 – Large Lot to Place Type P3 – Neighborhood was heard in August, but the application was withdrawn.

Overview of Applicant's Request & Background

The subject property currently consists of a single-family home located at the northeast corner of North Drive and W. Lake Drive. The subject property is shown to be developed as one parcel but is platted as six individual lots that have all come under one ownership. The Hague Heights Addition (1915) shows how the subject property and surrounding properties in the addition were intended to develop in a denser configuration. The applicant previously requested a zoning amendment request from Place Type P2.5 – Large Lot to Place Type P3 – Neighborhood in August 2024. The Planning & Zoning Commission voted to table the item, and the applicant elected to remove the item from the City Council agenda.

The applicant is now requesting an Infill Neighborhood Plan to change to Place Type P3 – Neighborhood with modification to the allowable building types to exclude Cottage Court and limit the building height to two-stories to develop the remaining five lots as platted. This would allow the applicant to develop the 6 lots with the P3 Neighborhood standards, limiting the maximum building height to 2 stories and excluding the Cottage Court building type. Cottage Court would allow for a group of individual, small dwelling units on a single lot or on individual lots that share a central courtyard

Location:

The subject property is a corner lot located at 1120 W. Lake Drive at the northeast corner of North Drive and West Lake Drive. The subject property is bounded by W. Lake Drive to the South, North Drive to the West, and Fisher Street to the North.

Physical and Natural Features:

The subject property consists of the six lots combined into a 1.240-acre parcel. The subject property consists of one single family home and additions of garage space.

Growth Sector Designations:

The subject property is located within Infill Growth Sector (G-5). The Infill Growth Sector is made up of areas that are already developed and served by infrastructure. They are also intended for higher intensity uses than are currently in place (Envision Taylor Comprehensive Plan page 50).

Future Land Use and Zoning Designations:

The subject property is assigned the Future Land Use designation of Market Center – Neighborhood. The Market Centers are mixed-use areas anchored by a retail destination where surrounding residents go to get daily goods and services. As with all centers, Market Centers are envisioned as mixed-use and walkable places. Neighborhood, Community, and Regional Market Centers differ by the scale of the development but not in the types of uses or the walkable nature.

The subject property is currently zoned Place Type P2.5 – Large Lot. The P2.5 Large Lot Place Type includes properties that are one (1) acre or greater. Generally, the lots contain one (1) or more structures with large setbacks from the street. Properties may be entirely fenced but can be seen from the street edge. Generally, the properties are residential in nature. The proposed P3 Place Type does not have an area minimum and permits Detached and Attached Dwellings, Cottage Courts, Duplexes, Accessory Dwelling Units, Residential Accessories, and Civic Buildings.

The surrounding properties are:

Direction	Zoning District	Growth Sector	Future Land Use	Existing Land Use
North	P3 Neighborhood	Infill Neighborhood Sector (G-4)	Neighborhood Infill	Single Family Home
South	P3 Neighborhood	Infill Neighborhood Sector (G-4)	Neighborhood Infill	Single Family Home
East	P2.5 Large Lot	Infill Growth Sector (G-5)	Market: Neighborhood	Single Family Home
West	P2.5 Large Lot	Infill Growth Sector (G-5)	Market: Neighborhood	Single Family Home

Transportation

The subject property gains access off North Drive for the existing home (lot 10). Lots 8 and 9 will also take access off W. Lake Drive and Lots 11, 12, and 13 will have access off Fisher Street. W. Lake Drive is identified as a Community Boulevard and North Drive is identified as a Neighborhood Avenue. Community Boulevards connect neighborhoods with the surrounding community and are typically low-speed, moderate volume streets. Community Boulevards are primary routes connecting major areas of the city and they are typically moderate-speed, high-volume streets. Turning movements should be limited on Community Boulevards when possible.

Utilities

The subject property is located within the City of Taylor water and wastewater CCN. There are adequate water lines bounding the subject property (8-inch – 12-inch lines). There is a 6-inch wastewater line on W. Lake Drive and North Drive.

Public Notification

As required by the Local Government Code and the City of Taylor Code of Ordinances, all property owners within a 200-foot radius of the subject property were notified of the zoning map amendment request. Twenty-one (21) notices were mailed to the property owners and a legal notice advertising the public hearing was placed in the Taylor Press on Sunday, September 22, 2024.

Staff Analysis

The requested Infill Neighborhood Plan to change to Place Type P3 – Neighborhood with modification to the allowable building types to exclude cottage court and limit the building height to two-stories, aims to provide housing that would be like the existing housing in the surrounding neighborhood. Upon analyzing the request, the proposed rezoning is consistent with the surrounding zoning of neighborhood homes to the north and south immediately adjacent to the subject property. The plat from 1915 depicts a total of six lots (8, 9, 10, 11, 12, and 13) that were merged into one parcel. The request is to activate the acreage as it was intended and previously platted to create further density in the area. A change of Place Type is required as the individual lots will be less than the area minimum (1 acre) required in the existing P2.5 Place Type. P3 aligns more accurately with the configuration of the platted lots from the 1915 Hague Heights Addition. Additionally, access along W. Lake Drive for lots 8 and 9 shall be limited to consist of one shared access off W. Lake Drive.

- **LU9** – Promote development patterns that maximize the use of existing infrastructure and land before expanding infrastructure to underdeveloped areas.
 - The proposed development on the subject property an existing lot as a neighborhood infill redevelopment with existing utilities.

Staff Recommendation

For the reasons stated in the staff analysis, staff recommends approval of the requested rezoning with the additional condition of access limitation along W. Lake Drive.

Planning and Zoning Commission Recommendation

The Planning and Zoning Commission is charged with reviewing all requests for rezonings and recommends to City Council either in favor of or opposition to each request. In determining a

recommendation on a rezoning request, the Planning and Zoning Commission members consider the following factors:

Is the rezoning consistent with the Comprehensive Plan?

- The rezoning request is partially consistent with the Comprehensive Plan. It seeks to provide an increase in density and housing to the large lot but the P3 – Neighborhood Place Type will not activate

Is the rezoning compatible with the surrounding area?

- The rezoning is compatible with other adjacent properties as they are primarily P3 – Neighborhood Place Type which is to the north and south of the subject property.

Does the rezoning promote the public health, safety, or general welfare?

- Staff analysis determined that the rezoning of the subject property will promote public health, safety, and the general welfare. In addition, the Planning Principles and Policy Guide in the Comprehensive Plan aim to promote health, safety, and general welfare by managing growth, while promoting safe and orderly development.

Is adequate infrastructure available or planned to meet the needs of the proposed land use?

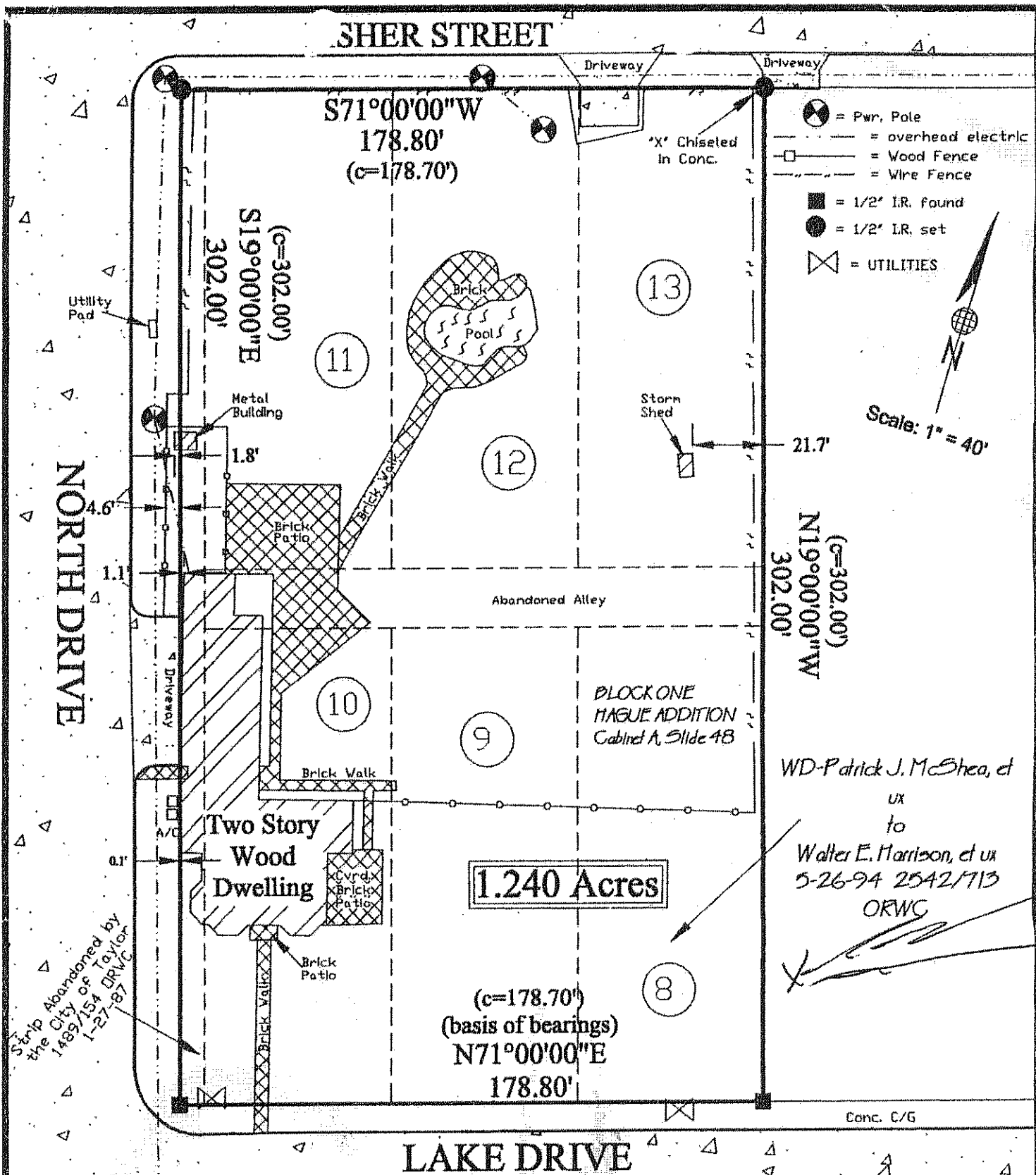
- There are existing public utilities adequate for infill development.

Do current conditions indicate that a rezoning is necessary?

- To proceed with development as platted in the Hague Heights Addition the request to rezone to P3 is necessary to meet minimum area requirements. Currently, P2.5 – Large Lot requires a minimum of one acre. The individual lots as platted will all be less than an acre and thus a rezoning is required.

Attachments:

1. Survey from Applicant
2. Hague Heights Addition Plat circa 1915
3. Location Map
4. Existing Zoning Map
5. Proposed Zoning Map
6. Growth Sectors Map
7. Future Land Use Map
8. Notification Map
9. Citizen Responses
10. Draft Ordinance
 - a. Exhibit A – Survey
 - b. Exhibit B – Proposed Zoning Map
 - c. Exhibit C – Proposed Infill Neighborhood Plan Standards



This is to certify that, on this date, a true and accurate survey was made on the ground under my supervision of property located at No. 1120 Lake Drive in Williamson County, Texas, described as follows: Lots 8, 9, 10, 11, 12 and 13, Block 1, Hague Heights, according to the map or plat thereof recorded in Cabinet A, Slide 48, of the Plat Records of Williamson County, Texas, and those certain tracts of land abandoned by City of Taylor as conveyed in Deed recorded in Volume 1489, Page 157, Official Records Williamson County, Texas.

FLOOD NOTE:

The property depicted hereon is / is not within a Special Flood Hazard Area as determined by the Federal Emergency Management Agency, the flood area being identified on F. I. R. M. Panel No. 48491 C0266 C effective 9-27-91 located in Zone "X".

BRYAN TECHNICAL SERVICES, INC.
P. O. BOX 1371
TAYLOR, TEXAS 76574
512-365-2524
512-365-2526 (Fax)
bbryan@ccai.com

NOTE: This tract is subject to easements, covenants, conditions, restrictions and right to create additional public utility easements across subject property as shown on foregoing plat.

To: First American Title Insurance Company of Texas
Re: Harrison, G.F.# 03010296, Proj. No. 01-752

SURVYOR'S CERTIFICATE

The plat shown hereon is a true, correct and accurate representation of the property as determined by survey, the lines and dimensions of said property being as indicated by the plat; the size, location and type of buildings are as shown, all improvements being within the boundaries of the property, set back from the property lines the distances indicated. There are no encroachments, conflicts or protrusions, except as shown hereon, and said property has access to and from a dedicated roadway.

DATE: December 28, 2001

State of Texas,

County of Williamson.

Know All Men By These Presents:-

That we, Annie Hague and Ed. R. Hague, husband and wife of Williamson County, Texas, do hereby certify that the above and foregoing map is a true and correct map of an addition made by us to the City of Taylor, Texas, in above said county, to be known as "Hague Heights" consisting of Blocks Numbered 1 and 2, and each Block consisting of 20 lots, numbered consecutively from 1 to 20, and all sales of said lots will be made with reference to said map.

In witness whereof we have hereto set our hands this the 30th day of March A.D. 1915.

Annie N. Hague,

Ed R Hague.

State of Texas,

County of Williamson.

Before me, Fred I Fisher, a Notary

Public in and for Williamson County,

Texas, on this day personally appeared Ed R Hague and Annie Hague, wife of said Ed R Hague known to me to be the persons whose names are subscribed to the foregoing instrument, and acknowledged to me that they executed the same for the purposes and consideration therein expressed. And the said Annie Hague wife of the said Ed R Hague, having been examined by me privily and apart from her husband, and having the same fully explained to her she the said Annie Hague acknowledged such instrument to be her act and deed, and declared that she had willingly signed the same for the purposes and consideration therein expressed, and that she did not wish to retract it.

Given under my hand and seal of office, this 30th day of March A. D. 1915.

(L.S.)
623.

Fred I Fisher, Notary Public, in and for

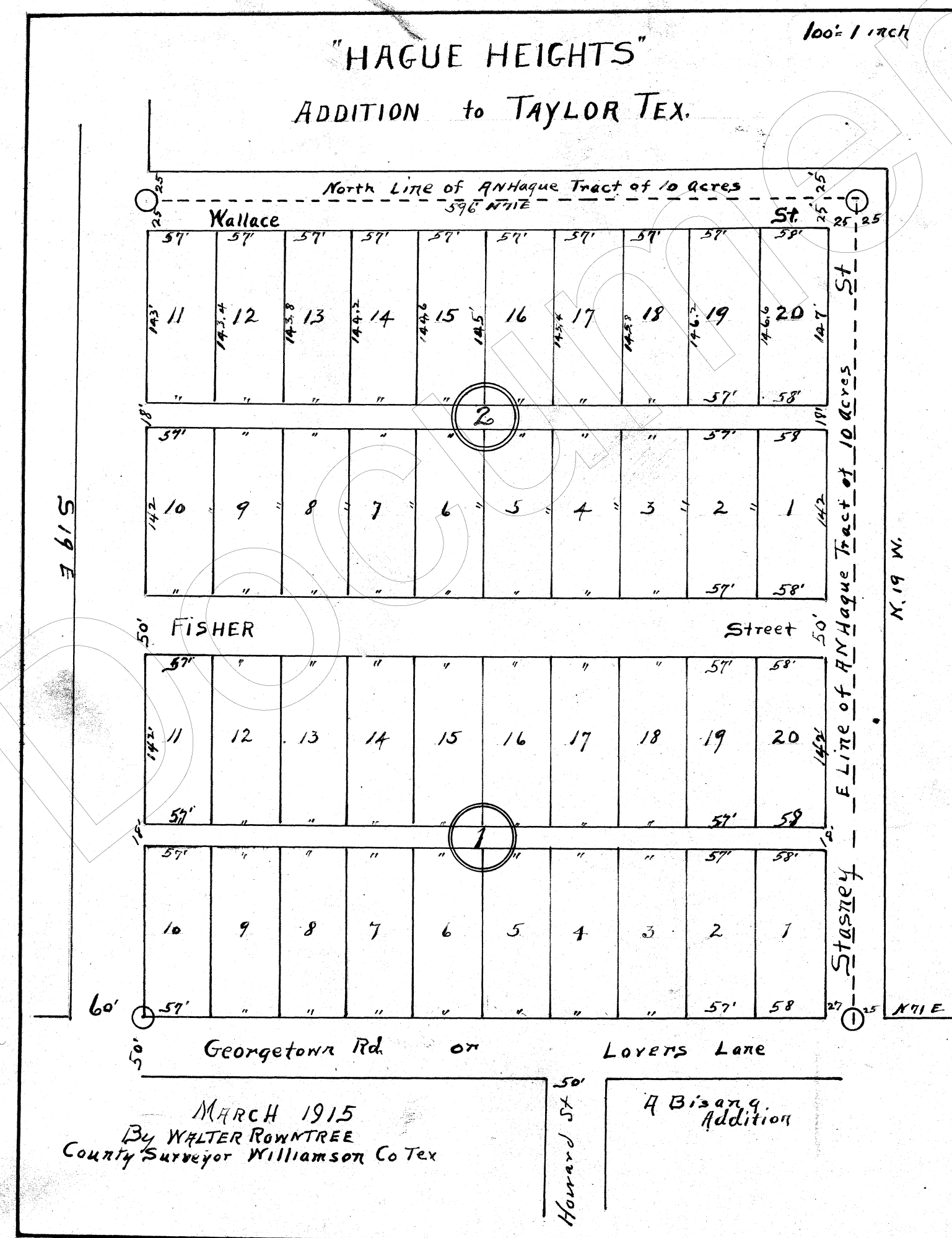
Williamson County, Texas.

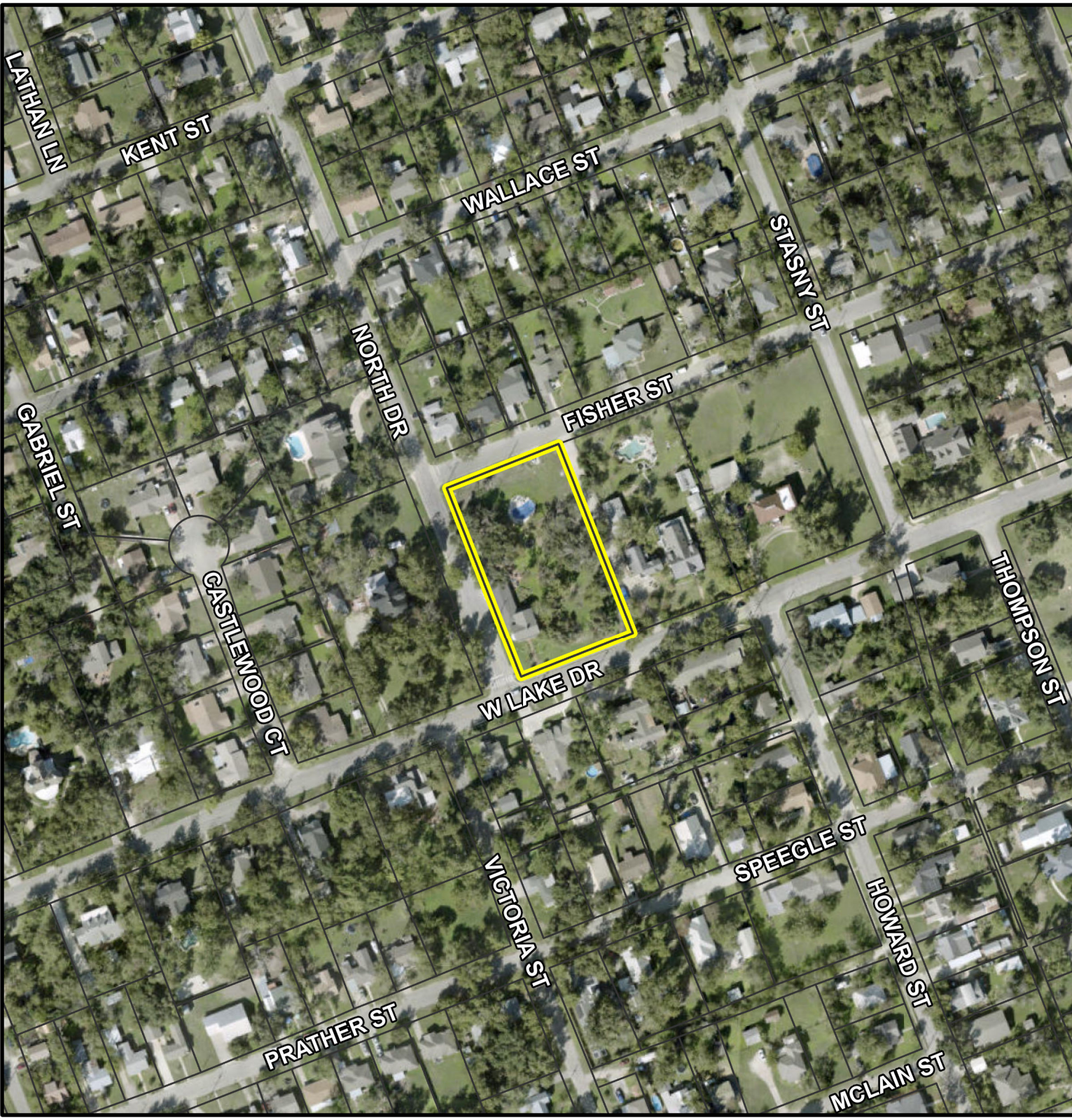
Filed for Record April 1st, 1915, at 8 o'clock A. M.

Recorded April 1st, 1915, at 9:30 o'clock A. M.

County Clerk, Williamson County, Texas.

By _____ Dept.

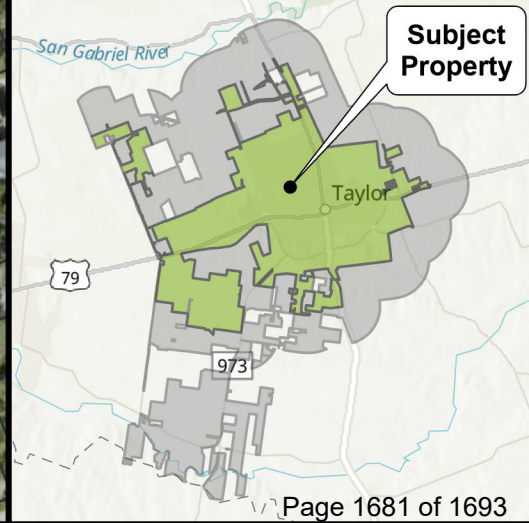




PZ-2024-2212

1120 W Lake Drive
Rezoning
Location Map
Approximately 1.24 acres

-  City Limits
-  Subject Property
-  Parcels

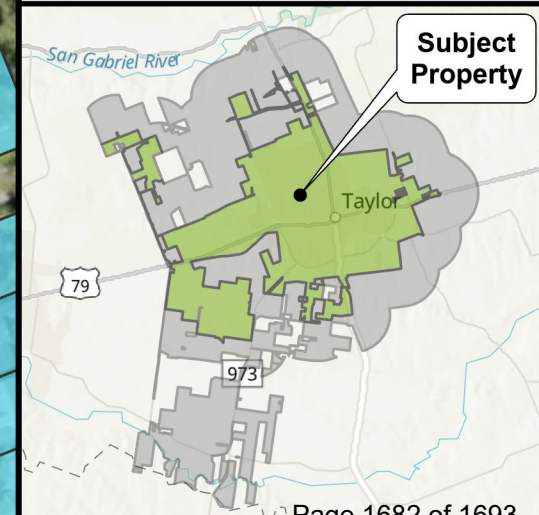




PZ-2024-2212

1120 W Lake Drive
Rezoning
Current Zoning Map
Approximately 39 acres

-  City Limits
-  Subject Property
-  Parcels
- Place Type Zoning**
 -  P2.5: Large Lot
 -  P3: Neighborhood





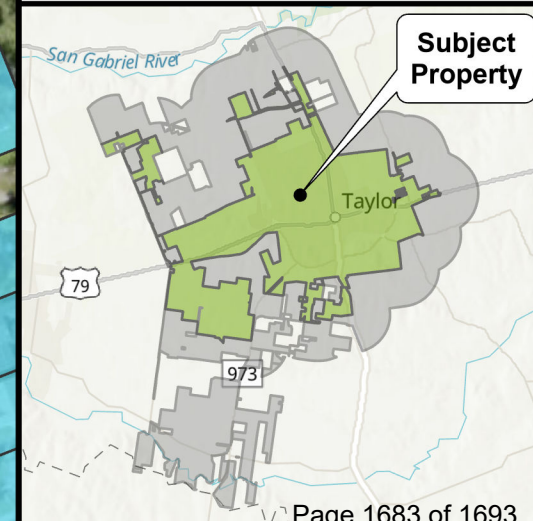
PZ-2024-2212

**1120 W Lake Drive
Rezoning
Proposed Zoning Map
Approximately 1.24 acres**

-  City Limits
-  Subject Property
-  Parcels

Place Type Zoning

-  P2.5: Large Lot
-  P3: Neighborhood





PZ-2024-2212

**1120 W Lake Drive
Rezoning
Growth Sector Map
Approximately 1.24 acres**

 City Limits

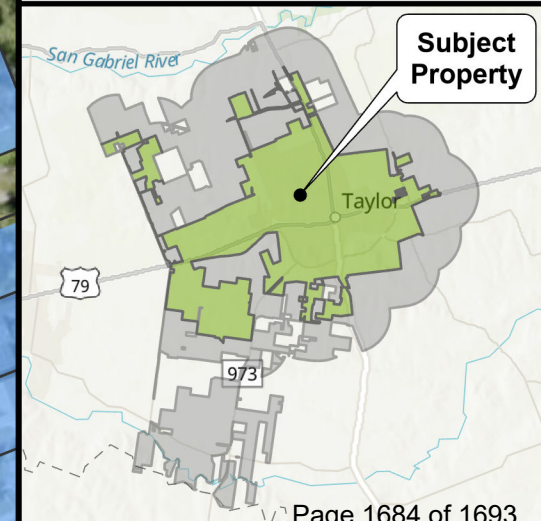
 Subject Property

 Parcels

Growth Sector

 Infill Neighborhood Sector (G-4)

 Infill Growth Sector (G-5)

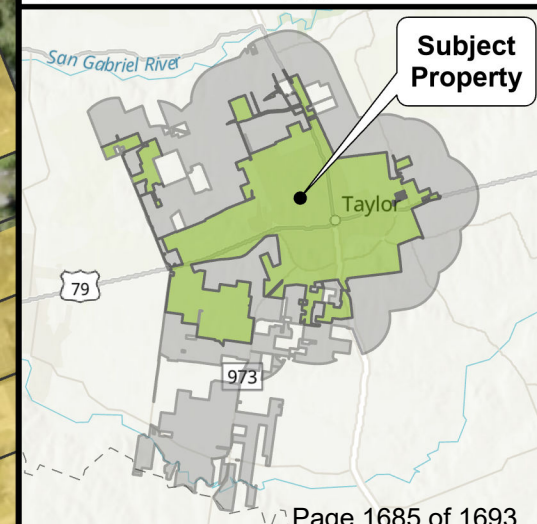




PZ-2024-2212

1120 W Lake Drive
Rezoning
Future Land Use Map
Approximately 1.24 acres

-  City Limits
-  Subject Property
-  Parcels
- Future Land Use**
 -  Neighborhood Infill
 -  Market: Neighborhood

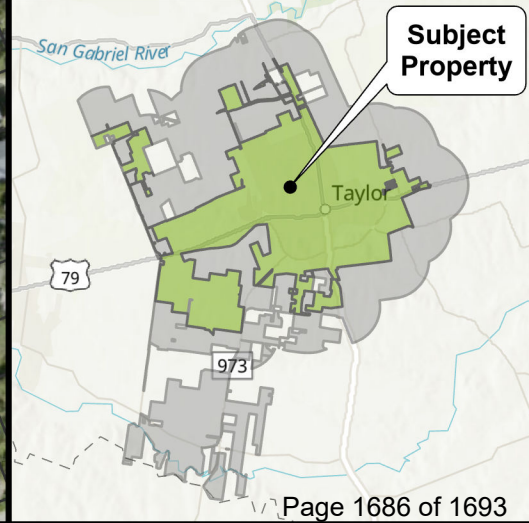




PZ-2024-2212

**1120 W Lake Drive
Rezoning
Notification Map
Approximately 1.24 acres**

- City Limits
- Subject Property
- 200-ft. Buffer
- Notified Properties
- Parcels



ORDINANCE NO. 2024-38

AN ORDINANCE APPROVING AN INFILL NEIGHBORHOOD PLAN ON PROPERTY GENERALLY LOCATED AT 1120 W. LAKE DRIVE, LEGALLY DESCRIBED AS HAGUE HEIGHTS ADDITION, BLOCK ONE, LOTS EIGHT THROUGH THIRTEEN, CONSISTING OF APPROXIMATELY 1.24 ACRES OF LAND, MORE PARTICULARLY DESCRIBED BY THE WILLIAMSON CENTRAL APPRAISAL DISTRICT PARCEL R017151, TAYLOR, WILLIAMSON COUNTY, TEXAS, AMENDING THE OFFICIAL ZONING MAP OF THE CITY OF TAYLOR, TEXAS, TO SHOW THE ZONING CHANGE ADOPTED HEREIN; PROVIDING A SAVINGS CLAUSE.

WHEREAS, the Taylor City Council, after proper notice, conducted a public hearing on October 24, 2024, to consider the request made by the applicant, whose property is legally described in Exhibit “A” attached hereto and incorporated by reference herein for all purposes (“Properties”), to approve an Infill Neighborhood Plan; and

WHEREAS, the Planning and Zoning Commission, after proper notice, conducted a public hearing on October 8, 2024, to consider the zoning request, and recommended the Infill Neighborhood Plan to the City Council; and

WHEREAS, the City Council, after the public hearing, approves the request for the Infill Neighborhood Plan and Property zoning change.

NOW, THEREFORE, BE IT ORDAINED BY THE CITY COUNCIL OF THE CITY OF TAYLOR, TEXAS, that:

SECTION 1. The facts and recitations contained in the preamble of this Ordinance are hereby declared to be true and correct and are incorporated by reference herein and made a part hereof, as if copied verbatim.

SECTION 2. The Official Zoning map of the City of Taylor, Texas, is changed to indicate that an Infill Neighborhood Plan with standards outline in Exhibit “B” has been approved for the Property and to show the allocation and location of Place Types on the Property as depicted in Exhibit “C” attached hereto and incorporated herein for all purposes.

SECTION 3. All other terms and conditions contained in the official zoning map, except as amended herein, shall continue and remain in full force and effect.

SECTION 4. Should any section, paragraph, clause, phrase, or provision of this Ordinance be adjudged invalid or held unconstitutional, the same shall not affect the validity of this Ordinance as a whole or any part of the provisions thereof, other than the part so decided to be invalid or unconstitutional.

SECTION 5. In accordance with Article VIII of the City Charter, Ordinance 2024-28 was introduced before the Taylor City Council on the 24th day of October 2024.

PASSED, APPROVED, and ADOPTED on the _____ day of _____, 2024.

Dwayne Ariola, Mayor

ATTEST:

Lucy Aldrich, City Clerk

APPROVED AS TO FORM:

Mark Schroeder,
City Attorney

CERTIFICATE

THE STATE OF TEXAS

COUNTY OF WILLIAMSON

I, Dianna McLean, being the current City Clerk of the City of Taylor, Texas, do hereby certify that the attached is a true and correct copy of Ordinance No. 2024-38, passed and approved by the City Council of the City of Taylor, Texas, on the _____ day of _____ 2024, and such Ordinance was duly introduced, passed, approved and adopted at meetings open to the public and notices of the meetings, giving the dates, places, and subject matter thereof, were posted as prescribed by Government Code Section 551.043.

Witness my hand and seal of office this _____ day of _____ 2024.

Dianna McLean
City Clerk

EXHIBIT A
(Metes & Bounds/Legal Description/Survey)

SHER STREET

S71°00'00"W
178.80'
(c=178.70')

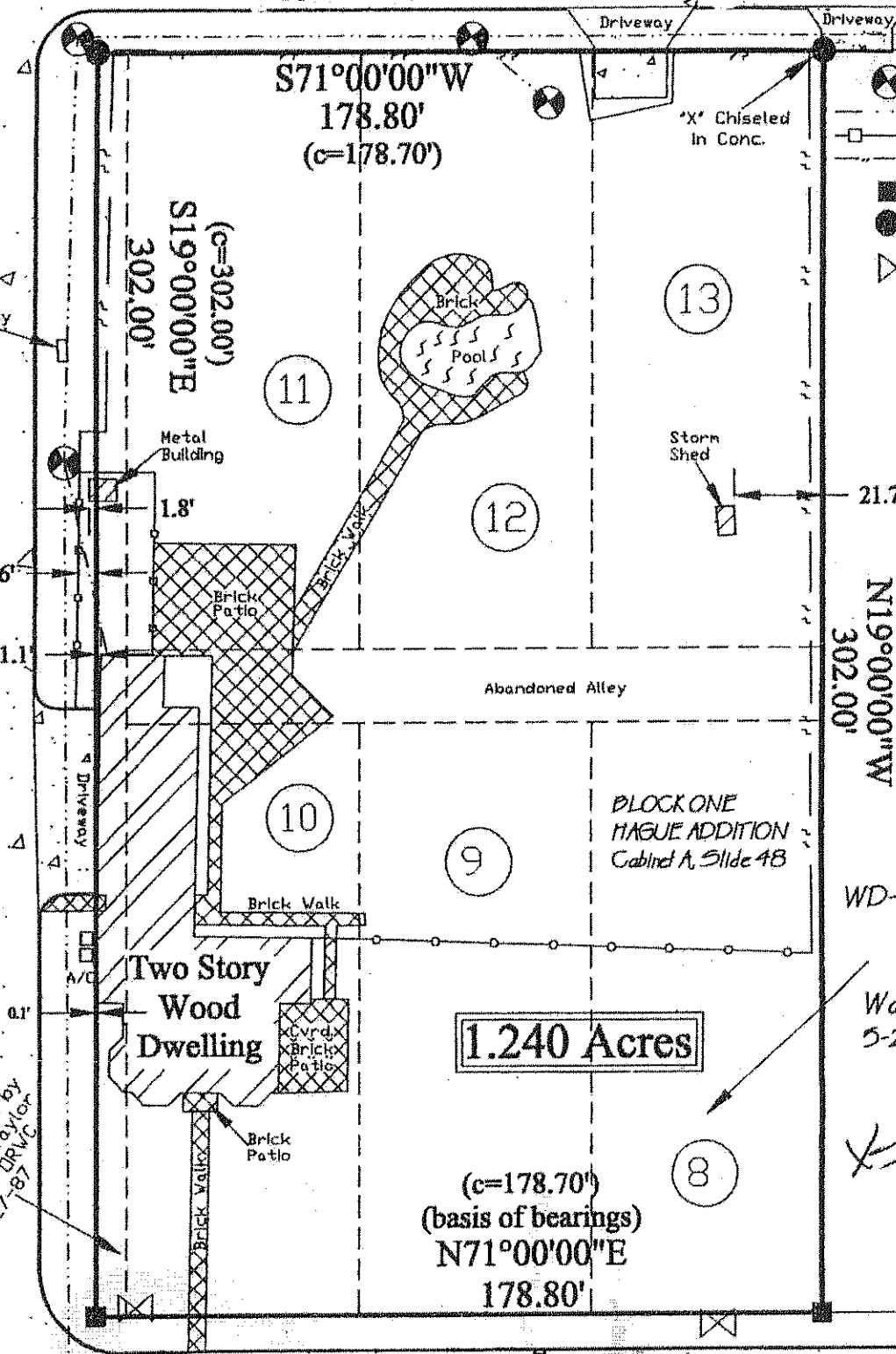
S19°00'00"E
(c=302.00')
302.00'

NORTH DRIVE

- ⊗ = Pwr. Pole
- = overhead electric
- = Wood Fence
- = Wire Fence
- = 1/2" I.R. found
- = 1/2" I.R. set
- ⊗ = UTILITIES

Scale: 1" = 40'

Strip Abandoned by
the City of Taylor
1-27-87
1489/154 DRWG



WD-Patrick J. McShea, et
ux
to
Walter E. Harrison, et ux
5-26-94 2542/713
ORWC

LAKE DRIVE

This is to certify that, on this date, a true and accurate survey was made on the ground under my supervision of property located at No. 1120 Lake Drive in Williamson County, Texas, described as follows: Lots 8, 9, 10, 11, 12 and 13, Block 1, Hague Heights, according to the map or plat thereof recorded in Cabinet A, Slide 48, of the Plat Records of Williamson County, Texas, and those certain tracts of land abandoned by City of Taylor as conveyed in Deed recorded in Volume 1489, Page 157, Official Records Williamson County, Texas.

FLOOD NOTE:

The property depicted hereon is / is not within a Special Flood Hazard Area as determined by the Federal Emergency Management Agency, the flood area being identified on F. I. R. M. Panel No. 48491 C0266 C effective 9-27-91 located in Zone "X".

BRYAN TECHNICAL SERVICES, INC.
P. O. BOX 1371
TAYLOR, TEXAS 76574
512-365-2524
512-365-2526 (Fax)
bbryan@ccai.com

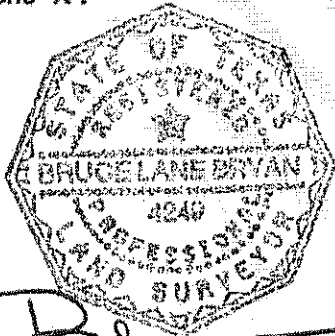
NOTE: This tract is subject to easements, covenants, conditions, restrictions and right to create additional public utility easements across subject property as shown on foregoing plat.

To: First American Title Insurance Company of Texas
Re: Harrison, G.F.# 03010296, Proj. No. 01-752

SURVYORS CERTIFICATE

The plat shown hereon is a true, correct and accurate representation of the property as determined by survey, the lines and dimensions of said property being as indicated by the plat; the size, location and type of buildings are as shown, all improvements being within the boundaries of the property, set back from the property lines the distances indicated. There are no encroachments, conflicts or protrusions, except as shown hereon, and said property has access to and from a dedicated roadway.

DATE: December 28, 2001



Blocks	
Block Length (maximum)	300 ft
Block Perimeter (maximum)	1,320 ft

Lots	
Width (minimum)	None
Area (minimum)	None
Lot Coverage (maximum)	60%
Build-to-Line (range)	5 ft – 20 ft
Façade Buildout (minimum)	50%
Stories (maximum)	2 Stories

Parking Location	
First Layer	Not Permitted
Second Layer	Permitted
Third Layer	Permitted
On-Street	Permitted
RV Storage	Permitted

Private Frontage Types	
Common Yard	Permitted
Porch with Optional Fence	Permitted
Stoop	Permitted
Gallery	Not Permitted
Arcade	Not Permitted
Shopfront	Not Permitted
Forecourt	Permitted
Terrace	Permitted

Building Disposition	
Edgeyard	Permitted
Sideyard	Permitted
Courtyard	Permitted
Rearyard	Not Permitted

Building Types	
Detached Dwelling	Permitted
Attached Dwelling	Permitted
Cottage Court	Not Permitted
Duplex	Permitted
Triplex	Not Permitted
Quadruplex	Not Permitted
Small Apartment	Not Permitted
Large Apartment	Not Permitted
Manufactured Home/Park	Not Permitted
Accessory Dwelling Unit	Permitted
Recreation Vehicle Park	Not Permitted
Residential Accessory	Permitted
Small Commercial	Not Permitted
Live/Work	Not Permitted
Mixed-Use Commercial	Not Permitted
Large Commercial	Not Permitted
Highway Commercial	Not Permitted
Industrial	Not Permitted
Carts, Kiosks, Food Trailers	Not Permitted
Commercial Accessory	Not Permitted
Civic Buildings	Permitted



PZ-2024-2212

**1120 W Lake Drive
Rezoning
Proposed Zoning Map
Approximately 1.24 acres**

-  City Limits
-  Subject Property
-  Parcels

Place Type Zoning

-  P2.5: Large Lot
-  P3: Neighborhood

